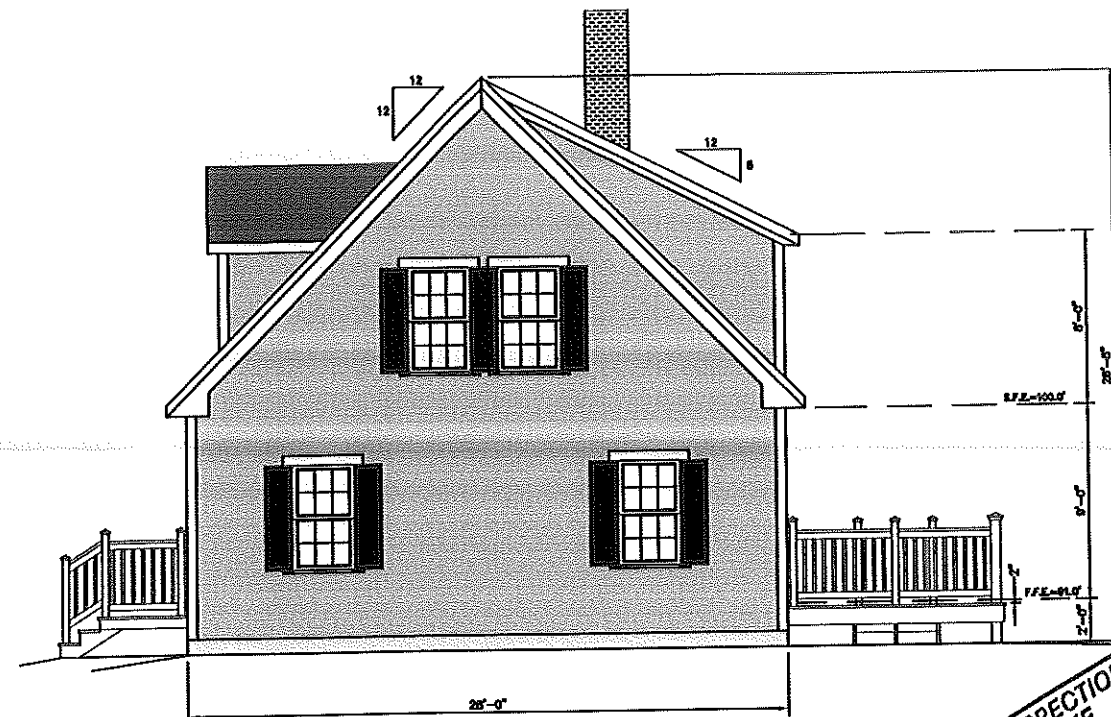
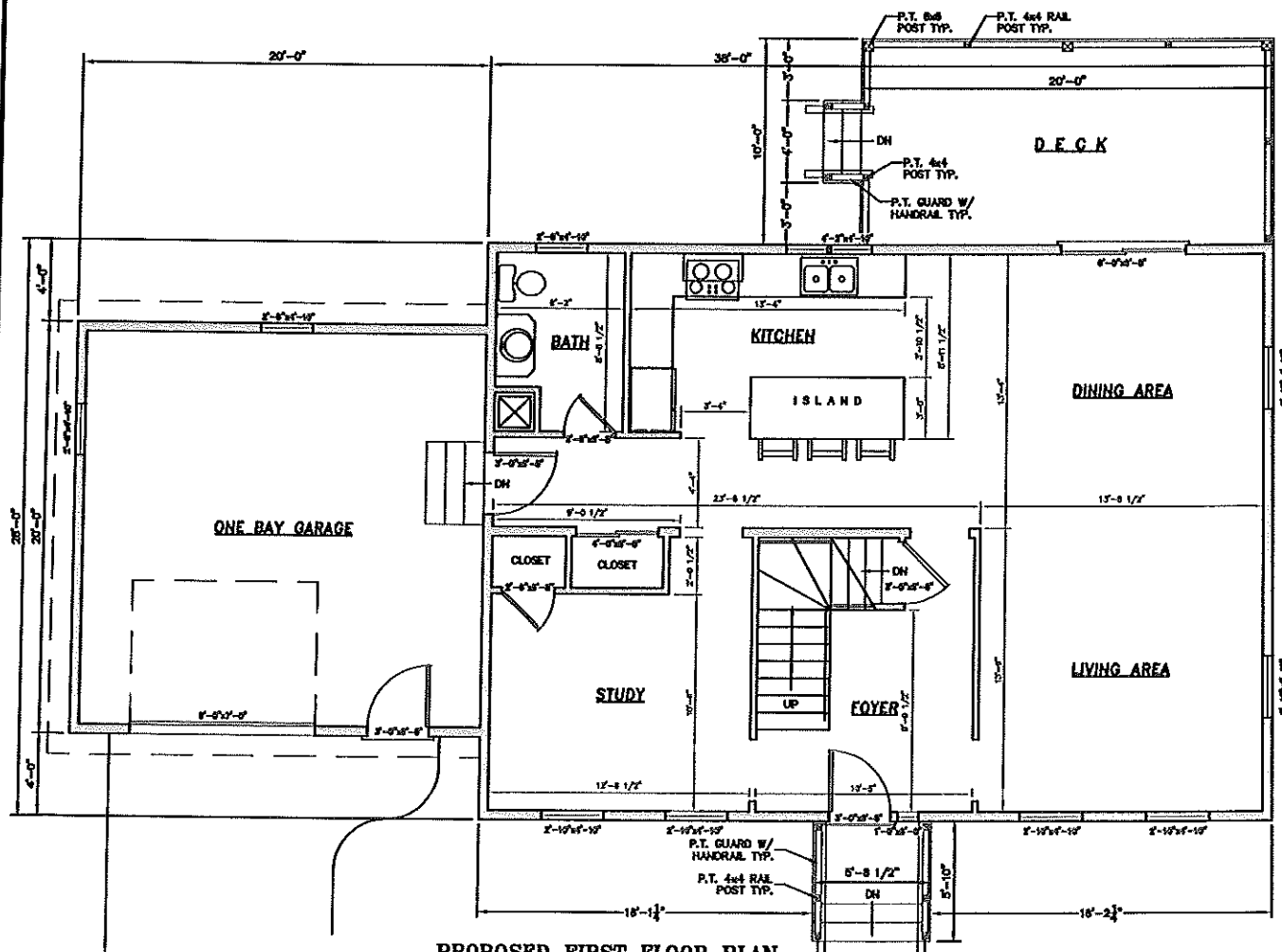




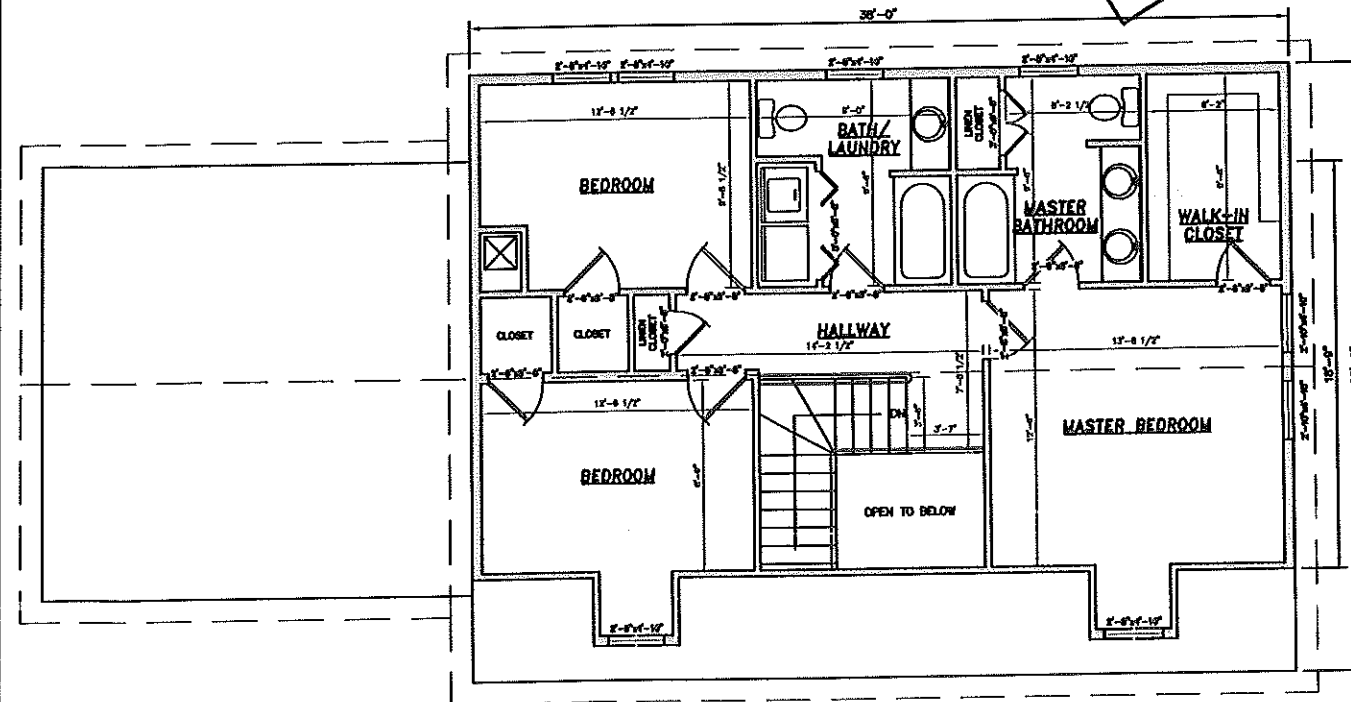
FRONT ELEVATION
SCALE 1/4" = 1'-0"



SIDE ELEVATION
SCALE 1/4" = 1'-0"



PROPOSED FIRST FLOOR PLAN
SCALE 1/4" = 1'-0"



PROPOSED SECOND FLOOR PLAN
SCALE 1/4" = 1'-0"

DEPT. OF BUILDING INSPECTION
 CITY OF PORTLAND, ME
 JUN 28 2005
 RECEIVED

| | | |
|--|---|---------|
| NO. | REVISIONS | DATE |
| 1 | ADDED DOOR & WINDOW DIMENSIONS REVISED DECK & STAIRS | 6/25/05 |
| SHEET TITLE: PROPOSED FLOOR PLANS AND ELEVATIONS | | |
| CLIENT/PROJECT: O'BRIEN RESIDENCE | | |
| SCALE: | AS SHOWN | |
| LOCATION: | PORTLAND COUNTY, CUMBERLAND, MAINE | |
| DATE: | JANUARY 07, 2004 | |
| TOWN: | PORTLAND | |
| 432 Camp Road PO Box 4487 Augusta, ME 04306 Tel: 603-733-9972 | | |
| 598 Union Street PO Box 1871 Camden, ME 04843 Tel: 603-282-4465 | | |
| | | |
| James E. Coffin PROJECT NO. 03618 | | |
| A-1 | | |

GENERAL NOTES:

PART 1 - GENERAL

- 1.01 GENERAL
A. NO PROVISIONS HAVE BEEN MADE FOR ANY TEMPORARY CONDITIONS THAT MAY ARISE DURING CONSTRUCTION PRIOR TO THE COMPLETION OF THE STRUCTURE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ADEQUATE DESIGN AND CONSTRUCTION OF ALL FORMS, SHORING AND TEMPORARY BRACING DURING THE PROGRESS OF THE PROJECT.
B. PRINCIPAL OPENINGS THROUGH THE FOUNDATION ARE NOT SHOWN ON THESE DRAWINGS. THE GENERAL CONTRACTOR SHALL EXAMINE THE DRAWINGS TO DETERMINE THE REQUIRED OPENINGS, AS HE SHALL PROVIDE FOR ALL OPENINGS AND SHALL VERIFY SIZE AND LOCATION OF ALL OPENINGS WITH OTHER PROJECT REQUIREMENTS. ANY DEVIATION FROM THE OPENINGS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE BROUGHT TO THE ENGINEER'S ATTENTION FOR APPROVAL.
C. ALTERNATE CONNECTION DETAILS MAY BE USED IF SUCH DETAILS ARE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW AND ACCEPTANCE IS GRANTED. HOWEVER, THE STRUCTURAL ENGINEER SHALL BE THE SOLE JUDGE OF ACCEPTABILITY AND THE CONTRACTOR'S BID SHALL ANTICIPATE THE USE OF THOSE SPECIFIC DETAILS SHOWN ON THE DRAWINGS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF ANY ALTERNATE DETAILS, WHICH HE PROPOSES.
D. WORK NOT INDICATED ON A PART OF THE DRAWINGS, BUT REASONABLY INFERRED TO BE SIMILAR TO THAT SHOWN AT CORRESPONDING PLACES SHALL BE INCLUDED.
E. THE CONTRACTOR SHALL BE COMPLETELY RESPONSIBLE FOR THE SAFETY OF ADJACENT STRUCTURES, PROPERTY, AND THE PUBLIC. THE CONTRACTOR SHALL COMPLY WITH ALL FEDERAL, STATE, AND LOCAL REQUIREMENTS.
F. ANY MODIFICATION OR ALTERATION OF THESE CONSTRUCTION DOCUMENTS OR CHANGES IN CONSTRUCTION FROM THE INTENT OF THESE DOCUMENTS BY THE CONTRACTOR WITHOUT WRITTEN APPROVAL OF THE ENGINEER SHALL REMOVE ALL PROFESSIONAL AND LIABLE RESPONSIBILITY ON THE PART OF THE ENGINEER.
G. ALL CONTRACTORS ARE REQUIRED TO EXAMINE THE DRAWINGS AND SPECIFICATIONS CAREFULLY, VISIT THE SITE AND FULLY INFORM THEMSELVES AS TO ALL EXISTING CONDITIONS AND LIMITATIONS, PRIOR TO SUBMITTING THE PROPOSAL. FAILURE TO VISIT THE SITE AND FAMILIARIZE THEMSELVES WITH THE EXISTING CONDITIONS AND LIMITATIONS WILL IN NO WAY RELIEVE THE SUCCESSFUL BIDDER FROM FURNISHING ANY MATERIALS OR PERFORMING ANY WORK IN ACCORDANCE WITH DRAWINGS AND SPECIFICATIONS WITHOUT ADDITIONAL COST TO THE OWNER.
H. DO NOT SCALE FROM DRAWINGS.
I. SEE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.
J. CONTRACTOR TO VERIFY BUILDING DRAWING DIMENSIONS WITH MANUFACTURER BUILDING DRAWING DIMENSIONS AND/OR ARCHITECTURAL DRAWINGS.
K. INTERIOR FLOOR DRAIN LOCATIONS AND OUTFALL TO BE DETERMINED BY OTHERS.

CONCRETE NOTES

PART 1 - GENERAL

- 1.01 GENERAL
A. ADHERE TO ACI OLD WEATHER CONCRETE SPECIFICATIONS, WHEN APPLICABLE.
B. ALL DIMENSIONS AND CONDITIONS MUST BE VERIFIED IN THE FIELD. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH THE INTENDED PART OF THE WORK.
C. CONTRACTOR SHALL TAKE NECESSARY PRECAUTIONS TO MAINTAIN STABILITY AND PREVENT UNDERMINING OF EXISTING FOUNDATIONS AT ALL TIMES.
D. NO FOUNDATIONS SHALL BE PLACED IN WATER OR ON FROZEN GROUND.
E. ALL FOOTINGS ARE TO BE EXCAVATED USING A BUCKET WITH A SMOOTH TOOTHLESS CUTTING EDGE. FOOTING EXCAVATIONS ARE TO BE FINISHED BY HAND FOR NOT LESS THAN THE LAST SIX INCHES.
F. ALL FINISHED FOUNDATION EXCAVATIONS SHALL BE INSPECTED AND APPROVED BY THE STRUCTURAL ENGINEER OR HIS DESIGNATE BEFORE ANY CONCRETE IS PLACED.
G. THE OWNER, THE STRUCTURAL ENGINEER AND THEIR CONSULTANTS ASSUME NO RESPONSIBILITY FOR THE VALIDITY OF THE SUBSURFACE CONDITIONS DESCRIBED ON THE DRAWINGS, SPECIFICATIONS, TEST BORINGS OR TEST PITS.
H. DETAILING OF CONCRETE REINFORCEMENT AND ACCESSORIES SHALL BE IN ACCORDANCE WITH ACI 315 - "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES," LATEST EDITION.

PART 2 - PRODUCTS

- 2.01 MATERIAL
A. REINFORCING:
1. SHALL BE GRADE 60, NEW DEFORMED BARS AND SHALL CONFORM TO ASTM A615. ALL REINFORCING BARS TO BE WELDED SHALL CONFORM TO ASTM A706.
2. REINFORCING BARS MAY NOT BE WELDED EXCEPT WHERE DESIGNATED BY THE STRUCTURAL ENGINEER.
3. ALL WELDED WIRE FABRIC (W.W.F.) SHALL CONFORM TO ASTM 105. W.W.F. SHALL BE PROVIDED IN FLAT SHEETS.
4. ALL LAPS IN W.W.F. SHALL BE ONE MESH PLUS TWO INCHES AT SPICES. W.W.F. SHALL BE #3@16"X16" (TYP., UNO)
5. CONCRETE PROTECTION FOR REINFORCEMENT SHALL BE PROVIDED AS FOLLOWS:
A. SURFACES CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH - 3 INCHES (CLEAR)
B. FORMED SURFACES EXPOSED TO EARTH OR WEATHER
1. #6 THROUGH #10 BARS - 2 INCHES
2. #6 BARS & SMALLER 1 1/2 INCHES
C. FORMED SURFACES NOT EXPOSED TO EARTH OR WEATHER SLABS, WALLS, JOISTS - 2 INCHES
D. BEAMS, COLUMNS - 2 INCHES
6. ALL HOOKS SHOWN ON DRAWINGS SHALL BE STANDARD HOOKS UNLESS NOTED OTHERWISE.
7. WHERE CONTINUOUS BARS ARE CALLED FOR, THEY SHALL RUN CONTINUOUSLY AROUND CORNERS AND LAPPED AT NECESSARY SPICES, OR HOOKED AT DISCONTINUOUS ENDS. LAP LENGTHS SHALL BE AS GIVEN IN THE SPICE AND DEVELOPMENT TABLE. LAP BEAM TOP BARS AT MID-SPAN AND BEAM BOTTOM BARS AT SUPPORTS, UNLESS NOTED OTHERWISE.
B. FOUNDATION WALLS & FOOTING MIX DESIGN:
1. 3000 PSI
2. 3/4" STONE
3. SLUMP 4" +/- 1"
4. 5% AIR ENTRAINMENT
C. SLAB MIX DESIGN:
1. 4000 PSI
2. 3/4" STONE
3. SLUMP 8" +/- 1"
4. NO AIR

PART 3 - EXECUTION

- 3.01 SUBGRADE
A. ALL GRADING SHALL BE ACHIEVED AT SUBGRADE TO PROVIDE A CONSTANT THICKNESS OF CONCRETE.
B. STRUCTURAL FILL SHALL BE COMPACTED IN 6" LIFTS TO 95% OF ITS MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D1557.
C. SUBGRADE TO CONSIST OF AT LEAST 1/2" OF COMPACTED SAND OR GRAVEL. THIS MATERIAL SHALL BE:
SCREEN OR SIEVE SIZE PERCENT FINER BY WEIGHT
4" 100%
4" 3/4" 100%
4" 3/8" 75%
4" 1/4" 25%
NO. 40 0-25%
NO. 200 0-5%
D. DRAINAGE STONE SHALL CONSIST OF CLEAN ANGULAR FRAGMENTS OF QUARRIED ROCK WITH UNIFORM QUALITY AND BE GRADED AS FOLLOWS:
SCREEN OR SIEVE SIZE PERCENT FINER BY WEIGHT
2" 100%
2" 3/4" 100%
2" 1/2" 0-100%
1" 0-30%
3/4" 0-5%
3.02 PLACEMENT
A. CONCRETE SLAB ON GRADE SHALL BE PLACED IN ONE CONTINUOUS PLACEMENT, WITH NO COLD JOINTS. IF COLD JOINTS ARE DESIRED, CONTRACTOR MUST PROVIDE PLACEMENT SEQUENCE AND JOINT DETAIL FOR ENGINEER'S APPROVAL, PRIOR TO PLACEMENT.
B. APPLY CONCRETE BEVELER AFTER THE SLAB HAS CURED FOR 30 DAYS.
C. VAPOR BARRIERS WILL BE USED UNDER SLAB TO PREVENT MOISTURE MIGRATION INTO THE SLAB, AND TO PROVIDE A BARRIER TO RADON PENETRATION.
D. VAPOR BARRIER:
1. 6 MIL POLYETHYLENE
2. PERMEANCE LESS THAN 0.3 PERMS DETERMINED IN ACCORDANCE WITH ASTM E 96.
3. BARRIER SHOULD NOT BE INTERRUPTED DURING CONSTRUCTION ACTIVITIES.
4. EDGES SHOULD BE LAPPED A MINIMUM OF 6", TAPPED, AND SHOULD BE CAREFULLY FITTED AROUND OPENINGS.
E. ALL CONCRETE EXPOSED TO THE WEATHER SHALL CONTAIN 5% - 7% AIR ENTRAINMENT ADJUSTURE.
F. ALL FOOTINGS SHALL BE PLACED MONOLITHICALLY.
G. PIPES OR CONDUITS PLACED IN SLABS ON GRADE SHALL NOT BE PLACED CLOSER THAN 3 DIAMETERS ON CENTER AND SHALL HAVE AN OUTSIDE DIAMETER LESS THAN 1/3 OF THE SLAB THICKNESS. ALUMINUM COMPONENTS SHALL NOT BE PLACED IN CONCRETE. NO CONDUITS SHALL BE PLACED IN SLABS ON METAL DECK.
H. AT LOCATIONS WHERE ANY PART OF FOOTING BEARS DIRECTLY ON LEDGE, SURFACE LEDGE SHALL BE REMOVED TO PROVIDE A LEVEL-BEARING SURFACE IN ALL DIRECTIONS. THOROUGHLY CLEAN LEDGE SURFACE PRIOR TO PLACING CONCRETE.
I. WHERE FOUNDATION ELEMENTS ARE TO HAVE FILL ON BOTH SIDES, EACH SIDE SHALL BE FILLED SIMULTANEOUSLY, MAINTAINING A COMMON ELEVATION.
J. CONTRACTOR SHALL PROVIDE CONTINUOUS DRAINAGE BY MECHANICAL METHODS TO CONTROL SURFACE AND UNDERGROUND WATER AS REQUIRED DURING CONSTRUCTION, SO THAT ALL EXCAVATIONS ARE DRY.
K. ALL LOCATIONS WHERE BEDROCK IS REMOVED SHALL BE FREE DRAINING SO THAT NO POCKETS OF UNDERGROUND WATER COLLECT.
L. ALL EXPOSED EDGES OF CONCRETE MEMBERS SHALL BE CHAMFERED 3/4" UNLESS SHOWN OTHERWISE ON DRAWINGS.
M. INTERIOR CONCRETE SLABS SHALL BE MOST CURED CONTINUOUSLY FOR 7 DAYS.
N. CONCRETE SHALL BE MAINTAINED ABOVE 50 DEGREES F, AND IN MOST CONDITION FOR AT LEAST THE FIRST SEVEN DAYS AFTER PLACEMENT.
O. ALL EMBEDMENTS IN CONCRETE, INCLUDING ANCHOR BOLTS, SHALL BE FIRMLY SECURED BY THE WIRE TO PREVENT MOVEMENT DURING CONCRETE PLACEMENT.
P. CONSOLIDATE ALL CONCRETE WITH A VIBRATOR OR OTHER MEANS RECOMMENDED BY ACI 301. HONEYCOMBED SURFACES WILL NOT BE PERMITTED.
Q. SEE ARCHITECTURAL DRAWINGS FOR DOOR AND WINDOW OPENINGS, DRIPS, WASHES, REGLETS, CONCRETE FINISHES, MASONRY ANCHORS, AND FOR MISCELLANEOUS EMBEDDED FINISHES, BOLTS, ANCHORS, ANGLES, ETC. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS OF DRAINS. SLOPE SLABS UNIFORMLY TO DRAINS.
3.03 CONTROL JOINTS
A. PLACE CONTROL JOINTS WHERE SHOWN ON THE PLANS. SLAB SECTIONS FORMED WITH CONTROL JOINTS SHOULD BE SQUARE OR NEARLY SQUARE.
B. SAW CUT JOINTS IN CONCRETE, AT EACH CONTROL JOINT LOCATION, AS SOON AS SLAB WILL SUPPORT THE WEIGHT OF THE SOFT-CUT SAW AND OPERATOR. (USUALLY WITHIN 2 HOURS AFTER FINISHING AT CONTROL JOINT LOCATION). THE DEPTH OF CUT SHALL BE 1" TO 1 1/4". USE 3/8" DIAMETER SONOFLOAM CLOSED CELL BACKER-ROD AND SPALLASTIC SL 2 SEALANT.
C. SEAL CONTROL JOINTS TO PREVENT SPALLING OF THE CONCRETE.
3.04 CONCRETE TESTING
A. FOUR CONCRETE TEST CYLINDERS TO BE SET ASIDE FOR LABORATORY TESTING EITHER EVERY 60 CUBIC YARDS FOR ONE CONTINUOUS PLACEMENT OR EACH NEW DAY PLACEMENT, WHICH EVER PRODUCES THE MOST CYLINDERS.
B. THE TESTING FOR THE FOUR CONCRETE CYLINDERS ARE TO CONSIST OF A (1) 7-DAY TEST, (2) 28-DAY TEST, AND (3) HOLD CYLINDER. THE TESTS SHALL INCLUDE TESTING OF THE WET AND DRY DENSITY OF THE CONCRETE AND THE COMPRESSIVE STRENGTH OF EACH SPECIMEN.
C. ALL TESTING SHALL BE PERFORMED BY A LABORATORY IN COMPLIANCE WITH ASTM C495.

DESIGN LOADING

PART 1 - LOADING

- 1.01 DESIGN SOIL BEARING PRESSURE
A. THE DESIGN SOIL BEARING PRESSURE IS ASSUMED TO BE 2,800 PSF.
1.02 DEAD LOAD
A. 1ST FLOOR=10 PSF
B. 2ND FLOOR=10 PSF
C. ROOF=15 PSF
1.03 LIVE LOAD
A. 1ST FLOOR=40 PSF
B. 2ND FLOOR=40 PSF
C. ROOF SNOW LOAD(S)=50 PSF (P)=35 PSF

STRUCTURAL STEEL NOTES

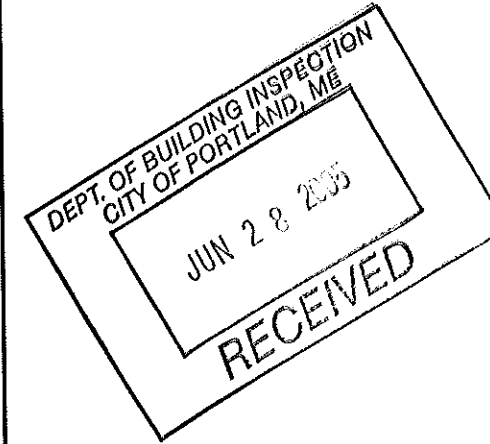
PART 1 - GENERAL

- 1.01 STANDARD SPECIFICATIONS
A. FABRICATION, ERECTION AND WELDING IN ACCORDANCE WITH THE SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS. ALLOWABLE STRESS DESIGN AND PLASTIC DESIGN ADOPTED JUNE 1989. INCLUDING ALL PUBLISHED SUPPLEMENTS.
B. WELDING SHALL BE DONE IN ACCORDANCE WITH THE AMERICAN WELDING SOCIETY "STRUCTURAL WELDING CODE", AND D11.1 (LATEST EDITION). ALL WELDING 1/4" FILLET OR LARGER.
C. BOLTING OF STRUCTURAL JOISTS SHALL BE IN ACCORDANCE WITH "ASCO SPECIFICATIONS FOR STRUCTURAL JOISTS USING ASTM A325 OR A490 BOLTS" (LATEST EDITION).
D. NO CHANGE IN SIZE OR POSITION OF THE STRUCTURAL ELEMENTS SHALL BE MADE WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER.
1.02 SUBMITTALS:
A. SUBMIT SHOP DRAWINGS FOR REVIEW.
1.03 PRODUCT HANDLING:
A. STORE STRUCTURAL STEEL MEMBERS AT THE PROJECT SITE ABOVE GROUND ON PLATFORMS, SKIDS, OR OTHER SUPPORTS.
B. PROTECT STEEL FROM CORROSION.
PART 2 - PRODUCTS
2.01 MATERIALS:
A. STEEL SHAPES, BARS, AND PLATES WILL BE ASTM A-36.
B. STRUCTURAL TUBING AND COLUMNS WILL BE ASTM 500, GRADE B.
C. ANCHOR BOLTS WILL BE ASTM A-307, GRADE A.
D. HIGH STRENGTH BOLTS WILL BE ASTM A-325, TYPE 1 OR 2.
E. WELDING WILL BE PERFORMED WITH 70 KSI WIRE OR ELECTRODES.
F. SHOP PAINT TO BE THENED 80 RD METAL PRIMER, OR EQUIVALENT APPROVED BY THE ENGINEER.
G. NON-SHRINK GROUT SHALL BE 7000 PSI (MIN.) COMPRESSIVE STRENGTH.
H. PLACE NON-SHRINK GROUT UNDER ALL COLUMN BASE PLATES BEFORE APPLYING ANY TENSILE LOADS.
I. ALL WORK AND MATERIALS PERTAINING TO METAL DECK SHALL CONFORM TO STEEL DECK INSTITUTE CODES, SPECIFICATIONS AND RECOMMENDATIONS. METAL DECK SHALL BE GALVANIZED.
PART 3 - EXECUTION
3.01 FABRICATION:
A. FABRICATE STRUCTURAL STEEL IN ACCORDANCE WITH THE REQUIREMENTS OF THE DRAWINGS.
B. SHOP PAINT SURFACES OF ALL STEEL WORK WITH FABRICATOR'S STANDARD RUST INHIBITIVE PAINT, MINIMUM 2.0 MIL THICK.
C. SHOP CONNECTIONS, UNLESS OTHERWISE NOTED, SHALL BE MADE BY WELDING.
D. ALL STRUCTURAL STEEL SHALL BE SHOP PRIMED EXCEPT THAT STRUCTURAL STEEL TO BE FIREPROOFED SHALL NOT BE PRIMED.
E. CONNECTIONS SHOWN ON THESE DRAWINGS ARE GENERALLY SCHEMATIC. THEY ARE INTENDED TO DEFINE THE SPATIAL RELATIONSHIP OF THE FRAMED MEMBERS AND SHOW A FEASIBLE METHOD OF MAKING THE CONNECTION. A LICENSED PROFESSIONAL ENGINEER IN THE STATE OF MAINE SHALL BE RETAINED BY THE FABRICATOR SHALL DESIGN ANY CONNECTION THAT IS NOT SHOWN, OR IS NOT COMPLETELY DETAILED ON THE STRUCTURAL DRAWINGS.
F. SUBSTANTIAL ALTERATIONS OF SCHEMATIC CONNECTION DETAILS MAY IMPACT ARCHITECTURAL CONCEPT AND SHALL NOT BE MADE WITHOUT PRIOR WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER.
G. MINIMUM CONNECTION PLATE THICKNESS SHALL BE 1/4" INCH FOR ALL NON-COMPOSITE BEAMS, THE BEAM-TO-BEAM AND BEAM-TO-COLUMN CONNECTION SHALL DEVELOP THE END REACTION OF THE CONNECTED BEAM. THE END REACTION OF THE CONNECTED BEAM SHALL BE ASSUMED EQUAL TO ONE-HALF THE UNIFORM LOAD CARRYING CAPACITY OF THE BEAM ASSUMING FULL LATERAL SUPPORT, AS GIVEN IN PART 2 (BEAMS AND GIRDERS) OF ASO MANUAL, 9TH EDITION. A MINIMUM SHEAR CAPACITY OF 12 KIPS SHALL BE PROVIDED FOR ALL BEAMS GREATER THAN 8" DEEP AND 8 KIPS FOR BEAMS 8" DEEP OR LESS.
H. CUTS, HOLES, CORNERS, ETC. REQUIRED FOR WORK OF OTHER TRADES SHALL BE SHOWN ON THE SHOP DRAWINGS AND MADE IN THE SHOP. CUTS OR BURNING OF HOLES IN STRUCTURAL STEEL MEMBERS IN THE FIELD WILL NOT BE PERMITTED, UNLESS APPROVED IN WRITING BY THE STRUCTURAL ENGINEER.
I. ALL BEAMS AND COLUMNS ENCASED IN MASONRY OR CONCRETE SHALL BE COVERED WITH A COAL TAR EPOXY COATING, 1/8" THICK, OR SHALL BE GALVANIZED.
3.02 ERECTION:
A. THE STRUCTURAL STEEL SHALL BE ERECTED PLUMB AND TRUE TO THE LINES AND ELEVATIONS INDICATED ON THE DRAWINGS.
B. ERECTION TOLERANCES SHALL BE WITHIN THE LIMITS SPECIFIED IN THE "ASCO CODE OF STANDARD PRACTICE" (LATEST EDITION).
C. TEMPORARY CONNECTIONS SHALL BE ADEQUATE TO SAFELY SUPPORT ALL DEAD LOAD AND ERECTION IMPOSED STRESSES.
D. TEMPORARY BRACING SHALL BE PROVIDED, WHEREVER NECESSARY TO HOLD THE STEEL IN A HORIZONTAL AND VERTICAL PLANE UNTIL PERMANENT BOLTING HAS BEEN COMPLETED.
E. BOLTS SHALL BE INSTALLED IN PROPERLY ALIGNED HOLES AND BROUGHT TO TIGHT CONDITION.
F. ENLARGEMENT OF HOLES BY BURNING WITH A TORCH SHALL NOT BE ALLOWED. ALL STEEL WITH BURNT HOLE ENLARGEMENTS SHALL BE REMOVED AND REPLACED AT THE CONTRACTOR'S EXPENSE.
G. STRUCTURAL STEEL WEB PENETRATIONS SHALL NOT EXCEED 2" DIAMETER. PENETRATIONS SHALL BE DRILLED.

WOOD TRUSS NOTES

PART 1 - GENERAL

- 1.01 STANDARD SPECIFICATION
A. THE LATEST ADDITION OF WTC SHALL APPLY.
B. TRUSS MANUFACTURER TO DESIGN LATERAL LONGITUDINAL BRACING. FINAL TRUSS PLANS TO BE STAMPED BY MANUFACTURER'S ENGINEER.
C. TRUSSES TO BE DESIGNED BASED ON DESIGN LOADS.
PART 2 - PRODUCTS
2.01 MATERIAL
A. ALL METAL TIE-BR CONNECTORS SHALL BE HOT-DIPPED GALVANIZED.
B. ALL BOLTS, NUTS, AND WASHERS ARE TO BE ASTM A307, HOT-DIPPED GALVANIZED.
C. ALL NAILS ARE COMBINATION WIRE, EXCEPT FOR PLYWOOD SHEATHING (BARBED), GALVANIZED FOR EXPOSED FRAMING. STAINLESS STEEL NAILS REQUIRED FOR BIRCH AND TRSA.
PART 3 - ERECTION
3.01 PRODUCT STORAGE:
A. STORE TRUSSES AT THE PROJECT SITE ABOVE GROUND ON PLATFORMS, SKIDS, OR OTHER SUPPORTS.
B. PROTECT FROM CORROSION.
3.02 FABRICATION:
A. FABRICATE TRUSSES IN ACCORDANCE WITH THE REQUIREMENTS OF THE DRAWINGS.
3.03 ERECTION:
A. THE TRUSSES SHALL BE ERECTED PLUMB AND TRUE TO THE LINES AND ELEVATIONS INDICATED ON THE DRAWINGS.
D. TEMPORARY BRACING SHALL BE PROVIDED, WHEREVER NECESSARY TO HOLD THE TRUSSES IN A HORIZONTAL AND VERTICAL PLANE UNTIL PERMANENT ATTACHMENT AND BRACING HAS BEEN COMPLETED.
E. INSTALL PERMANENT BRACING PER MANUFACTURER'S DRAWINGS.
WOOD NOTES:
PART 1 - GENERAL
1.01 STANDARD SPECIFICATIONS
A. THE CURRENT AIO SPECIFICATION SHALL APPLY.
PART 2 - PRODUCTS
2.01 MATERIAL
A. ALL TIE-BR IN CONTACT WITH MASONRY AND CONCRETE OR FRAMING LABELED "P.T." SHALL BE PRESSURE TREATED SOUTHERN PINE GRADE #1 WITH A MINIMUM Fb OF 1,350 PSI AND E OF 1,500 KSI OR BETTER.
B. CUT ENDS OF PRESSURE TREATED (P.T.) LUMBER AND TIE-BR POSTS AND SILLS SHALL BE DIPPED IN A PRESERVATIVE TO COMPLY WITH ANPA M4.
C. ALL TYPICAL FRAMING TIE-BR IS TO BE SPRUCE-PINE-FIR GRADE #2 WITH A MINIMUM Fb OF 750 PSI AND E OF 1,100 KSI OR BETTER.
D. ALL FRAMING TIE-BR LABELED "L.V." SHALL BE BOISE CASCADE LAMINATED VENEER LUMBER WITH A Fb OF 2800 PSI AND E OF 2,000 KSI OR BETTER.
E. ALL FRAMING LABELED "BO" SHALL BE BOISE CASCADE "EASTERN ENGINEERED WOOD PRODUCTS" WITH DESIGN PROPERTIES EQUAL OR BETTER THEN THE SPECIFIED MODEL PROPERTIES.
F. ALL FRAMING LABELED "MP PSL" SHALL BE WOLMANIZED PARALLAM PSL SERVICE LEVEL 2 BY TRUSSTOOST WITH A Fb OF 2088 PSI AND E OF 1740 KSI OR BETTER.
G. ALL METAL TIE-BR CONNECTORS INDICATED ON THE DRAWINGS SHALL BE HOT-DIPPED GALVANIZED.
H. ALL BOLTS, NUTS, AND WASHERS ARE TO BE ASTM A307, HOT-DIPPED GALVANIZED.
I. ALL NAILS ARE COMBINATION WIRE, EXCEPT FOR PLYWOOD SHEATHING (BARBED), GALVANIZED FOR EXPOSED FRAMING. STAINLESS STEEL NAILS REQUIRED FOR BIRCH AND TRSA.
J. REFERENCE TO "SIMPSON" ON DRAWINGS INDICATES METAL CONNECTORS MANUFACTURED BY SIMPSON STRONG-TIE.
PART 3 - EXECUTION
3.01 ERECTION
A. PROVIDE SAME SIZE SOLID BRIDGING/BLOCKING AT MID SPAN FOR ALL JOISTS.
B. FOR EXTERIOR WALLS (2X6) PROVIDE:
1. 3-2X6'S AT CORNERS
2. DOUBLE PLATE WITH 4" MIN. SPICE SEPARATION. ALL SPICES SHALL OCCUR OVER STUDS.
C. AT LOCATIONS WHERE PORTIONS OF WOOD FLOOR OR ROOF DECK ARE ADDED OR REPLACED, THE FINISH FLOOR ELEVATION OF THE NEW WOOD DECK SHALL MATCH THE ADJACENT EXISTING WOOD DECK.
D. PLYWOOD FOR FLOORS AND ROOF SHALL BE INSTALLED WITH BOTH ADHESIVE AND 100 NAILS AT 6" O.C. AT SUPPORTED EDGES AND 12" O.C. ELSEWHERE.
E. FLOOR FRAMING AROUND CHASE OPENINGS FOR MECHANICAL DUCTS SHALL CONSIST OF THE FOLLOWING:
A. DOUBLE FLOOR-LENGTH JOISTS EACH SIDE OF OPENING WITH JOIST DEPTH SAME AS ADJACENT FLOOR FRAMING.
B. MEMBERS CONNECTED WITH SIMPSON DOUBLE JOIST HANGERS.

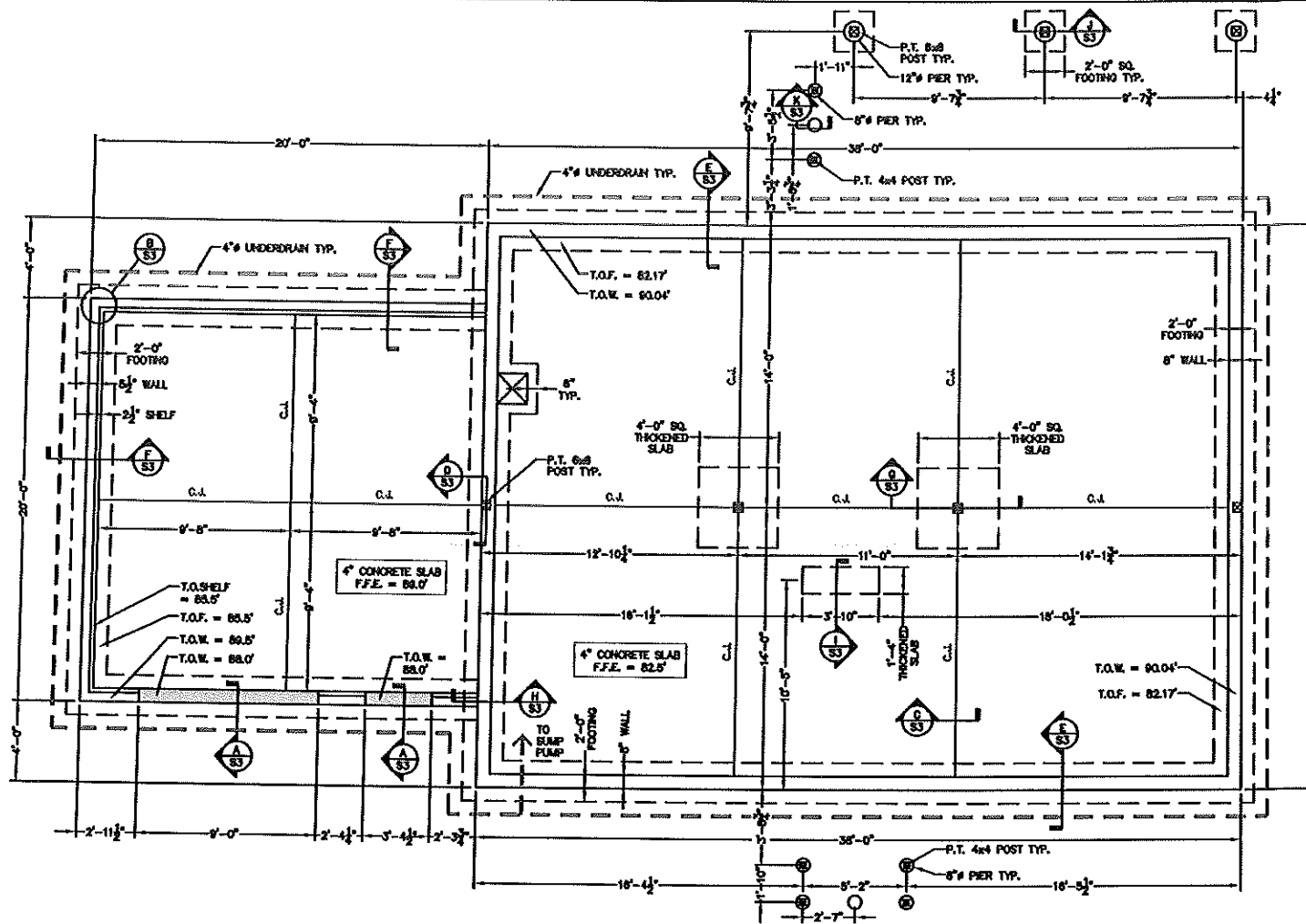


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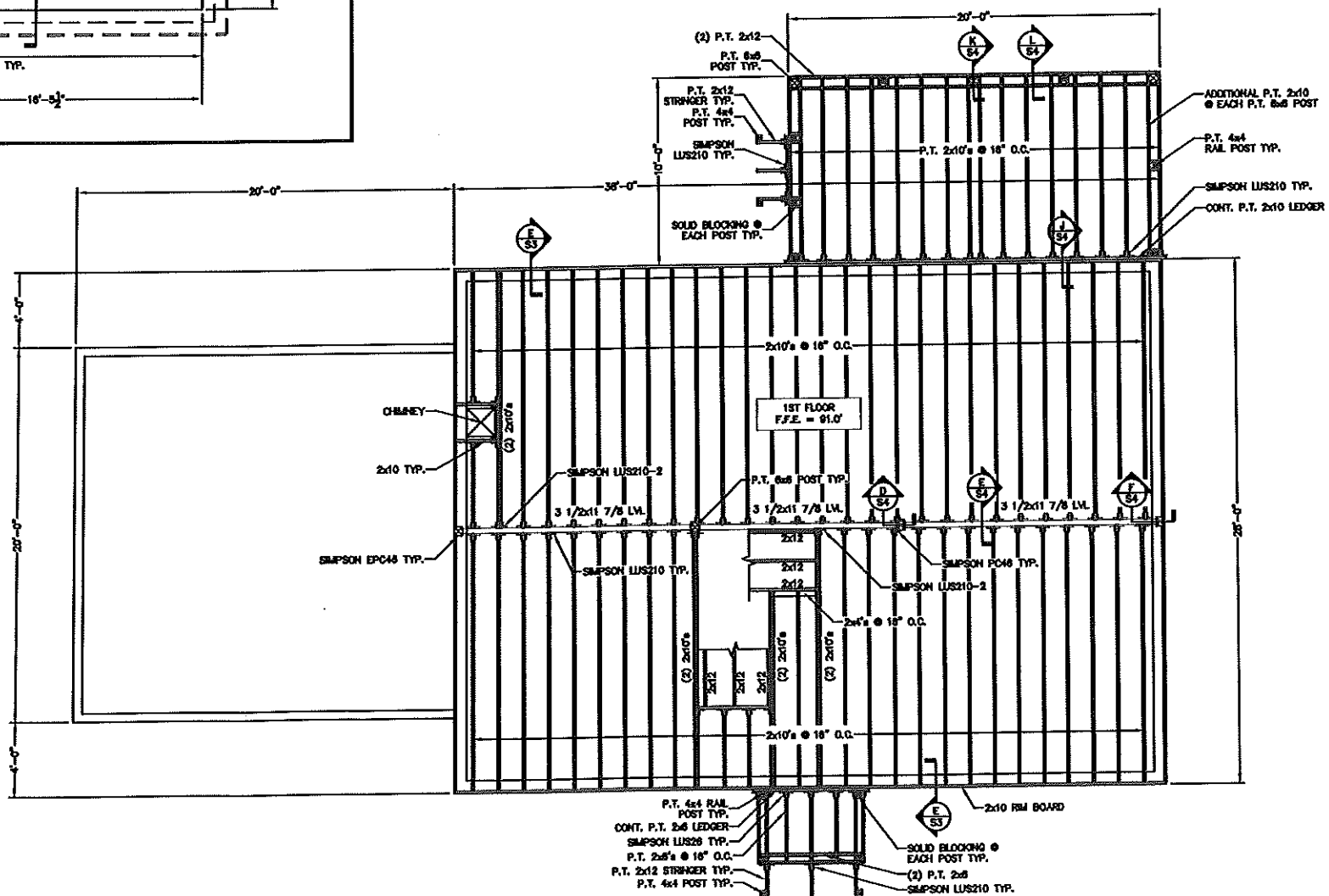
Table with columns for NO., REVISIONS, DATE, and other project details.

NOTES DRAWN BY: FFD CHECKED BY: BEH SCALE: NO SCALE DATE: JUNE 23, 2005

O'BRIEN RESIDENCE LOCATION: FISHER STREET TOWN: PORTLAND COUNTY: CUMBERLAND STATE: MAINE PROJ. NO. 2005-049

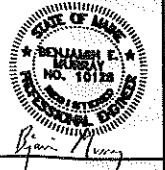


FOUNDATION PLAN
SCALE 1/4" = 1'-0"



1ST FLOOR FRAMING PLAN
SCALE 1/4" = 1'-0"

DEPT. OF BUILDING INSPECTION
 CITY OF PORTLAND, ME
 JUN 28 2005
 RECEIVED

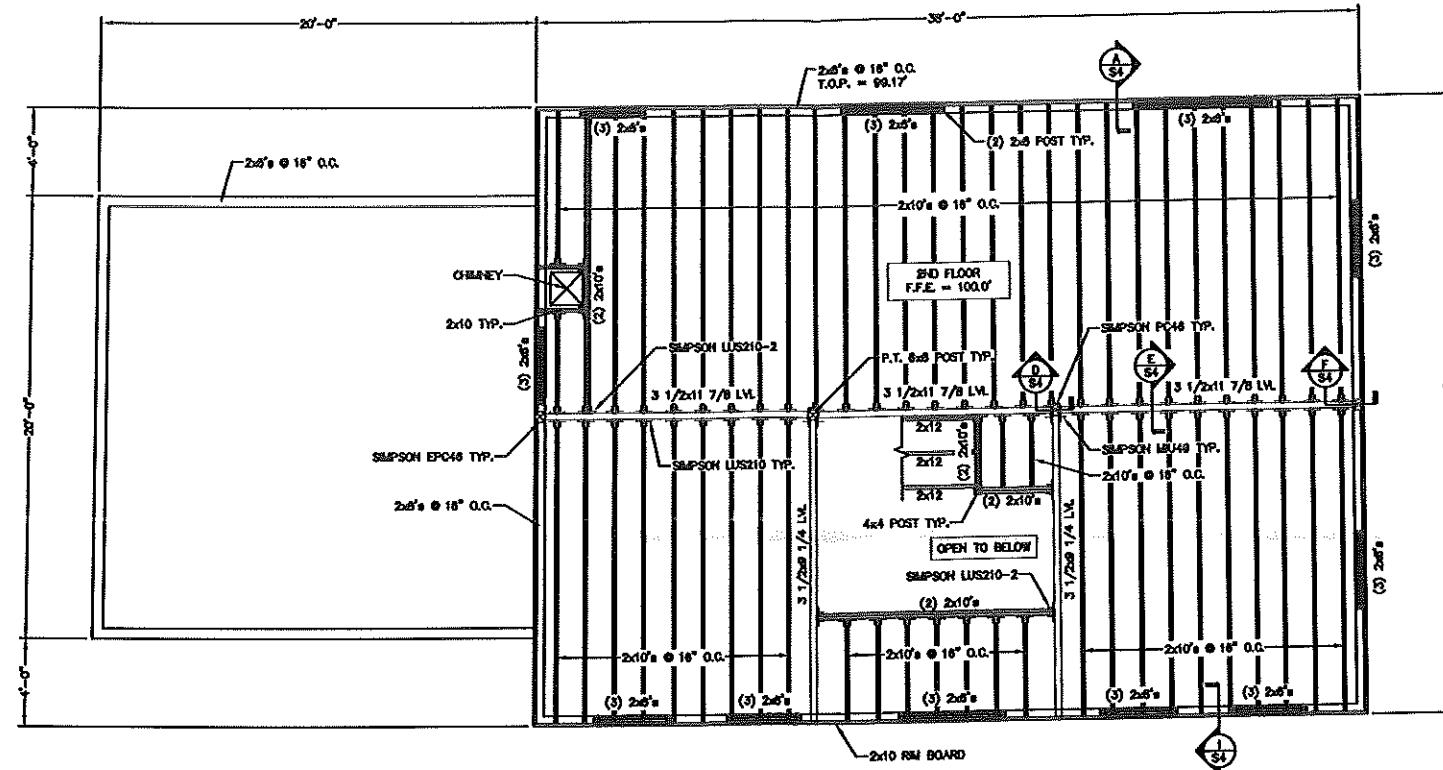


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432 Camp Road, P.O. Box 4687 Augusta, Maine 04330
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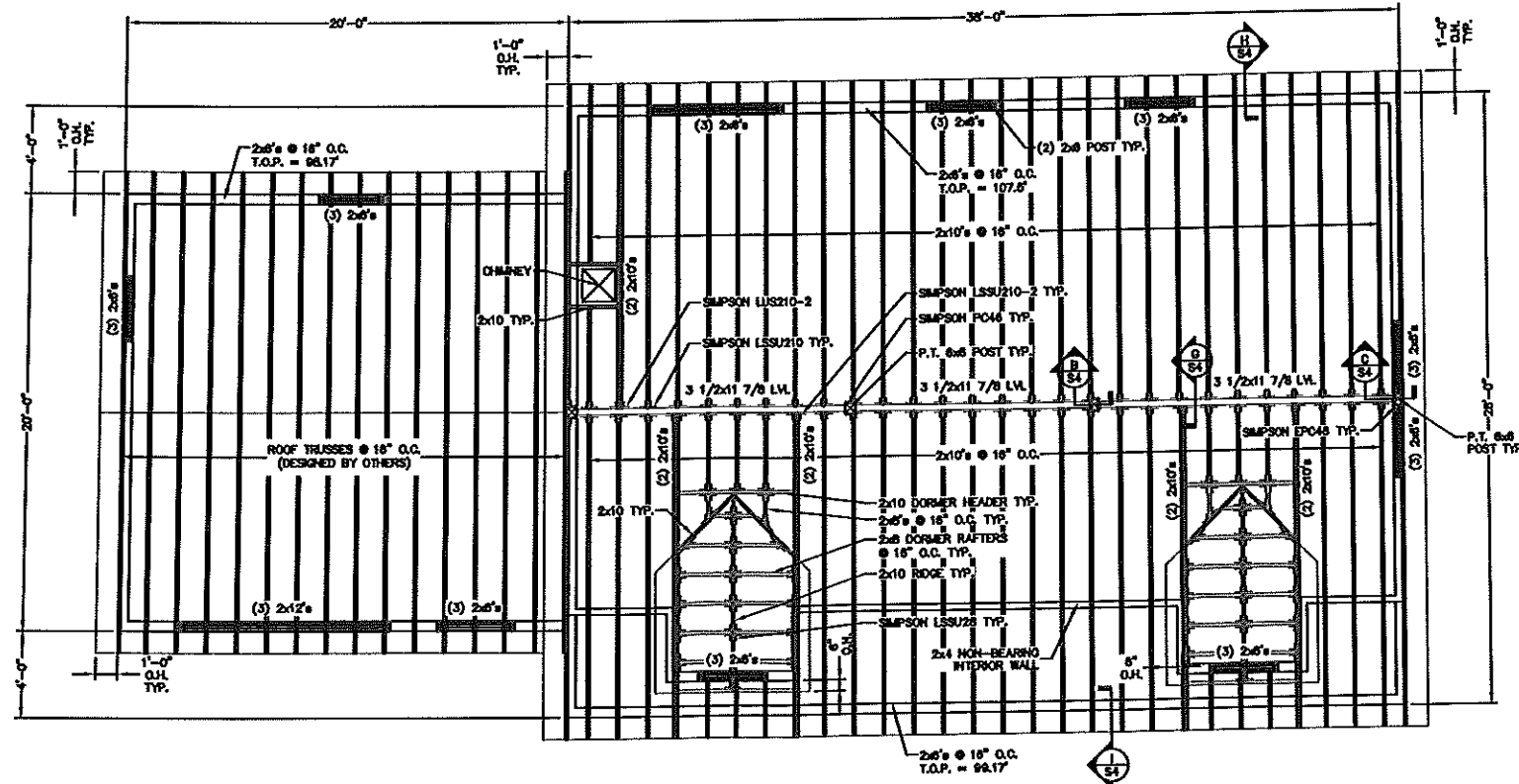
| NO. | DATE | REVISIONS |
|-----|---------|---|
| 1 | 8/22/05 | ADDED DECK & STAIR FRAMING, CONCRETE PIER |

FOUNDATION PLAN & 1ST FLOOR FRAMING PLAN

O'BRIEN RESIDENCE
 CLIENT/PROJECT: FISHER STREET
 LOCATION: PORTLAND, CUMBERLAND COUNTY, MAINE
 DRAWN BY: FRD
 CHECKED BY: BEJA
 SCALE: 1/4" = 1'-0"
 DATE: MARCH 14, 2005

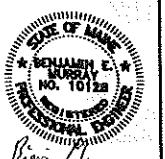


2ND FLOOR FRAMING PLAN
SCALE 1/4" = 1'-0"



ROOF FRAMING PLAN
SCALE 1/4" = 1'-0"

DEPT. OF BUILDING INSPECTION
 CITY OF PORTLAND, ME
 JUN 28 2005
 RECEIVED



E.S. COFFIN
 ENGINEERING & SURVEYING, INC.
 432 Corey Road, P.O. Box 4687, Augusta, Maine 04300
 Ph. (207) 623-4471 Fax (207) 623-4016 Toll Free 1-800-244-6475

| NO. | REVISIONS | DATE |
|-----|--------------------|---------|
| 1 | MOVED NOTES TO S-0 | 6/22/05 |

SUBJECT: **2ND FLOOR FRAMING PLAN & ROOF FRAMING PLAN**
 SCALE: 1/4" = 1'-0"
 DATE: MARCH 14, 2005
 DRAWN BY: FRD
 CHECKED BY: BEM

CLIENT/PROJECT: **O'BRIEN RESIDENCE**
 LOCATION: **FISHER STREET**
 TOWN: **PORTLAND** COUNTY: **CUMBERLAND** STATE: **MAINE**

PROJ. NO. 2005-049

