

PERMIT ISSUED

City of Portland, Maine - Building or Use Permit Application

389 Congress Street, 04101 Tel: (207) 874-8703, Fax: (207) 874-8716

Permit No: 04-1694	Issue Date: NOV 15 2004	GBL: 317 B005001
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Location of Construction: 375 Riverside St	Owner Name: Reynolds Marianne M	Owner Address: Po Box 99	City of Portland	Phone: 317 B005001
Business Name:	Contractor Name: Patco Construction	Contractor Address: 1293 Main St Sanford	Phone: 2073245574	
Lessee/Buyer's Name	Phone:	Permit Type: Foundation Only/Commercial	Zone:	

Past Use: Commercial/ Big Moose Harley	Proposed Use: Big Moose Harley/ Addition - Foundation only	Permit Fee:	Cost of Work: \$0.00	CEO District: 5
Proposed Project Description: Big Moose Harley/ Addition - Foundation only		FIRE DEPT: <input type="checkbox"/> Approved <input checked="" type="checkbox"/> Denied	INSPECTION: Use Group: <i>Foundations</i> Type: <i>Foundations</i>	

Signature: <i>N/A</i>	Signature: <i>Foundations</i>
PEDESTRIAN ACTIVITIES DISTRICT (P.A.D.)	Date: <i>11/15/04</i>
Action: <input type="checkbox"/> Approved <input type="checkbox"/> Approved w/Conditions <input type="checkbox"/> Denied	Date:

Permit Taken By: Idobson	Date Applied For: 11/15/2004	Zoning Approval	
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1. This permit application does not preclude the Applicant(s) from meeting applicable State and Federal Rules.	Special Zone or Reviews	Zoning Appeal	Historic Preservation
2. Building permits do not include plumbing, septic or electrical work.	<input type="checkbox"/> Shoreland	<input type="checkbox"/> Variance	<input type="checkbox"/> Not in District or Landmark
3. Building permits are void if work is not started within six (6) months of the date of issuance. False information may invalidate a building permit and stop all work..	<input type="checkbox"/> Wetland	<input type="checkbox"/> Miscellaneous	<input type="checkbox"/> Does Not Require Review
	<input type="checkbox"/> Flood Zone	<input type="checkbox"/> Conditional Use	<input type="checkbox"/> Requires Review
	<input type="checkbox"/> Subdivision	<input type="checkbox"/> Interpretation	<input type="checkbox"/> Approved
	<input type="checkbox"/> Site Plan	<input type="checkbox"/> Approved	<input type="checkbox"/> Approved w/Conditions
	Major <input type="checkbox"/> Minor <input type="checkbox"/> Minor <input type="checkbox"/> Major <input type="checkbox"/> Major <input type="checkbox"/> Major <input type="checkbox"/> Major	<input type="checkbox"/> Denied	<input type="checkbox"/> Denied
	Date: <i>11/15/04</i>	Date:	Date:

CERTIFICATION

I hereby certify that I am the owner of record of the named property, or that the proposed work is authorized by the owner of record and that I have been authorized by the owner to make this application as his authorized agent and I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in the application is issued, I certify that the code official's authorized representative shall have the authority to enter all areas covered by such permit at any reasonable hour to enforce the provision of the code(s) applicable to such permit.

SIGNATURE OF APPLICANT _____ ADDRESS _____ DATE _____ PHONE _____

RESPONSIBLE PERSON IN CHARGE OF WORK, TITLE _____ DATE _____ PHONE _____

DISPLAY THIS CARD ON PRINCIPAL FRONTAGE OF WORK CITY OF PORTLAND

Please Read
Application And
Notes, If Any,
Attached

BUILDING INSPECTION PERMIT

Permit Number: 041694

PERMIT ISSUED

NOV 15 2004

This is to certify that Reynolds Marianne M/Patco
has permission to Big Moose Harley/ Addition
AT 375 Riverside St

Construction Foundation July
317 B005001

provided that the person or persons, firm or corporation accepting this permit shall comply with all of the provisions of the Statutes of the City of Portland regulating the construction, maintenance and use of buildings and structures, and of the application on file in this department.

Apply to Public Works for street line and grade if nature of work requires such information.

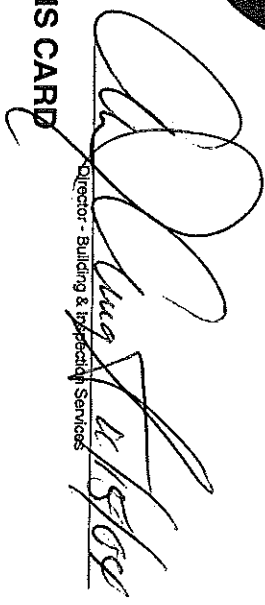
Notification and with this permit shall be closed-in. A NOTICE IS REQUIRED.

A certificate of occupancy must be procured by owner before this building or part thereof is occupied.

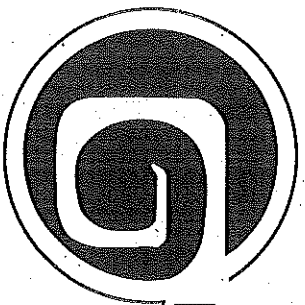
OTHER REQUIRED APPROVALS

Fire Dept _____
Health Dept _____
Appeal Board _____
Other _____
Department Name _____

PENALTY FOR REMOVING THIS CARD


Director - Building & Inspection Services

317 B5



R. W. Gillespie & Associates, Inc.
Geotechnical Engineering • Geohydrology • Materials Testing Services

09 June 2003

Mr. Calvin Reynolds
Big Moose Harley Davidson
375 Riverside Street
Portland, Maine 04101

Subject: Geotechnical Investigation
Additions to Big Moose Harley Davidson
Portland, Maine
RWG&A Project No. 235-883

Dear Mr. Reynolds:

In accordance with our Proposal No. P-5024 GI dated 17 April 2003, R. W. Gillespie & Associates, Inc., (RWG&A) has conducted a geotechnical investigation for the proposed addition to Big Moose Harley Davidson in Portland, Maine. The purpose of this investigation was to obtain information regarding subsurface soil and groundwater conditions on which to base recommendations for design and construction of foundations, ground floor slabs, and frost mitigation.

Subsoils at the site consist of a 2-foot thick layer of fill over silty clay to depths of 46 to 54 feet below the local ground surface. In addition, a 4-foot thick layer of silty sand was encountered below the fill in borings B-101 and B-104. Isolated column and continuous wall footings are recommended for support of the addition in conjunction with a shallow, frost-protected foundation system for the exterior. Soil supported, slab-on-grade construction is considered appropriate for the ground floors. Foundation drainage, consisting of perimeter footing and underslab drains, is recommended for the addition.

INTRODUCTION

The site is located at 375 Riverside Street in Portland, Maine, about ½ mile south of its intersection with Warren Avenue, as shown on Figure 1, *Site Locus Map*. The new addition will be located on the north side of the existing showroom.

Corporate Office - 86 Industrial Park Rd., Ste 4 • Saco, ME 04072 • 207-286-8008 • Fax 207-286-2882
Branch Office - 200 International Dr., Ste 170 • Portsmouth, NH 03801 • 603-427-0244 • Fax 603-430-2041

The project is a 3,050 square foot addition that will have wood framed exterior walls with a wood truss roof. Floors are expected to be slab-on-grade. Loads will be predominately live loads from wind and snow.

SUBSURFACE EXPLORATION

The subsurface exploration program for this investigation consisted of four soil borings drilled to depths of 22 to 58 feet below the local ground surface at the locations shown on Figure 2, *Exploration Location Plan*. Drilling was performed by Great Works Pump & Test Boring, Inc., of Rollinsford, New Hampshire. Explorations were made with a truck-mounted, rotary drill rig using cased hole methodologies.

Standard penetration resistance tests were taken at ground surface and at 5-foot intervals thereafter to depths of 10 to 20 feet. In addition, thin walled tube samples were taken in boring B-102 and field vane shear tests were performed in borings B-102 and B-103. Recovered samples and auger cuttings were used to describe the soils and prepare the boring logs presented in Appendix A. Stratification lines shown on the boring logs represent the approximate boundaries between soil types encountered; the actual transitions will be more gradual and vary over short distances. The standard penetration and field vane shear tests were performed and the thin walled tube samples obtained in general accordance with the following standards; *ASTM D1586, Standard Test Method for Penetration Test and Split-Barrel Sampling of Soils, ASTM D2573, Standard Test Method for Field Vane Shear Test in Cohesive Soil, and ASTM D1587, Standard Practice for Thin-Walled Tube Geotechnical Sampling of Soils.*

LABORATORY TESTING

All samples were visually examined and, when necessary, reclassified using the procedures outlined in *ASTM D2488, Standard Practice for Description and Identification of Soils (Visual Manual Procedure)*. Moisture content, laboratory miniature vane shear, and one-dimensional consolidation tests were performed on selected samples to estimate the engineering properties of undrained shear strength and consolidation for the soils encountered in the test borings. The laboratory testing was performed in general accordance with *ASTM D2216, Standard Test Method for Laboratory Determination of Water (Moisture) Content of Soil and Rock by Mass, ASTM D4648, Standard Test Method for Laboratory Miniature Vane Shear Test for Saturated Fine-Grained Clayey Soil, and ASTM D2435, Standard Test Method for One-Dimensional Consolidation Properties of Soils*; test results are present in Appendix A.

SUBSURFACE CONDITIONS

Subsoils

Below the surficial cover of the asphaltic pavement sections, the subsoils consist of fill over silty clay to depths of 48 to 58 feet below local ground surface. In addition 4 feet of silty sand was encountered below the fill in borings B-101 and B-104. The thickness of the fill is about 2 feet in all the borings and is a medium dense gravelly sand. The silty sand encountered in B-101 and B-104 is a medium dense, fine grained sand. The silty clay is generally medium stiff in the upper 5 to 10 feet and then becomes soft to very soft for the balance of the stratum.

Groundwater

Free water was observed in the completed boreholes at depths of about 0.2 to 2.0 feet below the local ground surface. In, general groundwater levels across the site will fluctuate due to season, temperature, precipitation, and construction activity in the area; therefore, water levels during and following construction will vary from those observed in the subsurface explorations.

EVALUATION OF GEOTECHNICAL DATA

The site is considered appropriate for the proposed addition from a geotechnical standpoint. Subsurface conditions are suitable for the use of shallow, frost-protected foundations consisting of spread footings at shallow depth with exterior insulation and drainage. The footings may bear on undisturbed, naturally-deposited soils or compacted structural fill where utilities or other features require foundation subgrades to be undercut. Post-construction settlements between the addition and the existing building are anticipated to be less than 1 inch. Foundation drainage is recommended to reduce water levels around and within the area of the proposed addition.

RECOMMENDATIONS

Recommendations pertaining to foundation design and construction, and site development are presented in the following sections. Foundation requirements and site development considerations are significantly affected by the subsurface conditions present at the proposed site. RWG&A recommends that foundation design and construction be in accordance with all applicable codes. It is understood that design of the addition will be subject to the requirements of 2000 *International Building Code*®.

Excavation and Filling

1. All topsoil, organic material, debris, pavements, utilities, fill, and other unsuitable foundation bearing materials should be removed from the areas receiving new constructed facilities.
2. Site grading should provide positive drainage away from constructed facilities both during and after construction.
3. Dewatering requirements will vary across the site based on groundwater levels encountered during construction and soil type. In general, it should be practical to accomplish construction dewatering from within excavations by open pumping methods to depths of one to two feet below groundwater. Dewatering to greater depths below groundwater will likely require the use of wells and/or well points. Surface runoff and infiltration of groundwater should be controlled so that excavation, filling, and foundation construction can be completed in-the-dry.
4. Structural fill for support of footings and floor slabs, and for use as backfill, should be a clean, well-graded sand and gravel mixture meeting the following gradation.

Structural Fill Gradation

Screen or Sieve Size	Percent Passing
6 inches	100
3 inches	70 - 100
No. 4	35 - 70
No. 40	5 - 35
No. 200	0 - 5

Note: Maximum particle size limited to 3 inches within two feet of walls and ground floor slabs.

5. In open areas, structural fill should be placed in level, uniform lifts not exceeding 9 inches in uncompacted thickness and be compacted with self-propelled compaction equipment. In confined areas, structural fill should be placed in lifts not exceeding 6 inches in uncompacted thickness (note: maximum particle size 3 inches) and be compacted with hand-operated compaction equipment. Structural fill should be compacted to at least 92 percent of the maximum dry density as determined by *ASTM D1557, Test Method for*

Laboratory Compaction Characteristics of Soil Using Modified Effort (56,000 ft-lbf/ft³ (2,700 kN-m/m³)).

Compaction with large roller compactors might produce vibrations that are noticeable to occupants of the existing building. It is recommended that this issue be reviewed prior to construction.

6. Only structural fill should be used to raise grade and support slab-on-grade floors for the proposed addition.

Foundations

7. The proposed building may be supported on spread and/or continuous footings bearing on undisturbed, naturally deposited soil or compacted structural fill. Footings should be designed for a maximum contact pressure of 1,500 pounds per square foot. For footings with bearing areas having a least lateral dimension smaller than three feet, the allowable bearing pressure shall be 1/3 of the above maximum times a footing's least lateral dimension in feet. Minimum footing width should be in accordance with concrete design and building code requirements, and not less than two feet.
8. A smooth edged bucket should be used during footing excavation to minimize disturbance to the clay subgrade. The exposed subgrade should be protected from further disturbance, moisture, and freezing until the footings are placed. Areas where fill is encountered should be over excavated to undisturbed soil and replaced with compacted structural fill. Any soft areas or areas where moisture has accumulated should be over excavated and replaced with compacted structural fill.
9. Exterior footings may be located at a depth of 2 feet below exterior finished grade and should be frost protected with a minimum of 2 inches of rigid insulation placed on top of the footings and extending 4 feet outward from the foundation walls. It is recommended that two pieces of 1-inch insulation with lapped joints be used to reduce vertical moisture infiltration and gaps in the insulation. The insulation should be pitched away from the foundation to reduce water accumulation.

At heated interior locations, footings may be designed to bear a minimum of 18 inches below the top of the ground floor slab. If exposure to freezing is anticipated, either during or following construction, then interior footings should be lowered and protected in accordance with the above recommendations for exterior footings.
10. The building foundation should be design to withstand lateral, uplift, and overturning forces due to earthquakes. In accordance with 2000 International Building Code® the site is classified as Site Class E.

11. Lateral foundation loads from wind and earthquake may be resisted by cohesion between the bottom of the spread footing and bearing subgrade. A cohesion value of 1,000 psf is recommended for use in design.
12. Perimeter and underslab drainage should be provided around the foundation and under the floor slab. The perimeter drain should be installed at an elevation equal to the bottom of the exterior footings. Invert of the underslab drain pipes should be located at least a foot below the bottom of the floor slab. Two outlets should be provided so as to not be reliant upon a single flow path. The outlets should provide free flow of water under all runoff conditions and, at a minimum, be above the 100-year flood level.

Ground Floor Slabs

13. Subsurface conditions are suitable for slab-on-grade floors. A minimum of 12 inches of structural fill should be placed beneath ground floor slabs and compacted in accordance with the above recommended criteria. A modulus of subgrade reaction of 150 pounds per cubic inch may be used in the design of slab-on-grade floors.

14. A vapor barrier should be installed beneath interior ground floors to minimize moisture infiltration. It is anticipated that details of the type, thickness, depth, bedding, and cover of the vapor barrier will be provided by the Architect or Structural Engineer.

Utilization of On-Site Soils

15. Excavated, on-site soils should be segregated and stockpiled during construction. Laboratory testing will be needed to verify the suitability of on-site soils for reuse as structural or common fill and compaction characteristics.

Geotechnical Observation

16. Since the above geotechnical recommendations are based on limited numbers of observations and tests, the Owner should be particularly sensitive to the potential need for adjustments in the field. It would be in the best interest of the Owner and project to retain RWG&A to observe geotechnical construction aspects of the project, observe general compliance with the design concepts, specifications, and recommendations, and to assist in development of design changes should subsurface conditions differ from those anticipated. Such observation increases the likelihood of the design intent being considered adequately during construction and will allow RWG&A to confirm its design recommendations.

CLOSURE


This report has been prepared for specific application to the Proposed Addition to Big Moose Harley Davidson in Portland, Maine, and for the exclusive use of Big Moose Harley Davidson. This work has been completed in accordance with generally accepted soil and foundation engineering practices. No other warranty, expressed or implied, is made. In the event any changes are made in the nature or location of the project, the conclusions and recommendations of this report should be reviewed by RWG&A.

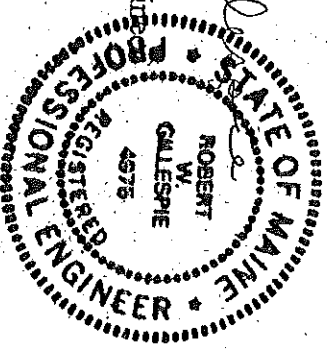
The recommendations presented are based on the results of the referenced soil explorations. The nature of variation between the explorations may not become evident until construction has begun. If significant variations are encountered, it will be necessary for RWG&A to re-evaluate the recommendations presented in this report. RWG&A requests an opportunity for a general review of the final design and specifications in order to determine that foundation recommendations have been interpreted in the manner in which they were intended.

We appreciate the opportunity to be of service to Big Moose Harley Davidson on this project. Please do not hesitate to contact us if you have any questions or if we can be of further service.

Very truly yours,
R. W. GILLESPIE & ASSOCIATES, INC.

Matthew P. Lilley, E. I.
Geotechnical Engineer

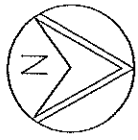
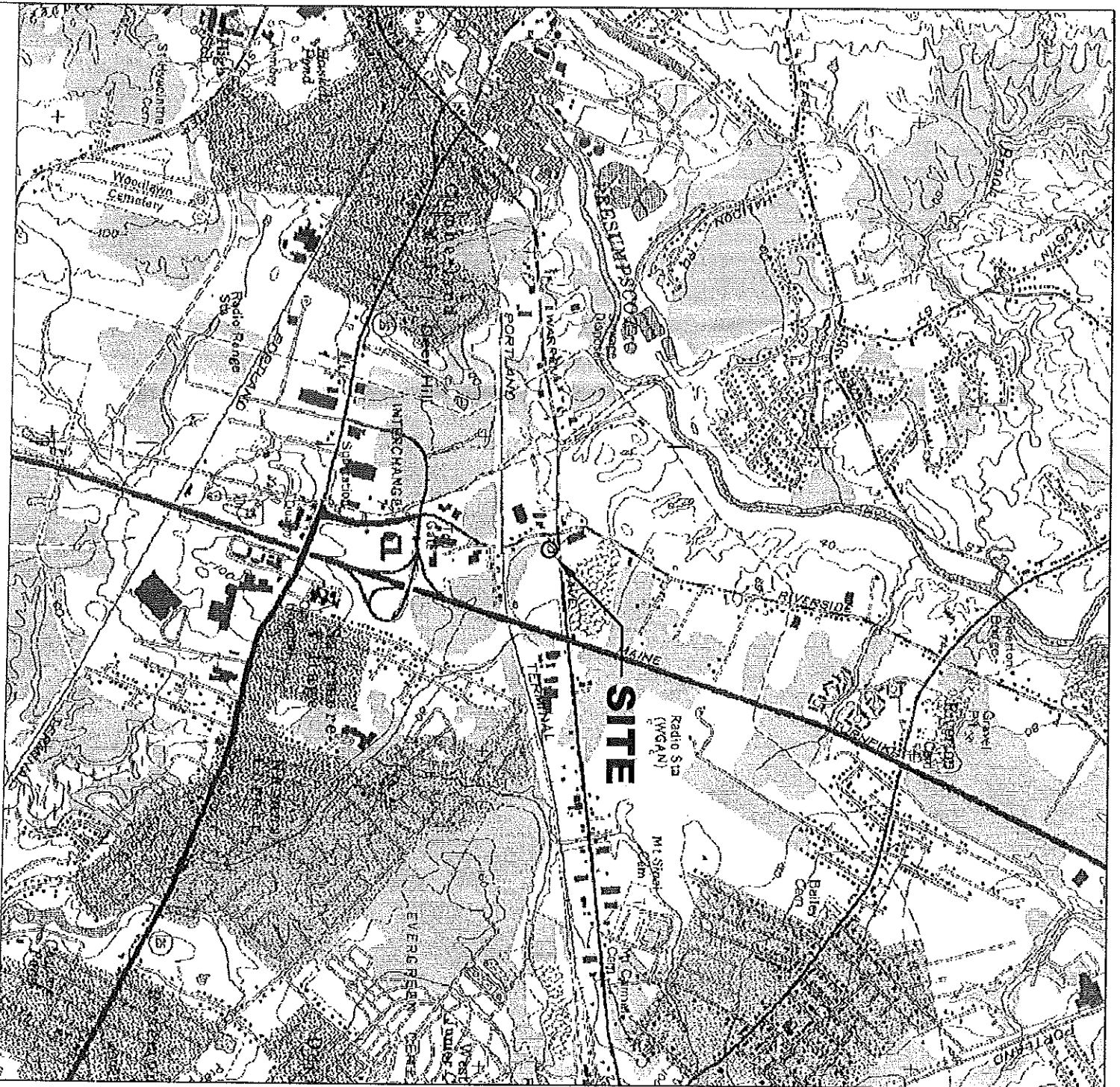

Robert W. Gillespie, P. E.
Principal Geotechnical Engineer



MPL/RWG:ci
In Duplicate
cc: PATCO Construction, Inc. (2)
Attn: Dennis Waters

Attachments:

- Figure 1 - Site Locus Map
- Figure 2 - Exploration Location Plan
- Appendix A - Test Boring Logs
- Appendix B - Laboratory Test Results



SCALE, FEET

SOURCE:
USGS 7.5-MINUTE TOPOGRAPHIC QUADRANGLE
OF PORTLAND WEST, ME, DATED 1978.

FIGURE 1
SITE LOCUS MAP
ADDITION TO BIG MOOSE HARLEY DAVIDSON
PORTLAND, MAINE

JUNE 2003

PROJECT NO. 235-883



R.W. Gillespie & Associates, Inc.
CONSULTING GEOTECHNICAL & ENVIRONMENTAL SPECIALISTS

86 Industrial Park Rd., Suite 4 Seaco, Maine 04072 (207) 286-8008
Fax: (207) 286-2882 E-mail: rwg-c@rwg-a.com

R. W. Gillespie & Associates, Inc.

APPENDIX A

TEST BORING LOGS

**Geotechnical Investigation
Addition to Big Moose Harley Davidson
Portland, Maine**

BORING LOG B-101

Project: Big Moose Harley Davidson
 Location: Portland, Maine

Approximate Surface Elevation:
 Ground Water Depth: 2'±

Client: Calvin Reynolds

Date: 09 May 2003

Project No. 235-883

DEPTH, FT.	SYMBOL	SAMPLES	DEPTH, FT.	DESCRIPTION OF MATERIAL	SAMPLE RECOVERY, IN.	BLOWS PER 6"	SPT-N BLOWS PER FT.	MOISTURE CONTENT	Lab Tests
0			0	ASPHALTIC PAVEMENT (1.5 inches).	14	10	19		
				FILL: Gravelly Sand, medium dense, moist, coarse to medium sand, some gravel, trace silt, brown.		10			
				SILTY SAND (SP); medium dense, wet, fine sand, some silt, gray with thin (<1") clay seams, stratified.		9			
		S-1	5		19	12	30	24.1	
				SILTY CLAY (CL); medium stiff then soft, wet, gray with thin (<1") sand seams.		15			
						8			
		S-2	10		24	2	5	32.2	
						3			
		S-3	15		24	2			
						2			
		S-4	20		24			46.6	
		S-5	25		24			44.1	
				-GLACIAL MARINE DEPOSITS-					
				Probed with "A" rod and hydraulic push from 22' to 54'.					
			30						
			35						

BORING LOG B-101

Project: Big Moose Harley Davidson
 Location: Portland, Maine

Approximate Surface Elevation:
 Ground Water Depth: 2'±

Client: Calvin Reynolds

Date: 09 May 2003

Project No. 235-883

DEPTH, FT.	SYMBOL	SAMPLES	SAMPLE #	DESCRIPTION OF MATERIAL	SAMPLE RECOVERY, IN.	BLOWS PER 6"	SPT-N BLOWS PER FT.	MOISTURE CONTENT	Lab Tests										
40				<p style="text-align: center;">SAND (SP); logged from change in probe resistance and hammer blow count.</p> <table style="width: 100%; border-collapse: collapse;"> <tr> <td style="text-align: left;">Depth</td> <td style="text-align: right;">Blows</td> </tr> <tr> <td style="text-align: left;">54.0 - 55.0</td> <td style="text-align: right;">48</td> </tr> <tr> <td style="text-align: left;">55.0 - 56.0</td> <td style="text-align: right;">44</td> </tr> <tr> <td style="text-align: left;">56.0 - 57.0</td> <td style="text-align: right;">38</td> </tr> <tr> <td style="text-align: left;">57.0 - 58.0</td> <td style="text-align: right;">38</td> </tr> </table> <p>Bottom of Exploration at 58'; not refusal, boring terminated 4' into sand.</p>	Depth	Blows	54.0 - 55.0	48	55.0 - 56.0	44	56.0 - 57.0	38	57.0 - 58.0	38					
Depth	Blows																		
54.0 - 55.0	48																		
55.0 - 56.0	44																		
56.0 - 57.0	38																		
57.0 - 58.0	38																		
45																			
50																			
55																			
60																			
65																			
70																			

BORING LOG B-102

Project: Big Moose Harley Davidson
 Location: Portland, Maine

Approximate Surface Elevation:
 Ground Water Depth: 0.2'

Client: Calvin Reynolds

Date: 09 May 2003

Project No. 235-883

DEPTH, FT.	SYMBOL	SAMPLES	SAMPLE #	DESCRIPTION OF MATERIAL	SAMPLE RECOVERY, IN.	BLOWS PER 6"	SPT-N BLOWS PER FT.	MOISTURE CONTENT	Lab Tests
0			S-1	ASPHALTIC PAVEMENT (2 inches). FILL: Gravelly Sand, medium dense, wet, coarse to fine sand, some gravel, trace silt, brown. SILTY CLAY (CL); medium stiff then soft, wet, gray with thick (2"-4") sand seams.	NR	10 12 8	20		
5			S-2		20	4 2 3 2	5	30.9	
10			U-1		NR			45.9	Lab Vane
15			U-1A		22/ 24				
15			FV	Field Vane: Undrained Shear Strength; Su = 0.44 ksf; Residual, Su = 0.03 ksf.					
15			FV	Field Vane: Undrained Shear Strength; Su = 0.68 ksf; Residual; Su = 0.03 ksf.					
25			FV	Field Vane: Undrained Shear Strength; Su = 0.68 ksf; Residual; Su = 0.03 ksf.					
25			FV	Field Vane: Undrained Shear Strength; Su = 0.68 ksf; Residual; Su = 0.03 ksf.					
20			U-2		23/ 24			47.8	Cons Lab Vane
30									
35									

BORING LOG B-102

Project: Big Moose Harley Davidson
 Location: Portland, Maine

Approximate Surface Elevation:
 Ground Water Depth: 0.2'

Client: Calvin Reynolds

Date: 09 May 2003

Project No. 235-883

DEPTH, FT.	SYMBOL	SAMPLES	SAMPLE #	DESCRIPTION OF MATERIAL	SAMPLE RECOVERY, IN.	BLOWS PER 6"	SPT-N BLOWS PER FT.	MOISTURE CONTENT	Lab Tests
70									
65									
60									
55				SAND (SP); (logged from change in probe resistance and/or hammer blow count)					
50									
45									
40									
35									
30									
25									
20									
15									
10									
5									
0									
Depth Blows 57.0 - 58.0 62 Bottom of Exploration at 58'; not refusal, boring terminated 4.5" into sand.									

R.W. Gillespie & Associates
 Saco, Maine

BORING LOG B-103

Project: Big Moose Harley Davidson
 Location: Portland, Maine

Approximate Surface Elevation:
 Ground Water Depth: 0.2'

Client: Calvin Reynolds

Date: 09 May 2003

Project No. 235-883

DEPTH, FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SAMPLE RECOVERY, IN.	BLOWS PER 6"	SPT-N BLOWS PER FT.	MOISTURE CONTENT	Lab Tests
0		S-1	ASPHALTIC PAVEMENT (2 inches).	11	9	15		
			FILL: Gravelly Sand, medium dense, wet, coarse to fine sand, some gravel, trace silt, brown.	8	7			
			SILTY CLAY (Cl); medium stiff to soft, wet, olive brown then gray.	7	2			
5		S-2		24	2	4	39.4	
10		S-3		24	2	3	48.6	
					2			
					1			
15		FV	Field Vane: Undrained Shear Strength; Su = 0.38 ksf, Residual; Su = 0					
20		FV	Field Vane: Undrained Shear Strength; Su = 0.53 ksf, Residual; Su = 0.01 ksf.					
25		FV	Field Vane: Undrained Shear Strength; Su = 0.46 ksf, Residual; Su = 0					
30			-GLACIAL MARINE DEPOSITS- Probed with "A" rod and hydraulic push from 27' to 46.5'.					
35								

BORING LOG B-103

Project: Big Moose Harley Davidson
 Location: Portland, Maine

Approximate Surface Elevation:
 Ground Water Depth: 0.2'

Client: Calvin Reynolds

Date: 09 May 2003

Project No. 235-883

DEPTH, FT.	SYMBOL	SAMPLES	SAMPLE #	DESCRIPTION OF MATERIAL	SAMPLE RECOVERY, IN.	BLOWS PER 6"	SPT-N BLOWS PER FT.	MOISTURE CONTENT	Lab Tests
45				SAND (SP); (logged from change in probe resistance and hammer blow count). Depth Blows 46.5 - 47.5 33 47.5 - 48.5 42 Bottom of Exploration at 48.5'; not refusal, boring terminated 2' into sand.					
40									
35									
30									
25									
20									
15									
10									
5									
0									

APPENDIX B

LABORATORY TEST RESULTS

Geotechnical Investigation
Addition to Big Moose Harley Davidson
Portland, Maine

Laboratory Vane Shear Test Results

Project: Addition to Big Moose Harley Davidson Client: Big Moose Harley Davidson
Project No.: 235-883

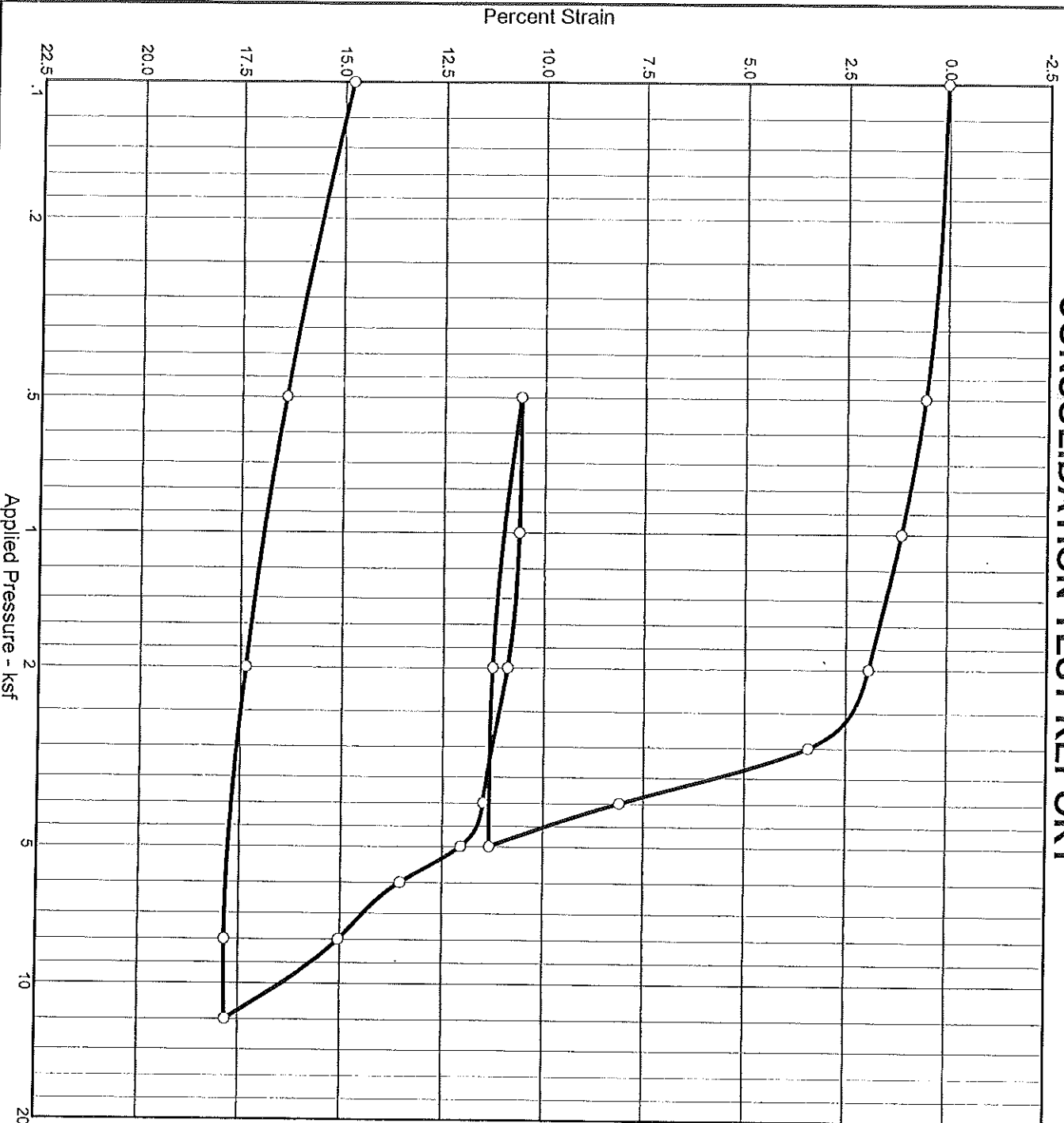
Boring No.	B-102	Lab No.	6621F
Sample No.	12.5' - 14.5'		
Test No.	S_u (Undisturbed)	S_u (Residual)	Moisture Content
1	480 psf	40 psf	45.9%
2	640 psf	100 psf	37.3%
3	240 psf	80 psf	48.5%

Laboratory Vane Shear Test Results

Project: Addition to Big Moose Harley Davidson Client: Big Moose Harley Davidson
Project No.: 235-883

Boring No.	B-102	Lab No.	6621G
Sample No.	20' - 22'		
Test No.	S _u (Undisturbed)	S _u (Residual)	Moisture Content
1	320 psf	0 psf	46.0%
2	340 psf	10 psf	47.8%
3	360 psf	0 psf	48.4%

CONSOLIDATION TEST REPORT



Natural		Dry Dens. (pcf)	LL	PI	Sp. Gr.	Overburden (ksf)	P _c (ksf)	C _c	C _r	Initial Void Ratio
Saturation	Moisture	75.7			2.77		2.94	0.87	0.03	1.283
94.7 %	43.9 %									

MATERIAL DESCRIPTION

Silty Clay

Project No. 235-883 **Client:** Big Moose Harley Davidson

Project: Addition to Big Moose Harley Davidson

Location: B-102, U-2 (20'-22')

Remarks:
Tested by MPL

CONSOLIDATION TEST REPORT

R.W. Gillespie & Associates, Inc.

Lab No. 6621G



Planning and Development Department
Lee D. Urban, Director

Planning Division
Alexander Jaegerman, Director

November 9, 2004

Calvin Reynolds
President
Big Moose Harley-Davidson
375 Riverside Street
Portland, ME 04102

RE: Big Moose Harley-Davidson Addition, 375 Riverside Street
ID #2004-0203, CBL #317-B-005

Dear Mr. Reynolds:

On November 5, 2004, the Portland Planning Authority granted minor site plan approval for a 3,050 sq. ft. addition located at 375 Riverside Street, as shown on the approved plan. Where submission drawings are available in electronic form, the applicant shall submit any available electronic CADD.DXF files with seven sets of final plans.

The approval is based on the submitted site plan. If you need to make any modifications to the approved site plan, you must submit a revised site plan for staff review and approval.

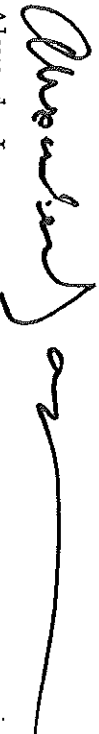
Please note the following provisions and requirements for all site plan approvals:

1. The site plan approval will be deemed to have expired unless work in the development has commenced within one (1) year of the approval or within a time period agreed upon in writing by the City and the applicant. A one year extension may be granted by this department if requested by the applicant in writing prior to the expiration date of the site plan.
2. A performance guarantee in a form acceptable to the City of Portland and an inspection fee equal to 2.0% of the performance guarantee will have to be posted before beginning any site construction or issuance of a building permit.
3. A defect guarantee, consisting of 10% of the performance guarantee, must be posted before the performance guarantee will be released.
4. Prior to construction, a pre-construction meeting shall be held at the project site with the contractor, development review coordinator, Public Works representative and owner to review the construction schedule and critical aspects of the site work. At that time, the site/building contractor shall provide three (3) copies of a detailed construction schedule to the attending City representatives. It shall be the contractor's responsibility to arrange a mutually agreeable time for the pre-construction meeting.

5. If work will occur within the public right-of-way such as utilities, curb, sidewalk and driveway construction, a street opening permit(s) is required for your site. Please contact Carol Merritt at 874-8300, ext. 8822. (Only excavators licensed by the City of Portland are eligible.)
6. Where submission drawings are available in electronic form, the applicant shall submit any available electronic CADD.DXF files with seven sets of final plans.
7. The Development Review Coordinator must be notified five (5) working days prior to date required for final site inspection. The Development Review Coordinator can be reached at the Planning Division at 874-8632. Please note that no Certificates of Occupancy will be issued until all site improvements have been completed and inspected in the field by the Development Review Coordinator.

If there are any questions, please contact Kandice Talbot at 874-8901.

Sincerely,



Alexander Jaegerman
Planning Division Director

cc: Lee D. Urban, Planning and Development Department Director
Sarah Hopkins, Development Review Program Manager
Kandice Talbot, Planner
Jay Reynolds, Development Review Coordinator
Marge Schmuckel, Zoning Administrator
Gayle Guertin, Inspections
Michael Bobinsky, Public Works Director
Traffic Division
Eric Labelle, City Engineer
Jeff Tarling, City Arborist
Penny Littell, Associate Corporation Counsel
Lt. Gaylen McDougall, Fire Prevention
Assessor's Office
Approval Letter File



Letter of Certification

377BS

Date: 9/20/2004

Time: 10:36:32 AM

Page: 1 of 2

Letter of Certification

Contact: Bill Rudman or Ron Mercier
Name: PATCO Construction Inc
Address: 1295 Main St

Project: Big Moose Harley Davidson
Builder PO #: 2663
Jobsite: 375 Riverside St

City, State: Sanford , Maine 04073
Country: United States

City, State: Portland, Maine 04103
Country, Country: Cumberland, United States

This is to certify that the above referenced VP BUILDINGS project has been designed for the applicable portions of the following Building Code and in accordance with the order documents which have stipulated the following applied environmental loads and conditions:

Overall Building Description

Shape	Overall Width	Overall Length	Floor Area (sq. ft.)	Wall Area (sq. ft.)	Roof Area (sq. ft.)	Max. Eave Height	Min. Eave Height 2	Max. Roof Pitch	Min. Roof Pitch	Peak Height
Reynolds Motorsports	70/0/0	43/0/0	3010	3620	3261	11/6/0	11/6/0	5,000:12	5,000:12	26/1/0

Loads and Codes - Shape: Reynolds Motorsports

City: Portland
Building Code: 2003 International Building Code
Building Use: Standard Occupancy Structure

State: Maine
Built Up: 89AISC
Cold Form: 96AISI

Country: United States
Rainfall: 4.00 in per hour
Allow. Overstress:

Frm: 1.03, Sec: 1.03, Brc: 1.03

Dead and Collateral Loads

Collateral Gravity: 5.00 psf
Collateral Uplift: 0.00 psf

Roof Covering + Second. Dead Load: 7.50 psf
Frame Weight (assumed for seismic): 3.00 psf

Live Load
Live Load: 20.00 psf Not Reducible
LL for Below Eave Canopy: N/A

Wind Load

Wind Speed: 90.00 mph
Wind Exposure (Factor): B (0.701)
Parts Wind Exposure Factor: 0.701

Snow Load
Ground Snow Load: 70.00 psf
Design Snow (Sloped): 32.15 psf
Snow Exposure Category (Factor): 1 Fully Exposed
(0.90)

Seismic Load
Mapped Spectral Response - Ss: 37.36 %g
Mapped Spectral Response - S1: 19.98 %g
Seismic Hazard / Use Group: Group 1

Wind Enclosure: Enclosed
Wind Importance Factor: 1.000
Hurricane Prone Region
Base Elevation: 0/0/0
Primary Zone Strip Width: 8/7/3
Parts / Portions Zone Strip Width: 4/3/10
Basic Wind Pressure: 12.35 psf

Snow Importance: 1.000
Thermal Category (Factor): Heated (1.00)
Ground / Roof Conversion: 0.70
% Snow Used in Seismic: 20.00
Seismic Snow Load: 8.82 psf
Unobstructed, Slippery Roof

Seismic Importance: 1.000
Seismic Performance / Design Category: C
System NOT detailed for Seismic
Framing Seismic Period: 0.0000
Bracing Seismic Period: 0.0000
Framing R-Factor: 3.0000
Bracing R-Factor: 3.0000
Soil Profile Type: Stiff soil (D, 4)
Frame Redundancy Factor: 1.0000
Brace Redundancy Factor: 1.0000
Frame Seismic Factor (Cs): 0.0556
Brace Seismic Factor (Cb): 0.0500

Per Article 2.9 in the Builder Agreement, VP Buildings assumes that the Builder has called the local Building Official or Project Engineer to obtain all code and loading information for this specific building site.

The steel design is in accordance with VP BUILDINGS standard design practices, which have been established based upon pertinent procedures and recommendations of the following organizations:

- American Institute of Steel Construction (AISC)
- American Iron and Steel Institute (AISI)
- American Welding Society (AWS) [D1.1]
- American Society for Testing and Materials (ASTM)
- Metal Building Manufacturers Association (MBMA)
- AISC Category MB Manufacturer Certification.

This certification DOES NOT apply to the design of the foundation or other on-site structures or components not supplied by VP BUILDINGS, nor does it apply to unauthorized modifications to framing systems provided by VP BUILDINGS. Furthermore, it is specified that certification is based upon the premise that all components furnished by VP BUILDINGS will be erected or constructed in strict compliance with drawings, specifications, and cuttings furnished by VP BUILDINGS.

Since 1985
VP BUILDINGS
AISC-REGISTERED
ENGINEER

VP BUILDINGS

3200 Plazek's Club Circle, Memphis TN 38125-8843

P.E. Prepared by:

Reviewed by:



Letter of Certification

The Structural Design and/or Manufacture of this VP BUILDINGS building will be or has been at one of the following VP Buildings locations:

Rainsville, AL.....	VP Alabama Plant.....	[Manufacture Only]
Memphis, TN.....	VP Headquarters.....	[Design Only]
Pine Bluff, AR.....	VP Arkansas Service Center.....	[Design and Manufacture]
Turlock, CA.....	VP California Service Center.....	[Design and Manufacture]
St. Joseph, MO.....	VP Missouri Service Center.....	[Design and Manufacture]
Kemerville, NC.....	VP North Carolina Service Center.....	[Design and Manufacture]
Van Wert, OH.....	VP Ohio Service Center.....	[Design Only]
Evansville, WI.....	VP Wisconsin Service Center.....	[Design and Manufacture]
Monterey, Mx.....	VP Mexico Service Center.....	[Design and Manufacture]

Additional Structural Material may be fabricated and provided for use in a VP Buildings building by one of the following fabricators:

BAR JOISTS-			
SML, Inc.	Hope, AR	STRUCTURAL STEEL FABRICATION	
SML, Inc.	Fallon, NV	Addison Steel, Inc.	Orlando, FL
SML, Inc.	Starke, FL	PKM Steel Service, Inc.	Salina, KS
SML, Inc.	Iowa Falls, IA		
SML, Inc.	Cayce West Columbia, SC	Qualico Steel Co. Inc.	Webb, AL
Hancock	Salem, VA		
Canam	Washington, MO		
Vulcraft	Grapeland, TX		
Vulcraft	Norfolk, NE		
Vulcraft	Florence, SC		
Vulcraft	Brigham City, UT		
ISP	El Paso, TX		
Socar	Florence, SC		
Quincy	Quincy, FL		
New Millennium Building Systems	Butler, IN		

(This information is presented in compliance with VP Buildings' AISC Certification responsibilities.)

ELECTRICAL PERMIT City of Portland, Me.



To the Chief Electrical Inspector, Portland Maine:
The undersigned hereby applies for a permit to make electrical installations in accordance with the laws of Maine, the City of Portland Electrical Ordinance, National Electrical Code and the following specifications:

Date 1. 11. 05

Permit # 20054638

CBL# 317 B 005

LOCATION: 575 Riverside St

METER MAKE & #

CMP ACCOUNT #

OWNER

TENANT B.G. moose Harley

PHONE #

							TOTAL EACH FEE
OUTLETS	18	Receptacles	8	Switches	<input checked="" type="checkbox"/>	Smoke Detector	20
FIXTURES		Incandescent	34	Fluorescent		Strips	20
SERVICES		Overhead		Underground		TTL AMPS	<800 15.00
		Overhead		Underground		>800	25.00
Temporary Service		Overhead		Underground		TTL AMPS	25.00
METERS		(number of)					25.00
MOTORS		(number of)					1.00
RESID/COM		Electric units		Interior		Exterior	2.00
HEATING		oil/gas units		Ranges		Wall Ovens	5.00
		Ranges		Water heaters		Fans	2.00
APPLIANCES		Insta-Hot		Disposals		Dishwasher	2.00
		Dryers		Spa		Washing Machine	2.00
MISC. (number of)		Others (denote)					2.00
		Air Cond/win				Pools	3.00
		Air Cond/cent				Thermostat	10.00
		HVAC		EMS			5.00
		Signs					10.00
		Alarms/res					5.00
		Alarms/com					15.00
		Heavy Duty(CRKT)					2.00
		Circus/Carnv					25.00
		Alterations					5.00
		Fire Repairs					15.00
		E Lights					1.00
		E Generators					20.00
PANELS		Service		Remote		Main	4.00
TRANSFORMER		0-25 Kva					5.00
		25-200 Kva					8.00
		Over 200 Kva					10.00
		MINIMUM FEE/COMMERCIAL	45.00			TOTAL AMOUNT DUE	
		MINIMUM FEE				MINIMUM FEE	35.00

DEPT. OF BUILDING INSPECTION
CITY OF PORTLAND, ME
JAN 11 2005
RECEIVED

CONTRACTORS NAME James R THURASAN MASTER LIC. # MS66016638
 ADDRESS 720 Box 853 Miller Hill Rd LIMITED LIC. #
 TELEPHONE 603 652-7664
 SIGNATURE OF CONTRACTOR Jim R Thyras

White Copy - Office • Yellow Copy - Applicant