

<b>ERITowerBeta</b>  <b>Electronics Research Inc.</b> 7777 Gardner Road Chandler, IN Phone: 812-925-6000 FAX: 812-925-4026	Job	Portland, ME 531' Guyed Tower	Page	30 of 82
	Project	12590 48" Face Run#2	Date	11:18:49 10/18/04
	Client	SAGA Communications	Designed by	M. Maurer

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	klf	
T15 191.000-171.000	0.279	1.645	C	0.286	2.334	0.612	0.85	1	14.731	1.159	0.058	B
			A	0.279	2.354	0.61	0.85	1	14.299			
			B	0.356	2.158	0.635	0.85	1	19.010			
T16 171.000-151.000	0.531	1.645	C	0.286	2.334	0.612	0.85	1	14.731	1.533	0.077	B
			A	0.386	2.093	0.646	0.85	1	20.961			
			B	0.487	1.918	0.691	0.85	1	28.291			
T17 151.000-131.000	0.783	1.774	C	0.286	2.334	0.612	0.85	1	14.731	1.807	0.090	B
			A	0.525	1.87	0.71	0.85	1	31.382			
			B	0.566	1.829	0.734	0.85	1	34.971			
T18 131.000-111.000	0.922	1.824	C	0.295	2.31	0.614	0.85	1	15.231	1.616	0.081	B
			A	0.53	1.864	0.713	0.85	1	31.796			
			B	0.658	1.779	0.791	0.85	1	43.844			
T19 111.000-91.000	0.922	1.774	C	0.297	2.305	0.615	0.85	1	15.348	1.598	0.080	B
			A	0.525	1.87	0.71	0.85	1	31.382			
			B	0.654	1.781	0.788	0.85	1	43.333			
T20 91.000-71.000	0.922	1.774	C	0.295	2.31	0.614	0.85	1	15.231	1.598	0.080	B
			A	0.525	1.87	0.71	0.85	1	31.382			
			B	0.654	1.781	0.788	0.85	1	43.333			
T21 71.000-51.000	0.922	2.016	C	0.295	2.31	0.614	0.85	1	15.231	1.630	0.082	B
			A	0.532	1.862	0.714	0.85	1	32.150			
			B	0.66	1.779	0.792	0.85	1	44.237			
T22 51.000-31.000	0.922	2.016	C	0.303	2.288	0.617	0.85	1	15.809	1.630	0.082	B
			A	0.532	1.862	0.714	0.85	1	32.150			
			B	0.66	1.779	0.792	0.85	1	44.237			
T23 31.000-11.000	0.922	2.016	C	0.303	2.288	0.617	0.85	1	15.809	1.630	0.082	B
			A	0.532	1.862	0.714	0.85	1	32.150			
			B	0.66	1.779	0.792	0.85	1	44.237			
T24 11.000-1.000	0.461	1.014	C	0.303	2.288	0.617	0.85	1	15.809	0.925*	0.093	B
			A	0.991	2.081	1	0.85	1	22.135			
			B	1	2.1	1	0.85	1	27.552			
Sum Weight:	9.251	46.616	C	0.504	1.895 *2A <sub>g</sub> limit	0.7	0.85	1	7.878	32.381		

### Tower Forces - With Ice - Wind Normal To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	klf	
L1 528.000-512.205	0.024	1.252	A	0.371	2.125	0.64	1	1	12.571	0.858	0.054	C
			B	0.371	2.125	0.64	1	1	12.571			
			C	0.422	2.022	0.661	1	1	14.682			
L2 512.205-496.410	0.024	1.251	A	0.37	2.125	0.64	1	1	12.562	0.858	0.054	C
			B	0.37	2.125	0.64	1	1	12.562			
			C	0.422	2.023	0.661	1	1	14.673			
L3 496.410-480.615	0.024	1.244	A	0.369	2.129	0.639	1	1	12.310	0.845	0.053	C
			B	0.369	2.129	0.639	1	1	12.310			
			C	0.42	2.025	0.66	1	1	14.428			
L4 480.615-470.000	0.058	1.280	A	0.552	1.841	0.726	1	1	14.700	0.782	0.074	A
			B	0.449	1.975	0.673	1	1	11.452			
			C	0.5	1.9	0.698	1	1	12.992			
T1 470.000-451.000	0.119	2.473	A	0.395	2.073	0.65	1	1	22.431	1.417	0.075	C
			B	0.317	2.252	0.621	1	1	17.647			
			C	0.452	1.971	0.674	1	1	26.212			
T2 451.000-	0.170	1.964	A	0.347	2.179	0.631	1	1	18.786	1.279	0.064	C

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	Project	12590 48" Face Run#2	Date	11:18:49 10/18/04
	Client	SAGA Communications	Designed by	M. Maurer

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	klf	
431.000			B	0.324	2.233	0.624	1	1	17.366			
			C	0.404	2.055	0.653	1	1	22.685			
T3 431.000-411.000	0.236	1.964	A	0.347	2.179	0.631	1	1	18.786	1.331	0.067	C
			B	0.393	2.077	0.649	1	1	21.905			
			C	0.423	2.02	0.661	1	1	24.026			
T4 411.000-391.000	0.253	1.964	A	0.347	2.179	0.631	1	1	18.786	1.397	0.070	C
			B	0.393	2.077	0.649	1	1	21.905			
			C	0.446	1.98	0.672	1	1	25.720			
T5 391.000-371.000	0.253	1.964	A	0.347	2.179	0.631	1	1	18.786	1.397	0.070	C
			B	0.393	2.077	0.649	1	1	21.905			
			C	0.446	1.98	0.672	1	1	25.720			
T6 371.000-351.000	0.253	2.169	A	0.364	2.14	0.637	1	1	19.904	1.301	0.065	C
			B	0.41	2.044	0.656	1	1	23.102			
			C	0.456	1.963	0.676	1	1	26.497			
T7 351.000-331.000	0.253	1.964	A	0.347	2.179	0.631	1	1	18.786	1.274	0.064	C
			B	0.393	2.077	0.649	1	1	21.905			
			C	0.446	1.98	0.672	1	1	25.720			
T8 331.000-311.000	0.282	1.964	A	0.347	2.179	0.631	1	1	18.786	1.383	0.069	C
			B	0.393	2.077	0.649	1	1	21.905			
			C	0.486	1.919	0.69	1	1	28.792			
T9 311.000-291.000	0.320	1.964	A	0.382	2.101	0.644	1	1	21.135	1.388	0.069	C
			B	0.393	2.077	0.649	1	1	21.905			
			C	0.488	1.916	0.691	1	1	28.959			
T10 291.000-271.000	0.363	1.964	A	0.425	2.016	0.662	1	1	24.178	1.388	0.069	C
			B	0.393	2.077	0.649	1	1	21.905			
			C	0.488	1.916	0.691	1	1	28.959			
T11 271.000-251.000	0.377	1.964	A	0.425	2.016	0.662	1	1	24.178	1.388	0.069	C
			B	0.412	2.041	0.657	1	1	23.224			
			C	0.488	1.916	0.691	1	1	28.959			
T12 251.000-231.000	0.401	2.085	A	0.443	1.985	0.67	1	1	25.465	1.231	0.062	C
		TA 1.578	B	0.453	1.969	0.674	1	1	26.196			
			C	0.5	1.9	0.698	1	1	29.950			
T13 231.000-211.000	0.426	1.964	A	0.425	2.016	0.662	1	1	24.178	1.308	0.065	C
			B	0.435	1.999	0.667	1	1	24.889			
			C	0.53	1.864	0.713	1	1	32.422			
T14 211.000-191.000	0.426	1.964	A	0.425	2.016	0.662	1	1	24.178	1.308	0.065	C
			B	0.435	1.999	0.667	1	1	24.889			
			C	0.53	1.864	0.713	1	1	32.422			
T15 191.000-171.000	0.693	1.964	A	0.425	2.016	0.662	1	1	24.178	1.482	0.074	B
			B	0.56	1.835	0.73	1	1	37.327			
			C	0.53	1.864	0.713	1	1	32.422			
T16 171.000-151.000	1.287	1.964	A	0.55	1.844	0.724	1	1	36.506	2.070	0.103	B
			B	0.712	1.777	0.828	1	1	53.824			
			C	0.53	1.864	0.713	1	1	32.422			
T17 151.000-131.000	1.881	2.108	A	0.71	1.777	0.827	1	1	53.683	2.545	0.127	B
			B	0.805	1.82	0.901	1	1	64.620			
			C	0.538	1.855	0.718	1	1	33.141			
T18 131.000-111.000	2.209	2.169	A	0.719	1.778	0.834	1	1	54.577	2.438	0.122	B
			B	0.918	1.951	1	1	1	78.802			
			C	0.54	1.853	0.719	1	1	33.310			
T19 111.000-91.000	2.209	2.108	A	0.71	1.777	0.827	1	1	53.683	2.386	0.119	B
			B	0.909	1.938	0.992	1	1	77.660			
			C	0.538	1.855	0.718	1	1	33.141			
T20 91.000-71.000	2.209	2.108	A	0.71	1.777	0.827	1	1	53.683	2.386	0.119	B
			B	0.909	1.938	0.992	1	1	77.660			
			C	0.538	1.855	0.718	1	1	33.141			
T21 71.000-51.000	2.209	2.359	A	0.717	1.778	0.832	1	1	54.568	2.429	0.121	B
			B	0.915	1.946	0.997	1	1	78.726			
			C	0.545	1.848	0.722	1	1	33.923			
T22 51.000-	2.209	2.359	A	0.717	1.778	0.832	1	1	54.568	2.429	0.121	B

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	<b>Project</b> 12590 48" Face Run#2		<b>Date</b> 11:18:49 10/18/04	
	<b>Client</b> SAGA Communications		<b>Designed by</b> M. Maurer	

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	klf	
31.000			B	0.915	1.946	0.997	1	1	78.726			
			C	0.545	1.848	0.722	1	1	33.923			
T23 31.000-11.000	2.209	2.359	A	0.717	1.778	0.832	1	1	54.568	2.429	0.121	B
			B	0.915	1.946	0.997	1	1	78.726			
			C	0.545	1.848	0.722	1	1	33.923			
T24 11.000-1.000	1.105	1.149	A	1	2.1	1	1	1	29.438	0.735*	0.074	B
			B	1	2.1	1	1	1	37.980			
			C	0.828	1.839	0.919	1	1	17.639			
Sum Weight:	22.482	55.579			*2A <sub>g</sub> limit					43.464		

### Tower Forces - With Ice - Wind 60 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	klf	
L1 528.000-512.205	0.024	1.252	A	0.371	2.125	0.64	0.8	1	12.354	0.846	0.054	C
			B	0.371	2.125	0.64	0.8	1	12.354			
			C	0.422	2.022	0.661	0.8	1	14.466			
L2 512.205-496.410	0.024	1.251	A	0.37	2.125	0.64	0.8	1	12.345	0.845	0.054	C
			B	0.37	2.125	0.64	0.8	1	12.345			
			C	0.422	2.023	0.661	0.8	1	14.456			
L3 496.410-480.615	0.024	1.244	A	0.369	2.129	0.639	0.8	1	12.202	0.838	0.053	C
			B	0.369	2.129	0.639	0.8	1	12.202			
			C	0.42	2.025	0.66	0.8	1	14.320			
L4 480.615-470.000	0.058	1.280	A	0.552	1.841	0.726	0.8	1	14.067	0.749	0.071	A
			B	0.449	1.975	0.673	0.8	1	10.819			
			C	0.5	1.9	0.698	0.8	1	12.358			
T1 470.000-451.000	0.119	2.473	A	0.395	2.073	0.65	0.8	1	21.587	1.371	0.072	C
			B	0.317	2.252	0.621	0.8	1	16.803			
			C	0.452	1.971	0.674	0.8	1	25.367			
T2 451.000-431.000	0.170	1.964	A	0.347	2.179	0.631	0.8	1	18.786	1.279	0.064	C
			B	0.324	2.233	0.624	0.8	1	17.366			
			C	0.404	2.055	0.653	0.8	1	22.685			
T3 431.000-411.000	0.236	1.964	A	0.347	2.179	0.631	0.8	1	18.786	1.331	0.067	C
			B	0.393	2.077	0.649	0.8	1	21.905			
			C	0.423	2.02	0.661	0.8	1	24.026			
T4 411.000-391.000	0.253	1.964	A	0.347	2.179	0.631	0.8	1	18.786	1.397	0.070	C
			B	0.393	2.077	0.649	0.8	1	21.905			
			C	0.446	1.98	0.672	0.8	1	25.720			
T5 391.000-371.000	0.253	1.964	A	0.347	2.179	0.631	0.8	1	18.786	1.397	0.070	C
			B	0.393	2.077	0.649	0.8	1	21.905			
			C	0.446	1.98	0.672	0.8	1	25.720			
T6 371.000-351.000	0.253	2.169	A	0.364	2.14	0.637	0.8	1	19.904	1.301	0.065	C
			B	0.41	2.044	0.656	0.8	1	23.102			
			C	0.456	1.963	0.676	0.8	1	26.497			
T7 351.000-331.000	0.253	1.964	A	0.347	2.179	0.631	0.8	1	18.786	1.274	0.064	C
			B	0.393	2.077	0.649	0.8	1	21.905			
			C	0.446	1.98	0.672	0.8	1	25.720			
T8 331.000-311.000	0.282	1.964	A	0.347	2.179	0.631	0.8	1	18.786	1.383	0.069	C
			B	0.393	2.077	0.649	0.8	1	21.905			
			C	0.486	1.919	0.69	0.8	1	28.792			
T9 311.000-291.000	0.320	1.964	A	0.382	2.101	0.644	0.8	1	21.135	1.388	0.069	C
			B	0.393	2.077	0.649	0.8	1	21.905			
			C	0.488	1.916	0.691	0.8	1	28.959			

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	Project	12590 48" Face Run#2	Date	11:18:49 10/18/04
	Client	SAGA Communications	Designed by	M. Maurer

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	klf	
T10 291.000-271.000	0.363	1.964	A	0.425	2.016	0.662	0.8	1	24.178	1.388	0.069	C
			B	0.393	2.077	0.649	0.8	1	21.905			
			C	0.488	1.916	0.691	0.8	1	28.959			
T11 271.000-251.000	0.377	1.964	A	0.425	2.016	0.662	0.8	1	24.178	1.388	0.069	C
			B	0.412	2.041	0.657	0.8	1	23.224			
			C	0.488	1.916	0.691	0.8	1	28.959			
T12 251.000-231.000	0.401	2.085	A	0.443	1.985	0.67	0.8	1	25.465	1.231	0.062	C
		TA 1.578	B	0.453	1.969	0.674	0.8	1	26.196			
			C	0.5	1.9	0.698	0.8	1	29.950			
T13 231.000-211.000	0.426	1.964	A	0.425	2.016	0.662	0.8	1	24.178	1.308	0.065	C
			B	0.435	1.999	0.667	0.8	1	24.889			
			C	0.53	1.864	0.713	0.8	1	32.422			
T14 211.000-191.000	0.426	1.964	A	0.425	2.016	0.662	0.8	1	24.178	1.308	0.065	C
			B	0.435	1.999	0.667	0.8	1	24.889			
			C	0.53	1.864	0.713	0.8	1	32.422			
T15 191.000-171.000	0.693	1.964	A	0.425	2.016	0.662	0.8	1	24.178	1.415	0.071	B
			B	0.56	1.835	0.73	0.8	1	35.639			
			C	0.53	1.864	0.713	0.8	1	32.422			
T16 171.000-151.000	1.287	1.964	A	0.55	1.844	0.724	0.8	1	34.818	1.925	0.096	B
			B	0.712	1.777	0.828	0.8	1	50.074			
			C	0.53	1.864	0.713	0.8	1	32.422			
T17 151.000-131.000	1.881	2.108	A	0.71	1.777	0.827	0.8	1	49.933	2.357	0.118	B
			B	0.805	1.82	0.901	0.8	1	59.858			
			C	0.538	1.855	0.718	0.8	1	33.141			
T18 131.000-111.000	2.209	2.169	A	0.719	1.778	0.834	0.8	1	50.827	2.252	0.113	B
			B	0.918	1.951	1	0.8	1	72.802			
			C	0.54	1.853	0.719	0.8	1	33.310			
T19 111.000-91.000	2.209	2.108	A	0.71	1.777	0.827	0.8	1	49.933	2.202	0.110	B
			B	0.909	1.938	0.992	0.8	1	71.660			
			C	0.538	1.855	0.718	0.8	1	33.141			
T20 91.000-71.000	2.209	2.108	A	0.71	1.777	0.827	0.8	1	49.933	2.202	0.110	B
			B	0.909	1.938	0.992	0.8	1	71.660			
			C	0.538	1.855	0.718	0.8	1	33.141			
T21 71.000-51.000	2.209	2.359	A	0.717	1.778	0.832	0.8	1	50.818	2.244	0.112	B
			B	0.915	1.946	0.997	0.8	1	72.726			
			C	0.545	1.848	0.722	0.8	1	33.923			
T22 51.000-31.000	2.209	2.359	A	0.717	1.778	0.832	0.8	1	50.818	2.244	0.112	B
			B	0.915	1.946	0.997	0.8	1	72.726			
			C	0.545	1.848	0.722	0.8	1	33.923			
T23 31.000-11.000	2.209	2.359	A	0.717	1.778	0.832	0.8	1	50.818	2.244	0.112	B
			B	0.915	1.946	0.997	0.8	1	72.726			
			C	0.545	1.848	0.722	0.8	1	33.923			
T24 11.000-1.000	1.105	1.149	A	1	2.1	1	0.8	1	27.563	0.735'	0.074	B
			B	1	2.1	1	0.8	1	34.980			
			C	0.828	1.839	0.919	0.8	1	17.639			
Sum Weight:	22.482	55.579			2A <sub>g</sub> limit					41.844		

### Tower Forces - With Ice - Wind 90 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	klf	
L1 528.000-512.205	0.024	1.252	A	0.371	2.125	0.64	0.85	1	12.408	0.849	0.054	C
			B	0.371	2.125	0.64	0.85	1	12.408			

<b>ERITowerBeta</b>  <b>Electronics Research Inc.</b> 7777 Gardner Road Chandler, IN Phone: 812-925-6000 FAX: 812-925-4026	Job	Portland, ME 531' Guyed Tower	Page	34 of 82
	Project	12590 48" Face Run#2	Date	11:18:49 10/18/04
	Client	SAGA Communications	Designed by	M. Maurer

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	klf	
L2 512.205-496.410	0.024	1.251	C	0.422	2.022	0.661	0.85	1	14.520	0.848	0.054	C
			A	0.37	2.125	0.64	0.85	1	12.399			
			B	0.37	2.125	0.64	0.85	1	12.399			
L3 496.410-480.615	0.024	1.244	C	0.422	2.023	0.661	0.85	1	14.510	0.840	0.053	C
			A	0.369	2.129	0.639	0.85	1	12.229			
			B	0.369	2.129	0.639	0.85	1	12.229			
L4 480.615-470.000	0.058	1.280	C	0.42	2.025	0.66	0.85	1	14.347	0.757	0.071	A
			A	0.552	1.841	0.726	0.85	1	14.225			
			B	0.449	1.975	0.673	0.85	1	10.977			
T1 470.000-451.000	0.119	2.473	C	0.5	1.9	0.698	0.85	1	12.517	1.383	0.073	C
			A	0.395	2.073	0.65	0.85	1	21.798			
			B	0.317	2.252	0.621	0.85	1	17.014			
T2 451.000-431.000	0.170	1.964	C	0.452	1.971	0.674	0.85	1	25.578	1.279	0.064	C
			A	0.347	2.179	0.631	0.85	1	18.786			
			B	0.324	2.233	0.624	0.85	1	17.366			
T3 431.000-411.000	0.236	1.964	C	0.404	2.055	0.653	0.85	1	22.685	1.331	0.067	C
			A	0.347	2.179	0.631	0.85	1	18.786			
			B	0.393	2.077	0.649	0.85	1	21.905			
T4 411.000-391.000	0.253	1.964	C	0.423	2.02	0.661	0.85	1	24.026	1.397	0.070	C
			A	0.347	2.179	0.631	0.85	1	18.786			
			B	0.393	2.077	0.649	0.85	1	21.905			
T5 391.000-371.000	0.253	1.964	C	0.446	1.98	0.672	0.85	1	25.720	1.397	0.070	C
			A	0.347	2.179	0.631	0.85	1	18.786			
			B	0.393	2.077	0.649	0.85	1	21.905			
T6 371.000-351.000	0.253	2.169	C	0.446	1.98	0.672	0.85	1	25.720	1.301	0.065	C
			A	0.364	2.14	0.637	0.85	1	19.904			
			B	0.41	2.044	0.656	0.85	1	23.102			
T7 351.000-331.000	0.253	1.964	C	0.456	1.963	0.676	0.85	1	26.497	1.274	0.064	C
			A	0.347	2.179	0.631	0.85	1	18.786			
			B	0.393	2.077	0.649	0.85	1	21.905			
T8 331.000-311.000	0.282	1.964	C	0.446	1.98	0.672	0.85	1	25.720	1.383	0.069	C
			A	0.347	2.179	0.631	0.85	1	18.786			
			B	0.393	2.077	0.649	0.85	1	21.905			
T9 311.000-291.000	0.320	1.964	C	0.486	1.919	0.69	0.85	1	28.792	1.388	0.069	C
			A	0.382	2.101	0.644	0.85	1	21.135			
			B	0.393	2.077	0.649	0.85	1	21.905			
T10 291.000-271.000	0.363	1.964	C	0.488	1.916	0.691	0.85	1	28.959	1.388	0.069	C
			A	0.425	2.016	0.662	0.85	1	24.178			
			B	0.393	2.077	0.649	0.85	1	21.905			
T11 271.000-251.000	0.377	1.964	C	0.488	1.916	0.691	0.85	1	28.959	1.388	0.069	C
			A	0.425	2.016	0.662	0.85	1	24.178			
			B	0.412	2.041	0.657	0.85	1	23.224			
T12 251.000-231.000	0.401	2.085 TA 1.578	C	0.488	1.916	0.691	0.85	1	28.959	1.231	0.062	C
			A	0.443	1.985	0.67	0.85	1	25.465			
			B	0.453	1.969	0.674	0.85	1	26.196			
T13 231.000-211.000	0.426	1.964	C	0.5	1.9	0.698	0.85	1	29.950	1.308	0.065	C
			A	0.425	2.016	0.662	0.85	1	24.178			
			B	0.435	1.999	0.667	0.85	1	24.889			
T14 211.000-191.000	0.426	1.964	C	0.53	1.864	0.713	0.85	1	32.422	1.308	0.065	C
			A	0.425	2.016	0.662	0.85	1	24.178			
			B	0.435	1.999	0.667	0.85	1	24.889			
T15 191.000-171.000	0.693	1.964	C	0.53	1.864	0.713	0.85	1	32.422	1.431	0.072	B
			A	0.425	2.016	0.662	0.85	1	24.178			
			B	0.56	1.835	0.73	0.85	1	36.061			
T16 171.000-151.000	1.287	1.964	C	0.53	1.864	0.713	0.85	1	32.422	1.962	0.098	B
			A	0.55	1.844	0.724	0.85	1	35.240			
			B	0.712	1.777	0.828	0.85	1	51.011			
T17 151.000-131.000	1.881	2.108	C	0.53	1.864	0.713	0.85	1	32.422	2.404	0.120	B
			A	0.71	1.777	0.827	0.85	1	50.871			
			B	0.805	1.82	0.901	0.85	1	61.048			

<b>ERITowerBeta</b>  Electronics Research Inc. 7777 Gardner Road Chandler, IN Phone: 812-925-6000 FAX: 812-925-4026	Job	Portland, ME 531' Guyed Tower	Page	35 of 82
	Project	12590 48" Face Run#2	Date	11:18:49 10/18/04
	Client	SAGA Communications	Designed by	M. Maurer

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	klf	
T18 131.000-111.000	2.209	2.169	C	0.538	1.855	0.718	0.85	1	33.141	2.299	0.115	B
			A	0.719	1.778	0.834	0.85	1	51.765			
			B	0.918	1.951	1	0.85	1	74.302			
T19 111.000-91.000	2.209	2.108	C	0.54	1.853	0.719	0.85	1	33.310	2.248	0.112	B
			A	0.71	1.777	0.827	0.85	1	50.871			
			B	0.909	1.938	0.992	0.85	1	73.160			
T20 91.000-71.000	2.209	2.108	C	0.538	1.855	0.718	0.85	1	33.141	2.248	0.112	B
			A	0.71	1.777	0.827	0.85	1	50.871			
			B	0.909	1.938	0.992	0.85	1	73.160			
T21 71.000-51.000	2.209	2.359	C	0.538	1.855	0.718	0.85	1	33.141	2.290	0.115	B
			A	0.717	1.778	0.832	0.85	1	51.755			
			B	0.915	1.946	0.997	0.85	1	74.226			
T22 51.000-31.000	2.209	2.359	C	0.545	1.848	0.722	0.85	1	33.923	2.290	0.115	B
			A	0.717	1.778	0.832	0.85	1	51.755			
			B	0.915	1.946	0.997	0.85	1	74.226			
T23 31.000-11.000	2.209	2.359	C	0.545	1.848	0.722	0.85	1	33.923	2.290	0.115	B
			A	0.717	1.778	0.832	0.85	1	51.755			
			B	0.915	1.946	0.997	0.85	1	74.226			
T24 11.000-1.000	1.105	1.149	C	0.828	1.839	0.919	0.85	1	17.639	0.735'	0.074	B
			A	1	2.1	1	0.85	1	28.032			
			B	1	2.1	1	0.85	1	35.730			
Sum Weight:	22.482	55.579								42.249		

**Discrete Appurtenance Pressures - No Ice**  $G_H = 1.061$

Description	Aiming Azimuth °	Weight K	Offset <sub>x</sub> ft	Offset <sub>y</sub> ft	z ft	K <sub>r</sub>	q <sub>r</sub> psf	C <sub>A</sub> A <sub>c</sub> Front ft <sup>2</sup>	C <sub>A</sub> A <sub>c</sub> Side ft <sup>2</sup>
Torque Arm Face C	180.0000	0.000	0.000	4.330	241.000	1.765	28.916	6.998	17.266
Torque Arm Face B	60.0000	0.000	3.750	-2.165	241.000	1.765	28.916	6.998	17.266
Torque Arm Face A	300.0000	0.000	-3.750	-2.165	241.000	1.765	28.916	6.998	17.266
Red A-2/3 lighting Kit w/ A-3 Spur	0.0000	0.175	0.000	0.000	528.000	2.208	36.179	12.000	12.000
Mid Beacon Level	0.0000	0.200	0.000	0.000	265.000	1.813	29.711	6.000	6.000
ERI---A-3 Lightning Spur	0.0000	0.150	0.000	0.000	265.000	1.813	29.711	8.000	8.000
SHPX-5AE-Radomes (3" Coax)	240.0000	0.810	-3.665	2.116	501.650	2.176	35.653	48.000	48.000
(3) DB224 w/ Long Arm Mounts (7/8" Coax)	0.0000	0.630	0.000	0.000	440.000	2.096	34.342	27.000	27.000
Ice Shield (4' x 6')	0.0000	0.250	0.000	-2.809	329.200	1.929	31.611	10.000	10.000
DCR-C 4 Bay w/ domes (3" Coax)	120.0000	0.425	4.165	2.405	300.000	1.879	30.783	50.000	50.000
(12) 5' x 1' Panels (1 5/8" Coax)	0.0000	2.400	0.000	0.000	180.000	1.624	26.602	109.000	109.000
(12) 5' x 1' Panels (1 5/8" Coax)	0.0000	2.400	0.000	0.000	160.000	1.570	25.722	109.000	109.000
(12) 5' x 1' Panels (1 5/8" Coax)	0.0000	2.400	0.000	0.000	140.000	1.511	24.759	109.000	109.000
Sum Weight:		9.840							

<b>ERITowerBeta</b>  <b>Electronics Research Inc.</b> 7777 Gardner Road Chandler, IN Phone: 812-925-6000 FAX: 812-925-4026	Job	Portland, ME 531' Guyed Tower	Page	36 of 82
	Project	12590 48" Face Run#2	Date	11:18:49 10/18/04
	Client	SAGA Communications	Designed by	M. Maurer

**Discrete Appurtenance Pressures - With Ice**  $G_H = 1.061$

Description	Aiming Azimuth °	Weight K	Offset <sub>x</sub> ft	Offset <sub>y</sub> ft	z ft	K <sub>z</sub>	q <sub>z</sub> psf	C <sub>Ac</sub> Front ft <sup>2</sup>	C <sub>Ac</sub> Side ft <sup>2</sup>	t <sub>z</sub> in
Torque Arm Face C	180.0000	0.000	0.000	4.330	241.000	1.765	22.139	7.898	19.069	0.500
Torque Arm Face B	60.0000	0.000	3.750	-2.165	241.000	1.765	22.139	7.898	19.069	0.500
Torque Arm Face A	300.0000	0.000	-3.750	-2.165	241.000	1.765	22.139	7.898	19.069	0.500
Red A-2/3 lighting Kit w/ A-3 Spur	0.0000	0.000	0.000	0.000	528.000	2.208	27.699	17.000	17.000	0.500
Mid Beacon Level	0.0000	0.250	0.000	0.000	265.000	1.813	22.747	8.000	8.000	0.500
ERI---A-3 Lightning Spur	0.0000	0.225	0.000	0.000	265.000	1.813	22.747	13.000	13.000	0.500
SHPX-5AE-Radomes (3" Coax)	240.0000	1.585	-3.665	2.116	501.650	2.176	27.297	56.000	56.000	0.500
(3) DB224 w/ Long Arm Mounts (7/8" Coax)	0.0000	0.885	0.000	0.000	440.000	2.096	26.293	39.000	39.000	0.500
Ice Shield (4' x 6')	0.0000	0.350	0.000	-2.809	329.200	1.929	24.202	12.000	12.000	0.500
DCR-C 4 Bay w/ domes (3" Coax)	120.0000	0.565	4.165	2.405	300.000	1.879	23.568	67.000	67.000	0.500
(12) 5' x 1' Panels (1 5/8" Coax)	0.0000	3.600	0.000	0.000	180.000	1.624	20.368	128.000	128.000	0.500
(12) 5' x 1' Panels (1 5/8" Coax)	0.0000	3.600	0.000	0.000	160.000	1.570	19.694	128.000	128.000	0.500
(12) 5' x 1' Panels (1 5/8" Coax)	0.0000	3.600	0.000	0.000	140.000	1.511	18.956	128.000	128.000	0.500
Sum Weight:		14.660								

**Dish Pressures - No Ice**

Elevation ft	Dish Description	Aiming Azimuth °	Weight K	Offset <sub>x</sub> ft	Offset <sub>y</sub> ft	K <sub>z</sub>	A <sub>d</sub> ft <sup>2</sup>	q <sub>z</sub> psf
420.000	6' Grid (7/8" Coax)	120.0000	0.130	2.866	1.655	2.068	28.274	33.889
330.000	4' Grid (7/8" Coax)	0.0000	0.100	0.000	-3.309	1.931	12.566	31.633
260.000	4' Grid (7/8" Coax)	240.0000	0.100	-2.866	1.655	1.804	12.566	29.550
235.000	4' Grid (7/8" Coax)	120.0000	0.100	2.866	1.655	1.752	12.566	28.708
	Sum Weight:		0.430					

**Dish Pressures - With Ice**

Elevation ft	Dish Description	Aiming Azimuth °	Weight K	Offset <sub>x</sub> ft	Offset <sub>y</sub> ft	K <sub>z</sub>	A <sub>d</sub> ft <sup>2</sup>	q <sub>z</sub> psf	t <sub>z</sub> in
420.000	6' Grid (7/8" Coax)	120.0000	0.275	2.866	1.655	2.068	29.065	25.946	0.500
330.000	4' Grid (7/8" Coax)	0.0000	0.175	0.000	-3.309	1.931	13.095	24.219	0.500
260.000	4' Grid (7/8" Coax)	240.0000	0.175	-2.866	1.655	1.804	13.095	22.624	0.500
235.000	4' Grid (7/8" Coax)	120.0000	0.175	2.866	1.655	1.752	13.095	21.980	0.500
	Sum Weight:		0.800						

**Force Totals (Does not include forces on guys)**

<b>ERITowerBeta</b>  <b>Electronics Research Inc.</b> 7777 Gardner Road Chandler, IN Phone: 812-925-6000 FAX: 812-925-4026	Job	Portland, ME 531' Guyed Tower	Page	37 of 82
	Project	12590 48" Face Run#2	Date	11:18:49 10/18/04
	Client	SAGA Communications	Designed by	M. Maurer

Load Case	Vertical Forces	Sum of Forces	Sum of Forces	Sum of Torques
	K	X K	Z K	kip-ft
Leg Weight	30.031			
Bracing Weight	16.585			
Total Member Self-Weight	46.616			
Guy Weight	9.366			
Total Weight	75.503			
Wind 0 deg - No Ice		0.000	-49.978	1.857
Wind 90 deg - No Ice		49.864	0.000	7.758
Wind 180 deg - No Ice		0.000	49.826	-1.857
Member Ice	8.963			
Guy Ice	5.025			
Total Weight Ice	107.912			
Wind 0 deg - Ice		0.000	-61.439	4.806
Wind 90 deg - Ice		60.225	0.000	8.649
Wind 180 deg - Ice		0.000	59.820	-4.806

### Load Combinations

Comb. No.	Description
1	Dead Only
2	Dead+Wind 0 deg - No Ice+Guy
3	Dead+Wind 90 deg - No Ice+Guy
4	Dead+Wind 180 deg - No Ice+Guy
5	Dead+Ice+Temp+Guy
6	Dead+Wind 0 deg+Ice+Temp+Guy
7	Dead+Wind 90 deg+Ice+Temp+Guy
8	Dead+Wind 180 deg+Ice+Temp+Guy

### Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft	
L1	528 - 512.205	Latticed Pole Leg	Max Tension	8	6.257	-0.025	0.055	
			Max. Compression	6	-6.754	0.026	-0.056	
			Max. Mx	7	4.380	0.056	-0.002	
			Max. My	6	-6.754	0.026	-0.056	
			Max. Vy	7	0.806	-0.012	-0.006	
			Max. Vx	6	-0.862	0.002	0.016	
			Latticed Pole Diagonal	Max Tension	8	0.853	0.000	0.000
				Max. Compression	6	-0.862	0.000	0.000
				Max. Mx	3	0.752	-0.002	0.000
				Max. Vy	7	0.003	-0.002	0.000
			Latticed Pole K-Brace	Max Tension	8	0.975	0.000	0.000
				Max. Compression	6	-0.950	0.000	0.000
				Max. Mx	6	0.160	0.001	0.000
			Latticed Pole Top Girt	Max. Vy	6	0.002	0.000	0.000
Max Tension	8	0.196		-0.001	-0.000			



<b>ERITowerBeta</b>  <b>Electronics Research Inc.</b> 7777 Gardner Road Chandler, IN Phone: 812-925-6000 FAX: 812-925-4026	Job	Portland, ME    531' Guyed Tower	Page	38 of 82
	Project	12590    48" Face    Run#2	Date	11:18:49 10/18/04
	Client	SAGA Communications	Designed by	M. Maurer

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L2	512.205 - 496.41	Latticed Pole Bottom Girt	Max. Compression	6	-0.195	-0.001	0.000
			Max. Mx	8	-0.100	-0.001	0.000
			Max. Vy	8	0.004	-0.001	0.000
			Max Tension	6	0.867	0.000	0.000
		Latticed Pole Mid Girt	Max. Compression	8	-0.871	0.000	0.000
			Max. Mx	6	0.270	-0.001	-0.000
			Max. Vy	6	0.004	-0.001	-0.000
			Max Tension	6	0.037	-0.001	0.000
		Latticed Pole Leg	Max. Compression	8	-0.034	-0.001	-0.000
			Max. Mx	6	-0.017	-0.001	-0.000
			Max. Vy	6	0.003	-0.001	-0.000
			Max Tension	8	21.375	-0.061	0.072
		Latticed Pole Diagonal	Max. Compression	6	-22.300	0.064	-0.073
			Max. Mx	7	15.894	0.087	0.013
			Max. My	2	7.854	-0.017	-0.104
			Max. Vy	7	1.541	-0.046	-0.022
			Max. Vx	6	-1.581	0.003	0.058
			Max Tension	4	1.886	0.000	0.000
		Latticed Pole K-Brace	Max. Compression	6	-1.897	0.000	0.000
			Max. Mx	3	1.623	-0.002	0.000
			Max. My	4	-1.848	-0.001	0.001
			Max. Vy	7	0.003	-0.002	0.000
			Max. Vx	4	-0.001	-0.001	0.001
			Max Tension	6	2.019	0.000	0.000
Latticed Pole Top Girt	Max. Compression	6	-1.995	0.000	0.000		
	Max. Mx	6	0.232	0.001	0.000		
	Max. Vy	6	0.002	0.000	0.000		
	Max Tension	8	0.926	-0.001	-0.000		
Latticed Pole Bottom Girt	Max. Compression	6	-0.913	-0.001	-0.000		
	Max. Mx	8	0.268	-0.002	0.000		
	Max. Vy	8	0.004	-0.002	0.000		
	Max Tension	6	1.820	0.000	0.000		
Latticed Pole Mid Girt	Max. Compression	4	-1.811	0.000	0.000		
	Max. Mx	6	0.687	-0.002	-0.000		
	Max. My	4	1.730	-0.001	-0.001		
	Max. Vy	6	0.004	-0.002	-0.000		
Latticed Pole Leg	Max Tension	6	0.098	-0.001	-0.000		
	Max. Compression	8	-0.088	-0.001	-0.001		
	Max. Mx	6	-0.069	-0.001	-0.000		
	Max. My	4	-0.083	-0.001	-0.001		
Latticed Pole Diagonal	Max. Vy	6	0.003	-0.001	-0.000		
	Max Tension	8	45.405	0.001	-0.124		
	Max. Compression	6	-46.864	0.046	-0.005		
	Max. Mx	7	-19.442	-0.176	-0.026		
	Max. My	6	-22.299	-0.059	0.190		
	Max. Vy	7	2.276	-0.099	-0.046		
Latticed Pole Diagonal	Max. Vx	6	-2.308	0.002	0.125		
	Max Tension	4	2.947	0.000	0.000		
	Max. Compression	6	-2.947	0.000	0.000		
	Max. Mx	7	2.570	-0.003	0.000		
L3	496.41 - 480.615	Latticed Pole Leg	Max. My	4	-2.697	0.000	0.002
			Max. Vy	4	0.000	0.000	0.000

<b>ERITowerBeta</b>  <i>Electronics Research Inc.</i> 7777 Gardner Road Chandler, IN Phone: 812-925-6000 FAX: 812-925-4026	Job	Portland, ME 531' Guyed Tower	Page	39 of 82
	Project	12590 48" Face Run#2	Date	11:18:49 10/18/04
	Client	SAGA Communications	Designed by	M. Maurer

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
			Max. Vy	7	0.003	-0.003	0.000
			Max. Vx	4	-0.001	0.000	0.002
		Latticed Pole K-Brace	Max Tension	6	3.031	0.000	0.000
			Max. Compression	2	-3.010	0.000	0.000
			Max. Mx	6	0.162	0.001	0.000
			Max. Vy	6	0.002	0.000	0.000
		Latticed Pole Top Girt	Max Tension	8	1.886	-0.000	-0.001
			Max. Compression	6	-1.881	-0.002	-0.001
			Max. Mx	8	-0.821	-0.003	-0.000
			Max. My	2	-1.866	-0.002	-0.001
			Max. Vy	8	-0.005	-0.003	-0.000
			Max. Vx	2	0.001	-0.002	-0.001
		Latticed Pole Bottom Girt	Max Tension	6	2.742	0.000	0.000
			Max. Compression	4	-2.720	0.000	0.000
			Max. Mx	6	1.085	-0.001	-0.001
			Max. My	4	2.601	-0.001	-0.002
			Max. Vy	6	0.003	-0.001	-0.001
			Max. Vx	4	0.001	0.000	0.000
		Latticed Pole Mid Girt	Max Tension	6	0.197	-0.001	-0.001
			Max. Compression	8	-0.188	-0.001	-0.002
			Max. Mx	6	-0.163	-0.001	-0.001
			Max. My	4	-0.176	-0.001	-0.002
			Max. Vy	6	0.003	-0.001	-0.001
			Max. Vx	4	0.001	0.000	0.000
L4	480.615 - 470	Latticed Pole Leg	Max Tension	8	65.856	0.004	-0.043
			Max. Compression	6	-68.426	0.000	0.000
			Max. Mx	7	-40.759	-0.290	-0.048
			Max. My	6	-46.861	-0.097	0.317
			Max. Vy	7	2.640	0.000	0.000
			Max. Vx	8	3.055	0.000	0.000
		Latticed Pole Diagonal	Max Tension	4	3.179	0.000	0.000
			Max. Compression	6	-3.210	0.000	0.000
			Max. Mx	7	0.228	-0.003	0.001
			Max. My	4	-3.007	0.001	0.003
			Max. Vy	7	0.004	-0.003	0.001
			Max. Vx	4	-0.002	0.001	0.003
		Latticed Pole K-Brace	Max Tension	8	3.602	0.000	0.000
			Max. Compression	6	-3.601	0.000	0.000
			Max. Mx	6	0.181	0.001	0.000
			Max. Vy	6	-0.002	0.000	0.000
		Latticed Pole Top Girt	Max Tension	4	2.809	-0.000	-0.002
			Max. Compression	6	-2.826	-0.001	-0.002
			Max. Mx	8	-1.270	-0.002	-0.001
			Max. My	2	-2.818	-0.001	-0.002
			Max. Vy	8	-0.003	-0.002	-0.001
			Max. Vx	2	0.001	-0.001	-0.002
		Latticed Pole Bottom Girt	Max Tension	4	3.386	0.223	-0.002
			Max. Compression	8	-3.961	0.000	0.000
			Max. Mx	8	3.355	0.240	-0.002
			Max. My	8	3.355	0.240	-0.002
			Max. Vy	8	0.199	0.000	0.000
			Max. Vx	8	-0.001	0.000	0.000
		Latticed Pole Mid	Max Tension	7	0.266	-0.001	0.000

<b>ERITowerBeta</b>  <b>Electronics Research Inc.</b> 7777 Gardner Road Chandler, IN Phone: 812-925-6000 FAX: 812-925-4026	<b>Job</b> Portland, ME      531' Guyed Tower		<b>Page</b> 40 of 82
	<b>Project</b> 12590      48" Face      Run#2		<b>Date</b> 11:18:49 10/18/04
	<b>Client</b> SAGA Communications		<b>Designed by</b> M. Maurer

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
		Girt	Max. Compression	8	-0.272	-0.001	0.000
			Max. Mx	6	-0.219	-0.001	-0.001
			Max. My	4	-0.235	-0.001	-0.002
			Max. Vy	6	0.003	-0.001	-0.001
			Max. Vx	4	0.002	0.000	0.000
T1	470 - 451	Leg	Max Tension	8	48.786	-0.000	-0.000
			Max. Compression	6	-55.900	-0.015	0.012
			Max. Mx	7	38.905	-0.542	0.151
			Max. My	8	15.206	0.078	-0.604
			Max. Vy	7	6.500	-0.542	0.151
			Max. Vx	8	7.251	0.078	-0.604
		Diagonal	Max Tension	8	2.746	0.000	0.000
			Max. Compression	8	-2.700	0.000	0.000
			Max. Mx	7	-1.204	-0.003	0.000
			Max. My	7	1.165	-0.002	0.017
			Max. Vy	7	0.004	-0.003	0.000
			Max. Vx	7	-0.007	-0.002	0.017
		K-Brace	Max Tension	8	2.655	0.000	0.000
			Max. Compression	8	-2.110	0.000	0.000
			Max. Mx	6	-2.073	0.001	0.000
			Max. Vy	6	-0.003	0.000	0.000
		Horizontal	Max Tension	8	0.858	0.000	0.000
			Max. Compression	1	0.000	0.000	0.000
			Max. Mx	6	0.217	0.005	0.000
			Max. Vy	6	-0.005	0.000	0.000
		Secondary Horizontal	Max Tension	7	0.000	0.000	0.000
			Max. Compression	3	-0.000	0.000	0.000
			Max. Mx	6	0.000	0.001	0.000
			Max. Vy	7	0.026	0.000	0.000
			Max. Vx	7	-0.024	0.000	0.000
		Top Girt	Max Tension	6	9.824	0.156	0.000
			Max. Compression	2	-0.159	0.000	0.000
			Max. Mx	8	4.689	0.348	-0.000
			Max. My	6	4.171	0.318	-0.002
			Max. Vy	8	-0.226	0.000	0.000
			Max. Vx	6	0.002	0.000	0.000
		Bottom Girt	Max Tension	8	1.995	-0.002	-0.001
			Max. Compression	8	-1.854	-0.002	-0.001
			Max. Mx	7	-0.784	-0.002	-0.000
			Max. My	8	1.995	-0.002	-0.001
			Max. Vy	7	0.005	-0.002	-0.000
		Guy A	Bottom Tension	8	36.890		
			Top Tension	8	38.271		
			Top Cable Vert	8	32.964		
			Top Cable Norm	8	19.443		
			Top Cable Tan	8	0.003		
			Bot Cable Vert	8	-30.339		
			Bot Cable Norm	8	20.986		
			Bot Cable Tan	8	0.003		
		Guy B	Bottom Tension	6	29.020		
			Top Tension	6	30.403		
			Top Cable Vert	6	26.173		
			Top Cable Norm	6	15.441		
			Top Cable Tan	6	0.938		
			Bot Cable Vert	6	-23.962		
			Bot Cable Norm	6	16.325		
			Bot Cable Tan	6	1.203		
		Guy C	Bottom Tension	7	35.701		
			Top Tension	7	37.134		

<b>ERITowerBeta</b>  <b>Electronics Research Inc.</b> 7777 Gardner Road Chandler, IN Phone: 812-925-6000 FAX: 812-925-4026	Job	Portland, ME 531' Guyed Tower	Page	41 of 82
	Project	12590 48" Face Run#2	Date	11:18:49 10/18/04
	Client	SAGA Communications	Designed by	M. Maurer

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft	
T2	451 - 431	Index Plate	Top Cable Vert	7	32.301			
			Top Cable Norm	7	18.313			
			Top Cable Tan	7	0.475			
			Bot Cable Vert	7	-29.712			
			Bot Cable Norm	7	19.780			
			Bot Cable Tan	7	0.686			
			Max Tension	8	7.256	-28.164	0.591	
			Max. Compression	1	0.000	0.000	0.000	
			Max. Mx	6	3.820	29.952	-0.525	
			Max. My	8	1.674	1.291	-4.560	
		Max. Vy	6	51.876	29.951	-0.532		
		Max. Vx	8	-2.784	1.291	-4.560		
		Leg	Max Tension	1	0.000	0.000	0.000	
			Max. Compression	2	-35.316	-0.021	-0.012	
			Max. Mx	7	-20.172	0.185	0.015	
			Max. My	8	-8.486	0.087	0.185	
			Max. Vy	7	-1.281	-0.050	-0.056	
			Max. Vx	8	-1.759	0.019	0.038	
			Diagonal	Max Tension	8	2.233	0.000	0.000
				Max. Compression	8	-2.246	0.000	0.000
				Max. Mx	7	-0.871	-0.002	0.000
				Max. My	7	0.692	-0.002	0.017
		Max. Vy		7	0.003	-0.002	0.000	
		K-Brace	Max. Vx	7	-0.007	-0.002	0.017	
			Max Tension	8	2.377	0.000	0.000	
			Max. Compression	8	-2.405	0.000	0.000	
			Max. Mx	6	-0.134	0.002	0.000	
			Max. Vy	6	-0.003	0.000	0.000	
		Horizontal	Max Tension	8	0.625	0.000	0.000	
			Max. Compression	1	0.000	0.000	0.000	
			Max. Mx	6	0.555	0.005	0.000	
		Secondary Horizontal	Max. Vy	6	-0.005	0.000	0.000	
			Max Tension	7	0.000	0.000	0.000	
			Max. Compression	3	-0.000	0.000	0.000	
		Top Girt	Max. Mx	6	0.000	0.001	0.000	
			Max. Vy	7	0.023	0.000	0.000	
			Max. Vx	7	-0.022	0.000	0.000	
			Max Tension	8	1.947	0.000	0.000	
			Max. Compression	8	-1.952	0.000	0.000	
			Max. Mx	8	-0.822	-0.003	-0.000	
Max. My	8		-1.952	-0.002	-0.001			
Max. Vy	8		-0.006	-0.003	-0.000			
Max. Vx	8		0.001	0.000	0.000			
Bottom Girt	Max Tension		8	1.287	-0.001	-0.001		
	Max. Compression	8	-1.186	-0.001	-0.001			
	Max. Mx	7	0.126	0.002	0.000			
	Max. My	8	1.287	-0.001	-0.001			
	Max. Vy	5	-0.005	-0.001	-0.000			
Leg	Max Tension	1	0.000	0.000	0.000			
	Max. Compression	2	-31.615	-0.001	0.092			
	Max. Mx	7	-20.565	0.296	0.037			
	Max. My	8	-19.491	0.042	0.461			
	Max. Vy	7	0.972	-0.021	-0.042			
	Max. Vx	8	-1.029	-0.019	-0.039			
	Diagonal	Max Tension	8	1.377	0.000	0.000		
		Max. Compression	7	-1.629	0.000	0.000		
		Max. Mx	7	0.549	-0.002	0.000		
		Max. My	7	-0.276	-0.002	0.016		
Max. Vy		7	0.003	-0.002	0.000			
T3	431 - 411	Leg	Max. Vx	7	-0.007	-0.002	0.016	

<b>ERITowerBeta</b>  <i>Electronics Research Inc.</i> 7777 Gardner Road Chandler, IN Phone: 812-925-6000 FAX: 812-925-4026	Job	Portland, ME    531' Guyed Tower	Page	42 of 82
	Project	12590    48" Face    Run#2	Date	11:18:49 10/18/04
	Client	SAGA Communications	Designed by	M. Maurer

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft		
T4	411 - 391	K-Brace	Max Tension	8	1.474	0.000	0.000		
			Max. Compression	8	-1.502	0.000	0.000		
		Horizontal	Max. Mx	6	1.044	0.002	0.000	0.000	
			Max. Vy	6	-0.003	0.000	0.000	0.000	
			Max Tension	6	0.697	0.000	0.000	0.000	
			Max. Compression	1	0.000	0.000	0.000	0.000	
			Max. Mx	5	0.217	0.005	0.000	0.000	
			Max. Vy	5	-0.005	0.000	0.000	0.000	
		Secondary Horizontal	Max Tension	7	0.000	0.000	0.000	0.000	
			Max. Compression	3	-0.000	0.000	0.000	0.000	
		Top Girt	Max. Mx	6	0.000	0.001	0.000	0.000	
			Max. Vy	7	0.024	0.000	0.000	0.000	
			Max. Vx	7	-0.020	0.000	0.000	0.000	
			Max Tension	8	1.239	0.000	0.000	0.000	
		Bottom Girt	Max. Compression	8	-1.187	0.000	0.000	0.000	
			Max. Mx	7	-0.106	-0.003	0.000	0.000	
			Max. My	8	-1.187	-0.003	-0.001	0.000	
			Max. Vy	7	0.006	-0.003	0.000	0.000	
			Max Tension	7	1.132	0.000	0.000	0.000	
			Max. Compression	7	-1.127	0.000	0.000	0.000	
		Leg	Max. Mx	7	-1.127	0.002	-0.000	-0.000	
			Max. Vy	5	-0.005	-0.001	-0.000	-0.000	
			Max Tension	1	0.000	0.000	0.000	0.000	
			Max. Compression	2	-42.729	-0.001	0.134	0.000	
			Max. Mx	7	-14.735	0.118	-0.039	0.000	
			Max. My	2	-31.616	-0.052	0.146	0.000	
			Max. Vy	7	1.538	-0.064	-0.064	0.000	
			Max. Vx	6	-1.589	0.000	0.126	0.000	
			Diagonal	Max Tension	2	1.488	0.000	0.000	0.000
				Max. Compression	7	-2.213	0.000	0.000	0.000
			K-Brace	Max. Mx	6	1.368	-0.002	-0.000	0.000
				Max. My	7	-1.775	-0.002	0.015	0.000
		Max. Vy		6	0.003	-0.002	-0.000	0.000	
		Max. Vx		7	-0.006	-0.002	0.015	0.000	
		Horizontal	Max Tension	7	1.944	0.000	0.000	0.000	
			Max. Compression	7	-1.918	0.000	0.000	0.000	
			Max. Mx	6	1.037	0.002	0.000	0.000	
		Secondary Horizontal	Max. Vy	6	-0.003	0.000	0.000	0.000	
			Max Tension	8	0.673	0.000	0.000	0.000	
			Max. Compression	1	0.000	0.000	0.000	0.000	
		Top Girt	Max. Mx	5	0.226	0.005	0.000	0.000	
			Max. Vy	5	-0.005	0.000	0.000	0.000	
Max Tension	7		0.000	0.000	0.000	0.000			
Max. Compression	3		-0.000	0.000	0.000	0.000			
Max. Mx	6		0.000	0.001	0.000	0.000			
Max. Vy	7		0.024	0.000	0.000	0.000			
Bottom Girt	Max. Vx	7	-0.018	0.000	0.000	0.000			
	Max Tension	7	1.152	-0.003	-0.000	0.000			
	Max. Compression	7	-1.151	-0.003	-0.000	0.000			
	Max. Mx	7	-1.151	-0.003	-0.000	0.000			
Leg	Max. Vy	7	-0.006	-0.003	-0.000	0.000			
	Max Tension	7	1.578	0.000	0.000	0.000			
	Max. Compression	7	-1.571	0.000	0.000	0.000			
	Max. Mx	8	0.328	0.002	0.000	0.000			
	Max. My	4	1.204	-0.001	-0.001	0.000			
	Max. Vy	5	-0.005	-0.001	-0.000	0.000			
T5	391 - 371	Leg	Max Tension	4	10.324	-0.016	-0.016		
			Max. Compression	6	-65.453	-0.009	0.248		
			Max. Mx	7	-37.382	-0.192	-0.064		

<b>ERITowerBeta</b>  <i>Electronics Research Inc.</i> 7777 Gardner Road Chandler, IN Phone: 812-925-6000 FAX: 812-925-4026	Job	Portland, ME 531' Guyed Tower	Page 43 of 82
	Project	12590 48" Face Run#2	Date 11:18:49 10/18/04
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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T6	371 - 351	Diagonal	Max. My	6	-42.351	-0.023	0.259
			Max. Vy	7	2.070	-0.167	-0.088
			Max. Vx	6	-2.234	-0.009	0.248
			Max Tension	7	1.924	0.000	0.000
			Max. Compression	7	-2.768	0.000	0.000
			Max. Mx	6	1.849	-0.003	0.000
			Max. My	7	-2.333	-0.002	0.013
			Max. Vy	6	0.004	-0.003	0.000
			Max. Vx	7	-0.005	-0.002	0.013
			Max Tension	7	2.420	0.000	0.000
			Max. Compression	7	-2.393	0.000	0.000
			Max. Mx	6	1.582	0.002	0.000
		K-Brace	Max. Vy	6	-0.003	0.000	0.000
			Max. Vx	7	0.917	0.000	0.000
			Max Tension	8	0.000	0.000	0.000
			Max. Compression	1	0.000	0.000	0.000
			Max. Mx	5	0.233	0.005	0.000
			Max. Vy	5	-0.005	0.000	0.000
		Horizontal	Max Tension	7	0.000	0.000	0.000
			Max. Compression	3	-0.000	0.000	0.000
			Max. Mx	6	0.000	0.002	0.000
		Secondary Horizontal	Max. Vy	7	0.023	0.000	0.000
			Max. Vx	7	-0.015	0.000	0.000
			Max Tension	7	1.629	-0.003	-0.001
			Max. Compression	7	-1.626	-0.003	-0.001
		Top Girt	Max. Mx	8	0.260	-0.003	0.000
			Max. My	2	-1.353	-0.002	-0.001
			Max. Vy	8	0.006	-0.003	0.000
			Max Tension	7	1.966	0.000	0.000
			Max. Compression	7	-1.958	0.000	0.000
			Max. Mx	8	0.360	0.002	0.000
		Bottom Girt	Max. My	4	1.461	-0.001	-0.001
			Max. Vy	6	0.005	-0.002	-0.000
			Max Tension	4	17.666	0.002	0.052
			Max. Compression	6	-79.301	0.001	0.010
			Max. Mx	7	-57.049	-0.340	-0.103
			Max. My	6	-65.454	-0.034	0.434
		Diagonal	Max. Vy	7	2.070	-0.340	-0.103
			Max. Vx	8	-2.285	-0.039	-0.202
			Max Tension	7	2.554	0.000	0.000
			Max. Compression	7	-3.020	0.000	0.000
			Max. Mx	7	-1.872	-0.005	-0.009
Max. My	7		-2.986	-0.002	0.013		
K-Brace	Max. Vy	7	0.005	-0.005	-0.009		
	Max. Vx	7	-0.005	-0.002	0.013		
	Max Tension	7	2.563	0.000	0.000		
	Max. Compression	7	-2.596	0.000	0.000		
	Max. Mx	6	2.131	0.002	0.000		
	Max. Vy	6	-0.003	0.000	0.000		
Horizontal	Max Tension	8	1.473	0.000	0.000		
	Max. Compression	6	-0.469	0.000	0.000		
	Max. Mx	5	0.422	0.005	0.000		
Secondary Horizontal	Max. Vy	5	-0.005	0.000	0.000		
	Max Tension	7	0.000	0.000	0.000		
	Max. Compression	3	-0.000	0.000	0.000		
Top Girt	Max. Mx	6	0.000	0.002	0.000		
	Max. Vy	7	0.019	0.000	0.000		
	Max. Vx	7	-0.016	0.000	0.000		
	Max Tension	7	2.108	-0.003	-0.001		
	Max. Compression	7	-2.098	-0.003	-0.001		

<b>ERITowerBeta</b>  <i>Electronics Research Inc.</i> 7777 Gardner Road Chandler, IN Phone: 812-925-6000 FAX: 812-925-4026	Job	Portland, ME 531' Guyed Tower	Page	44 of 82
	Project	12590 48" Face Run#2	Date	11:18:49 10/18/04
	Client	SAGA Communications	Designed by	M. Maurer

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft		
T7	351 - 331	Bottom Girt	Max. Mx	8	0.283	-0.004	0.000		
			Max. My	7	2.108	-0.003	-0.001		
			Max. Vy	8	0.006	-0.004	0.000		
			Max Tension	8	1.893	-0.001	-0.000		
			Max. Compression	8	-1.777	-0.001	-0.000		
			Max. Mx	8	-0.413	0.002	-0.000		
			Max. My	2	1.856	-0.001	-0.001		
			Max. Vy	5	0.005	-0.001	-0.000		
			Guy A	Bottom Tension	8	27.221			
				Top Tension	8	28.070			
				Top Cable Vert	8	22.130			
				Top Cable Norm	8	17.268			
		Top Cable Tan		8	0.001				
		Bot Cable Vert		8	-20.314				
		Guy B	Bot Cable Norm	8	18.120				
			Bot Cable Tan	8	0.001				
			Bottom Tension	6	20.282				
			Top Tension	6	21.133				
			Top Cable Vert	6	16.718				
			Top Cable Norm	6	12.908				
		Guy C	Top Cable Tan	6	0.705				
			Bot Cable Vert	6	-15.169				
			Bot Cable Norm	6	13.437				
			Bot Cable Tan	6	0.833				
			Bottom Tension	7	26.264				
			Top Tension	7	27.155				
		Top Guy Pull-Off	Top Cable Vert	7	21.827				
			Top Cable Norm	7	16.151				
			Top Cable Tan	7	0.347				
			Bot Cable Vert	7	-20.009				
			Bot Cable Norm	7	17.007				
			Bot Cable Tan	7	0.455				
		Leg	Max Tension	7	7.236	0.000	0.000	0.000	
			Max. Compression	1	0.000	0.000	0.000	0.000	
			Max. Mx	5	3.484	0.012	0.000	0.000	
			Max. Vy	5	-0.012	0.000	0.000	0.000	
			Max Tension	1	0.000	0.000	0.000	0.000	
			Max. Compression	6	-68.978	-0.011	-0.026	-0.026	
			Max. Mx	7	-42.686	-0.153	-0.127	-0.127	
			Max. My	6	-68.941	0.012	0.257	0.257	
			Max. Vy	7	-1.692	-0.143	-0.129	-0.129	
			Max. Vx	8	-2.283	-0.011	-0.011	-0.011	
Diagonal	Max Tension		8	2.010	0.000	0.000	0.000		
	Max. Compression		2	-2.105	0.000	0.000	0.000		
	Max. Mx		6	-0.884	-0.003	-0.000	-0.000		
K-Brace	Max. My		7	0.728	-0.002	0.010	0.010		
	Max. Vy		6	0.004	-0.003	-0.000	-0.000		
	Max. Vx		7	-0.004	-0.002	0.010	0.010		
	Max Tension		8	2.088	0.000	0.000	0.000		
	Max. Compression		8	-2.116	0.000	0.000	0.000		
	Max. Mx	6	-1.096	0.002	0.000	0.000			
Horizontal	Max. Vy	6	-0.003	0.000	0.000	0.000			
	Max Tension	8	1.099	0.000	0.000	0.000			
	Max. Compression	1	0.000	0.000	0.000	0.000			
Secondary Horizontal	Max. Mx	5	0.616	0.005	0.000	0.000			
	Max. Vy	5	-0.005	0.000	0.000	0.000			
	Max Tension	7	0.000	0.000	0.000	0.000			
	Max. Compression	3	-0.000	0.000	0.000	0.000			
	Max. Mx	6	0.000	0.002	0.000	0.000			
			Max. Vy	7	0.014	0.000	0.000		

<b>ERITowerBeta</b>  <i>Electronics Research Inc.</i> 7777 Gardner Road Chandler, IN Phone: 812-925-6000 FAX: 812-925-4026	Job	Portland, ME 531' Guyed Tower	Page	45 of 82
	Project	12590 48" Face Run#2	Date	11:18:49 10/18/04
	Client	SAGA Communications	Designed by	M. Maurer

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T8	331 - 311	Top Girt	Max. Vx	7	-0.010	0.000	0.000
			Max Tension	2	1.773	0.000	0.000
			Max. Compression	8	-1.682	0.000	0.000
			Max. Mx	8	-0.298	-0.004	-0.000
			Max. My	4	-1.643	-0.002	-0.001
			Max. Vy	8	-0.006	-0.004	-0.000
		Bottom Girt	Max Tension	4	1.532	-0.001	-0.001
			Max. Compression	2	-1.480	-0.001	-0.001
			Max. Mx	7	-0.842	0.002	0.000
			Max. My	2	1.526	-0.001	-0.001
			Max. Vy	5	0.005	-0.001	-0.000
			Max Tension	1	0.000	0.000	0.000
		Leg	Max. Compression	8	-52.733	0.002	0.163
			Max. Mx	7	-34.168	0.179	0.086
			Max. My	8	-41.212	0.029	0.264
			Max. Vy	7	-1.193	0.042	-0.095
			Max. Vx	8	-1.682	-0.001	0.124
			Max Tension	2	1.463	0.000	0.000
		Diagonal	Max. Compression	7	-1.701	0.000	0.000
			Max. Mx	8	0.031	-0.003	0.000
			Max. My	6	-0.894	-0.001	-0.010
			Max. Vy	8	0.004	-0.003	0.000
			Max. Vx	6	-0.004	0.000	0.000
			Max Tension	2	1.772	0.000	0.000
		K-Brace	Max. Compression	2	-1.794	0.000	0.000
			Max. Mx	6	-0.609	0.002	0.000
			Max. Vy	6	-0.003	0.000	0.000
			Max Tension	7	1.036	0.000	0.000
			Max. Compression	1	0.000	0.000	0.000
			Max. Mx	8	0.747	0.005	0.000
Horizontal	Max. Vy	8	-0.005	0.000	0.000		
	Max Tension	7	0.000	0.000	0.000		
	Max. Compression	3	-0.000	0.000	0.000		
	Max. Mx	6	0.000	0.002	0.000		
	Max. Vy	7	0.013	0.000	0.000		
	Max. Vx	8	0.011	0.000	0.000		
Top Girt	Max Tension	2	1.498	0.000	0.000		
	Max. Compression	2	-1.410	0.000	0.000		
	Max. Mx	7	-0.830	-0.004	0.000		
	Max. My	4	-1.348	-0.002	-0.001		
	Max. Vy	7	0.006	-0.004	0.000		
	Max Tension	2	0.721	-0.001	-0.000		
Bottom Girt	Max. Compression	2	-0.659	-0.001	-0.000		
	Max. Mx	7	0.217	0.002	0.000		
	Max. Vy	5	0.005	-0.001	-0.000		
	Max Tension	1	0.000	0.000	0.000		
	Max. Compression	8	-53.586	-0.013	0.026		
	Max. Mx	7	-46.973	0.139	-0.095		
Leg	Max. My	8	-50.457	0.031	0.223		
	Max. Vy	7	0.889	-0.034	-0.075		
	Max. Vx	6	-0.908	-0.002	0.123		
	Max Tension	6	0.799	0.000	0.000		
	Max. Compression	7	-1.637	0.000	0.000		
	Max. Mx	8	-0.176	-0.003	-0.000		
Diagonal	Max. My	6	-0.716	-0.001	-0.010		
	Max. Vy	8	0.004	-0.003	-0.000		
	Max. Vx	6	-0.004	0.000	0.000		
	Max Tension	3	1.165	0.000	0.000		
	Max. Compression	3	-1.148	0.000	0.000		
	Max. Mx	7	1.142	0.002	0.000		



<b>ERITowerBeta</b>  <i>Electronics Research Inc.</i> 7777 Gardner Road Chandler, IN Phone: 812-925-6000 FAX: 812-925-4026	Job	Portland, ME    531' Guyed Tower	Page	46 of 82
	Project	12590    48" Face    Run#2	Date	11:18:49 10/18/04
	Client	SAGA Communications	Designed by	M. Maurer

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft	
T10	291 - 271	Horizontal	Max. Vy	7	-0.003	0.000	0.000	
			Max Tension	7	1.064	0.000	0.000	
			Max. Compression	1	0.000	0.000	0.000	
			Max. Mx	8	0.395	0.005	0.000	
		Secondary Horizontal	Max. Vy	8	-0.005	0.000	0.000	
			Max Tension	7	0.000	0.000	0.000	
			Max. Compression	3	-0.000	0.000	0.000	
			Max. Mx	6	0.000	0.002	0.000	
		Top Girt	Max. Vy	7	0.014	0.000	0.000	
			Max. Vx	8	0.011	0.000	0.000	
			Max Tension	2	0.696	0.000	0.000	
			Max. Compression	2	-0.598	0.000	0.000	
		Bottom Girt	Max. Mx	7	-0.163	-0.004	0.000	
			Max. Vy	7	-0.006	-0.004	0.000	
			Max Tension	6	0.971	0.000	0.000	
			Max. Compression	3	-0.939	0.000	0.000	
		Leg	Max. Mx	7	-0.914	0.002	-0.000	
			Max. Vy	5	-0.005	-0.001	0.000	
			Max Tension	1	0.000	0.000	0.000	
			Max. Compression	6	-63.452	0.001	0.192	
			Max. Mx	7	-33.694	0.160	-0.081	
			Max. My	8	-35.253	0.051	0.236	
			Max. Vy	7	1.724	0.017	-0.072	
			Max. Vx	6	-1.781	0.001	0.192	
			Diagonal	Max Tension	8	1.963	0.000	0.000
				Max. Compression	7	-2.710	0.000	0.000
				Max. Mx	6	0.362	-0.003	0.000
				Max. My	8	-0.079	-0.002	-0.010
		K-Brace	Max. Vy	6	0.004	-0.003	0.000	
			Max. Vx	8	0.004	-0.002	-0.010	
			Max Tension	7	2.124	0.000	0.000	
			Max. Compression	7	-2.100	0.000	0.000	
		Horizontal	Max. Mx	7	1.174	0.002	0.000	
			Max. Vy	7	-0.003	0.000	0.000	
			Max Tension	7	1.097	0.000	0.000	
			Max. Compression	1	0.000	0.000	0.000	
		Secondary Horizontal	Max. Mx	8	0.546	0.005	0.000	
			Max. Vy	8	-0.005	0.000	0.000	
			Max Tension	7	0.000	0.000	0.000	
			Max. Compression	3	-0.000	0.000	0.000	
Top Girt	Max. Mx	6	0.000	0.002	0.000			
	Max. Vy	7	0.015	0.000	0.000			
	Max. Vx	8	0.010	0.000	0.000			
	Max Tension	8	1.016	-0.003	-0.000			
Bottom Girt	Max. Compression	3	-0.980	-0.003	-0.000			
	Max. Mx	7	-0.962	-0.004	-0.000			
	Max. Vy	7	-0.006	-0.004	-0.000			
	Max Tension	6	1.806	0.000	0.000			
Leg	Max. Compression	7	-1.715	0.000	0.000			
	Max. Mx	8	0.667	0.002	-0.000			
	Max. My	8	-1.671	-0.001	-0.001			
	Max. Vy	5	-0.005	-0.001	0.000			
Diagonal	Max Tension	1	0.000	0.000	0.000			
	Max. Compression	6	-91.961	0.000	0.277			
	Max. Mx	7	-54.679	-0.231	-0.109			
	Max. My	6	-63.454	0.053	0.341			
	Max. Vy	7	2.724	-0.059	-0.030			
	Max. Vx	8	2.891	0.000	-0.019			
Diagonal	Max Tension	8	2.502	0.000	0.000			

<b>ERITowerBeta</b>  <i>Electronics Research Inc.</i> 7777 Gardner Road Chandler, IN Phone: 812-925-6000 FAX: 812-925-4026	Job	Portland, ME      531' Guyed Tower	Page	47 of 82
	Project	12590      48" Face      Run#2	Date	11:18:49 10/18/04
	Client	SAGA Communications	Designed by	M. Maurer

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T12	251 - 231	K-Brace	Max. Compression	7	-3.824	0.000	0.000
			Max. Mx	6	1.433	-0.003	-0.000
			Max. My	8	-0.018	-0.002	-0.008
			Max. Vy	6	0.004	-0.003	-0.000
			Max. Vx	8	0.003	-0.002	-0.008
			Max Tension	7	3.229	0.000	0.000
			Max. Compression	7	-3.203	0.000	0.000
			Max. Mx	7	2.163	0.002	0.000
			Max. Vy	7	-0.003	0.000	0.000
			Max Tension	8	1.368	0.000	0.000
		Horizontal	Max. Compression	1	0.000	0.000	0.000
			Max. Mx	8	0.748	0.005	0.000
			Max. Vy	8	-0.005	0.000	0.000
			Max Tension	7	0.000	0.000	0.000
		Secondary Horizontal	Max. Compression	3	-0.000	0.000	0.000
			Max. Mx	6	0.000	0.001	0.000
		Top Girt	Max. Vy	7	0.014	0.000	0.000
			Max. Vx	6	-0.006	0.000	0.000
			Max Tension	8	1.858	-0.003	-0.001
			Max. Compression	7	-1.767	-0.004	-0.001
			Max. Mx	7	1.785	-0.004	-0.001
		Bottom Girt	Max. My	6	1.815	-0.003	-0.001
			Max. Vy	7	0.006	-0.004	-0.001
			Max Tension	7	2.634	0.000	0.000
			Max. Compression	7	-2.611	0.000	0.000
			Max. Mx	8	0.486	0.003	-0.000
		Leg	Max. My	3	-2.267	0.000	-0.001
			Max. Vy	7	-0.005	-0.001	0.000
			Max. Vx	3	0.001	0.000	0.000
			Max Tension	4	4.462	0.008	-0.110
			Max. Compression	6	-109.677	-0.009	-0.116
			Max. Mx	7	-79.547	-0.388	-0.151
			Max. My	6	-91.963	0.033	0.514
			Max. Vy	7	2.726	-0.286	-0.004
			Max. Vx	6	3.347	-0.010	0.491
			Max Tension	8	2.732	0.000	0.000
		Diagonal	Max. Compression	7	-3.980	0.000	0.000
			Max. Mx	6	-2.064	-0.004	-0.000
			Max. My	8	-0.642	-0.002	-0.007
			Max. Vy	6	0.004	-0.004	-0.000
Max. Vx	8		0.003	-0.002	-0.007		
K-Brace	Max Tension	7	3.289	0.000	0.000		
	Max. Compression	7	-3.321	0.000	0.000		
Horizontal	Max. Mx	7	3.289	0.002	0.000		
	Max. Vy	7	-0.002	0.000	0.000		
	Max Tension	7	1.716	0.000	0.000		
Secondary Horizontal	Max. Compression	1	0.000	0.000	0.000		
	Max. Mx	8	0.744	0.005	0.000		
	Max. Vy	8	-0.005	0.000	0.000		
	Max Tension	7	0.000	0.000	0.000		
Top Girt	Max. Compression	3	-0.000	0.000	0.000		
	Max. Mx	6	0.000	0.002	0.000		
	Max. Vy	7	0.010	0.000	0.000		
	Max. Vx	6	-0.005	0.000	0.000		
	Max Tension	7	2.706	-0.004	-0.001		
	Max. Compression	7	-2.686	-0.004	-0.001		
	Max. Mx	8	0.360	-0.005	-0.000		
	Max. My	3	-2.322	-0.003	-0.001		
	Max. Vy	8	-0.007	-0.005	-0.000		

<b>ERITowerBeta</b>  <b>Electronics Research Inc.</b> 7777 Gardner Road Chandler, IN Phone: 812-925-6000 FAX: 812-925-4026	Job	Portland, ME 531' Guyed Tower	Page	48 of 82
	Project	12590 48" Face Run#2	Date	11:18:49 10/18/04
	Client	SAGA Communications	Designed by	M. Maurer

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
		Bottom Girt	Max. Vx	3	0.001	0.000	0.000
			Max Tension	6	2.204	0.000	0.000
			Max. Compression	6	-2.303	0.000	0.000
		Guy A	Max. Mx	8	-0.110	0.003	0.000
			Max. Vy	8	0.005	0.000	0.000
			Bottom Tension	8	16.007		
		Guy B	Top Tension	8	16.355		
			Top Cable Vert	8	10.852		
			Top Cable Norm	8	12.235		
			Top Cable Tan	8	0.008		
			Bot Cable Vert	8	-9.948		
			Bot Cable Norm	8	12.541		
			Bot Cable Tan	8	0.009		
			Bottom Tension	6	11.801		
			Top Tension	6	12.150		
			Top Cable Vert	6	8.150		
		Guy C	Top Cable Norm	6	9.002		
			Top Cable Tan	6	0.418		
			Bot Cable Vert	6	-7.337		
			Bot Cable Norm.	6	9.231		
			Bot Cable Tan	6	0.478		
			Bottom Tension	7	15.282		
			Top Tension	7	15.656		
			Top Cable Vert	7	10.849		
			Top Cable Norm	7	11.286		
			Top Cable Tan	7	0.186		
		Top Guy Pull-Off	Bot Cable Vert	7	-9.906		
			Bot Cable Norm	7	11.634		
			Bot Cable Tan	7	0.234		
		Bottom Guy Pull-Off	Max Tension	8	9.838	0.000	0.000
			Max. Compression	6	-4.055	0.000	0.000
			Max. Mx	5	1.585	0.012	0.000
		Torque Arm Top	Max. Vy	5	-0.012	0.000	0.000
			Max Tension	8	14.365	0.000	0.000
			Max. Compression	1	0.000	0.000	0.000
		Torque Arm Bottom	Max. Mx	7	13.571	-0.109	0.000
			Max. Vy	7	0.061	0.000	0.000
			Max Tension	6	6.328	0.000	0.000
		Leg	Max. Compression	7	-18.132	0.000	0.000
			Max. Mx	8	-3.473	-0.110	0.000
			Max. Vy	8	0.061	0.000	0.000
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	6	-83.797	0.003	-0.039
			Max. Mx	7	-40.236	0.210	-0.117
			Max. My	6	-48.140	-0.009	0.350
			Max. Vy	7	-2.532	-0.119	-0.114
			Max. Vx	6	3.349	-0.001	0.212
			Max Tension	6	2.512	0.000	0.000
		Diagonal	Max. Compression	7	-3.329	0.000	0.000
			Max. Mx	6	-2.117	-0.003	0.000
			Max. My	8	-0.960	-0.002	-0.005
			Max. Vy	6	0.004	-0.003	0.000
			Max. Vx	8	0.002	-0.002	-0.005
		K-Brace	Max Tension	6	2.685	0.000	0.000
			Max. Compression	6	-2.714	0.000	0.000
			Max. Mx	6	1.990	0.002	0.000

T13 231 - 211

<b>ERITowerBeta</b> Electronics Research Inc. 7777 Gardner Road Chandler, IN Phone: 812-925-6000 FAX: 812-925-4026		Job Portland, ME 531' Guyed Tower	Project 12590 48" Face Run#2	Client SAGA Communications Designed by M. Maurer
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Section No.	Elevation	Component	Type	Gov.	Load Comb.	Force	Major Axis	Minor Axis
	ft					K	kip-ft	kip-ft

T14	211 - 191	Horizontal	Max. Tension	6	1.505	0.000	0.000	0.000
			Max. Mx	8	0.000	0.000	0.000	0.000
			Max. Vy	7	0.004	0.000	0.000	0.000
			Max. Vx	8	0.003	0.000	0.000	0.000
			Max. Compression	1	0.000	0.000	0.000	0.000
			Max. Mx	8	0.653	0.000	0.000	0.000
		Secondary	Max. Vy	8	-0.005	0.000	0.000	0.000
			Max. Vx	8	-0.005	0.000	0.000	0.000
			Max. Tension	7	0.000	0.000	0.000	0.000
			Max. Compression	3	-0.000	0.000	0.000	0.000
			Max. Mx	6	0.000	0.001	0.000	0.000
			Max. Vy	6	0.004	0.000	0.000	0.000
T15	191 - 171	Horizontal	Max. Tension	6	1.360	0.000	0.000	0.000
			Max. Mx	8	0.004	0.000	0.000	0.000
			Max. Vy	7	0.007	0.000	0.000	0.000
			Max. Vx	8	0.004	0.000	0.000	0.000
			Max. Compression	6	0.004	0.000	0.000	0.000
			Max. Mx	6	0.007	0.005	0.000	0.000
		Top Girt	Max. Tension	6	1.970	-0.003	-0.000	-0.000
			Max. Mx	8	0.004	0.000	0.000	0.000
			Max. Vy	6	0.007	0.000	0.000	0.000
			Max. Vx	6	0.006	-0.005	0.000	0.000
			Max. Compression	6	0.067	-0.005	0.000	0.000
			Max. Mx	6	0.006	-0.005	0.000	0.000
Bottom Girt	Max. Tension	6	1.360	-0.000	-0.000	-0.000		
	Max. Mx	6	0.038	-0.003	-0.000	-0.000		
	Max. Vy	6	-0.005	0.000	0.000	0.000		
	Max. Vx	6	-0.005	0.000	0.000	0.000		
	Max. Compression	6	-1.302	-0.000	-0.000	-0.000		
	Max. Mx	6	-0.038	-0.003	-0.000	-0.000		
T15	191 - 171	Leg	Max. Tension	2	9.113	0.000	0.166	0.051
			Max. Mx	8	-47.312	-0.575	0.038	-0.376
			Max. Vy	3	-1.395	0.064	-0.030	0.022
			Max. Vx	6	2.009	0.001	0.000	0.000
			Max. Compression	6	-2.099	0.000	0.000	0.000
			Max. Mx	6	-1.465	0.000	0.000	0.000
		Diagonal	Max. Tension	6	1.465	0.000	0.000	0.000
			Max. Mx	6	0.000	0.000	0.000	0.000
			Max. Vy	6	0.001	0.000	0.000	0.000
			Max. Vx	6	0.001	0.000	0.000	0.000
			Max. Compression	6	-2.099	0.000	0.000	0.000
			Max. Mx	6	-1.465	0.000	0.000	0.000

<b>ERITowerBeta</b> Electronics Research Inc. 7777 Gardner Road Chandler, IN Phone: 812-925-6000 FAX: 812-925-4026		Job Portland, ME 531' Guyed Tower	Project 12590 48" Face Run#2	Client SAGA Communications	Page 50 of 82
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Section No.	Elevation	Component Type	Condition	Gov. Load	Comb. Load	Force	Major Axis Moment	Minor Axis Moment	
T16	171 - 151	Leg	Max. Mx	8	-0.520	-0.004	0.000	0.000	
			Max. My	6	-1.503	-0.003	0.004	0.004	
			Max. Vy	8	0.004	-0.004	0.004	0.004	
		K-Brace	Max. Tension	6	1.700	1.700	0.000	0.000	0.000
			Max. Vy	8	0.004	-0.004	0.004	0.004	0.004
			Max. Vx	6	0.002	-0.002	0.002	0.002	0.002
		Top Girt	Max. Tension	6	1.506	1.506	0.000	0.000	0.000
			Max. Vy	8	0.004	-0.004	0.004	0.004	0.004
			Max. Vx	6	0.004	-0.004	0.004	0.004	0.004
		Horizontal	Max. Tension	6	-1.291	-1.291	0.000	0.000	0.000
			Max. Vy	8	-0.007	-0.007	0.000	0.000	0.000
			Max. Vx	6	-0.006	-0.006	0.000	0.000	0.000
Bottom Girt	Max. Tension	8	0.670	0.670	0.000	0.000	0.000		
	Max. Vy	6	-0.007	-0.007	0.000	0.000	0.000		
	Max. Vx	6	-0.006	-0.006	0.000	0.000	0.000		
T17	151 - 131	Leg	Max. Mx	8	-0.520	-0.004	0.000	0.000	
			Max. My	6	-1.503	-0.003	0.004	0.004	
			Max. Vy	8	0.004	-0.004	0.004	0.004	
		K-Brace	Max. Tension	6	1.700	1.700	0.000	0.000	0.000
			Max. Vy	8	0.004	-0.004	0.004	0.004	0.004
			Max. Vx	6	0.002	-0.002	0.002	0.002	0.002
		Top Girt	Max. Tension	6	1.506	1.506	0.000	0.000	0.000
			Max. Vy	8	0.004	-0.004	0.004	0.004	0.004
			Max. Vx	6	0.004	-0.004	0.004	0.004	0.004
		Horizontal	Max. Tension	6	-1.291	-1.291	0.000	0.000	0.000
			Max. Vy	8	-0.007	-0.007	0.000	0.000	0.000
			Max. Vx	6	-0.006	-0.006	0.000	0.000	0.000
Bottom Girt	Max. Tension	8	0.670	0.670	0.000	0.000	0.000		
	Max. Vy	6	-0.007	-0.007	0.000	0.000	0.000		
	Max. Vx	6	-0.006	-0.006	0.000	0.000	0.000		
T17	151 - 131	Leg	Max. Mx	8	-0.520	-0.004	0.000	0.000	
			Max. My	6	-1.503	-0.003	0.004	0.004	
			Max. Vy	8	0.004	-0.004	0.004	0.004	
		K-Brace	Max. Tension	6	1.700	1.700	0.000	0.000	0.000
			Max. Vy	8	0.004	-0.004	0.004	0.004	0.004
			Max. Vx	6	0.002	-0.002	0.002	0.002	0.002
		Top Girt	Max. Tension	6	1.506	1.506	0.000	0.000	0.000
			Max. Vy	8	0.004	-0.004	0.004	0.004	0.004
			Max. Vx	6	0.004	-0.004	0.004	0.004	0.004
		Horizontal	Max. Tension	6	-1.291	-1.291	0.000	0.000	0.000
			Max. Vy	8	-0.007	-0.007	0.000	0.000	0.000
			Max. Vx	6	-0.006	-0.006	0.000	0.000	0.000
Bottom Girt	Max. Tension	8	0.670	0.670	0.000	0.000	0.000		
	Max. Vy	6	-0.007	-0.007	0.000	0.000	0.000		
	Max. Vx	6	-0.006	-0.006	0.000	0.000	0.000		
T17	151 - 131	Leg	Max. Mx	8	-0.520	-0.004	0.000	0.000	
			Max. My	6	-1.503	-0.003	0.004	0.004	
			Max. Vy	8	0.004	-0.004	0.004	0.004	
		K-Brace	Max. Tension	6	1.700	1.700	0.000	0.000	0.000
			Max. Vy	8	0.004	-0.004	0.004	0.004	0.004
			Max. Vx	6	0.002	-0.002	0.002	0.002	0.002
		Top Girt	Max. Tension	6	1.506	1.506	0.000	0.000	0.000
			Max. Vy	8	0.004	-0.004	0.004	0.004	0.004
			Max. Vx	6	0.004	-0.004	0.004	0.004	0.004
		Horizontal	Max. Tension	6	-1.291	-1.291	0.000	0.000	0.000
			Max. Vy	8	-0.007	-0.007	0.000	0.000	0.000
			Max. Vx	6	-0.006	-0.006	0.000	0.000	0.000
Bottom Girt	Max. Tension	8	0.670	0.670	0.000	0.000	0.000		
	Max. Vy	6	-0.007	-0.007	0.000	0.000	0.000		
	Max. Vx	6	-0.006	-0.006	0.000	0.000	0.000		
T17	151 - 131	Leg	Max. Mx	8	-0.520	-0.004	0.000	0.000	
			Max. My	6	-1.503	-0.003	0.004	0.004	
			Max. Vy	8	0.004	-0.004	0.004	0.004	
		K-Brace	Max. Tension	6	1.700	1.700	0.000	0.000	0.000
			Max. Vy	8	0.004	-0.004	0.004	0.004	0.004
			Max. Vx	6	0.002	-0.002	0.002	0.002	0.002
		Top Girt	Max. Tension	6	1.506	1.506	0.000	0.000	0.000
			Max. Vy	8	0.004	-0.004	0.004	0.004	0.004
			Max. Vx	6	0.004	-0.004	0.004	0.004	0.004
		Horizontal	Max. Tension	6	-1.291	-1.291	0.000	0.000	0.000
			Max. Vy	8	-0.007	-0.007	0.000	0.000	0.000
			Max. Vx	6	-0.006	-0.006	0.000	0.000	0.000
Bottom Girt	Max. Tension	8	0.670	0.670	0.000	0.000	0.000		
	Max. Vy	6	-0.007	-0.007	0.000	0.000	0.000		
	Max. Vx	6	-0.006	-0.006	0.000	0.000	0.000		
T17	151 - 131	Leg	Max. Mx	8	-0.520	-0.004	0.000	0.000	
			Max. My	6	-1.503	-0.003	0.004	0.004	
			Max. Vy	8	0.004	-0.004	0.004	0.004	
		K-Brace	Max. Tension	6	1.700	1.700	0.000	0.000	0.000
			Max. Vy	8	0.004	-0.004	0.004	0.004	0.004
			Max. Vx	6	0.002	-0.002	0.002	0.002	0.002
		Top Girt	Max. Tension	6	1.506	1.506	0.000	0.000	0.000
			Max. Vy	8	0.004	-0.004	0.004	0.004	0.004
			Max. Vx	6	0.004	-0.004	0.004	0.004	0.004
		Horizontal	Max. Tension	6	-1.291	-1.291	0.000	0.000	0.000
			Max. Vy	8	-0.007	-0.007	0.000	0.000	0.000
			Max. Vx	6	-0.006	-0.006	0.000	0.000	0.000
Bottom Girt	Max. Tension	8	0.670	0.670	0.000	0.000	0.000		
	Max. Vy	6	-0.007	-0.007	0.000	0.000	0.000		
	Max. Vx	6	-0.006	-0.006	0.000	0.000	0.000		
T17	151 - 131	Leg	Max. Mx	8	-0.520	-0.004	0.000	0.000	
			Max. My	6	-1.503	-0.003	0.004	0.004	
			Max. Vy	8	0.004	-0.004	0.004	0.004	
		K-Brace	Max. Tension	6	1.700	1.700	0.000	0.000	0.000
			Max. Vy	8	0.004	-0.004	0.004	0.004	0.004
			Max. Vx	6	0.002	-0.002	0.002	0.002	0.002
		Top Girt	Max. Tension	6	1.506	1.506	0.000	0.000	0.000
			Max. Vy	8	0.004	-0.004	0.004	0.004	0.004
			Max. Vx	6	0.004	-0.004	0.004	0.004	0.004
		Horizontal	Max. Tension	6	-1.291	-1.291	0.000	0.000	0.000
			Max. Vy	8	-0.007	-0.007	0.000	0.000	0.000
			Max. Vx	6	-0.006	-0.006	0.000	0.000	0.000
Bottom Girt	Max. Tension	8	0.670	0.670	0.000	0.000	0.000		
	Max. Vy	6	-0.007	-0.007	0.000	0.000	0.000		
	Max. Vx	6	-0.006	-0.006	0.000	0.000	0.000		

<b>FRITOWERBeta</b> Electronics Research Inc. 7777 Gardner Road Chandler, IN Phone: 812-925-6000 FAX: 812-925-4026		Job Portland, ME 531' Guyed Tower	Project 12590 48" Face Run#2	Client SAGA Communications	Page 51 of 82
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Section No.	Elevation	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T18	131 - 111	Leg	Max. Compression	7	-89.094	-0.175	-0.163
			Max. Tension	4	3.806	0.001	0.083
			Max. Vx	8	0.005	0.000	0.000
			Max. Vy	8	0.131	0.003	0.000
		Bottom Girt	Max. Compression	7	-4.132	0.000	0.000
			Max. Tension	7	4.156	0.000	0.000
			Max. Vx	6	0.007	-0.006	-0.000
			Max. Vy	6	0.238	-0.006	-0.000
		Top Girt	Max. Compression	7	-2.406	-0.005	-0.000
			Max. Tension	7	2.461	-0.005	-0.000
			Max. Vx	8	0.004	0.000	0.000
			Max. Vy	7	0.019	0.000	0.000
Diagonal	Max. Mx	6	0.000	0.001	0.000		
	Max. My	8	-1.522	-0.004	-0.005		
	Max. Mx	7	1.865	-0.005	-0.000		
	Max. Compression	7	-5.970	0.000	0.000		
K-Brace	Max. Tension	7	5.095	0.000	0.000		
	Max. Compression	7	-5.065	0.000	0.000		
	Max. Vx	8	0.002	0.000	0.000		
	Max. Vy	7	0.005	-0.005	-0.000		
Horizontal	Max. Tension	8	2.205	0.000	0.000		
	Max. Vy	7	-0.003	0.000	0.000		
	Max. Mx	7	2.967	0.002	0.000		
	Max. Compression	7	-2.967	0.000	0.000		
Secondary	Max. Tension	8	1.211	0.005	0.000		
	Max. Vy	8	-0.005	0.000	0.000		
	Max. Mx	8	5.753	0.001	0.033		
	Max. Compression	7	-4.871	-0.005	-0.068		
Top Girt	Max. Mx	8	-83.706	0.007	0.716		
	Max. My	6	4.874	-0.411	0.061		
	Max. Vx	7	5.756	-0.002	-0.446		
	Max. Tension	8	4.719	0.000	0.000		
Diagonal	Max. Compression	7	-6.204	0.000	0.000		
	Max. Mx	7	-3.954	-0.007	0.002		
	Max. My	8	-1.924	-0.005	-0.005		
	Max. Vy	7	0.006	-0.007	0.002		
K-Brace	Max. Tension	7	5.149	0.000	0.000		
	Max. Compression	7	-5.184	0.000	0.000		
	Max. Mx	7	5.149	0.002	0.000		
	Max. Vy	7	-0.003	0.000	0.000		
Horizontal	Max. Tension	8	2.829	0.000	0.000		
	Max. Compression	1	0.000	0.000	0.000		
	Max. Mx	8	2.687	0.005	0.000		
	Max. Vy	8	-0.005	0.000	0.000		
Secondary	Max. Tension	7	0.000	0.000	0.000		
	Max. Compression	3	-0.000	0.000	0.000		
	Max. Mx	6	-0.002	0.002	0.000		
	Max. Vy	7	0.017	0.000	0.000		
Top Girt	Max. Vx	8	0.000	0.000	0.000		
	Max. Tension	7	4.227	-0.005	-0.001		
	Max. Compression	7	-4.202	-0.005	-0.001		
	Max. Vy	7	0.000	0.000	0.000		



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Section No.	Elevation	Component	Type	Gov.	Force	Major axis	Minor axis
	ft			Load	K	Moment	kip-ft
				Comb.			kip-ft
T20	91 - 71	Top Girt	Max. Tension	6	2.795	0.000	0.000
			Max. Compression	7	-2.691	0.000	0.000
			Max. Mx	8	0.120	-0.006	0.000
			Max. Vy	8	0.007	-0.006	0.000
		Bottom Girt	Max. Tension	7	2.048	0.002	0.000
			Max. Compression	7	-2.002	0.002	0.000
			Max. Mx	6	0.199	0.003	-0.000
			Max. Vy	6	0.005	0.000	0.000
		Leg	Max. Tension	1	101.923	0.232	-0.206
			Max. Compression	7	-77.269	0.321	-0.221
			Max. Mx	8	-71.193	0.000	0.445
			Max. Vy	7	-2.415	0.119	-0.149
Diagonal	Max. Tension	8	1.988	0.000	0.000		
	Max. Compression	7	-3.646	0.000	0.000		
	Max. Mx	7	0.739	-0.006	0.000		
	Max. Vy	7	-1.457	-0.004	0.004		
K-Brace	Max. Tension	7	2.428	0.000	0.000		
	Max. Compression	7	-2.465	0.000	0.000		
	Max. Mx	6	-1.156	0.002	0.000		
	Max. Vy	6	-0.003	0.000	0.000		
Horizontal	Max. Tension	6	2.873	0.000	0.000		
	Max. Compression	1	0.000	0.000	0.000		
	Max. Mx	8	1.138	0.005	0.000		
	Max. Vy	8	-0.005	0.000	0.000		
Secondary	Max. Tension	7	0.000	0.000	0.000		
	Max. Compression	3	-0.000	0.000	0.000		
	Max. Mx	6	-0.000	0.002	0.000		
	Max. Vy	7	0.012	0.000	0.000		
Top Girt	Max. Tension	6	2.047	0.000	0.000		
	Max. Compression	7	-1.973	0.000	0.000		
	Max. Mx	6	0.059	-0.006	-0.000		
	Max. Vy	6	0.007	-0.006	-0.000		
Bottom Girt	Max. Tension	8	1.200	0.000	0.000		
	Max. Compression	7	-1.059	0.002	0.000		
	Max. Mx	6	0.169	0.004	-0.000		
	Max. Vy	6	0.005	0.000	0.000		
Leg	Max. Tension	1	0.000	0.000	0.000		
	Max. Compression	7	-113.376	0.276	-0.235		
	Max. Mx	7	-101.925	0.346	-0.251		
	Max. Vy	8	-99.493	0.001	0.494		
Top Girt	Max. Tension	6	2.047	0.000	0.000		
	Max. Compression	7	-1.973	0.000	0.000		
	Max. Mx	6	0.059	-0.006	-0.000		
	Max. Vy	6	0.007	-0.006	-0.000		
Bottom Girt	Max. Tension	8	1.200	0.000	0.000		
	Max. Compression	7	-1.059	0.002	0.000		
	Max. Mx	6	0.169	0.004	-0.000		
	Max. Vy	6	0.005	0.000	0.000		
Leg	Max. Tension	1	0.000	0.000	0.000		
	Max. Compression	7	-113.376	0.276	-0.235		
	Max. Mx	7	-101.925	0.346	-0.251		
	Max. Vy	8	-99.493	0.001	0.494		
Diagonal	Max. Tension	8	1.133	0.000	0.000		
	Max. Compression	7	-2.353	0.000	0.000		
	Max. Mx	7	0.163	-0.006	0.000		
	Max. Vy	7	-0.004	0.004	0.000		
K-Brace	Max. Tension	8	0.006	-0.006	0.000		
	Max. Compression	7	-0.001	0.000	0.000		
	Max. Mx	8	1.347	0.000	0.000		
	Max. Vy	7	-1.383	0.000	0.000		
Horizontal	Max. Tension	6	2.580	0.000	0.000		
	Max. Compression	1	0.000	0.000	0.000		
	Max. Mx	6	-0.003	0.000	0.000		
	Max. Vy	6	0.010	0.002	0.000		
Secondary	Max. Tension	7	0.000	0.000	0.000		
	Max. Compression	6	-0.003	0.000	0.000		
	Max. Mx	6	0.003	0.000	0.000		
	Max. Vy	6	0.000	0.000	0.000		



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Project	12590	48" Face	Run#2
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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft	
T22	51 - 31	Secondary	Max. Vy	8	-0.005	0.000	0.000	
			Max. Tension	7	0.000	0.000	0.000	
		Horizontal	Max. Compression	3	-0.000	0.000	0.000	0.000
			Max. Mx	6	-0.000	0.002	0.000	0.000
		Top Girt	Max. Vy	7	0.016	0.000	0.000	0.000
			Max. Vx	8	0.004	0.000	0.000	0.000
		Bottom Girt	Max. Tension	8	1.188	0.000	0.000	0.000
			Max. Compression	7	-1.082	0.000	0.000	0.000
		Leg	Max. Mx	6	-0.129	-0.006	-0.000	-0.000
			Max. Vy	6	0.007	-0.006	-0.000	-0.000
		Bottom Girt	Max. Tension	8	0.405	-0.000	-0.000	-0.000
			Max. Compression	4	-0.123	-0.000	-0.000	-0.000
		Horizontal	Max. Mx	6	0.163	0.003	-0.000	-0.000
			Max. Vy	6	0.005	0.000	0.000	0.000
Leg	Max. Tension	1	0.000	0.000	0.000	0.000		
	Max. Compression	8	-114.399	0.000	0.107	0.107		
Horizontal	Max. Mx	7	-110.785	0.343	-0.251	-0.251		
	Max. Vy	8	-111.222	-0.003	0.486	0.486		
Diagonal	Max. Tension	6	0.978	0.000	0.000	0.000		
	Max. Compression	7	-2.185	0.000	0.000	0.000		
K-Brace	Max. Vy	8	-0.006	-0.006	0.000	0.000		
	Max. Vx	8	0.006	-0.006	0.000	0.000		
Horizontal	Max. Tension	6	1.340	0.000	0.000	0.000		
	Max. Compression	6	-1.313	0.000	0.000	0.000		
Horizontal	Max. Mx	6	1.160	0.002	0.000	0.000		
	Max. Vy	6	-0.003	0.000	0.000	0.000		
Horizontal	Max. Tension	6	2.589	0.000	0.000	0.000		
	Max. Compression	1	0.000	0.000	0.000	0.000		
Horizontal	Max. Mx	5	0.785	0.005	0.000	0.000		
	Max. Vy	5	-0.005	0.000	0.000	0.000		
Secondary	Max. Tension	7	0.000	0.000	0.000	0.000		
	Max. Compression	3	-0.000	0.000	0.000	0.000		
Horizontal	Max. Mx	6	-0.021	0.000	0.000	0.000		
	Max. Vy	7	0.004	0.000	0.000	0.000		
Top Girt	Max. Tension	7	0.415	0.000	0.000	0.000		
	Max. Compression	3	-0.127	0.000	0.000	0.000		
Bottom Girt	Max. Mx	6	0.161	-0.006	-0.000	-0.000		
	Max. Vy	6	0.007	-0.006	-0.000	-0.000		
Bottom Girt	Max. Tension	6	1.194	0.000	0.000	0.000		
	Max. Compression	6	-0.967	0.000	0.000	0.000		
Horizontal	Max. Mx	6	0.134	0.003	-0.000	-0.000		
	Max. Vy	6	0.005	0.000	0.000	0.000		
Leg	Max. Tension	1	0.000	0.000	0.000	0.000		
	Max. Compression	8	-111.266	0.001	-0.094	-0.094		
Diagonal	Max. Mx	7	-94.519	-1.050	0.522	0.522		
	Max. Vy	7	-93.972	0.047	-1.100	-1.100		
Diagonal	Max. Tension	8	9.068	-1.050	0.522	0.522		
	Max. Compression	8	-0.006	0.000	0.000	0.000		
Diagonal	Max. Mx	8	-1.067	-0.004	-0.004	-0.004		
	Max. Vy	8	0.005	-0.006	-0.000	-0.000		
Diagonal	Max. Tension	8	-0.002	0.000	0.000	0.000		
	Max. Compression	8	0.002	0.000	0.000	0.000		

Section No.	Elevation ft	Component Type	Condition	Gov. Load	Force	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
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T24	11 - 1	K-Brace	Max. Tension	6	2.249	0.000	0.000	
			Max. Compression	6	-2.218	0.000	0.000	
			Max. Mix	6	1.962	0.002	0.000	
		Horizontal	Max. Mix	6	-0.003	0.000	0.000	0.000
			Max. Tension	6	2.457	0.000	0.000	0.000
			Max. Compression	1	0.000	0.000	0.000	0.000
		Horizontal	Max. Mix	5	0.806	0.005	0.000	0.000
			Max. Tension	5	4.881	0.003	-0.000	-0.000
			Max. Compression	6	0.002	-0.002	-0.000	-0.000
		Leg	Max. Tension	1	0.000	0.000	0.000	0.000
			Max. Compression	7	-98.129	-0.122	0.589	0.000
			Max. Mix	7	-94.405	-0.170	0.071	0.071
Top Girt	Max. Mix	6	-93.546	-0.055	-1.151	-1.151		
	Max. Tension	7	11.275	1.170	0.966	0.966		
	Max. Vx	6	1.070	-0.098	0.000	0.000		
Diagonal	Max. Tension	1	0.000	0.000	0.000	0.000		
	Max. Compression	6	-4.797	0.000	0.000	0.000		
	Max. Mix	6	-0.407	0.009	0.000	0.000		
Horizontal	Max. Mix	6	0.009	0.000	0.000	0.000		
	Max. Tension	6	1.917	0.000	0.000	0.000		
	Max. Compression	1	0.000	0.000	0.000	0.000		
Top Girt	Max. Mix	5	0.598	0.006	0.000	0.000		
	Max. Tension	5	-0.008	0.000	0.000	0.000		
	Max. Compression	6	7.203	0.000	0.000	0.000		
Horizontal	Max. Mix	1	0.000	0.000	0.000	0.000		
	Max. Tension	6	4.998	0.012	0.000	0.000		
	Max. Compression	5	0.000	0.000	0.000	0.000		

### Maximum Reactions

Location	Condition	Gov. Load	Vertical K	Horizontal, X K	Horizontal, Z K
Mast	Max. Vert	7	245.294	-3.337	0.034
	Max. Hx	6	244.629	0.029	3.723
	Max. Hy	6	244.629	0.029	3.723
	Max. Mx	1	0.000	-0.000	0.010
	Max. My	1	0.000	-0.000	0.010
	Max. Mz	1	0.000	-0.000	0.010
	Min. Vert	1	147.439	-0.000	0.010
	Min. Hx	7	245.294	-3.337	0.034
	Min. Hy	8	240.595	-0.001	-3.370
	Min. Mx	1	0.000	-0.000	0.010
	Min. My	1	0.000	-0.000	0.010
	Min. Mz	1	0.000	-0.000	0.010
	Comb.				

# ERITowerBeta

Electronics Research Inc.  
 7777 Gardner Road  
 Chandler, IN  
 Phone: 812-925-6000  
 FAX: 812-925-4026

Job	Portland, ME	531' Guyed Tower
Project	12590	48" Face
Client	SAGA Communications	
Date	11:18:49 10/18/04	Run#2
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Location	Condition	Gov. Load	Vertical	Horizontal, X	Horizontal, Z
Guy C @ 295 ft	Elev -10.3 ft	Min. Torsion	6	-0.511	0.029
Azimuth 240 deg	Max. H <sub>x</sub>	4	-13.321	-8.429	3.471
	Max. H <sub>z</sub>	7	-49.721	-32.429	17.405
	Min. H <sub>x</sub>	7	-49.721	-32.429	17.405
	Min. H <sub>z</sub>	4	-13.321	-8.429	3.471
Guy B @ 295 ft	Max. H <sub>x</sub>	4	-13.321	-8.429	3.471
	Max. H <sub>z</sub>	7	-49.721	-32.429	17.405
	Min. H <sub>x</sub>	7	-49.721	-32.429	17.405
	Min. H <sub>z</sub>	4	-13.321	-8.429	3.471
Azimuth 120 deg	Max. H <sub>x</sub>	6	-39.131	24.757	16.644
	Max. H <sub>z</sub>	6	-39.131	24.757	16.644
	Min. H <sub>x</sub>	6	-39.131	24.757	16.644
	Min. H <sub>z</sub>	3	-5.984	2.997	2.425
Guy A @ 295 ft	Max. H <sub>x</sub>	2	-3.183	-0.001	-1.730
	Max. H <sub>z</sub>	3	-5.984	2.997	2.425
	Min. H <sub>x</sub>	3	-5.984	2.997	2.425
	Min. H <sub>z</sub>	2	-3.183	-0.001	-1.730
Azimuth 0 deg	Max. H <sub>x</sub>	4	-39.901	0.003	-30.223
	Max. H <sub>z</sub>	2	-3.183	-0.001	-1.730
	Min. H <sub>x</sub>	8	-50.652	0.002	-39.106
	Min. H <sub>z</sub>	7	-27.506	-2.353	-20.556
Guy C @ 280 ft	Max. H <sub>x</sub>	4	-6.965	-9.665	4.747
	Max. H <sub>z</sub>	7	-32.506	-45.088	25.233
	Min. H <sub>x</sub>	7	-32.506	-45.088	25.233
	Min. H <sub>z</sub>	4	-6.965	-9.665	4.747
Elev 5.7 ft	Max. H <sub>x</sub>	4	-6.965	-9.665	4.747
	Max. H <sub>z</sub>	7	-32.506	-45.088	25.233
	Min. H <sub>x</sub>	7	-32.506	-45.088	25.233
	Min. H <sub>z</sub>	4	-6.965	-9.665	4.747
Azimuth 120 deg	Max. H <sub>x</sub>	6	-24.168	36.602	22.775
	Max. H <sub>z</sub>	6	-24.168	36.602	22.775
	Min. H <sub>x</sub>	6	-24.168	36.602	22.775
	Min. H <sub>z</sub>	3	-2.297	3.544	2.405
Guy A @ 280 ft	Max. H <sub>x</sub>	2	-0.711	-0.000	-2.305
	Max. H <sub>z</sub>	3	-2.297	3.544	2.405
	Min. H <sub>x</sub>	3	-2.297	3.544	2.405
	Min. H <sub>z</sub>	2	-0.711	-0.000	-2.305
Elev 8 ft	Max. H <sub>x</sub>	2	-0.711	-0.000	-2.305
	Max. H <sub>z</sub>	3	-2.297	3.544	2.405
	Min. H <sub>x</sub>	3	-2.297	3.544	2.405
	Min. H <sub>z</sub>	2	-0.711	-0.000	-2.305
Azimuth 0 deg	Max. H <sub>x</sub>	2	-0.711	-0.000	-2.305
	Max. H <sub>z</sub>	3	-2.297	3.544	2.405
	Min. H <sub>x</sub>	3	-2.297	3.544	2.405
	Min. H <sub>z</sub>	2	-0.711	-0.000	-2.305
Guy B @ 280 ft	Max. H <sub>x</sub>	7	-16.419	-1.713	-28.852
	Max. H <sub>z</sub>	8	-31.016	-0.009	-54.080
	Min. H <sub>x</sub>	7	-16.419	-1.713	-28.852
	Min. H <sub>z</sub>	8	-31.016	-0.009	-54.080

## Tower Mast Reaction Summary

Load	Vertical	Shear <sub>x</sub>	Shear <sub>y</sub>	Overtwring	Overtwring	Torque
Combination	K	K	K	kip-ft	kip-ft	kip-ft
Dead Only	147.439	0.000	-0.014	-0.010	0.000	0.192
Dead+Wind 0 deg - No	183.187	0.000	-0.014	-2.773	0.000	0.371

Job	Portland, ME	531' Guyed Tower
Project	12590	48" Face Run#2
Client	SAGA Communications	
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Load	Combination	Vertical	Shear <sub>x</sub>	Shear <sub>y</sub>	Overtwring Moment, M <sub>x</sub>	Overtwring Moment, M <sub>y</sub>	Torque
Ice+Guy	Ice+Guy	183,504	2,695	-0.052	0.000	0.000	0.420
Dead+Wind 90 deg - No	Ice+Guy	179,091	0.004	2.792	0.000	0.000	0.096
Dead+Wind 180 deg - No	Ice+Guy	192,143	-0.003	-0.010	0.000	0.000	0.289
Dead+Wind 0	deg+Ice+Temp+Guy	244,629	-0.029	-3.723	0.000	0.000	0.511
Dead+Wind 90	deg+Ice+Temp+Guy	245,294	3.337	-0.034	0.000	0.000	0.413
Dead+Wind 180	deg+Ice+Temp+Guy	240,595	0.001	3.370	0.000	0.000	0.229
deg+Ice+Temp+Guy							

**Solution Summary**

Load	Comb.	Sum of Applied Forces	Sum of Reactions	% Error
1	2	0.000	-0.000	0.011%
2	3	0.035	-75.982	0.024%
3	4	59.487	-75.539	0.023%
4	5	-0.035	-75.021	0.020%
5	6	0.000	-107.910	0.019%
6	7	0.060	-108.733	0.023%
7	8	76.225	-107.975	0.023%
8		-0.060	-107.087	0.022%

**Non-Linear Convergence Results**

Load	Converged	Number of Cycles	Displacement Tolerance	Force Tolerance
1	Yes	23	0.00010000	0.00001341
2	Yes	167	0.00009753	0.00004077
3	Yes	153	0.00009708	0.00004591
4	Yes	57	0.00009502	0.00005093
5	Yes	17	0.00010000	0.00005015
6	Yes	185	0.00009812	0.00004372
7	Yes	173	0.00009914	0.00004887
8	Yes	41	0.00009526	0.00006190

**Maximum Tower Deflections - Design Wind**

Section No.	Elevation	Horz. Deflection in ft	Gov. Load Comb.	Tilt	Twist
L1	528 - 512.205	49.828	6	1.2681	1.6088
L2	512.205 - 496.41	45.637	6	1.2538	1.5860
L3	496.41 - 480.615	41.557	6	1.1795	1.5102
L4	480.615 - 470	37.872	6	0.9934	1.3683

<b>FRITOWERBeta</b> Electronics Research Inc. 7777 Gardner Road Chandler, IN Phone: 812-925-6000 FAX: 812-925-4026		Job Portland, ME 531' Guyed Tower	Project 12590 48" Face Run#2	Client SAGA Communications Designed by M. Maurer
Page	58 of 82	Date	11:18:49 10/18/04	

Section No.	Elevation	Horz. Deflection	Gov. Load	Tilt	Twist
T1	470 - 451	35.838	6	0.7783	1.2508
T2	451 - 431	32.984	6	0.7013	1.1802
T3	431 - 411	30.141	6	0.6818	1.1053
T4	411 - 391	27.281	6	0.6801	1.0164
T5	391 - 371	24.716	7	0.6457	0.9106
T6	371 - 351	22.596	7	0.5466	0.8049
T7	351 - 331	21.024	7	0.4008	0.7353
T8	331 - 311	19.881	7	0.3313	0.6559
T9	311 - 291	18.745	7	0.3128	0.6205
T10	291 - 271	17.474	7	0.3130	0.5380
T11	271 - 251	16.121	7	0.3011	0.3997
T12	251 - 231	14.923	7	0.2149	0.2471
T13	231 - 211	14.377	7	0.1132	0.1814
T14	211 - 191	14.230	7	0.1090	0.1580
T15	191 - 171	14.117	6	0.1683	0.1463
T16	171 - 151	13.807	6	0.3082	0.1359
T17	151 - 131	12.755	6	0.4159	0.1268
T18	131 - 111	11.260	6	0.4001	0.1208
T19	111 - 91	10.126	6	0.2648	0.1182
T20	91 - 71	9.411	6	0.2336	0.1189
T21	71 - 51	8.395	6	0.3239	0.1206
T22	51 - 31	6.757	6	0.4723	0.1232
T23	31 - 11	4.441	6	0.6198	0.1270
T24	11 - 1	1.560	6	0.7153	0.1311

## Critical Deflections and Radius of Curvature - Design Wind

Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
528.000	Red A-2/3 Lighting Kit w/ A-3 Spur	6	49.828	1.2681	1.6008	51456
522.700	SHPX-5AE-Radomes (3" Coax)	6	48.418	1.2669	1.5996	48544
517.438	SHPX-5AE-Radomes (3" Coax)	6	47.020	1.2630	1.5956	24358
512.175	SHPX-5AE-Radomes (3" Coax)	6	45.629	1.2538	1.5859	18543
506.913	SHPX-5AE-Radomes (3" Coax)	6	44.247	1.2369	1.5682	16061
501.650	SHPX-5AE-Radomes (3" Coax)	6	42.885	1.2120	1.5427	10661
496.388	SHPX-5AE-Radomes (3" Coax)	6	41.551	1.1793	1.5100	7672
491.125	SHPX-5AE-Radomes (3" Coax)	6	40.258	1.1373	1.4705	5313
485.863	SHPX-5AE-Radomes (3" Coax)	6	39.024	1.0782	1.4236	3985
480.600	SHPX-5AE-Radomes (3" Coax)	6	37.669	0.9931	1.3681	3293
469.917	Guy	6	35.824	0.7770	1.2501	3359
440.000	(3) DB224 w/ Long Arm Mounts	6	31.427	0.6931	1.1438	55639
420.000	6' Grid (7/8" Coax)	6	28.569	0.6800	1.0576	31357
361.000	Guy	7	21.737	0.4712	0.7692	7227
330.000	4' Grid (7/8" Coax)	7	19.827	0.3296	0.6532	34331
329.200	Ice Shield (4' x 6')	7	19.783	0.3283	0.6513	32230
319.200	DCR-C 4 Bay w/ domes (3" Coax)	7	19.226	0.3173	0.6346	23339
313.714	DCR-C 4 Bay w/ domes (3" Coax)	7	18.907	0.3140	0.6261	20578
308.229	DCR-C 4 Bay w/ domes (3" Coax)	7	18.577	0.3118	0.6134	20107
302.743	DCR-C 4 Bay w/ domes (3" Coax)	7	18.236	0.3099	0.5948	21584
297.257	DCR-C 4 Bay w/ domes (3" Coax)	7	17.885	0.3074	0.5709	23364
291.771	DCR-C 4 Bay w/ domes (3" Coax)	7	17.525	0.3124	0.5424	25710
286.286	DCR-C 4 Bay w/ domes (3" Coax)	7	17.159	0.3149	0.5097	31025
280.800	DCR-C 4 Bay w/ domes (3" Coax)	7	16.787	0.3140	0.4733	39550
260.000	Mid Beacon Level	7	15.720	0.2845	0.3503	11116
	4' Grid (7/8" Coax)	7	15.405	0.2649	0.3098	7541

# FRITOWERBETA

Electronics Research Inc.  
 7777 Gardner Road  
 Chandler, IN  
 Phone: 812-925-6000  
 FAX: 812-925-1026

Job	Portland, ME	531' Guyed Tower
Project	12590	48" Face Run#2
Client	SAGA Communications	
Page	59 of 82	Designed by M. Maurer

Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of Curvature
241.000	Guy	7	14.569	0.1444	0.2040	6093
235.000	4' Grid (7/8" Coax)	7	14.438	0.1073	0.1888	7300
180.000	(12) 5' x 1' Panels (1 5/8" Coax)	6	14.034	0.2432	0.1405	6046
160.000	(12) 5' x 1' Panels (1 5/8" Coax)	6	13.315	0.3775	0.1306	6408
140.000	(12) 5' x 1' Panels (1 5/8" Coax)	6	11.934	0.4281	0.1231	20013
121.000	Guy	6	10.622	0.3331	0.1191	5805

## Guy Design Data

Section No.	Elevation	Initial Tension	Breaking Load	Actual T	Allowable T <sub>0</sub>	Required S.F.	Actual S.F.
T1	469.917 (A)	10.450	104.500	38.271	41.800	2.500	2.731 ✓
	469.917 (B)	10.450	104.500	30.403	41.800	2.500	3.437 ✓
	469.917 (C)	10.450	104.500	37.134	41.800	2.500	2.814 ✓
T6	361.000 (A)	7.970	79.700	28.070	31.880	2.500	2.839 ✓
	361.000 (B)	7.970	79.700	21.133	31.880	2.500	3.771 ✓
	361.000 (C)	7.970	79.700	27.155	31.880	2.500	2.935 ✓
T12	241.000 (A)	4.240	42.400	16.355	16.960	2.500	2.593 ✓
	241.000 (B)	4.240	42.400	11.602	16.960	2.500	3.655 ✓
	241.000 (C)	4.240	42.400	15.282	16.960	2.500	2.775 ✓
	241.000 (A)	4.240	42.400	12.150	16.960	2.500	3.490 ✓
	241.000 (B)	4.240	42.400	15.656	16.960	2.500	2.708 ✓
	241.000 (C)	4.240	42.400	31.763	41.800	2.500	3.290 ✓
T18	121.000 (A)	10.450	104.500	27.267	41.800	2.500	3.833 ✓
	121.000 (B)	10.450	104.500	31.861	41.800	2.500	3.280 ✓
	121.000 (C)	10.450	104.500				

## Compression Checks

## Leg Design Data (Compression)

<b>FRITowerBeta</b>		Electronics Research Inc. 7777 Gardner Road Chandler, IN Phone: 812-925-6000 FAX: 812-925-4026	
Job	Portland, ME	531' Guyed Tower	
Project	12590	48" Face	Run#2
Client	SAGA Communications		
	Designed by M. Maurer		
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Section No.	Elevation	Size	L	L <sub>n</sub>	K/L <sub>n</sub>	Fast Stability Index	ksi	in <sup>2</sup>	Actual P	Allow. P <sub>n</sub>	Ratio P/P <sub>n</sub>
L1	528 - 512.205	2	15.795	1.755	42.1	0.98	24.960	3.142	-6.754	78.415	0.086
L2	512.205 - 496.41	2	15.795	1.755	42.1	0.99	25.258	3.142	-22.300	79.350	0.281
L3	496.41 - 480.615	2	15.795	1.755	42.1	0.99	25.357	3.142	-46.864	79.660	0.588
L4	480.615 - 470	2	10.615	1.769	42.5	0.99	25.259	3.142	-68.426	79.353	0.862
T1	470 - 451	2 1/2	19.000	2.833	54.4	0.97	23.052	4.909	-55.900	113.154	0.494
T2	451 - 431	2 1/2	20.000	2.833	54.4	0.96	22.786	4.909	-35.316	111.853	0.316
T3	431 - 411	2 1/2	20.000	2.833	54.4	0.95	22.573	4.909	-31.615	110.804	0.285
T4	411 - 391	2 1/2	20.000	2.833	54.4	0.96	22.804	4.909	-42.729	111.938	0.382
T5	391 - 371	2 1/2	20.000	2.833	54.4	0.97	22.960	4.909	-65.453	112.704	0.581
T6	371 - 351	2 1/2	20.000	2.833	54.4	0.94	22.297	4.909	-79.301	109.453	0.725
T7	351 - 331	2 1/2	20.000	2.833	54.4	0.93	22.084	4.909	-68.978	108.403	0.636
T8	331 - 311	2 1/2	20.000	2.833	54.4	0.91	21.487	4.909	-52.733	105.475	0.500
T9	311 - 291	2 1/2	20.000	2.833	54.4	0.91	21.500	4.909	-53.586	105.537	0.508
T10	291 - 271	2 1/2	20.000	2.833	54.4	0.92	21.734	4.909	-63.452	106.686	0.595
T11	271 - 251	2 1/2	20.000	2.833	54.4	0.94	22.252	4.909	-91.961	109.230	0.842
T12	251 - 231	2 1/2	20.000	2.833	54.4	0.94	22.273	4.909	-109.677	109.331	1.003
T13	231 - 211	2 1/2	20.000	2.833	54.4	0.92	21.722	4.909	-83.797	106.630	0.786
T14	211 - 191	2 1/2	20.000	2.833	54.4	0.92	21.809	4.909	-91.724	107.053	0.857
T15	191 - 171	2 1/2	20.000	2.833	54.4	0.93	21.904	4.909	-101.213	107.522	0.941
T16	171 - 151	2 1/2	20.000	2.833	54.4	0.92	21.843	4.909	-98.558	107.223	0.919
T17	151 - 131	2 1/2	20.000	2.833	54.4	0.91	21.451	4.909	-89.094	105.295	0.846
T18	131 - 111	2 1/2	20.000	2.833	54.4	0.92	21.748	4.909	-117.455	106.754	1.100
T19	111 - 91	2 1/2	20.000	2.833	54.4	0.91	21.432	4.909	-100.921	105.201	0.959
T20	91 - 71	2 1/2	20.000	2.833	54.4	0.90	21.341	4.909	-101.923	104.758	0.973
T21	71 - 51	2 3/4	20.000	2.833	49.5	0.89	21.805	5.940	-113.376	129.514	0.875
T22	51 - 31	2 3/4	20.000	2.833	49.5	0.89	21.817	5.940	-114.399	129.586	0.883

# ERITowerBeta

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7777 Gardner Road  
Chandler, IN  
Phone: 812-925-6000  
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Job

Portland, ME 531' Guyed Tower

Project

12590 48" Face Run#2

Client

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Section No.	Elevation	Size	L	L <sub>n</sub>	K/r	Mass Stability Index	F <sub>o</sub>	A	Actual P	Allow. P <sub>a</sub>	Ratio P/P <sub>a</sub>
T23	31 - 11	2 3/4	20,000	2,833	49.5	0.89	21,710	5,940	-111,266	128,946	0.863
T24	11 - 1	2 3/4	10,263	2,104	36.7	0.82	21,654	5,940	-98,129	128,614	0.763

## Diagonal Design Data (Compression)

Section No.	Elevation	Size	L	L <sub>n</sub>	K/r	F <sub>o</sub>	A	Actual P	Allow. P <sub>a</sub>	Ratio P/P <sub>a</sub>
L1	528 - 512.205	3/4	3,476	1,738	105.0	13,532	0.442	-0.862	5,978	0.144
L2	512.205 - 496.41	3/4	3,476	1,738	105.0	13,532	0.442	-1,897	5,978	0.317
L3	496.41 - 480.615	3/4	3,476	1,738	105.0	13,532	0.442	-2,947	5,978	0.493
L4	480.615 - 470	3/4	3,483	1,741	105.1	13,511	0.442	-3,210	5,969	0.538
T1	470 - 451	3/4	4,902	2,451	141.2	7,493	0.442	-2,700	3,310	0.816
T2	451 - 431	3/4	4,902	2,451	141.2	7,493	0.442	-2,246	3,310	0.678
T3	431 - 411	3/4	4,902	2,451	141.2	7,493	0.442	-1,629	3,310	0.492
T4	411 - 391	3/4	4,902	2,451	141.2	7,493	0.442	-2,213	3,310	0.669
T5	391 - 371	3/4	4,902	2,451	141.2	7,493	0.442	-2,768	3,310	0.836
T6	371 - 351	7/8	4,902	2,451	121.0	10,199	0.601	-3,020	6,133	0.492
T7	351 - 331	3/4	4,902	2,451	141.2	7,493	0.442	-2,105	3,310	0.636
T8	331 - 311	3/4	4,902	2,451	141.2	7,493	0.442	-1,701	3,310	0.514
T9	311 - 291	3/4	4,902	2,451	141.2	7,493	0.442	-1,637	3,310	0.494
T10	291 - 271	3/4	4,902	2,451	141.2	7,493	0.442	-2,710	3,310	0.819
T11	271 - 251	3/4	4,902	2,451	141.2	7,493	0.442	-3,824	3,310	1.155
T12	251 - 231	3/4	4,902	2,451	141.2	7,493	0.442	-3,980	3,310	1.202
T13	231 - 211	3/4	4,902	2,451	141.2	7,493	0.442	-3,329	3,310	1.006
T14	211 - 191	3/4	4,902	2,451	141.2	7,493	0.442	-2,774	3,310	0.838
T15	191 - 171	3/4	4,902	2,451	141.2	7,493	0.442	-2,099	3,310	0.634



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### K-Brace Design Data (Compression)

Section No.	Elevation	Size	L	L <sub>n</sub>	K/lr	F <sub>a</sub>	A	Actual P	Allow. P <sub>a</sub>	Ratio
	f		f	f	ksi	in <sup>2</sup>		K	K	P <sub>a</sub>
T16	171 - 151	3/4	4.902	2.451	141.2	7.493	0.442	-3.673	3.310	1.110
T17	151 - 131	7/8	4.902	2.451	121.0	10.199	0.601	-5.970	6.133	0.973
T18	131 - 111	7/8	4.902	2.451	121.0	10.199	0.601	-6.204	6.133	1.012
T19	111 - 91	7/8	4.902	2.451	121.0	10.199	0.601	-4.522	6.133	0.737
T20	91 - 71	7/8	4.902	2.451	121.0	10.199	0.601	-3.646	6.133	0.595
T21	71 - 51	7/8	4.902	2.451	121.0	10.199	0.601	-2.353	6.133	0.384
T22	51 - 31	7/8	4.902	2.451	121.0	10.199	0.601	-2.185	6.133	0.356
T23	31 - 11	7/8	4.902	2.451	121.0	10.199	0.601	-3.301	6.133	0.538
T24	11 - 1	1 1/8	2.322	2.322	94.8	15.885	0.994	-4.797	15.790	0.304

Section No.	Elevation	Size	L	L <sub>n</sub>	K/lr	F <sub>a</sub>	A	Actual P	Allow. P <sub>a</sub>	Ratio
	f		f	f	ksi	in <sup>2</sup>		K	K	P <sub>a</sub>
L1	528 - 512.205	3/4	1.697	1.697	96.2	15.581	0.442	-0.950	6.883	0.138
L2	512.205 - 496.41	3/4	1.697	1.697	96.2	15.581	0.442	-1.995	6.883	0.290
L3	496.41 - 480.615	3/4	1.697	1.697	96.2	15.581	0.442	-3.010	6.883	0.437
L4	480.615 - 470	3/4	1.701	1.701	96.2	15.578	0.442	-3.601	6.882	0.523
T1	470 - 451	3/4	2.200	2.200	112.6	11.769	0.442	-2.110	5.199	0.406
T2	451 - 431	3/4	2.451	2.451	125.5	9.483	0.442	-2.405	4.189	0.574
T3	431 - 411	3/4	2.451	2.451	125.5	9.483	0.442	-1.502	4.189	0.359
T4	411 - 391	3/4	2.451	2.451	125.5	9.483	0.442	-1.918	4.189	0.458
T5	391 - 371	3/4	2.451	2.451	125.5	9.483	0.442	-2.393	4.189	0.571
T6	371 - 351	7/8	2.451	2.451	107.6	12.907	0.601	-2.596	7.761	0.335
T7	351 - 331	3/4	2.451	2.451	125.5	9.483	0.442	-2.116	4.189	0.505
T8	331 - 311	3/4	2.451	2.451	125.5	9.483	0.442	-1.794	4.189	0.428
T9	311 - 291	3/4	2.451	2.451	125.5	9.483	0.442	-1.148	4.189	0.274

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Section No.	Elevation	Size	L	L <sub>n</sub>	Kl/r	F <sub>a</sub>	A	Actual P	Allow. P <sub>a</sub>	Ratio
T10	291 - 271	3/4	2.451	2.451	125.5	9.483	0.442	-2.100	4.189	0.501 ✓
T11	271 - 251	3/4	2.451	2.451	125.5	9.483	0.442	-3.203	4.189	0.764 ✓
T12	251 - 231	3/4	2.451	2.451	125.5	9.483	0.442	-3.321	4.189	0.793 ✓
T13	231 - 211	3/4	2.451	2.451	125.5	9.483	0.442	-2.714	4.189	0.648 ✓
T14	211 - 191	3/4	2.451	2.451	125.5	9.483	0.442	-2.289	4.189	0.546 ✓
T15	191 - 171	3/4	2.451	2.451	125.5	9.483	0.442	-1.730	4.189	0.413 ✓
T16	171 - 151	3/4	2.451	2.451	125.5	9.483	0.442	-2.905	4.189	0.693 ✓
T17	151 - 131	7/8	2.451	2.451	107.6	12.907	0.601	-5.065	7.761	0.653 ✓
T18	131 - 111	7/8	2.451	2.451	107.6	12.907	0.601	-5.184	7.761	0.668 ✓
T19	111 - 91	7/8	2.451	2.451	107.6	12.907	0.601	-3.336	7.761	0.430 ✓
T20	91 - 71	7/8	2.451	2.451	107.6	12.907	0.601	-2.465	7.761	0.318 ✓
T21	71 - 51	7/8	2.451	2.451	107.6	12.907	0.601	-1.383	7.761	0.178 ✓
T22	51 - 31	7/8	2.451	2.451	107.6	12.907	0.601	-1.313	7.761	0.169 ✓
T23	31 - 11	7/8	2.451	2.451	107.6	12.907	0.601	-2.218	7.761	0.286 ✓

**Horizontal Design Data (Compression)**

Section No.	Elevation	Size	L	L <sub>n</sub>	Kl/r	F <sub>a</sub>	A	Actual P	Allow. P <sub>a</sub>	Ratio
T6	371 - 351	3/4	4.000	4.000	204.8	3.560	0.442	-0.469	1.573	0.298 ✓

Kl/r > 200 (C) - 719

**Secondary Horizontal Design Data (Compression)**

Section No.	Elevation	Size	L	L <sub>n</sub>	Kl/r	F <sub>a</sub>	A	Actual P	Allow. P <sub>a</sub>	Ratio
T1	470 - 451	5/8	2.000	2.000	122.9	9.866	0.307	-0.000	3.027	0.000 ✓
T2	451 - 431	5/8	2.000	2.000	122.9	9.866	0.307	-0.000	3.027	0.000 ✓

# ERTOWERBeta

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Job

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Section No.	Elevation	Size	L	L <sub>n</sub>	K/lr	F <sub>a</sub>	A	Actual P	Allow. P <sub>a</sub>	Ratio
T3	431 - 411	5/8	2.000	2.000	122.9	9.866	0.307	-0.000	3.027	0.000
T4	411 - 391	5/8	2.000	2.000	122.9	9.866	0.307	-0.000	3.027	0.000
T5	391 - 371	5/8	2.000	2.000	122.9	9.866	0.307	-0.000	3.027	0.000
T6	371 - 351	5/8	2.000	2.000	122.9	9.866	0.307	-0.000	3.027	0.000
T7	351 - 331	5/8	2.000	2.000	122.9	9.866	0.307	-0.000	3.027	0.000
T8	331 - 311	5/8	2.000	2.000	122.9	9.866	0.307	-0.000	3.027	0.000
T9	311 - 291	5/8	2.000	2.000	122.9	9.866	0.307	-0.000	3.027	0.000
T10	291 - 271	5/8	2.000	2.000	122.9	9.866	0.307	-0.000	3.027	0.000
T11	271 - 251	5/8	2.000	2.000	122.9	9.866	0.307	-0.000	3.027	0.000
T12	251 - 231	5/8	2.000	2.000	122.9	9.866	0.307	-0.000	3.027	0.000
T13	231 - 211	5/8	2.000	2.000	122.9	9.866	0.307	-0.000	3.027	0.000
T14	211 - 191	5/8	2.000	2.000	122.9	9.866	0.307	-0.000	3.027	0.000
T15	191 - 171	5/8	2.000	2.000	122.9	9.866	0.307	-0.000	3.027	0.000
T16	171 - 151	5/8	2.000	2.000	122.9	9.866	0.307	-0.000	3.027	0.000
T17	151 - 131	5/8	2.000	2.000	122.9	9.866	0.307	-0.000	3.027	0.000
T18	131 - 111	5/8	2.000	2.000	122.9	9.866	0.307	-0.000	3.027	0.000
T19	111 - 91	5/8	2.000	2.000	122.9	9.866	0.307	-0.000	3.027	0.000
T20	91 - 71	5/8	2.000	2.000	122.9	9.866	0.307	-0.000	3.027	0.000
T21	71 - 51	5/8	2.000	2.000	122.9	9.866	0.307	-0.000	3.027	0.000
T22	51 - 31	5/8	2.000	2.000	122.9	9.866	0.307	-0.000	3.027	0.000
T23	31 - 11	5/8	2.000	2.000	122.9	9.866	0.307	-0.000	3.027	0.000

## Top Girt Design Data (Compression)

Section No.	Elevation	Size	L	L <sub>n</sub>	K/lr	F <sub>a</sub>	A	Actual P	Allow. P <sub>a</sub>	Ratio
L1	528 - 512.205	1 1/2x1/2	3.000	3.000	199.5	3.751	0.750	-0.195	2.813	0.069

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Section No.	Elevation	Size	L	L <sub>u</sub>	K/lr	F <sub>o</sub>	A	Actual	Allow.	Ratio
	f <sub>i</sub>		f <sub>i</sub>	f <sub>i</sub>	ksi	m <sup>2</sup>	K	P	P <sub>o</sub>	P
L2	512.205	1 1/2x1/2	3.000	3.000	199.5	3.751	0.750	-0.913	2.813	0.325
L3	496.41	1 1/2x1/2	3.000	3.000	199.5	3.751	0.750	-1.881	2.813	0.669
L4	480.615 - 470	7/8	3.000	3.000	131.7	8.615	0.601	-2.826	5.180	0.546
T1	470 - 451	12x1	4.000	4.000	166.3	5.401	12.000	-0.159	64.814	0.002
T2	451 - 431	1	4.000	4.000	153.6	6.329	0.785	-1.952	4.971	0.393
T3	431 - 411	1	4.000	4.000	153.6	6.329	0.785	-1.187	4.971	0.239
T4	411 - 391	1	4.000	4.000	153.6	6.329	0.785	-1.151	4.971	0.231
T5	391 - 371	1	4.000	4.000	153.6	6.329	0.785	-1.626	4.971	0.327
T6	371 - 351	1	4.000	4.000	153.6	6.329	0.785	-2.098	4.971	0.422
T7	351 - 331	1	4.000	4.000	153.6	6.329	0.785	-1.682	4.971	0.338
T8	331 - 311	1	4.000	4.000	153.6	6.329	0.785	-1.410	4.971	0.284
T9	311 - 291	1	4.000	4.000	153.6	6.329	0.785	-0.598	4.971	0.120
T10	291 - 271	1	4.000	4.000	153.6	6.329	0.785	-0.980	4.971	0.197
T11	271 - 251	1	4.000	4.000	153.6	6.329	0.785	-1.767	4.971	0.356
T12	251 - 231	1	4.000	4.000	153.6	6.329	0.785	-2.686	4.971	0.540
T13	231 - 211	1	4.000	4.000	153.6	6.329	0.785	-2.048	4.971	0.412
T14	211 - 191	1	4.000	4.000	153.6	6.329	0.785	-1.738	4.971	0.350
T15	191 - 171	1	4.000	4.000	153.6	6.329	0.785	-1.291	4.971	0.260
T16	171 - 151	1	4.000	4.000	153.6	6.329	0.785	-0.570	4.971	0.115
T17	151 - 131	1	4.000	4.000	153.6	6.329	0.785	-2.406	4.971	0.484
T18	131 - 111	1	4.000	4.000	153.6	6.329	0.785	-4.202	4.971	0.845
T19	111 - 91	1	4.000	4.000	153.6	6.329	0.785	-2.691	4.971	0.541
T20	91 - 71	1	4.000	4.000	153.6	6.329	0.785	-1.973	4.971	0.397
T21	71 - 51	1	4.000	4.000	153.6	6.329	0.785	-1.082	4.971	0.218
T22	51 - 31	1	4.000	4.000	153.6	6.329	0.785	-0.127	4.971	0.026
T23	31 - 11	1	4.000	4.000	153.6	6.329	0.785	-0.903	4.971	0.182

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Section No.	Elevation	Size	L	L <sub>n</sub>	Kl/r	F <sub>a</sub>	A	Actual	Allow.	Ratio
	f <sub>i</sub>		f <sub>i</sub>	f <sub>i</sub>	ksi	in <sup>2</sup>	K	K	P <sub>a</sub>	P <sub>a</sub>

### Bottom Girt Design Data (Compression)

Section No.	Elevation	Size	L	L <sub>n</sub>	Kl/r	F <sub>a</sub>	A	Actual	Allow.	Ratio
L1	528 - 512.205	1 1/2x1/2	3.000	199.5	3.751	0.750	-0.871	2.813	0.309	0.309
L2	512.205 - 496.41	1 1/2x1/2	3.000	199.5	3.751	0.750	-1.811	2.813	0.644	0.644
L3	496.41 - 480.615	7/8	3.000	105.3	13.449	0.601	-2.720	8.087	0.336	0.336
L4	480.615 - 470	12x1	3.000	124.7	9.602	12.000	-3.961	115.225	0.034	0.034
T1	470 - 451	4.000	4.000	153.6	6.329	0.785	-1.854	4.971	0.373	0.373
T2	451 - 431	4.000	4.000	153.6	6.329	0.785	-1.186	4.971	0.239	0.239
T3	431 - 411	4.000	4.000	153.6	6.329	0.785	-1.127	4.971	0.227	0.227
T4	411 - 391	4.000	4.000	153.6	6.329	0.785	-1.571	4.971	0.316	0.316
T5	391 - 371	4.000	4.000	153.6	6.329	0.785	-1.958	4.971	0.394	0.394
T6	371 - 351	4.000	4.000	153.6	6.329	0.785	-1.778	4.971	0.358	0.358
T7	351 - 331	4.000	4.000	153.6	6.329	0.785	-1.480	4.971	0.298	0.298
T8	331 - 311	4.000	4.000	153.6	6.329	0.785	-0.659	4.971	0.133	0.133
T9	311 - 291	4.000	4.000	153.6	6.329	0.785	-0.939	4.971	0.189	0.189
T10	291 - 271	4.000	4.000	153.6	6.329	0.785	-1.715	4.971	0.345	0.345
T11	271 - 251	4.000	4.000	153.6	6.329	0.785	-2.611	4.971	0.525	0.525
T12	251 - 231	4.000	4.000	153.6	6.329	0.785	-2.303	4.971	0.463	0.463
T13	231 - 211	4.000	4.000	153.6	6.329	0.785	-1.804	4.971	0.363	0.363
T14	211 - 191	4.000	4.000	153.6	6.329	0.785	-1.302	4.971	0.262	0.262
T15	191 - 171	4.000	4.000	153.6	6.329	0.785	-0.628	4.971	0.126	0.126
T16	171 - 151	4.000	4.000	153.6	6.329	0.785	-2.376	4.971	0.478	0.478
T17	151 - 131	4.000	4.000	153.6	6.329	0.785	-4.132	4.971	0.831	0.831
T18	131 - 111	4.000	4.000	153.6	6.329	0.785	-2.777	4.971	0.559	0.559

Section No.	Elevation	f	L	L <sub>n</sub>	Kl/r	F <sub>a</sub>	A	Actual P	Allow. P <sub>a</sub>	Ratio P/P <sub>a</sub>
T19	111 - 91	f	4,000	4,000	153.6	6.329	0.785	-2.002	4.971	0.403 ✓
T20	91 - 71	f	4,000	4,000	153.6	6.329	0.785	-1.059	4.971	0.213 ✓
T21	71 - 51	f	4,000	4,000	153.6	6.329	0.785	-0.123	4.971	0.025 ✓
T22	51 - 31	f	4,000	4,000	153.6	6.329	0.785	-0.967	4.971	0.195 ✓

**Mid Girt Design Data (Compression)**

Section No.	Elevation	f	L	L <sub>n</sub>	Kl/r	F <sub>a</sub>	A	Actual P	Allow. P <sub>a</sub>	Ratio P/P <sub>a</sub>
L1	528 - 512.205	f	3,000	3,000	105.3	13.449	0.601	-0.034	8.087	0.004 ✓
L2	512.205 - 496.41	f	3,000	3,000	105.3	13.449	0.601	-0.088	8.087	0.011 ✓
L3	496.41 - 480.615	f	3,000	3,000	105.3	13.449	0.601	-0.188	8.087	0.023 ✓
L4	480.615 - 470	f	3,000	3,000	131.7	8.615	0.601	-0.272	5.180	0.053 ✓

**Top Guy Pull-Off Design Data (Compression)**

Section No.	Elevation	L	L <sub>n</sub>	Kl/r	F <sub>a</sub>	A	Actual P	Allow. P <sub>a</sub>	Ratio P/P <sub>a</sub>	
T12	251 - 231	f	4,000	4,000	153.6	6.329	1.227	-4.055	7.767	0.522 ✓

**Bottom Guy Pull-Off Design Data (Compression)**

Section No.	Elevation	L	L <sub>n</sub>	Kl/r	F <sub>a</sub>	A	Actual P	Allow. P <sub>a</sub>	Ratio P/P <sub>a</sub>	
T12	251 - 231	f	4,000	4,000	153.6	6.329	1.227	-6.445	7.767	0.830 ✓

**Torque-Arm Bottom Design Data**

<b>ERITowerBeta</b> Electronics Research Inc. 7777 Gardner Road Chandler, IN Phone: 812-925-6000 FAX: 812-925-4026		Job Portland, ME 531' Guyed Tower	Project 12590 48" Face Run#2	Client SAGA Communications	Page 68 of 82
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**Tension Checks**

Section No.	Elevation	Size	L	L <sub>n</sub>	K/r	F <sub>a</sub>	A	Actual P	Allow. P <sub>a</sub>	Ratio
T12	251 - 231 (1976)	L4x4x1/2	7.236	7.236	111.0	11.531	3.750	-15.047	43.243	0.348
T12	251 - 231 (1977)	L4x4x1/2	7.236	7.236	111.0	11.531	3.750	-16.850	43.243	0.390
T12	251 - 231 (1984)	L4x4x1/2	7.236	7.236	111.0	11.531	3.750	-8.162	43.243	0.189
T12	251 - 231 (1985)	L4x4x1/2	7.236	7.236	111.0	11.531	3.750	-18.132	43.243	0.419
T12	251 - 231 (1990)	L4x4x1/2	7.236	7.236	111.0	11.531	3.750	-14.695	43.243	0.340
T12	251 - 231 (1991)	L4x4x1/2	7.236	7.236	111.0	11.531	3.750	-17.052	43.243	0.394

**Leg Design Data (Tension)**

Section No.	Elevation	Size	L	L <sub>n</sub>	f <sub>i</sub>	f <sub>t</sub>	F <sub>a</sub>	A	Actual P	Allow. P <sub>a</sub>	Ratio
L1	528 - 512.205	2	15.795	42.1	1.755	42.1	30.000	3.142	6.257	94.248	0.066
L2	512.205 - 496.41	2	15.795	42.1	1.755	42.1	30.000	3.142	21.375	94.248	0.227
L3	496.41 - 480.615	2	15.795	42.1	1.755	42.1	30.000	3.142	45.405	94.248	0.482
L4	480.615 - 470	2	10.615	42.5	1.769	42.5	30.000	3.142	65.856	94.248	0.699
T1	470 - 451	2 1/2	19.000	54.4	2.833	54.4	30.000	4.909	48.786	147.262	0.331
T5	391 - 371	2 1/2	20.000	54.4	2.833	54.4	30.000	4.909	10.324	147.262	0.070
T6	371 - 351	2 1/2	20.000	54.4	2.833	54.4	30.000	4.909	17.666	147.262	0.120
T12	251 - 231	2 1/2	20.000	54.4	2.833	54.4	30.000	4.909	4.462	147.262	0.030
T15	191 - 171	2 1/2	20.000	54.4	2.833	54.4	30.000	4.909	9.113	147.262	0.062
T16	171 - 151	2 1/2	20.000	54.4	2.833	54.4	30.000	4.909	6.636	147.262	0.045
T18	131 - 111	2 1/2	20.000	54.4	2.833	54.4	30.000	4.909	3.806	147.262	0.026

**Diagonal Design Data (Tension)**

<b>FRITOWERBETA</b>		Electronics Research Inc. 7777 Gardner Road Chandler, IN Phone: 812-925-6000 FAX: 812-925-4026	
Job	Portland, ME	531' Guyed Tower	
Project	12590	48" Face	Run#2
Client	SAGA Communications		
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Section No.	Elevation	Size	L	L <sub>n</sub>	Kl/r	F <sub>a</sub>	A	Actual	Allow.	Ratio
	<i>f</i>		<i>f</i>	<i>f</i>	<i>f</i>	<i>ksi</i>	<i>in<sup>2</sup></i>	<i>K</i>	<i>K</i>	$\frac{P}{P_a}$

### K-Brace Design Data (Tension)

Section	Elevation	Size	L	L <sub>n</sub>	Kl/r	F <sub>a</sub>	A	Actual	Allow.	Ratio
L1	528 - 512.205	3/4	1.697	1.697	1.697	108.6	30.000	0.442	0.975	0.074
L2	512.205 - 496.41	3/4	1.697	1.697	1.697	108.6	30.000	0.442	2.019	0.152
L3	496.41 - 480.615	3/4	1.697	1.697	1.697	108.6	30.000	0.442	3.031	0.229
L4	480.615 - 470	3/4	1.701	1.701	1.701	108.8	30.000	0.442	3.602	0.272
T1	470 - 451	3/4	2.200	2.200	2.200	140.8	30.000	0.442	2.655	0.200
T2	451 - 431	3/4	2.451	2.451	2.451	156.9	30.000	0.442	2.377	0.179
T3	431 - 411	3/4	2.451	2.451	2.451	156.9	30.000	0.442	1.474	0.111
T4	411 - 391	3/4	2.451	2.451	2.451	156.9	30.000	0.442	1.944	0.147
T5	391 - 371	3/4	2.451	2.451	2.451	156.9	30.000	0.442	2.420	0.183
T6	371 - 351	7/8	2.451	2.451	2.451	134.5	30.000	0.601	2.563	0.142
T7	351 - 331	3/4	2.451	2.451	2.451	156.9	30.000	0.442	2.088	0.158
T8	331 - 311	3/4	2.451	2.451	2.451	156.9	30.000	0.442	1.772	0.134
T9	311 - 291	3/4	2.451	2.451	2.451	156.9	30.000	0.442	1.165	0.088
T10	291 - 271	3/4	2.451	2.451	2.451	156.9	30.000	0.442	2.124	0.160
T11	271 - 251	3/4	2.451	2.451	2.451	156.9	30.000	0.442	3.229	0.244
T12	251 - 231	3/4	2.451	2.451	2.451	156.9	30.000	0.442	3.289	0.248
T13	231 - 211	3/4	2.451	2.451	2.451	156.9	30.000	0.442	2.685	0.203
T14	211 - 191	3/4	2.451	2.451	2.451	156.9	30.000	0.442	2.259	0.170
T15	191 - 171	3/4	2.451	2.451	2.451	156.9	30.000	0.442	1.700	0.128
T16	171 - 151	3/4	2.451	2.451	2.451	156.9	30.000	0.442	2.928	0.221
T17	151 - 131	7/8	2.451	2.451	2.451	134.5	30.000	0.601	5.095	0.282
T18	131 - 111	7/8	2.451	2.451	2.451	134.5	30.000	0.601	5.149	0.285



<b>FRITowerBeta</b>		Electronics Research Inc. 7777 Gardner Road Chandler, IN Phone: 812-925-6000 FAX: 812-925-4026	
Job	Portland, ME	531' Guyed Tower	
Project	12590	48" Face	Run#2
Client	SAGA Communications		
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Section No.	Elevation	f	L	L <sub>n</sub>	K/lr	F <sub>a</sub>	A	Actual P	Allow. P <sub>a</sub>	Ratio P/P <sub>a</sub>
T19	111 - 91	2.451	2.451	134.5	30.000	0.601	3.302	18.040	0.183	✓
T20	91 - 71	2.451	2.451	134.5	30.000	0.601	2.428	18.040	0.135	✓
T21	71 - 51	2.451	2.451	134.5	30.000	0.601	1.347	18.040	0.075	✓
T22	51 - 31	2.451	2.451	134.5	30.000	0.601	1.340	18.040	0.074	✓
T23	31 - 11	2.451	2.451	134.5	30.000	0.601	2.249	18.040	0.125	✓

### Horizontal Design Data (Tension)

Section No.	Elevation	f	L	L <sub>n</sub>	K/lr	F <sub>a</sub>	A	Actual P	Allow. P <sub>a</sub>	Ratio P/P <sub>a</sub>
T1	470 - 451	4.000	4.000	256.0	30.000	0.442	0.858	13.254	0.065	✓
T2	451 - 431	4.000	4.000	256.0	30.000	0.442	0.625	13.254	0.047	✓
T3	431 - 411	4.000	4.000	256.0	30.000	0.442	0.697	13.254	0.053	✓
T4	411 - 391	4.000	4.000	256.0	30.000	0.442	0.673	13.254	0.051	✓
T5	391 - 371	4.000	4.000	256.0	30.000	0.442	0.917	13.254	0.069	✓
T6	371 - 351	4.000	4.000	256.0	30.000	0.442	1.473	13.254	0.111	✓
T7	351 - 331	4.000	4.000	256.0	30.000	0.442	1.099	13.254	0.083	✓
T8	331 - 311	4.000	4.000	256.0	30.000	0.442	1.036	13.254	0.078	✓
T9	311 - 291	4.000	4.000	256.0	30.000	0.442	1.064	13.254	0.080	✓
T10	291 - 271	4.000	4.000	256.0	30.000	0.442	1.097	13.254	0.083	✓
T11	271 - 251	4.000	4.000	256.0	30.000	0.442	1.368	13.254	0.103	✓
T12	251 - 231	4.000	4.000	256.0	30.000	0.442	1.716	13.254	0.129	✓
T13	231 - 211	4.000	4.000	256.0	30.000	0.442	1.505	13.254	0.114	✓
T14	211 - 191	4.000	4.000	256.0	30.000	0.442	1.833	13.254	0.138	✓
T15	191 - 171	4.000	4.000	256.0	30.000	0.442	2.176	13.254	0.164	✓
T16	171 - 151	4.000	4.000	256.0	30.000	0.442	2.191	13.254	0.165	✓
T17	151 - 131	4.000	4.000	256.0	30.000	0.442	2.205	13.254	0.166	✓

<b>FRITowerBeta</b>		Electronics Research Inc. 7777 Gardner Road Chandler, IN Phone: 812-925-6000 FAX: 812-925-4026	
Job	Portland, ME	531' Guyed Tower	
Project	12590	48" Face	Run#2
Client	SAGA Communications		
	Designed by	M. Maurer	
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Section No.	Elevation	Size	L	L <sub>n</sub>	KI/r	F <sub>a</sub>	A	h <sup>2</sup>	Actual P	Allow. P <sub>a</sub>	Ratio P/P <sub>a</sub>
T18	131 - 111	3/4	4.000	4.000	256.0	30.000	0.442	2.829	13.254	0.213	✓
T19	111 - 91	3/4	4.000	4.000	256.0	30.000	0.442	2.557	13.254	0.193	✓
T20	91 - 71	3/4	4.000	4.000	256.0	30.000	0.442	2.873	13.254	0.217	✓
T21	71 - 51	3/4	4.000	4.000	256.0	30.000	0.442	2.580	13.254	0.195	✓
T22	51 - 31	3/4	4.000	4.000	256.0	30.000	0.442	2.589	13.254	0.195	✓
T23	31 - 11	3/4	4.000	4.000	256.0	30.000	0.442	2.457	13.254	0.185	✓
T24	11 - 1	1/8	1.607	1.607	68.6	30.000	0.994	1.917	29.821	0.064	✓

### Secondary Horizontal Design Data (Tension)

Section No.	Elevation	Size	L	L <sub>n</sub>	f <sub>i</sub>	f <sub>a</sub>	KI/r	F <sub>a</sub>	A	h <sup>2</sup>	Actual P	Allow. P <sub>a</sub>	Ratio P/P <sub>a</sub>
T1	470 - 451	5/8	2.000	2.000	153.6	21.600	0.307	6.627	0.000	0.000	6.627	0.000	✓
T2	451 - 431	5/8	2.000	2.000	153.6	21.600	0.307	6.627	0.000	0.000	6.627	0.000	✓
T3	431 - 411	5/8	2.000	2.000	153.6	21.600	0.307	6.627	0.000	0.000	6.627	0.000	✓
T4	411 - 391	5/8	2.000	2.000	153.6	21.600	0.307	6.627	0.000	0.000	6.627	0.000	✓
T5	391 - 371	5/8	2.000	2.000	153.6	21.600	0.307	6.627	0.000	0.000	6.627	0.000	✓
T6	371 - 351	5/8	2.000	2.000	153.6	21.600	0.307	6.627	0.000	0.000	6.627	0.000	✓
T7	351 - 331	5/8	2.000	2.000	153.6	21.600	0.307	6.627	0.000	0.000	6.627	0.000	✓
T8	331 - 311	5/8	2.000	2.000	153.6	21.600	0.307	6.627	0.000	0.000	6.627	0.000	✓
T9	311 - 291	5/8	2.000	2.000	153.6	21.600	0.307	6.627	0.000	0.000	6.627	0.000	✓
T10	291 - 271	5/8	2.000	2.000	153.6	21.600	0.307	6.627	0.000	0.000	6.627	0.000	✓
T11	271 - 251	5/8	2.000	2.000	153.6	21.600	0.307	6.627	0.000	0.000	6.627	0.000	✓
T12	251 - 231	5/8	2.000	2.000	153.6	21.600	0.307	6.627	0.000	0.000	6.627	0.000	✓
T13	231 - 211	5/8	2.000	2.000	153.6	21.600	0.307	6.627	0.000	0.000	6.627	0.000	✓
T14	211 - 191	5/8	2.000	2.000	153.6	21.600	0.307	6.627	0.000	0.000	6.627	0.000	✓
T15	191 - 171	5/8	2.000	2.000	153.6	21.600	0.307	6.627	0.000	0.000	6.627	0.000	✓

<b>FRITowerBeta</b>		Electronics Research Inc. 7777 Gardner Road Chandler, IN Phone: 812-925-6000 FAX: 812-925-4026	
Job	Portland, ME	531' Guyed Tower	
Project	12590	48" Face	Run#2
Client	SAGA Communications		
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Section No.	Elevation	Size	L	L <sub>n</sub>	K/r	F <sub>a</sub>	A	Actual P	Allow. P <sub>a</sub>	Ratio
	f		f	f	ksi	ksi	in <sup>2</sup>	K	K	P <sub>a</sub>
T16	171 - 151	5/8	2.000	2.000	153.6	21.600	0.307	0.000	6.627	0.000
T17	151 - 131	5/8	2.000	2.000	153.6	21.600	0.307	0.000	6.627	0.000
T18	131 - 111	5/8	2.000	2.000	153.6	21.600	0.307	0.000	6.627	0.000
T19	111 - 91	5/8	2.000	2.000	153.6	21.600	0.307	0.000	6.627	0.000
T20	91 - 71	5/8	2.000	2.000	153.6	21.600	0.307	0.000	6.627	0.000
T21	71 - 51	5/8	2.000	2.000	153.6	21.600	0.307	0.000	6.627	0.000
T22	51 - 31	5/8	2.000	2.000	153.6	21.600	0.307	0.000	6.627	0.000
T23	31 - 11	5/8	2.000	2.000	153.6	21.600	0.307	0.000	6.627	0.000

### Top Girt Design Data (Tension)

Section No.	Elevation	Size	L	L <sub>n</sub>	K/r	F <sub>a</sub>	A	Actual P	Allow. P <sub>a</sub>	Ratio
	f		f	f	ksi	ksi	in <sup>2</sup>	K	K	P <sub>a</sub>
L1	528 - 512.205	1 1/2x1/2	3.000	249.4	21.600	0.750	0.196	0.012	16.200	0.012
L2	512.205 - 496.41	1 1/2x1/2	3.000	249.4	21.600	0.750	0.926	0.057	16.200	0.057
L3	496.41 - 480.615	1 1/2x1/2	3.000	249.4	21.600	0.750	1.886	0.116	16.200	0.116
L4	480.615 - 470	7/8	3.000	164.6	30.000	0.601	2.809	0.156	18.040	0.156
T1	470 - 451	12x1	4.000	166.3	30.000	12.000	9.824	0.027	360.000	0.027
T2	451 - 431	1	4.000	192.0	30.000	0.785	1.947	0.083	23.562	0.083
T3	431 - 411	1	4.000	192.0	30.000	0.785	1.239	0.053	23.562	0.053
T4	411 - 391	1	4.000	192.0	30.000	0.785	1.152	0.049	23.562	0.049
T5	391 - 371	1	4.000	192.0	30.000	0.785	1.629	0.069	23.562	0.069
T6	371 - 351	1	4.000	192.0	30.000	0.785	2.108	0.089	23.562	0.089
T7	351 - 331	1	4.000	192.0	30.000	0.785	1.773	0.075	23.562	0.075
T8	331 - 311	1	4.000	192.0	30.000	0.785	1.498	0.064	23.562	0.064
T9	311 - 291	1	4.000	192.0	30.000	0.785	0.696	0.030	23.562	0.030
T10	291 - 271	1	4.000	192.0	30.000	0.785	1.016	0.043	23.562	0.043

<b>ERITowerBeta</b>		Electronics Research Inc. 7777 Gardner Road Chandler, IN Phone: 812-925-6000 FAX: 812-925-4026	
Job	Portland, ME 531' Guyed Tower	Project	12590 48" Face Run#2
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Client		SAGA Communications	
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Section No.	Elevation	Size	L	L <sub>n</sub>	K/r	F <sub>o</sub>	A	Actual	Allow.	Ratio
	f		f	f	ksi	ksi	in <sup>2</sup>	K	P	P <sub>o</sub>
T11	271 - 251	1	4.000	4.000	192.0	30.000	0.785	1.858	23.562	0.079
T12	251 - 231	1	4.000	4.000	192.0	30.000	0.785	2.706	23.562	0.115
T13	231 - 211	1	4.000	4.000	192.0	30.000	0.785	2.355	23.562	0.100
T14	211 - 191	1	4.000	4.000	192.0	30.000	0.785	1.970	23.562	0.084
T15	191 - 171	1	4.000	4.000	192.0	30.000	0.785	1.506	23.562	0.064
T16	171 - 151	1	4.000	4.000	192.0	30.000	0.785	0.650	23.562	0.028
T17	151 - 131	1	4.000	4.000	192.0	30.000	0.785	2.461	23.562	0.104
T18	131 - 111	1	4.000	4.000	192.0	30.000	0.785	4.227	23.562	0.179
T19	111 - 91	1	4.000	4.000	192.0	30.000	0.785	2.795	23.562	0.119
T20	91 - 71	1	4.000	4.000	192.0	30.000	0.785	2.047	23.562	0.087
T21	71 - 51	1	4.000	4.000	192.0	30.000	0.785	1.188	23.562	0.050
T22	51 - 31	1	4.000	4.000	192.0	30.000	0.785	0.415	23.562	0.018
T23	31 - 11	1	4.000	4.000	192.0	30.000	0.785	1.082	23.562	0.046
T24	11 - 1	1 1/4	3.967	3.967	152.3	30.000	1.227	7.203	36.816	0.196

### Bottom Girt Design Data (Tension)

Section No.	Elevation	Size	L	L <sub>n</sub>	K/r	F <sub>o</sub>	A	Actual	Allow.	Ratio
	f		f	f	ksi	ksi	in <sup>2</sup>	K	P	P <sub>o</sub>
E1	528 - 512.205	1 1/2x1/2	3.000	3.000	249.4	21.600	0.750	0.867	16.200	0.054
L2	512.205 - 496.41	1 1/2x1/2	3.000	3.000	249.4	21.600	0.750	1.820	16.200	0.112
L3	496.41 - 480.615	7/8	3.000	3.000	164.6	30.000	0.601	2.742	18.040	0.152
L4	480.615 - 470	12x1	3.000	3.000	124.7	30.000	12.000	3.386	360.000	0.009
T1	470 - 451	1	4.000	4.000	192.0	30.000	0.785	1.995	23.562	0.085
T2	451 - 431	1	4.000	4.000	192.0	30.000	0.785	1.287	23.562	0.055
T3	431 - 411	1	4.000	4.000	192.0	30.000	0.785	1.132	23.562	0.048
T4	411 - 391	1	4.000	4.000	192.0	30.000	0.785	1.578	23.562	0.067

Section No.	Elevation	Size	L	L <sub>n</sub>	K/r	F <sub>s</sub>	A	Actual	Allow.	Ratio
	f		f	f	ksi	in <sup>2</sup>	K	P	K	P <sub>a</sub>
T5	391 - 371	1	4.000	4.000	192.0	30.000	0.785	1.966	23.562	0.083
T6	371 - 351	1	4.000	4.000	192.0	30.000	0.785	1.893	23.562	0.080
T7	351 - 331	1	4.000	4.000	192.0	30.000	0.785	1.532	23.562	0.065
T8	331 - 311	1	4.000	4.000	192.0	30.000	0.785	0.721	23.562	0.031
T9	311 - 291	1	4.000	4.000	192.0	30.000	0.785	0.971	23.562	0.041
T10	291 - 271	1	4.000	4.000	192.0	30.000	0.785	1.806	23.562	0.077
T11	271 - 251	1	4.000	4.000	192.0	30.000	0.785	2.634	23.562	0.112
T12	251 - 231	1	4.000	4.000	192.0	30.000	0.785	2.204	23.562	0.094
T13	231 - 211	1	4.000	4.000	192.0	30.000	0.785	1.867	23.562	0.079
T14	211 - 191	1	4.000	4.000	192.0	30.000	0.785	1.360	23.562	0.058
T15	191 - 171	1	4.000	4.000	192.0	30.000	0.785	0.670	23.562	0.028
T16	171 - 151	1	4.000	4.000	192.0	30.000	0.785	2.381	23.562	0.101
T17	151 - 131	1	4.000	4.000	192.0	30.000	0.785	4.156	23.562	0.176
T18	131 - 111	1	4.000	4.000	192.0	30.000	0.785	2.795	23.562	0.119
T19	111 - 91	1	4.000	4.000	192.0	30.000	0.785	2.048	23.562	0.087
T20	91 - 71	1	4.000	4.000	192.0	30.000	0.785	1.200	23.562	0.051
T21	71 - 51	1	4.000	4.000	192.0	30.000	0.785	0.405	23.562	0.017
T22	51 - 31	1	4.000	4.000	192.0	30.000	0.785	1.194	23.562	0.051
T23	31 - 11	1	4.000	4.000	192.0	30.000	0.785	6.071	23.562	0.258

**Mid Girt Design Data (Tension)**

Section No.	Elevation	Size	L	L <sub>n</sub>	K/r	F <sub>s</sub>	A	Actual	Allow.	Ratio
	f		f	f	ksi	in <sup>2</sup>	K	P	K	P <sub>a</sub>
L1	528 - 512.205	7/8	3.000	3.000	164.6	30.000	0.601	0.037	18.040	0.002
L2	512.205 - 496.41	7/8	3.000	3.000	164.6	30.000	0.601	0.098	18.040	0.005
L3	496.41 -	7/8	3.000	3.000	164.6	30.000	0.601	0.197	18.040	0.011

Job	Portland, ME	531' Guyed Tower
Project	12590	48" Face Run#2
Client	SAGA Communications	
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Section No.	Elevation	Size	L	L <sub>u</sub>	K/r	F <sub>o</sub>	A	Actual	Allow.	Ratio
L4	480.615	7/8	3.000	3.000	164.6	30.000	0.601	0.266	18.040	0.015 ✓

**Top Guy Pull-Off Design Data (Tension)**

Section No.	Elevation	Size	L	L <sub>u</sub>	K/r	F <sub>o</sub>	A	Actual	Allow.	Ratio
T6	371 - 351	1 1/4	4.000	4.000	153.6	30.000	1.227	7.236	36.816	0.197 ✓
T12	251 - 231	1 1/4	4.000	4.000	153.6	30.000	1.227	9.838	36.816	0.267 ✓
T18	131 - 111	1 1/4	4.000	4.000	153.6	30.000	1.227	13.702	36.816	0.372 ✓

**Bottom Guy Pull-Off Design Data (Tension)**

Section No.	Elevation	Size	L	L <sub>u</sub>	K/r	F <sub>o</sub>	A	Actual	Allow.	Ratio
T12	251 - 231	1 1/4	4.000	4.000	153.6	30.000	1.227	6.350	36.816	0.172 ✓

**Torque-Arm Top Design Data**

Section No.	Elevation	Size	L	L <sub>u</sub>	K/r	F <sub>o</sub>	A	Actual	Allow.	Ratio
T12	251 - 231 (1972)	L4x4x1/2	7.236	7.236	71.2	21.600	3.750	13.935	81.000	0.172 ✓
T12	251 - 231 (1973)	L4x4x1/2	7.236	7.236	71.2	21.600	3.750	13.622	81.000	0.168 ✓
T12	251 - 231 (1982)	L4x4x1/2	7.236	7.236	71.2	21.600	3.750	12.834	81.000	0.158 ✓
T12	251 - 231 (1983)	L4x4x1/2	7.236	7.236	71.2	21.600	3.750	12.652	81.000	0.156 ✓
T12	251 - 231 (1988)	L4x4x1/2	7.236	7.236	71.2	21.600	3.750	14.365	81.000	0.177 ✓
T12	251 - 231 (1989)	L4x4x1/2	7.236	7.236	71.2	21.600	3.750	9.779	81.000	0.121 ✓

# ERITowerBeta

Electronics Research Inc.  
 7777 Gardner Road  
 Chandler, IN  
 Phone: 812-925-6000  
 FAX: 812-925-4026

Job	Portland, ME	531' Guyed Tower
Project	12590	48" Face Run#2
Client	SAGA Communications	
Date	11:18:49	10/18/04
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## Torque-Arm Bottom Design Data

Section No.	Elevation	Size	L	L <sub>n</sub>	Kl/r	F <sub>a</sub>	A	Actual	Allow.	Ratio
	f	L4x4x1/2	f	f	ksi	in <sup>2</sup>	P	K	P <sub>a</sub>	P / P <sub>a</sub>
T12	251 - 231 (1976)	L4x4x1/2	7.236	7.236	71.2	21.600	3.750	2.888	81.000	0.036
T12	251 - 231 (1977)	L4x4x1/2	7.236	7.236	71.2	21.600	3.750	6.199	81.000	0.077
T12	251 - 231 (1984)	L4x4x1/2	7.236	7.236	71.2	21.600	3.750	5.685	81.000	0.070
T12	251 - 231 (1990)	L4x4x1/2	7.236	7.236	71.2	21.600	3.750	3.077	81.000	0.038
T12	251 - 231 (1991)	L4x4x1/2	7.236	7.236	71.2	21.600	3.750	6.328	81.000	0.078

## Element Map

Section No.	Section Elevation	Component Type	Element List
L1	528.000-512.205	Latticed Pole Leg	1-3
L2	496.410	Latticed Pole	28-39,55-78
		Diagonal	5-6-8-9,11-12,14-15,17-18,20-27,41-42,44-45,47-54,80-81,83-84,86-87
		Latticed Pole K-	4,7,10
		Brace	79,82,85
		Latticed Pole	13,16,19,40,43,46
		Bottom Girt	88-90
		Latticed Pole Mid	115-126,142-165
		Girt	92-93,95-96,98-99,101-102,104-105,107-114,128-129,131-132,134-141,167-
		Latticed Pole Top	168,170-171,173-174
		Girt	91,94,97
L3	480.615	Latticed Pole	166,169,172
		Bottom Girt	100,103,106,127,130,133
		Latticed Pole Leg	175-177
		Latticed Pole	202-213,229-252
		Diagonal	179-180,182-183,185-186,188-189,191-192,194-201,215-216,218-219,221-
		Latticed Pole K-	228,254-255,257-258,260-261
		Brace	178,181,184
		Latticed Pole Top	253,256,259
		Girt	187,190,193,214,217,220
		Latticed Pole Mid	

<b>ERITOWERBeta</b>		Electronics Research Inc. 7777 Gardner Road Chandler, IN Phone: 812-925-6000 FAX: 812-925-4026	
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Section No.	Section Elevation	Component Type	Element List
L4	470.000-480.615	Latticed Pole Leg	262-264
T1	470.000-451.000	Latticed Pole Diagonal Latticed Pole K- Brace Latticed Pole Top Girt Latticed Pole Bottom Girt Latticed Pole Mid Girt	289-312 266-267,269-270,272-273,275-276,278-279,281-288,314-315,317-318,320-321 265,268,271 313,316,319 274,277,280 322-324
T2	451.000-431.000	Diagonal K-Brace Horizontal Secondary Top Girt Bottom Girt Guy A Guy B Guy C Index Plate Leg	338-343,346-351,354-359,362-367,370-375,378-383 331-336,386-391 337,345,353,361,369,377,385 344,352,360,368,376,384 325-327 328-330 1964 1963 1962 1997-1999 392-394
T3	431.000-411.000	Diagonal K-Brace Horizontal Secondary Top Girt Bottom Girt Leg	408-413,416-421,424-429,432-437,440-445,448-453 401-406,456-461 407,415,423,431,439,447,455 414,422,430,438,446,454 395-397 398-400 462-464
T4	411.000-391.000	Diagonal K-Brace Horizontal Secondary Top Girt Bottom Girt Leg	548-553,556-561,564-569,572-577,580-585,588-593 541-546,596-601 547,555,563,571,579,587,595 554,562,570,578,586,594 535-537 538-540 602-604
T5	391.000-371.000	Diagonal K-Brace Horizontal Secondary Top Girt Bottom Girt Leg	618-623,626-631,634-639,642-647,650-655,658-663 611-616,666-671 617,625,633,641,649,657,665 624,632,640,648,656,664 605-607 608-610



<b>ERTOWERBeta</b> Electronics Research Inc. 777 Gardner Road Chandler, IN Phone: 812-925-6000 FAX: 812-925-4026		Job Portland, ME 531' Guyed Tower	Project 12590 48" Face Run#2	Client SAGA Communications	Designed by M. Maurer
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Section No.	Section Elevation	Component Type	Element List
T6	371.000-351.000	Leg	672-674
T7	351.000-331.000	Diagonal K-Brace Horizontal Secondary Top Girt Bottom Girt Guy A Guy B Guy C Top Guy Pull-Off Leg	688-693,696-701,704-709,712-717,720-725,728-733 681-686,736-741 687,695,703,719,727,735 694,702,710,718,726,734 675-677 678-680 1969 1968 1965 711,1966-1967 742-744
T8	331.000-311.000	Diagonal K-Brace Horizontal Secondary Top Girt Bottom Girt Leg	758-763,766-771,774-779,782-787,790-795,798-803 751-756,806-811 757,765,773,781,789,797,805 764,772,780,788,796,804 745-747 748-750 812-814
T9	311.000-291.000	Diagonal K-Brace Horizontal Secondary Top Girt Bottom Girt Leg	828-833,836-841,844-849,852-857,860-865,868-873 821-826,876-881 827,835,843,851,859,867,875 834,842,850,858,866,874 815-817 818-820 882-884
T10	291.000-271.000	Diagonal K-Brace Horizontal Secondary Top Girt Bottom Girt Leg	968-973,976-981,984-989,992-997,1000-1005,1008-1013 961-966,1016-1021 967,975,983,991,999,1007,1015 974,982,990,998,1006,1014 955-957 958-960 1022-1024
T11	271.000-251.000	Diagonal K-Brace Horizontal Secondary Top Girt Bottom Girt Leg	1038-1043,1046-1051,1054-1059,1062-1067,1070-1075,1078-1083 1031-1036,1086-1091 1037,1045,1053,1061,1069,1077,1085 1044,1052,1060,1068,1076,1084 1025-1027 1028-1030 1092-1094
T12	251.000-231.000	Diagonal	1108-1113,1116-1121,1124-1129,1132-1137,1140-1145,1148-1153

<b>Job</b>		Portland, ME	531' Guyed Tower	Page	80 of 82
<b>Project</b>		12590	48" Face	Run#2	Date
<b>Client</b>		SAGA Communications			
<b>Designed by</b>		M. Maurer			

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 Chandler, IN  
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 FAX: 812-925-4026

Section No.	Section Elevation	Component Type	Element List
T13	231,000-211,000	Diagonal K-Brace Horizontal Secondary Top Girt Bottom Girt Guy A Guy B Guy C Top Guy Pull-Off Bottom Guy Pull-Off Torque Arm Top Torque Arm Bottom Leg	1178-1183,1186-1191,1194-1199,1202-1207,1210-1215,1218-1223 1171-1176,1226-1231 1177,1185,1193,1201,1209,1217,1225 1184,1192,1200,1208,1216,1224 1165-1167 1168-1170 1232-1234 1248-1253,1256-1261,1264-1269,1272-1277,1280-1285,1288-1293 1241-1246,1296-1301 1247,1255,1263,1271,1279,1287,1295 1254,1262,1270,1278,1286,1294 1235-1237 1238-1240 1302-1304 Diagonal K-Brace Horizontal Secondary Top Girt Bottom Girt Leg
T14	211,000-191,000	Diagonal K-Brace Horizontal Secondary Top Girt Bottom Girt Leg	1318-1323,1326-1331,1334-1339,1342-1347,1350-1355,1358-1363 1311-1316,1366-1371 1317,1325,1333,1341,1349,1357,1365 1324,1332,1340,1348,1356,1364 1305-1307 1308-1310 1372-1374 Diagonal K-Brace Horizontal Secondary Top Girt Bottom Girt Leg
T15	191,000-171,000	Diagonal K-Brace Horizontal Secondary Top Girt Bottom Girt Leg	1388-1393,1396-1401,1404-1409,1412-1417,1420-1425,1428-1433 1381-1386,1436-1441 1387,1395,1403,1411,1419,1427,1435 1394,1402,1410,1418,1426,1434 1375-1377 1378-1380 1442-1444 Diagonal K-Brace Horizontal Secondary Top Girt Bottom Girt Leg
T16	171,000-151,000	Diagonal K-Brace Horizontal Secondary Top Girt Bottom Girt Leg	1458-1463,1466-1471,1474-1479,1482-1487,1490-1495,1498-1503 1451-1456,1506-1511 1457,1465,1473,1481,1489,1497,1505 1464,1472,1480,1488,1496,1504 1445-1447 1448-1450 1512-1514 Diagonal K-Brace Horizontal Secondary Top Girt Bottom Girt Leg
T17	151,000-131,000	Diagonal K-Brace Horizontal Secondary Top Girt Bottom Girt Leg	1388-1393,1396-1401,1404-1409,1412-1417,1420-1425,1428-1433 1381-1386,1436-1441 1387,1395,1403,1411,1419,1427,1435 1394,1402,1410,1418,1426,1434 1375-1377 1378-1380 1442-1444 Diagonal K-Brace Horizontal Secondary Top Girt Bottom Girt Leg
T18	131,000-131,000	Diagonal K-Brace Horizontal Secondary Top Girt Bottom Girt Leg	1458-1463,1466-1471,1474-1479,1482-1487,1490-1495,1498-1503 1451-1456,1506-1511 1457,1465,1473,1481,1489,1497,1505 1464,1472,1480,1488,1496,1504 1445-1447 1448-1450 1512-1514 Diagonal K-Brace Horizontal Secondary Top Girt Bottom Girt Leg

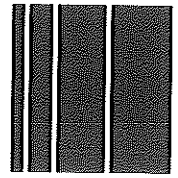
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Electronics Research Inc.  
7777 Gardner Road  
Chandler, IN  
Phone: 812-925-6000  
FAX: 812-925-4026

Job	Portland, ME	531' Guyed Tower	Page	81 of 82
Project	12590	48" Face	Date	11:18:49 10/18/04
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Section No.	Section Elevation	Component Type	Element List
T19	111,000-91,000	Diagonal K-Brace Horizontal Top Girt Bottom Girt Guy A Guy B Guy C Top Guy Pull-Off	1528-1533,1536-1541,1544-1549,1552-1557,1560-1565,1568-1573 1527,1535,1543,1559,1567,1575 1534,1542,1550,1558,1566,1574 1515-1517 1518-1520 1996 1995 1992 1551,1993-1994 1582-1584 1598-1603,1606-1611,1614-1619,1622-1627,1630-1635,1638-1643 1591-1596,1646-1651 1597,1605,1613,1621,1629,1637,1645 1604,1612,1620,1628,1636,1644 1585-1587 1588-1590 1652-1654 1668-1673,1676-1681,1684-1689,1692-1697,1700-1705,1708-1713 1661-1666,1716-1721 1667,1675,1683,1691,1699,1707,1715 1674,1682,1690,1698,1706,1714 1655-1657 1658-1660 Leg Bottom Girt Diagonal K-Brace Horizontal Secondary Top Girt Horizontal Top Girt
T20	91,000-71,000	Diagonal K-Brace Horizontal Secondary Top Girt Bottom Girt Leg	1722-1724 1738-1743,1746-1751,1754-1759,1762-1767,1770-1775,1778-1783 1731-1736,1786-1791 1737,1745,1753,1761,1769,1777,1785 1744,1752,1760,1768,1776,1784 1725-1727 1728-1730 1792-1794 1808-1813,1816-1821,1824-1829,1832-1837,1840-1845,1848-1853 1801-1806,1856-1861 1807,1815,1823,1831,1839,1847,1855 1814,1822,1830,1838,1846,1854 1795-1797 1798-1800 1862-1864 1878-1883,1886-1891,1894-1899,1902-1907,1910-1915,1918-1923 1871-1876,1926-1931 1877,1885,1893,1901,1909,1917,1925 1884,1892,1900,1908,1916,1924 1865-1867 1868-1870 Leg Bottom Girt Top Girt Horizontal Diagonal K-Brace Horizontal Secondary Top Girt
T21	71,000-51,000	Diagonal K-Brace Horizontal Secondary Top Girt Bottom Girt Leg	1725-1727 1728-1730 1792-1794 1808-1813,1816-1821,1824-1829,1832-1837,1840-1845,1848-1853 1801-1806,1856-1861 1807,1815,1823,1831,1839,1847,1855 1814,1822,1830,1838,1846,1854 1795-1797 1798-1800 1862-1864 1878-1883,1886-1891,1894-1899,1902-1907,1910-1915,1918-1923 1871-1876,1926-1931 1877,1885,1893,1901,1909,1917,1925 1884,1892,1900,1908,1916,1924 1865-1867 1868-1870 Leg Bottom Girt Top Girt Horizontal Diagonal K-Brace Horizontal Secondary Top Girt
T22	51,000-31,000	Diagonal K-Brace Horizontal Secondary Top Girt Bottom Girt Leg	1725-1727 1728-1730 1792-1794 1808-1813,1816-1821,1824-1829,1832-1837,1840-1845,1848-1853 1801-1806,1856-1861 1807,1815,1823,1831,1839,1847,1855 1814,1822,1830,1838,1846,1854 1795-1797 1798-1800 1862-1864 1878-1883,1886-1891,1894-1899,1902-1907,1910-1915,1918-1923 1871-1876,1926-1931 1877,1885,1893,1901,1909,1917,1925 1884,1892,1900,1908,1916,1924 1865-1867 1868-1870 Leg Bottom Girt Top Girt Horizontal Diagonal K-Brace Horizontal Secondary Top Girt
T23	31,000-11,000	Diagonal K-Brace Horizontal Secondary Top Girt Bottom Girt Leg	1725-1727 1728-1730 1792-1794 1808-1813,1816-1821,1824-1829,1832-1837,1840-1845,1848-1853 1801-1806,1856-1861 1807,1815,1823,1831,1839,1847,1855 1814,1822,1830,1838,1846,1854 1795-1797 1798-1800 1862-1864 1878-1883,1886-1891,1894-1899,1902-1907,1910-1915,1918-1923 1871-1876,1926-1931 1877,1885,1893,1901,1909,1917,1925 1884,1892,1900,1908,1916,1924 1865-1867 1868-1870 Leg Bottom Girt Top Girt Horizontal Diagonal K-Brace Horizontal Secondary Top Girt
T24	11,000-1,000	Diagonal K-Brace Horizontal Secondary Top Girt Bottom Girt Leg	1725-1727 1728-1730 1792-1794 1808-1813,1816-1821,1824-1829,1832-1837,1840-1845,1848-1853 1801-1806,1856-1861 1807,1815,1823,1831,1839,1847,1855 1814,1822,1830,1838,1846,1854 1795-1797 1798-1800 1862-1864 1878-1883,1886-1891,1894-1899,1902-1907,1910-1915,1918-1923 1871-1876,1926-1931 1877,1885,1893,1901,1909,1917,1925 1884,1892,1900,1908,1916,1924 1865-1867 1868-1870 Leg Bottom Girt Top Girt Horizontal Diagonal K-Brace Horizontal Secondary Top Girt
			Total number of elements: 1999

<b>ERITowerBeta</b> Electronics Research Inc. 7777 Gardner Road Portland, ME 04106 Phone: 812-925-6000 FAX: 812-925-4026		Program Version: 16/11/2004 Client: PortlandME/12590_PortlandME/12590v2_531_48_80.eri
Job Portland, ME 53' Guyed Tower	Project 12590 48" Face Run#2	Date 11:18:49 10/18/04
Page 82 of 82	Designed by M. Maurer	



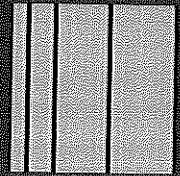
# **Report on Subsurface and Foundation Investigation**

## **Radio Tower Presumpscot Street Portland, Maine**

for

Portland Radio Group  
420 Western Avenue  
South Portland, ME 04106

May 3, 2004



May 3, 2004  
03497

Andrew T. Armstrong, Chief Engineer  
Portland Radio Group  
420 Western Avenue  
South Portland, ME 04106

**Report on Subsurface and Foundation Investigation  
Radio Tower, Presumpscot Street, Portland, Maine**

Dear Mr. Armstrong:

This report presents the results of our subsurface and foundation investigation for the proposed radio tower on Presumpscot Street near the St. Lawrence Cement Terminal in Portland, Maine.

In summary, we recommend that the tower base and guy anchor foundations be supported on steel H-piles or concrete filled steel pipe piles driven to end bearing in the glacial till or on the underlying bedrock. Uplift loads on the guy anchor foundations may be resisted by permanent rock anchors grouted into bedrock installed in permanent casings opposite the piles or, for the guy anchor in water, piles driven into glacial till. Specific recommendations regarding foundation design and construction considerations are presented below.

**Introduction**

The site, located near the St. Lawrence Cement Terminal at 189 Presumpscot Street, is the location of a former radio tower that experienced a failure of a guy anchor. Based on readily available plans, we understand that the former tower base was supported on a reinforced concrete footing and pier, and the tower was stabilized with three equally spaced guy anchors with foundations consisting of buried concrete deadmen. Ground surfaces at the site vary from approximately El. 0.0 to El. 20.0. Ground surface elevations in this report are in feet and referenced to North American Vertical Datum 1988 (NAVD).

**Proposed Construction**

We understand that the replacement tower will have a height of approximately 525 feet and the base and guy anchor locations will be at approximately the same locations as the original tower. The tower will be stabilized with three pairs of equally spaced guy anchors. Electronics Research, Inc., the tower designer, provided the following loads for the tower and guys:

Tower Base:	250 kips axial (downward) 6 kips lateral
Guys	
Inner (R = 305 ft.)	33 kips vertical (uplift) 63 kips lateral 71 kips resultant
Outer (R = 325 ft.)	51 kips vertical (uplift) 43 kips lateral 67 kips resultant

Two pairs of guy anchors will be located in the upland portion of the site and one pair will be located in the tidal marsh. We understand that the preferred guy anchor foundation, especially in the tidal marsh, will be a pile supported concrete cap (a "mini dolphin") to permit observation and preventive maintenance of the guy cable connections.

### Subsurface Explorations

During the period January 12 to January 16, 2004 and January 22 to January 23, 2004, Maine Test Borings, Inc. (MTB) drilled six borings, B101 to B106, at the site at locations shown on Sheet 3, Site Plan. MTB drilled the borings to depths below ground surface varying from 20.8 feet to 59.3 feet. Sebago Technics monitored the borings and prepared the logs included in Appendix A. Results of the borings are summarized on Table I.

Borings B102 and B104 were drilled using 2.5-inch diameter hollow stem augers. Borings B101, B103, B105 and B106 were drilled using 3-inch diameter casing. Standard Penetration Resistance (N) was measured at each sample interval in the overburden soil in accordance with ASTM Test Designation D1586. Samples were taken at 5-foot intervals. Bedrock was cored in borings B101, B103, B105 and B106.

Boring B106 was drilled at the edge of the upper marsh grass, approximately 40 feet from the previous guy anchor location. In our judgment, this was as close to the former anchor location as we could get with conventional drilling equipment. We estimate that to drill at the former guy anchor location would require mobilizing specialized water borne equipment and a permit from the Maine Department of Environmental Protection (MDEP).

Sebago Technics laid out the borings and determined as-drilled locations and ground surface elevations by survey methods.

The boring logs and related information depict subsurface conditions and water levels encountered at the locations and during the times indicated on the logs. Subsurface conditions at other locations may differ from those encountered in the explorations. The passage of time may result in a change in groundwater conditions at the exploration locations.

### Subsurface Conditions

The borings encountered three principal soil units overlying bedrock at the site: fill, marine deposit and glacial till. Encountered thickness and generalized descriptions of the strata encountered are presented below in order of increasing depth below ground surface.

**Fill** – Fill consists of loose, dark brown to brown to yellow brown silty SAND (SM); to poorly-graded SAND with gravel (SP); to sandy SILT (ML); to well-graded SAND with gravel (SW). Encountered thickness varied from 1.5 feet to 10.0 feet.

**Marine Deposit** – The marine deposit consists of sand, silt and clay. Marine sand consists of very loose to dense, olive brown to gray brown silty SAND (SM); to poorly-graded SAND (SP). Encountered thickness of marine sand varied from 3.3 feet to 14.2 feet. Marine silt consists of very soft to hard, olive brown to gray brown SILT (ML), trace shell fragments. Encountered thickness of silt varied from 0.1 foot to 19.5 feet. Marine clay consists of medium stiff to stiff, gray lean CLAY (CL), trace shell fragments, sand and black streaks. Encountered thickness of marine clay in B106 was 13.0 feet.

**Glacial Till** – Glacial till consists of very loose to very dense, brown to gray silty SAND with gravel (SM); to well-graded SAND with gravel (SW). Encountered thickness varied from 0.8 foot to 11.3 feet.

Bedrock was encountered at depths below ground surface varying from 20.8 feet to 48.7 feet. Bedrock consists of medium to moderately hard, fresh, reddish gray, fine to medium grained BIOTITE SCHIST with quartz veins. Joint spacing is very close to moderate to wide, moderately dipping, undulating, mostly fresh and tight with minor silt infilling. Core recovery varied from 93 to 100 percent, and rock quality designation (RQD) varied from 93 to 100 percent at the guy locations to 46 percent at the tower base. The upper 0.2 foot to 0.8 foot of bedrock was weathered.

Water was encountered in the borings at depths below ground surface varying from 7.4 feet to 15.7 feet in borings B101 to B105. B106 was drilled within the tidal zone. However, observations of water were made over a short period of time and do not represent the stabilized groundwater levels. In addition, groundwater levels at the site will vary with season, precipitation, temperature, tidal fluctuations and construction activities in the area. Therefore, water conditions during and following construction will vary from those observed in the borings.

### Recommendations for Foundation Design

#### Recommended Foundation Type and Design Criteria

The fill and loose marine soils are not considered suitable for support of the tower base or guy foundations. In our opinion, the tower base and guy anchors may be supported on steel piles driven to refusal in the underlying glacial till or bedrock. We recommend that uplift loads be resisted by permanent rock anchors drilled and grouted into the underlying bedrock for guy anchors on land and steel piles for the guy anchor located in the marsh..



In our opinion, steel H-piles or open-end steel pipe piles are the most appropriate pile type. Steel H-piles should consist of HP 12x53, 50-ksi steel, with design capacity of 100 kips and ultimate capacity of 300 kips. Steel pipe piles should consist of 12.75-inch diameter with 0.25-inch wall thickness, 50-ksi steel with design capacity of 100 kips and ultimate capacity of 300 kips. After driving, the inside of the pipe piles should be cleaned to the top of the bedrock surface and concreted. Piles should be fitted with driving points to protect the tips except for the uplift piles at the guy anchor in the marsh, which should be driven without driving tips. Anticipated pile lengths vary from approximately 20 feet to 55 feet. Piles should be spaced at least 3-feet on center when groups are required. Compression piles in the guy anchors located in the marsh should be considered as unsupported to a depth of 15 feet below mud line.

The bottoms of pile caps in the upland areas should be founded a minimum of 4.5 feet below the lowest surface exposed to freezing. In the marsh, we anticipate that the MDEP will require the bottom of pile cap to be no lower than 1 foot above the highest spring tide (El. 7.0), which would place the bottom of cap at El. 8. Piles for the guy anchor located in the marsh should be epoxy coated to at least 15 feet below mud line to minimize corrosion of the steel in the salt water. To resist lateral loads, piles should be driven at a batter no flatter than 5 horizontal to 12 vertical.

We anticipate that three piles will be required to support the tower at the base and that each upland guy anchor will require two piles in compression and two rock anchors in tension. The guy anchors in the marsh will require two piles in compression and three piles in tension.

The piles should be driven to bearing in the glacial till or bedrock with a hammer delivering a minimum of 25,000 ft. lbs. of energy per blow. A final penetration resistance equal to 10 blows per inch for the final 6 inches of driving should be required. If abrupt refusal is encountered, driving may be terminated when the pile penetration is less than ½-inch for 10 successive blows.

Prior to installation, one of the piles should be load tested to three times the design capacity. In our opinion, in lieu of a pile load test, the contractor may monitor the installation of a minimum of three production piles at separate locations using the Case-Goble Pile Driving Analyzer (PDA) equipment to verify that the piles achieve three times the design capacity with acceptable driving stresses. Monitoring with PDA in lieu of a load test will require the approval of local building officials.

We recommend that permanent rock anchors resist the uplift loads for guys located in the upland. For the guys located in the marsh, we recommend that the uplift loads for each guy be resisted by three piles. Rock anchors should consist of grouted, high strength continuous upset threaded steel bars with double corrosion protection. Grout should consist of Portland cement or epoxy resin, provided the resin is installed by tremie grouting. We do not recommend the use of epoxy resin installed in the anchor hole using "sausage" containers due to the uncertainty regarding even and complete distribution of resin around the bar and the potential impacts of long-term corrosion if not fully encapsulated. We recommend a working bond strength between grout (both Portland cement and epoxy resin) and rock of 50 lbs. per sq. in. (psi). Cement grout should consist of high strength cement grout having a minimum 28-day compressive strength of 6,000 psi.

We recommend that each anchor have a minimum design capacity of 140 kips and the design capacity be used to resist the ultimate uplift loads. This would require a bar with a minimum diameter of 1.375 in. with yield strength of 150 kips per sq. in. (ksi). Anchors should have a minimum bond length of 26 feet in bedrock.

Rock anchors should be installed to at least 26 feet below the top of rock. The portion of the anchor passing through the overburden soil will serve as the stressing length. Rock anchors will vary in length depending on the top of bedrock at the proposed location. We anticipate rock anchor lengths varying from approximately 46 feet to 80 feet. Additional bar length will be required to penetrate through the foundations and for lock off.

Anchors should be installed in minimum 4-inch diameter holes drilled into the bedrock using air percussion drilling techniques to provide a rough surface. Holes should be drilled at least 6 inches deeper than the depth required to provide for accumulation of drill cuttings and debris that is not removed from the holes. All rock anchors should be proof tested to 150% of design capacity and post-tensioned (locked off) at design capacity. Anchors should be installed with a minimum spacing of 3 feet on center. To resist lateral loads, anchors should be installed at a batter no flatter than 1 horizontal to 4 vertical. The batter direction should be opposite that of the piles. Each guy anchor will then be supported by two piles for compression and lateral resistance, two rock anchors for uplift and lateral resistance, battered in opposite directions.

Rock anchors will require permanent steel casings. Casings should be sized to permit drilling a 4-inch hole in bedrock, seated in bedrock and tremie grouted to the top of casing during anchor grouting.

### Construction Considerations

#### General

The primary purpose of this section of the report is to comment on items related to excavation, earthwork, pile driving, rock anchor installation, and related geotechnical aspects of proposed construction. It is written primarily for the engineer having responsibility for preparation of plans and specifications. Since it identifies potential construction problems related to foundations and earthwork, it will also aid personnel who monitor the construction activity.

#### Excavation, Lateral Support and Control of Water

We anticipate that foundation excavation can be accomplished with sloped open excavation through the overburden soils provided safe side slopes can be maintained. Some sloughing and raveling should be anticipated in temporary slopes. Temporary excavations should be made in accordance with all OSHA and other applicable regulatory agency requirements.

We anticipate that groundwater may be encountered at proposed subgrade level or bottom of pile caps. If encountered, open pumping from sumps can likely control groundwater. In general, the contractor should control groundwater and water from runoff and other sources by methods which prevent disturbance of bearing surfaces or adjacent soils and allow construction in-the-dry.

Pile Installation

Pile driving may encounter obstructions in the form of railroad ties and timber. Obstructions will likely be near the ground surface and may be removed by excavation.

Construction Monitoring

The foundation recommendations contained herein are based on the known and predictable behavior of a properly engineered and constructed foundation. Monitoring of the foundation construction is required to enable the geotechnical engineer to keep in contact with procedures and techniques used in construction. Therefore, we recommend that a person qualified by training and experience be present to provide monitoring at the site during pile driving and rock anchor installation and testing. Sebago Technics is available to provide these services.

Limitations of Recommendations

This report has been prepared for specific application to the subject project in accordance with generally accepted geotechnical engineering practices. In the event that any changes in the nature, design or location of the tower are planned, the conclusions and recommendations contained in this report should not be considered valid unless the changes are reviewed and the conclusions of this report modified or verified in writing.

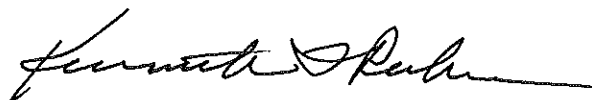
The recommendations presented herein are based in part upon the data obtained from the referenced test borings. The nature and extent of variations from that disclosed by the explorations may not become evident until construction. If variations then appear evident, it will be necessary to re-evaluate the recommendations of this report.

We recommend that we be provided with the opportunity for a general review of final design and specifications in order to determine that our earthwork and foundation recommendations have been interpreted and implemented in the design and specifications as they were intended.

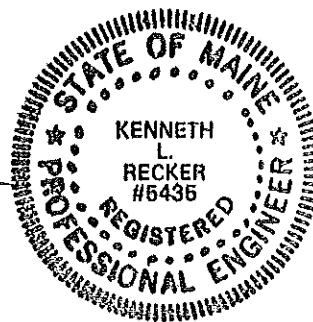
It has been a pleasure to work with you on this project. Please do not hesitate to contact us if you have questions or need more information.

Sincerely,

SEBAGO TECHNICS, INC.



Kenneth L. Recker  
Geotechnical Engineering Manager



KLR:klr/jc

Enclosures:

- |            |  |
|------------|--|
| Table I    | - Summary of Borings                   |
| Sheet 1    | - Site and Subsurface Exploration Plan |
| Appendix A | - Logs of Test Borings                 |

**TABLE I**  
**SUMMARY OF BORINGS**  
**PROPOSED RADIO TOWER**  
**PRESUMPCOT STREET**  
**PORTLAND, MAINE**

Boring No.	Depth (Ft.)	Ground Surface El. (Ft.)	Depth to Water (Ft.)	Strata Thickness (Ft.)							El. Top of Rock (Ft.)
				Fill	Marine Sand	Marine Silt	Marine Clay	Marine Sand	Glacial Till	Weathered Bedrock	
B101	34.6	8.1	NE	8.0	7.0	10.0	--	4.0	0.8	0.8	4.0*
B102	20.8	13.1	15.7	8.5	5.5	--	--	4.0	2.8	--	0.0*
B103	30.6	15.4	10.0	6.6	--	0.1	--	3.3	11.3	0.2	9.1*
B104	33.6	17.0	8.7	4.0	--	12.3	--	12.2	5.1	--	0.0*
B105	42.2	18.5	7.4	10.0	--	8.0	--	14.2	--	--	10.0*
B106	59.3	2.0	Tidal Zone	1.5	--	19.5	13.0	12.3	2.4	0.6	10.0*

## NOTES:

1. ELEVATIONS REFERENCED TO NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD 88).
2. -- INDICATES STRATUM NOT ENCOUNTERED WITHIN DEPTH OF BORINGS.
3. NE INDICATES GROUNDWATER NOT ENCOUNTERED WITHIN DEPTH OF BORING.
4. \* INDICATED DEPTH OF PENETRATION INTO STRATUM.

**Joint Coordinates and Temperatures**

	Label	X (ft)	Y (ft)	Z (ft)	Temp (F)	Detach From Diap...
1	N1	4.5	0	-2	0	
2	N2	4.5	0	2	0	
3	N3	14	0	-2	0	
4	N4	14	0	2	0	
5	N5	18	0	-2	0	
6	N6	18	0	2	0	
7	N7	22	0	-2	0	
8	N8	22	0	2	0	
9	N9	11	0	0	0	
10	N10	14.7	0	0	0	
11	N11	22	0	0	0	
12	N12	10	-18	-4.5	0	
13	N13	10	-18	4.5	0	
14	N14	8	-18	-2	0	
15	N15	8	-18	2	0	
16	N16	23.92	-18	-3	0	
17	N17	23.92	-18	3	0	
18	N18	26.9	-18	-5.5	0	
19	N19	26.9	-18	5.5	0	

**Member Primary Data**

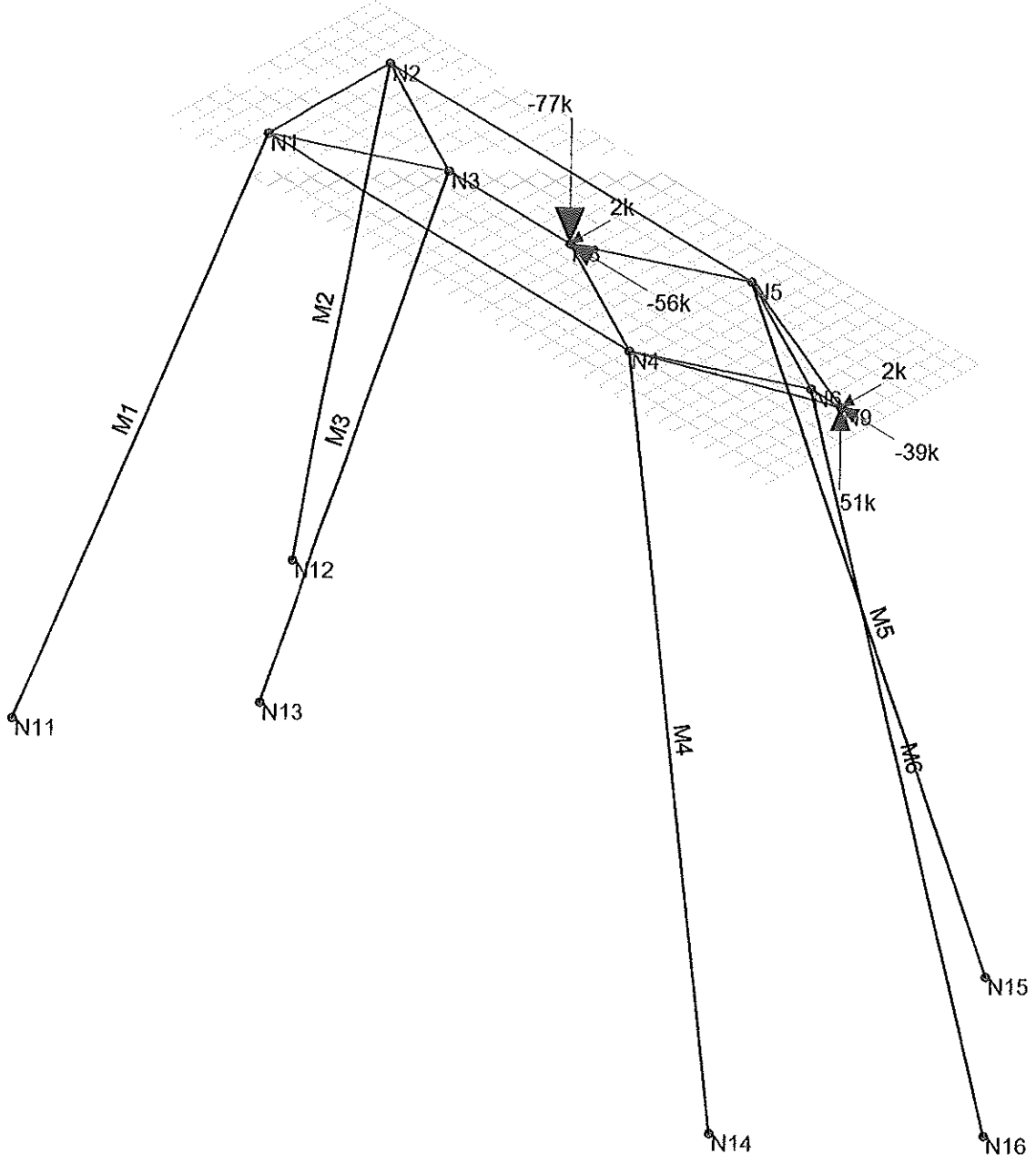
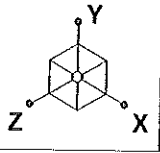
	Label	I Joint	J Joint	K Joint	Rotate(deg)	Section/Shape	Design List	Type	Material	Design Rules
1	M1	N1	N12			HP12X53	None	Column	A572Grad...	Typical
2	M2	N2	N13			HP12X53	None	Column	A572Grad...	Typical
3	M3	N3	N14			HP12X53	None	Column	A572Grad...	Typical
4	M4	N4	N15			HP12X53	None	Column	A572Grad...	Typical
5	M5	N5	N16			HP12X53	None	Column	A572Grad...	Typical
6	M6	N6	N17			HP12X53	None	Column	A572Grad...	Typical
7	M7	N7	N18			HP12X53	None	Column	A572Grad...	Typical
8	M8	N8	N19			HP12X53	None	Column	A572Grad...	Typical

**Joint Loads and Enforced Displacements (BLC 1 : Dead)**

	Joint Label	L,D,M	Direction	Magnitude[k,k-ft in,rad k*s^2/ft]
1	N9	L	Y	-122
2	N1	L	Y	-6.8
3	N2	L	Y	-6.8
4	N3	L	Y	-6.8
5	N4	L	Y	-6.8
6	N5	L	Y	-6.8
7	N6	L	Y	-6.8
8	N7	L	Y	-6.8
9	N8	L	Y	-6.8

**Joint Loads and Enforced Displacements (BLC 2 : Radial)**

	Joint Label	L,D,M	Direction	Magnitude[k,k-ft in,rad k*s^2/ft]
1	N10	L	X	-56
2	N11	L	X	-39



Loads: LC 2, D + R + V + T + SW

Associated Design Partner...  
 Bob Arledge  
 04053

Anchor 1 & 2 - 040924\_0700

Oct 13, 2004 at 11:22 AM  
 Anchors 1 & 2.r3d

**Joint Coordinates and Temperatures**

	Label	X (ft)	Y (ft)	Z (ft)	Temp (F)	Detach From Diap...
1	N1	2	0	2	0	
2	N2	2	0	-2	0	
3	N3	6	0	0	0	
4	N4	14	0	2	0	
5	N5	14	0	-2	0	
6	N6	18	0	0	0	
7	N8	10	0	0	0	
8	N9	19	0	0	0	
9	N11	-3.5	-18	4.5	0	
10	N12	-3.5	-18	-4.5	0	
11	N13	0	-18	0	0	
12	N14	19.5	-18	4.5	0	
13	N15	19.5	-18	-4.5	0	
14	N16	24	-18	0	0	

**Member Primary Data**

	Label	I Joint	J Joint	K Joint	Rotate(deg)	Section/Shape	Design List	Type	Material	Design Rules
1	M1	N1	N11			HP12X53	None	Column	A572Grad...	Typical
2	M2	N2	N12			HP12X53	None	Column	A572Grad...	Typical
3	M3	N3	N13			HP12X53	None	Column	A572Grad...	Typical
4	M4	N4	N14			HP12X53	None	Column	A572Grad...	Typical
5	M5	N5	N15			HP12X53	None	Column	A572Grad...	Typical
6	M6	N6	N16			HP12X53	None	Column	A572Grad...	Typical

**Joint Loads and Enforced Displacements (BLC 1 : Dead)**

	Joint Label	L,D,M	Direction	Magnitude(k,k-ft in,rad k*s^2/ft)
1	N8	L	Y	-109

**Joint Loads and Enforced Displacements (BLC 2 : Radial)**

	Joint Label	L,D,M	Direction	Magnitude(k,k-ft in,rad k*s^2/ft)
1	N8	L	X	-56
2	N9	L	X	-39

**Joint Loads and Enforced Displacements (BLC 3 : Tangential)**

	Joint Label	L,D,M	Direction	Magnitude(k,k-ft in,rad k*s^2/ft)
1	N8	L	Z	2
2	N9	L	Z	2

**Joint Loads and Enforced Displacements (BLC 4 : Vertical)**

	Joint Label	L,D,M	Direction	Magnitude(k,k-ft in,rad k*s^2/ft)
1	N9	L	Y	51
2	N8	L	Y	32

**Basic Load Cases**

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distributed Area (Me...	Surface (...)
1	Dead	DL				1			
2	Radial	WL				2			
3	Tangential	WL				2			

Company : Associated Design Partners, Inc.  
 Designer : Bob Arledge  
 Job Number : 04053

Anchor 1 & 2 - 040924\_0700

Oct 13, 2004  
 9:20 AM  
 Checked By: \_\_\_\_\_

**Basic Load Cases (Continued)**

BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distributed Area (Me...	Surface (...)
4 Vertical	WL				2			
5 Pile Selfweight	DL		-1					

**Load Combinations**

Description	Solve PD..	SRSS	BLC Factor	BLC Factor	BLC Factor	BLC Factor	BLC Factor	BLC Factor	BLC Factor	BLC Factor	BLC Factor	BLC Factor	BLC Factor	BLC Factor
1 D+R+V+SW	Yes		1	1	2	1	4	1	5	1				
2 D+R+V+T+..	Yes		1	1	2	1	3	1	4	1	5	1		
3 D+SW	Yes		1	1	5	1								

**Envelope Member Section Forces**

Member	Sec		Axial[k]	lc	y Shear[k]	lc	z Shear[k]	lc	Torque(k-ft)	lc	y-y Momen...	lc	z-z Momen...	lc	
1	M1	1	max	55.616	1	.478	1	.321	2	0	1	.22	3	6.044	1
2			min	20.435	3	.106	3	-.012	3	0	1	-6.09	2	-1.008	3
3		2	max	56.565	1	.159	1	.321	2	0	1	0	1	0	1
4			min	21.384	3	-.212	3	-.012	3	0	1	0	1	0	1
5	M2	1	max	69.553	2	.538	2	.234	2	0	1	.819	1	7.184	2
6			min	20.435	3	.106	3	-.043	1	0	1	-4.451	2	-1.008	3
7		2	max	70.502	2	.219	2	.234	2	0	1	0	1	0	1
8			min	21.384	3	-.212	3	-.043	1	0	1	0	1	0	1
9	M3	1	max	61.512	2	.504	2	.069	2	0	1	0	3	6.565	2
10			min	18.901	3	.097	3	0	1	0	1	-1.316	2	-1.154	3
11		2	max	62.461	2	.188	2	.069	2	0	1	0	1	0	1
12			min	19.851	3	-.219	3	0	1	0	1	0	1	0	1
13	M4	1	max	0	3	.051	2	.369	2	0	1	0	3	0	3
14			min	-72.197	1	-.073	1	0	3	0	1	-7.016	2	-4.408	1
15		2	max	0	3	0	3	.369	2	0	1	0	1	0	1
16			min	-71.247	1	-.391	1	0	3	0	1	0	1	0	1
17	M5	1	max	0	3	0	3	.283	2	0	1	.819	1	0	3
18			min	-101.058	2	-.197	2	-.043	1	0	1	-5.377	2	-6.768	2
19		2	max	0	3	0	3	.283	2	0	1	0	1	0	1
20			min	-100.108	2	-.516	2	-.043	1	0	1	0	1	0	1
21	M6	1	max	56.4	3	.26	3	0	1	0	1	0	3	1.936	3
22			min	0	1	0	1	0	3	0	1	0	1	0	1
23		2	max	57.349	3	0	1	0	1	0	1	0	1	0	1
24			min	0	1	-.056	3	0	3	0	1	0	1	0	1



---

# **Appendix A**

## **Logs of Test Borings**

PROJECT RADIO TOWER  
 LOCATION PRESUMPCOT STREET, PORTLAND, MAINE  
 CLIENT PORTLAND RADIO GROUP  
 CONTRACTOR MAINE TEST BORINGS, INC.  
 DRILLER T. SCHAEFER

STI JOB NO. 03497  
 PROJECT MGR. C. BROWN  
 FIELD REP. B. ESTES  
 DATE STARTED 1/15/04  
 DATE FINISHED 1/16/04

Elevation	8.1	ft	Datum	NAVD 88	Boring Location	See Plan
Item	Casing	Sampler	Core Barrel	Rig Make & Model	Mobile B-34	Hammer Type
Type	NW	SS	NQ	<input type="checkbox"/> Truck <input type="checkbox"/> Tripod	<input type="checkbox"/> Cat-Head	<input type="checkbox"/> Safety
Inside Diameter (in.)	3.0	1.375	2	<input type="checkbox"/> ATV <input type="checkbox"/> Geoprobe	<input checked="" type="checkbox"/> Winch	<input type="checkbox"/> Doughnut <input type="checkbox"/> Bentonite
Hammer Weight (lb.)	300	140		<input checked="" type="checkbox"/> Track <input type="checkbox"/> Air Track	<input checked="" type="checkbox"/> Roller Bit	<input type="checkbox"/> Automatic <input checked="" type="checkbox"/> None
Hammer Fall (in.)	16	30		<input type="checkbox"/> Skid <input type="checkbox"/>	<input type="checkbox"/> Cutting Head	<input checked="" type="checkbox"/>
Drilling Notes:						
						Casing Advance
						Type Method Depth
						NW/DRIVE/30.6 ft.

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel		Sand		Field Test					
								% Coarse	% Fine	% Coarse	% Medium	% Fine	Dilatancy	Toughness	Plasticity	Strength	
0	7 5 3 2	S1 1	1.0 3.0			SP	Loose, brown poorly-graded SAND with gravel (SP), mps = 1.4 in., moist, trace organics, frozen to 1.0 ft.	15	10	5	25	40	5				
							-FILL-										
5	WOH 7 10	S2 3	5.0 7.0		8.0	SP	Very loose, brown poorly-graded SAND with gravel (SP), mps = 1.4 in., moist, trace organics WOOD, possible voids below wood	15	10	5	25	40	5				
							-FILL-										
10	WOR WOR WOH WOH	S3 1	10.0 12.0			SM	Very loose, black silty SAND (SM), mps = 0.1 in., wet, strong petroleum odor, creosote, trace organics			10	70	20					
							-MARINE DEPOSIT-										
15	3 1 2 3	S4 14	15.0 17.0		15.0	ML	Soft, olive brown SILT with sand (ML), mps = 0.04 in., moist, trace organics				20	80	N	L	L		
20	1 1 2 4	S5 21	20.0 22.0			ML	Soft, gray brown SILT (ML), moist to wet, trace organics and roots			10	90	N	L	L			
							-MARINE DEPOSIT-										
25	4 8 8 10	S6 4	25.0 27.0		25.0	SM	Medium dense, gray brown silty SAND (SM), wet, freq. silt and clay layers, trace organics			60	40						
							-MARINE DEPOSIT-										
					29.0		Gravelly SAND layer										
					29.8		-GLACIAL TILL-										
30							Weathered BEDROCK Drifted with roller bit to 30.6 ft., begin NQ core at 30.6 ft.										

Water Level Data			Depth in feet to:			Sample ID		Well Diagram		Summary	
Date	Time	Elapsed Time (hr.)	Bottom of Casing	Bottom of Hole	Water	O	Open End Rod	<input type="checkbox"/>	Riser Pipe	Overburden (Linear ft.)	30.6
1/16/04	1130	--	Caved	8.4	Dry	T	Thin Wall Tube	<input type="checkbox"/>	Screen	Rock Cored (Linear ft.)	4.0
						U	Undisturbed Sample	<input type="checkbox"/>	Filter Sand	Number of Samples	6S, 1C
						S	Split Spoon Sample	<input type="checkbox"/>	Cuttings		
						G	Geoprobe	<input type="checkbox"/>	Grout		
								<input type="checkbox"/>	Concrete		
								<input type="checkbox"/>	Bentonite Seal		
										BORING NO.	B101

Field Tests Dilatancy: R - Rapid S - Slow N - None Plasticity: N - Nonplastic L - Low M - Medium H - High  
 Toughness: L - Low M - Medium H - High Dry Strength: N - None L - Low M - Medium H - High V - Very High  
 \*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.  
 NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

# CORE BORING REPORT

BORING NO.

**B101**

Page 2 of 2

Depth (ft)	Drilling Rate (min/ft)	Core No. Depth (ft)	Recovery RQD		Weathering	Well Diagram	Stratum Change (ft)	Visual Classification and Remarks
			(in)	(%)				
30								Begin NQ core boring at 30.6 ft. See Test Boring Report for overburden details.
	6:55 6:10 5:35 5:40	30.6 C1 34.6	45 45 22	94 94 46	Fresh Fresh Fresh		30.6	<p>Medium to moderately hard, fresh, reddish gray fine to medium grained BIOTITE SCHIST with quartz veins. Joint spacing is very close to moderate with several slightly weathered, highly fractured zones. Joints are moderately dipping, undulating and rough, slightly weathered, open, some filled with gray silt.</p> <p style="text-align: center;">-BEDROCK-</p>
35								Bottom of Exploration at 34.6 ft. below ground surface
40								
							JOB NO.	03497
							BORING NO.	B101

PROJECT RADIO TOWER  
 LOCATION PRESUMPCOT STREET, PORTLAND, MAINE  
 CLIENT PORTLAND RADIO GROUP  
 CONTRACTOR MAINE TEST BORINGS, INC.  
 DRILLER T. SCHAEFER

STI JOB NO. 03497  
 PROJECT MGR. C. BROWN  
 FIELD REP. B. ESTES  
 DATE STARTED 1/13/04  
 DATE FINISHED 1/13/04

Elevation	13.1	ft.	Datum	NAVD 88	Boring Location	See Plan		
Item	Casing	Sampler	Core Barrel	Rig Make & Model	Mobile B-34	Hammer Type	Drilling Mud	Casing Advance
Type	HSA	SS		<input type="checkbox"/> Truck <input type="checkbox"/> Tripod	<input type="checkbox"/> Cat-Head <input type="checkbox"/> Winch	<input type="checkbox"/> Safety <input checked="" type="checkbox"/> Doughnut	<input type="checkbox"/> Bentonite <input type="checkbox"/> Polymer	Type Method Depth
Inside Diameter (in.)	2.5	1.375		<input type="checkbox"/> ATV <input type="checkbox"/> Geoprobe	<input checked="" type="checkbox"/> Roller Bit	<input type="checkbox"/> Automatic <input checked="" type="checkbox"/>		
Hammer Weight (lb.)		140		<input checked="" type="checkbox"/> Track <input type="checkbox"/> Air Track	<input type="checkbox"/> Cutting Head			HSA/20.8 ft.
Hammer Fall (in.)		30		<input type="checkbox"/> Skid <input type="checkbox"/>				

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density, consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel					Sand					Field Test			
								% Coarse	% Fine	% Coarse	% Medium	% Fine	% Fines	Dilatancy	Toughness	Plasticity	Strength				
0	6 3 3 2	S1 4	0.0 2.0			SM	Loose, dark brown to brown silty SAND with gravel (SM), mps 0.5 in., moist to wet, trace organics, frozen to 0.5 ft.	5	10	15	20	25	25								
							-FILL-														
5	50/5 in.	S2 4	5.0 5.4			SM	Loose, dark brown to brown silty SAND with gravel (SM), mps 1.4 in., wet, trace organics SPT refusal on wood	5	10	15	20	25	25								
							-FILL-														
					8.5																
10	1 1 1 1	S3 1	10.0 12.0			SM	Very loose, gray brown silty SAND (SM), mps = 0.2 in., slight anaerobic odor, wet, trace organics		5	10	65	20									
					14.0		-MARINE DEPOSIT-														
15	4 12 29 35	S4 18	15.0 17.0			SM	Dense, gray silty SAND (SM), mps = 0.1 in., wet, trace organics				80	20									
					18.0		-MARINE DEPOSIT-														
20	30 30/4 in.	S5 11	20.0 20.8			SM	Very dense, brown silty SAND with gravel (SM), mps = 1.4 in., moist to wet	15	10	15	20	20	20								
					20.8		Spoon Refusal at 20.8 ft. below ground surface Judged to be BEDROCK														

Water Level Data					Sample ID	Well Diagram	Summary	
Date	Time	Elapsed Time (hr.)	Depth in feet to:				Overburden (Linear ft.)	Rock Cored (Linear ft.)
1/13/04	1240		Bottom of Casing	Bottom of Hole	Water	20.8	--	
						Number of Samples	5S	

Field Tests Dilatancy: R - Rapid S - Slow N - None Plasticity: N - Nonplastic L - Low M - Medium H - High  
 Toughness: L - Low M - Medium H - High Dry Strength: N - None L - Low M - Medium H - High V - Very High  
 \*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.  
 NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

TEST BORING REPORT

PROJECT RADIO TOWER  
 LOCATION PRESUMPCOT STREET, PORTLAND, MAINE  
 CLIENT PORTLAND RADIO GROUP  
 CONTRACTOR MAINE TEST BORINGS, INC.  
 DRILLER T. SCHAEFER

STI JOB NO. 03497  
 PROJECT MGR. C. BROWN  
 FIELD REP. B. ESTES  
 DATE STARTED 1/12/04  
 DATE FINISHED 1/13/04

Elevation	15.4	ft.	Datum	NAVD 88	Boring Location	See Plan		
Item	Casing	Sampler	Core Barrel	Rig Make & Model	Mobile B-34	Hammer Type	Drilling Mud	Casing Advance
Type	NW	SS	HQ	<input type="checkbox"/> Truck <input type="checkbox"/> Tripod	<input checked="" type="checkbox"/> Cat-Head	<input type="checkbox"/> Safety	<input type="checkbox"/> Bentonite	Type Method Depth NW/DRIVE/21.5 ft.
Inside Diameter (In.)	3.0	1.375	2.0	<input type="checkbox"/> ATV <input type="checkbox"/> Geoprobe	<input checked="" type="checkbox"/> Winch	<input checked="" type="checkbox"/> Doughnut	<input type="checkbox"/> Polymer	
Hammer Weight (lb.)	300	140		<input checked="" type="checkbox"/> Track <input type="checkbox"/> Air Track	<input checked="" type="checkbox"/> Roller Bit	<input type="checkbox"/> Automatic	<input checked="" type="checkbox"/> None	
Hammer Fall (In.)	16	30		<input type="checkbox"/> Skid <input type="checkbox"/>	<input type="checkbox"/> Cutting Head	Drilling Notes:		

Depth (ft.)	Sampler Blows per 6 In.	Sample No. & Recovery (In.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel					Sand					Field Test		
								% Coarse	% Fine	% Coarse	% Medium	% Fine	% Fines	Dilatancy	Toughness	Plasticity	Strength			
0	3 3 2 5	S1 14	1.0 3.0			SP	Loose, yellow brown poorly-graded SAND (SP), mps = 0.2 in., dry to moist, frozen to 0.5 ft.						5	10	85					
							-FILL-													
5	2 2 2 1	S2 13	5.0 7.0		6.6	SP	Very loose, yellow brown poorly-graded SAND (SP), mps = 0.2 in., moist						5	10	85					
					6.7	ML	Olive brown, SILT (ML), moist													
						SP	Very loose, yellow brown poorly-graded SAND (SP), mps = 0.2 in., wet						10	30	60					
							-MARINE DEPOSIT-													
10	1 WOH WOH 3	S3 1	10.0 12.0		10.0	SW	Very loose, gray well-graded SAND with gravel (SW), mps = 1.4 in., wet	15	10	20	25	30								
							-GLACIAL TILL-													
15	2 1 1 3	S4 0	15.0 17.0			SW	Very loose, gray well-graded SAND with gravel (SW), mps = 1.4 in., wet	15	10	20	25	30								
							-GLACIAL TILL-													
20	27 30/4 in.	S5 5	20.5 21.3		21.3	SM	Very dense brown silty SAND with gravel (SM), mps = 1.4 in., moist, mottled	15	10	15	20	20								
							Drove casing to 21.3 ft. Drilled with roller bit to 21.5 ft. Begin NQ core at 21.5 ft., see Core Boring Report													
25																				
30																				

Water Level Data

Date	Time	Elapsed Time (hr.)	Depth in feet to:		
			Bottom of Casing	Bottom of Hole	Water
1/13/04	0700		15.0	17.0	6.6
1/13/04	1110		21.3	30.6	10.0

Sample ID	Well Diagram	Summary
O Open End Rod	Riser Pipe	Overburden (Linear ft.) 21.5
T Thin Wall Tube	Screen	Rock Cored (Linear ft.) 9.1
U Undisturbed Sample	Filter Sand	Number of Samples 5S, 2C
S Split Spoon Sample	Cuttings	
G Geoprobe	Grout	
	Concrete	
	Bentonite Seal	

Field Tests Dilatancy: R - Rapid S - Slow N - None Toughness: L - Low M - Medium H - High Plasticity: N - Nonplastic L - Low M - Medium H - High Dry Strength: N - None L - Low M - Medium H - High V - Very High

\*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.  
 NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

# CORE BORING REPORT

BORING NO.

**B103**

Page 2 of 2

Depth (ft)	Drilling Rate (min/ft)	Core No. Depth (ft)	Recovery RQD		Weathering	Well Diagram	Stratum Change (ft)	Visual Classification and Remarks		
			(In)	(%)						
20							21.5	Begin NQ core boring at 21.5 ft. See Test Boring Report for overburden details.		
	4:15 4:10 3:40 3:35 3:40	21.5  C1  26.3	56	97	Fresh			Medium to moderately hard, fresh, reddish gray fine to medium grained BIOTITE SCHIST with quartz veins. Joint spacing is moderate to wide, moderately dipping, undulating, fresh, tight		
25		26.3	55	95				-BEDROCK-		
	2:30 2:40 3:05 3:40	26.3  C2  30.6	51	100	Fresh			Medium to moderately hard, fresh, reddish gray fine to medium grained BIOTITE SCHIST with quartz veins. Joint spacing is moderate to wide, moderately dipping, undulating, fresh, tight		
30								-BEDROCK-		
								Bottom of Exploration at 30.6 ft. below ground surface.		
							JOB NO.	03497	BORING NO.	B103

TEST BORING REPORT

Table with project details: PROJECT RADIO TOWER, LOCATION PRESUMPCOT STREET, PORTLAND, MAINE, CLIENT PORTLAND RADIO GROUP, CONTRACTOR MAINE TEST BORINGS, INC., DRILLER T. SCHAEFER, STI JOB NO. 03497, PROJECT MGR. C. BROWN, FIELD REP. B. ESTES, DATE STARTED 1/15/04, DATE FINISHED 1/15/04.

Table with drilling parameters: Elevation 17.0 ft, Datum NAVD 88, Boring Location See Plan, Rig Make & Model Mobile B-34, Hammer Type, Drilling Mud, Casing Advance, Inside Diameter (in.) 2.5, Sampler SS, Core Barrel, Type, and various checkboxes for rig features.

Main data table with columns: Depth (ft.), Sampler Blows per 6 in., Sample No. & Recovery (in.), Sample Depth (ft.), Well Diagram, Stratum Change (ft.), USCS Symbol, Visual-Manual Identification & Description, Gravel, Sand, and Field Test results. Includes soil layers like ML, SM, SP and -FILL-, -MARINE DEPOSIT-, -GLACIAL TILL-.

Summary table with sections: Water Level Data (Date, Time, Elapsed Time, Bottom of Casing, Bottom of Hole, Water), Sample ID (O, T, U, S, G), Well Diagram (Riser Pipe, Screen, Filter Sand, Cuttings, Grout, Concrete, Bentonite Seal), and Summary (Overburden, Rock Cored, Number of Samples, BORING NO. B104).

Field Tests Dilatancy: R - Rapid S - Slow N - None Toughness: L - Low M - Medium H - High Plasticity: N - Nonplastic L - Low M - Medium H - High Dry Strength: N - None L - Low M - Medium H - High V - Very High
NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.
NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (In.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel		Sand			Field Test				
								% Coarse	% Fine	% Coarse	% Medium	% Fine	% Fines	Dilatancy	Toughness	Plasticity	Strength
30	6 7 25 35	S7  24	30.0  32.0			SM	Dense, gray silty SAND with gravel (SM), mps = 1.25 in., wet, slightly bonded	10	10	10	20	30	20				
					33.6		-GLACIAL TILL-										
35							Auger Refusal at 33.6 ft. below ground surface Judged to be BEDROCK										
40																	
45																	
50																	
55																	
60																	
65																	
70																	

NOTES:

FILE NO.

03497

BORING NO.

B104

\*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.



PROJECT RADIO TOWER  
 LOCATION PRESUMPCOT STREET, PORTLAND, MAINE  
 CLIENT PORTLAND RADIO GROUP  
 CONTRACTOR MAINE TEST BORINGS, INC.  
 DRILLER T. SCHAEFER

STI JOB NO. 03497  
 PROJECT MGR. C. BROWN  
 FIELD REP. B. ESTES  
 DATE STARTED 1/13/04  
 DATE FINISHED 1/14/04

Elevation	18.5	ft.	Datum	NAVD 88	Boring Location	See Plan
Item	Casing	Sampler	Core Barrel	Rig Make & Model	Mobile B-34	Hammer Type
Type	NW	SS	NQ	<input type="checkbox"/> Truck <input type="checkbox"/> Tripod <input type="checkbox"/> Cat-Head	<input type="checkbox"/> Safety	<input type="checkbox"/> Bentonite
Inside Diameter (In.)	3.0	1.375	2.0	<input type="checkbox"/> ATV <input type="checkbox"/> Geoprobe	<input checked="" type="checkbox"/> Winch	<input type="checkbox"/> Polymer
Hammer Weight (lb.)	300	140		<input checked="" type="checkbox"/> Track <input type="checkbox"/> Air Track	<input checked="" type="checkbox"/> Roller Bit	<input type="checkbox"/> None
Hammer Fall (In.)	16	30		<input type="checkbox"/> Skid <input type="checkbox"/>	<input type="checkbox"/> Cutting Head	

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (In.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel			Sand			Field Test			
								% Coarse	% Fine	% Coarse	% Medium	% Fine	% Fines	Dilatancy	Toughness	Plasticity	Strength
0						SM	Dense, brown silty SAND with gravel (SM), mps = 1.0 in., moist to wet, trace asphalt and brick fragments, frozen to 0.5 ft.	10	10	10	20	30	20				
	15	S1	1.0		1.5		-FILL-										
	8					ML	Very stiff, olive brown SILT with sand (ML), mps = 0.1 in., moist				5	10	85	N	L	L	
	8																
	8	5	3.0														
5							-FILL-										
	2	S2	5.0		5.2	SW	Loose, yellow brown well-graded SAND with gravel (SW), mps = 0.75 in., moist	5	10	20	30	30	5				
	2																
	5																
	5	6	7.0														
10							-FILL-										
	11	S3	10.0		10.0	ML	Hard, olive brown SILT (ML), moist, slightly mottled						100	N	L	L	
	14																
	17																
	22	24	12.0														
15							-MARINE DEPOSIT-										
	6	S4	15.0			ML	Very stiff, olive brown SILT (ML), moist to wet, mottled with silty fine sand seams					5	95	N	L	L	
	8																
	10																
	14	24	17.0														
20							-MARINE DEPOSIT-										
	17	S5	20.0			SP/ML	Very dense, brown and gray brown interbedded poorly-graded SAND (SP) and SILT (ML), wet, occasional silt seams					80	10	N	L	L	
	14											10	90				
	31																
	37	13	22.0														
25							-MARINE DEPOSIT-										
	7	S6	25.0			SP/ML	Medium dense, brown and gray brown interbedded poorly-graded SAND (SP) and SILT (ML), wet, occasional silt seams					90	10	N	L	L	
	7											10	90				
	13																
	22	12	27.0		28.5												

Water Level Data			Depth in feet to:			Sample ID		Well Diagram		Summary	
Date	Time	Elapsed Time (hr.)	Bottom of Casing	Bottom of Hole	Water	O	T	U	S	G	Overburden (Linear ft.)
1/14/04	1425		Caved	11.4	7.4	Open End Rod	Thin Wall Tube	Undisturbed Sample	Split Spoon Sample	Geoprobe	32.2
											Rock Cored (Linear ft.)
											10.0
											Number of Samples
											7S, 2C
											BORING NO.
											B105

Field Tests Dilatancy: R - Rapid S - Slow N - None Plasticity: N - Nonplastic L - Low M - Medium H - High  
 Toughness: L - Low M - Medium H - High Dry Strength: N - None L - Low M - Medium H - High V - Very High  
 \*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.  
 NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel		Sand			Field Test					
								% Coarse	% Fine	% Coarse	% Medium	% Fine	% Fines	Dilatancy	Toughness	Plasticity	Strength	
30	1 for 26 in.	S7	30.0			SP/SM	Very loose, gray interbedded poorly-graded SAND (SP) and silty SAND (SM), wet, occasional silt seams				80	20						
	25/0 in.	22	32.2		32.2		-MARINE DEPOSIT-											
							Drove casing to 32.2 ft. Drilled with roller bit to 32.2 ft. Begin NQ core at 32.2 ft., see Core Boring Report											
35																		
40																		
45																		
50																		
55																		
60																		
65																		
70																		

NOTES:

FILE NO.

03497

BORING NO.

B105

\*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

# CORE BORING REPORT

Depth (ft)	Drilling Rate (min/ft)	Core No. Depth (ft)	Recovery RQD		Weathering	Well Diagram	Stratum Change (ft)	Visual Classification and Remarks
			(in)	(%)				
30								Begin NQ core boring at 32.2 ft. See Test Boring Report for overburden details.
		32.2	60	100	Fresh		32.2	Medium to moderately hard, fresh, reddish gray, fine to medium grained BIOTITE SCHIST with quartz veins and minor pyrite. Joint spacing is generally wide, moderately dipping, undulating, mostly fresh and tight, few with minor silt infilling.
35	4:40 3:30 3:10 3:35	C1						-BEDROCK-
		37.2	59	98				Medium to moderately hard, fresh, reddish gray, fine to medium grained BIOTITE SCHIST with quartz veins and minor pyrite. Joint spacing is generally wide, moderately dipping, undulating, mostly fresh and tight, few with minor silt infilling.
40	3:25 3:05 2:55 4:50	C2			Fresh			-BEDROCK-
		42.2	56	93				Bottom of Exploration at 42.2 ft. below ground surface.
45								

PROJECT	RADIO TOWER	STI JOB NO.	03497
LOCATION	PRESUMPCOT STREET, PORTLAND, MAINE	PROJECT MGR.	C. BROWN
CLIENT	PORTLAND RADIO GROUP	FIELD REP.	B. ESTES
CONTRACTOR	MAINE TEST BORINGS, INC.	DATE STARTED	1/22/04
DRILLER	J. RUDNICKI	DATE FINISHED	1/23/04

Elevation	2.0	ft	Datum	NAVD 88	Boring Location	See Plan		
Item	Casing	Sampler	Core Barrel	Rig Make & Model	Mobile B-53	Hammer Type	Drilling Mud	Casing Advance
Type	NW	SS	NQ	<input type="checkbox"/> Truck <input type="checkbox"/> Tripod	<input type="checkbox"/> Cat-Head	<input checked="" type="checkbox"/> Safety	<input type="checkbox"/> Bentonite	Type/Method/Depth
Inside Diameter (In.)	3.0	1.375	2.0	<input type="checkbox"/> ATV <input type="checkbox"/> Geoprobe	<input checked="" type="checkbox"/> Winch	<input type="checkbox"/> Doughnut	<input type="checkbox"/> Polymer	NW/DRIVE/48.7 ft.
Hammer Weight (lb.)	300	140		<input checked="" type="checkbox"/> Track <input type="checkbox"/> Air Track	<input checked="" type="checkbox"/> Roller Bit	<input type="checkbox"/> Automatic	<input checked="" type="checkbox"/> None	
Hammer Fall (in.)	16	30		<input type="checkbox"/> Skid <input type="checkbox"/>	<input type="checkbox"/> Cutting Head	Drilling Notes:		

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (In.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel					Sand					Field Test			
								% Coarse	% Fine	% Coarse	% Medium	% Fine	% Fines	Dilatancy	Toughness	Plasticity	Strength				
0	4 10 13 3	S1  3	0.0  2.0		1.5		Dark brown, sandy SILT with pieces of crushed rock and marsh grass, wet -FILL-														
						ML/OL	Soft, gray to black SILT (ML) and ORGANIC SOIL (OL), mps = 0.04 in., wet, trace grass -MARINE DEPOSIT-						5	5	90	N	L	L			
5	2 1 2 2	S2  1	5.0  7.0			ML	Soft, gray SILT (ML), mps = 0.04 in., wet							5	95	N	L	L			
10	WOH WOH WOH WOH	S3  22	10.0  12.0			ML	Very soft, gray SILT (ML), mps = 0.04 in., wet, trace grass, trace shell fragments, anaerobic odor -MARINE DEPOSIT-							5	95	N	L	L			
15	1 WOH 1 2	S4  3	15.0  17.0			ML	Very soft, olive brown SILT (ML), mps = 0.04 in., wet, trace shell fragments and organics -MARINE DEPOSIT-							5	95	N	L	L			
20	3 4 5 7	S5  24	20.0  22.0		21.0	ML ML/CL	Soft to stiff, gray SILT (ML), mps = 1.0 in., wet Stiff, olive brown and gray SILT to lean CLAY (ML to CL), mps = 0.1 in., wet, slightly mottled			5			5	5	85	N	L	L			
25	2 3 4 4	S6  24	25.0  27.0			ML/CL	Stiff, olive brown and gray SILT to lean CLAY (ML to CL), mps = 0.1 in., wet, slightly mottled, trace shell fragments -MARINE DEPOSIT-							5	95	N	L	L			
30																					

Water Level Data						Sample ID		Well Diagram		Summary				
Date	Time	Elapsed Time (hr.)	Depth In feet to:			O	Y	U	S	G	<input type="checkbox"/> Riser Pipe <input type="checkbox"/> Screen <input checked="" type="checkbox"/> Filter Sand <input checked="" type="checkbox"/> Cuttings <input type="checkbox"/> Grout <input type="checkbox"/> Concrete <input checked="" type="checkbox"/> Bentonite Seal	Overburden (Linear ft.)	Rock Cored (Linear ft.)	Number of Samples
			Bottom of Casing	Bottom of Hole	Water									
1/23/04	--	--	--	--	Tidal Zone							49.3	10.0	10S, 2C

Field Tests Dilatancy: R - Rapid S - Slow N - None Plasticity: N - Nonplastic L - Low M - Medium H - High  
 Toughness: L - Low M - Medium H - High Dry Strength: N - None L - Low M - Medium H - High V - Very High  
 \*NOTE: Maximum Particle Size Is determined by direct observation within the limitations of sampler size.  
 NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel		Sand			Field Test					
								% Coarse	% Fine	% Coarse	% Medium	% Fine	% Fines	Diabancy	Toughness	Plasticity	Strength	
30	1	S7	30.0			CL	Medium stiff, gray lean CLAY (CL), wet, freq. silty fine sand seams and black streaks  -MARINE DEPOSIT-					5	95	N	M	M		
	2																	
	3																	
	3	24	32.0															
					34.0													
35	4	S8	35.0			SM	Medium dense, gray brown silty SAND (SM), mps = 1.4 in., wet, freq. silt layers and seams  -MARINE DEPOSIT-	5				75	20					
	9																	
	15																	
	19	4	37.0															
					38.0													
40	WOH	S9	40.0			SP	Loose, light brown poorly-graded SAND (SP), mps = 0.08 in., wet  -MARINE DEPOSIT-			5	25	70						
	1																	
	4																	
	9	24	42.0															
45	2	S10	45.0			SP	Loose, light brown poorly-graded SAND (SP), mps = 0.04 in., wet  -MARINE DEPOSIT-			5	25	70						
	6																	
	14																	
	25	17	47.0															
					46.3													
						SM	Dense, gray silty SAND with gravel (SM), mps = 1.0 in., wet	5	10	15	20	30	20					
					48.7		-GLACIAL TILL-											
							Drove casing to 48.7 ft. Drilled with roller bit to 49.3 ft. Begin NQ core at 49.3 ft. See Core Boring Report											
50																		
55																		
60																		
65																		
70																		

NOTES:

FILE NO.

03497

BORING NO.

B106

\*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

# CORE BORING REPORT

BORING NO.

**B106**

Page 3 of 3

Depth (ft)	Drilling Rate (min/ft)	Core No. Depth (ft)	Recovery RQD		Weathering	Well Diagram	Stratum Change (ft)	Visual Classification and Remarks		
			(in)	(%)						
45										
50		49.3  C1	60	100	Fresh		49.3	Medium to moderately hard, fresh, reddish gray fine to medium grained BIOTITE SCHIST with quartz veins. Joint spacing is very close to moderate with several slightly weathered, highly fractured zones. Joints are moderately dipping, undulating and rough, slightly weathered, open, some filled with gray silt.		
		54.3	60	100				-BEDROCK-		
55		54.3  C2	60	100	Fresh			Medium to moderately hard, fresh, reddish gray fine to medium grained BIOTITE SCHIST with quartz veins. Joint spacing is very close to moderate with several slightly weathered, highly fractured zones. Joints are moderately dipping, undulating and rough, slightly weathered, open, some filled with gray silt.		
		59.3	60	100				-BEDROCK-		
60								Bottom of Exploration at 59.3 ft. below ground surface.		
							JOB NO.	03497	BORING NO.	B106

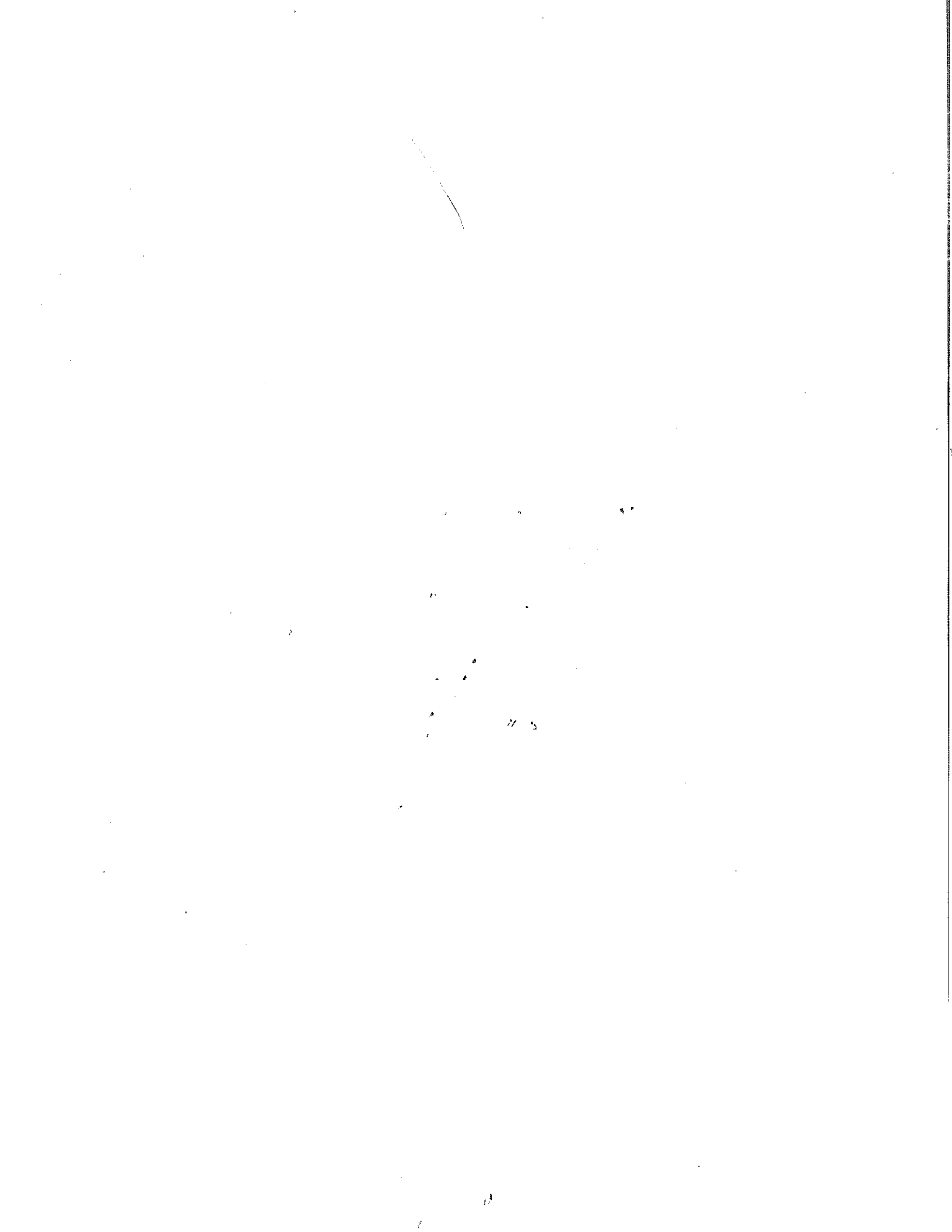


<b>Job</b>		Portland, ME 531' Guyed Tower		Page 69 of 82	
<b>Project</b>		12590 48" Face Run#2		Date 11:18:49 10/18/04	
<b>Client</b>		SAGA Communications			
<b>Job</b>		Portland, ME 531' Guyed Tower		Designed by M. Maurer	

**ERTOWERBeta**  
 Electronics Research Inc.  
 777 Gardner Road  
 Chandler, IN  
 Phone: 812-925-6000  
 FAX: 812-925-4026

Section No.	Elevation	Size	L	L <sub>n</sub>	K/r	F <sub>a</sub>	A	Actual P	Allow. P <sub>a</sub>	Ratio
	f		f	f	h	kt	h <sup>2</sup>	K	K	P <sub>a</sub>
L1	528 - 512.205	3/4	3.476	1.738	111.2	30.000	0.442	0.853	13.254	0.064
L2	512.205 - 496.41	3/4	3.476	1.738	111.2	30.000	0.442	1.886	13.254	0.142
L3	496.41 - 480.615	3/4	3.476	1.738	111.2	30.000	0.442	2.947	13.254	0.222
L4	480.615 - 470	3/4	3.483	1.741	111.4	30.000	0.442	3.179	13.254	0.240
T1	470 - 451	3/4	4.902	2.451	156.9	30.000	0.442	2.746	13.254	0.207
T2	451 - 431	3/4	4.902	2.451	156.9	30.000	0.442	2.233	13.254	0.168
T3	431 - 411	3/4	4.902	2.451	156.9	30.000	0.442	1.377	13.254	0.104
T4	411 - 391	3/4	4.902	2.451	156.9	30.000	0.442	1.488	13.254	0.112
T5	391 - 371	3/4	4.902	2.451	156.9	30.000	0.442	1.924	13.254	0.145
T6	371 - 351	7/8	4.902	2.451	134.4	30.000	0.601	2.554	18.040	0.142
T7	351 - 331	3/4	4.902	2.451	156.9	30.000	0.442	2.010	13.254	0.152
T8	331 - 311	3/4	4.902	2.451	156.9	30.000	0.442	1.463	13.254	0.110
T9	311 - 291	3/4	4.902	2.451	156.9	30.000	0.442	0.799	13.254	0.060
T10	291 - 271	3/4	4.902	2.451	156.9	30.000	0.442	1.963	13.254	0.148
T11	271 - 251	3/4	4.902	2.451	156.9	30.000	0.442	2.502	13.254	0.189
T12	251 - 231	3/4	4.902	2.451	156.9	30.000	0.442	2.732	13.254	0.206
T13	231 - 211	3/4	4.902	2.451	156.9	30.000	0.442	2.512	13.254	0.190
T14	211 - 191	3/4	4.902	2.451	156.9	30.000	0.442	2.043	13.254	0.154
T15	191 - 171	3/4	4.902	2.451	156.9	30.000	0.442	1.465	13.254	0.111
T16	171 - 151	3/4	4.902	2.451	156.9	30.000	0.442	2.380	13.254	0.180
T17	151 - 131	7/8	4.902	2.451	134.4	30.000	0.601	4.102	18.040	0.227
T18	131 - 111	7/8	4.902	2.451	134.4	30.000	0.601	4.719	18.040	0.262
T19	111 - 91	7/8	4.902	2.451	134.4	30.000	0.601	2.915	18.040	0.162
T20	91 - 71	7/8	4.902	2.451	134.4	30.000	0.601	1.988	18.040	0.110
T21	71 - 51	7/8	4.902	2.451	134.4	30.000	0.601	1.133	18.040	0.063
T22	51 - 31	7/8	4.902	2.451	134.4	30.000	0.601	0.978	18.040	0.054
T23	31 - 11	7/8	4.902	2.451	134.4	30.000	0.601	2.132	18.040	0.118







APPLICATION FOR EXEMPTION FROM SITE PLAN REVIEW

Applicant

Cambro Corp  
328 W. Commercial St

Application Date

11/10/04

Applicant's Mailing Address

Bob Seaman Blvd 553-288

Project Name/Description

189 Presumpscot St

Consultant/Agent/Phone Number

Address of Proposed Site

421-B-5-6  
CBL: 426-B-1-7

Description of Proposed Development:

to rebuild communications tower that collapsed on  
12/11/03 to no higher than the original 540'  
can rebuild within 1 year

Please Attach Sketch/Plan of Proposal/Development

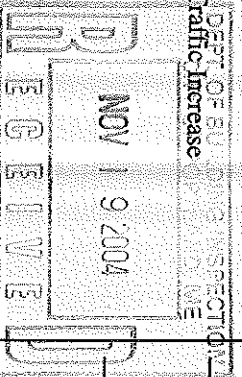
Applicant's Assessment  
(Yes, No, N/A)

Planning Office  
Use Only

Criteria for Exemptions:  
See Section 14-523 (4) on back side of form

- a) Within Existing Structures; No New Buildings, Demolitions or Additions
- b) Footprint Increase Less Than 500 Sq. Ft.
- c) No New Curb Cuts, Driveways, Parking Areas
- d) Curbs and Sidewalks in Sound Condition/Comply with ADA
- e) No Additional Parking/ No Traffic Increase
- f) No Stormwater Problems
- g) Sufficient Property Screening
- h) Adequate Utilities

Applicant's Assessment (Yes, No, N/A)	Planning Office Use Only
✓	✓
✓	✓
✓	✓
✓	✓
✓	✓
✓	✓
✓	✓
✓	✓
✓	✓
✓	✓



Planning Division Use Only

Exemption Granted

Partial Exemption

Exemption Denied

Planner's Signature

*[Handwritten Signature]*

Date 11/17/04

White - Planning Office

Pink - Inspections

Yellow - Applicant