Ryerson Number: 50 445519 03 01 001
Territory: 50
Load: OTX
Item Number: 03

PO 10248
J 4041

RYERSON TULL
Material Certification

To: JAMES A MCBRADY INC

PO BOX 8239
PORTLAND ME 04104

Your Order Number	Order Date
10248	09/27/2002

Item Description
PLT STNLS 304L A/P 3/8 x 4 x 0 FT 6

Single / Multiple Heat Number
43091

Slab / Coil Number (if applicable)
576651

Item Instructions
SEND CERTS WITH SHIPMENT
CERTIFIED

Part Number (if applicable)

Item Mark Instruction (if applicable)

A survey of our material sources has indicated that neither mercury nor radioactive substances is introduced into their products, or is used in any of their processes. While we make no independent tests for mercury or radiation, there is nothing in Ryerson's system, which could be expected to introduce contamination of either type.

This document certifies that the material described above was shipped on your order and that the attached data is a true copy of the test report furnished by the producer with said material.

10/01/2002

Authorized Agent of RYERSON TULL

FAFER
GROUPE USINOR
Téléphone : 00/32/71/44.16.64
Téléfax : 00/32/71/44.16.28
S.A. FABRIQUE DE FER DE CHARLEROI
B.P. 1713 - B-6000 CHARLEROI (Belgique)

TOLES LAMINEES A CHAUD
HOT [illegible] ED PLATES
WARMGEWALZTE BLECHE
LAMIERE LAMINATE A CALDO

Certificat de contrôle des produits par l'usine : 3.1.B.
Mill test certificate : EN 10204
Prüfzeugnis : ISO 10474
Certificato di controllo interno dei prodotti :

CLIENT / CUSTOMER / BESTELLER / CLIENTE

CHARLEROI-U.S.A., RYERSON-TULL
45 SARATOGA BLVD
USA-MA 01432 DEVENS

REFERENCE / PURCHASER ORDER / BESTELL NR / REFERENZA
50A12231 DU 17.12.99.

COMMANDE USINE NR / ORDER NR / WERKS NR / ORDINE NR
6355

CERTIFICATE - PRÜFZEUGNIS - CERTIFICATO
155699

BORDEREAU / DELIVERY / VERSANDANZEIGE / BORDERO
188769

FAFER (C.Q.)

PRODUIT - ARTICLE - PRÜFGEGENSTAND - PRODOTTO : TOLES - PLATES - BLECHE - LAMIERE

MATIERE / MATERIAL / WERKSTOFF / MATERIALE	SUIVANT / ACCORDING TO / ENTSPRECHEND / SECONDO
304L / S30403	A M B5511 F Spec. 7315-P (07.01.98)
SA 312-TP 304L / S30403	ASME II A Ed98+Ad99 according to SA 480
A 666-304L Anakl Ed96b / S30403	A S T M Spec. 7315-P (07.01.98)
A 312-TP 304L Ed95a / S30403	A S T M Spec. 7315-P (07.01.98)
A 240 304L Ed98a / S30403	A S T M Spec. 7315-P (07.01.98)
SA 240 304L / S30403	ASME II A Ed98+Ad99 according to SA 480

ETAT DE LIVRAISON / STATE OF DELIVERY / LIEFERZUSTAND / STATO DI FORNITURA

PROCEDE DE FABRICATION / MELTING PROCESS / PROCEDIMENTO DI ELABORAZIONE : E + VOD Finish no 1 - C2 - IIa

MARQUAGE / MARKING / KENNZEICHNUNG / IDENTIFICAZIONE

- VG = MARQUE DU PRODUCTEUR / BRAND OF THE MANUFACTURER
- RF = POINCON DE L'AGENT / INSPECTOR STAMP

Liste descriptive - Extract of material delivery - Umfang der Lieferung - Descrizione del materiale

Marque / Mark / Zeichen / Marca	Nombre / Number / Stückzahl / Quantità	Désignation du produit / Article / Gegenstand / Tipo di prodotto	N° de tôle / Plate N° / Blech Nr / Lamiera N°	N° coulée / Heat N° / Schmelze Nr / N° colata	N° éprouvette / Test N° / Probe Nr / Provetta N°	Kg
1- 731330	3	240.*x95.*:.375*	576651/1 >3	43091	576651.4001 576651.5001	3199

Résultats de l'inspection et du contrôle de dimensions : SANS REMARQUE
Results of surface examination and dimensional controls : NO REMARK
Ergebnis der Besichtigung und Maßkontrolle : OHNE BEANSTANDUNG
Risultati delle ispezione e del controllo di dimensioni : SENZA OSSERVAZIONI.

ESSAIS MECANIQUES - MECHANICAL TESTS - PRÜFUNGEN - PROVE MECCANICHE
ANALYSES CHIMIQUES - CHEMICAL ANALYSIS - CHEMISCHE ZUSAMMENSETZUNG - ANALISI CHIMICHE

Ep(mm)	COULEE HEAT	ESSAIS SAMPLES	T°(C) (F)	ETAT *	Traction transversale / Transverse tensile test				
					Rp0.2% ksi	Rp 1% ksi	Rm/UTS ksi	A 50 %	% Ra %
9.53	43091	576651.5001	20/ 68	0	41.7	52.1	87.2	74	61

Ep(mm)	COULEE HEAT	ESSAIS SAMPLES	T°(C) (F)	ETAT *	Dureté BRINELL 3000 Kg / BRINELL Hardness 3000 Kg			
					HB 10	HB 10	HB 10	HB MOY
9.53	43091	576651.5001	20/ 68	0	169	171	181	174

Ep(mm)	COULEE HEAT	ESSAIS SAMPLES	T°(C) (F)	ETAT *	Dureté ROCKWELL B / ROCKWELL B hardness			
					HRB	HRB	HRB	HRB
9.53	43091	576651.5001	20/ 68	0	86	85	84	85

Pliage transversale, branches parallèles (180°) > Satisfaisant Selon Norme
Transverse bend test (180°) > Satisfactory to Par Specification

Corrosion intergran A 262 p. E (sensibilisé) > Résultat Satisfaisant . Pas de Fissures
Intergrant corrosion A 262 p. E (sensitized) > Satisfactory . No Cracks

COULEE HEAT	C	Mn	P	S	Si	Cu	Ni	Cr	Mo	N	
43091	0.024	1.866	0.026	0.0003	0.554	0.363	8.520	18.340	0.337	0.07[illegible]	

PRODUIT	C	Mn	P	S	Si	Cu	Ni	Cr	Mo	N	
576651.4001	0.022	1.870	0.027	0.0006	0.552	0.260	8.712	18.201	0.331	0.0785	

*ETAT : 0 > Etat de livraison
Delivery condition

to the best of our knowledge, mercury has not been used in the fabrication or testing of this material
no weld repair. [illegible]

CERTIFIED MATERIAL TEST REPORT VERIFIED
JUN 20 2000
BY MCC

Nous certifions que les produits ci-dessus sont conformes aux prescriptions de la commande.
We hereby certify that the above mentioned products are consistent with the order prescriptions.
Wir bestätigen hiermit daß die obengenannten Erzeugnisse den Bestellungsvorschriften entsprechen.
Certifichiamo che i prodotti suddetti sono conformi alle prescrizioni dell' ordine.

Date - Datum - Data	Q. C. Manager	Mill Inspector
15-MAY-2000	DESCHEPPER, MICHELE	BAYET ALEXANDRE

FABRIQUE DE FER DE CHARLEROI

memo 5902
P.O. 10250

NUCOR STEEL - BERKELEY
P.O. Box 2259
Mt. Pleasant, S.C. 29464
Phone: (843) 336-6000

CERTIFIED MILL TEST REPORT

5/13/02 10:55:05
100% MELTED AND MANUFACTURED IN THE USA
All beams produced by Nucor-Berkeley are cast and rolled to a fully killed and fine grain practice.

Sold To: FEDERAL PIPE & STEEL CORP.
PO BOX 416

PLYMOUTH, MI 48170

Ship To: FEDERAL PIPE & STEEL CORP.
1641 NEW MARKET ST

SOUTH PLAINFIELD, NJ 07080

Customer #.: 451 - 2
Customer PO: 12563
B.o.L. #...: 234938
Invoice #..: 320831

SPECIFICATIONS: Tested in accordance with ASTM specification A6/A6M and A370.
AASHTO : M270-36-00/M270-50-00
ASME : SA-36
ASTM A992-01 A36-00A/A572-00-50/A709-00-50 A709-01-36/A709-01-[illegible]

Description	Heat # / Grade(s) / Test	Yield/ Tensile Ratio	Yield (MPa)	Tensile (MPa)	%	C							CE2
W10x26.0	2202771	.81	56600	69700	25.50	.0710	[illegible]	.0099	.0121	[illegible]	.1160	.0340	.2205
040' 00.00"	A992-01		390	481		.0250	.0000	.0049	.0062	.0002	.0275		.2585
W250X38.5		.82	57100	70300	26.30	.0001	.0008	.0001	.0001	.0061	.0000	3.1596	.1260
012.1920m			396	485		12 Piece(s)							
W10x26.0	1203048	.81	55900	68800	27.31	.0670	[illegible]	.0102	.0139	.1940	.0700	.0230	.2164
045' 00.00"	A992-01		385	474		.0190	[illegible]	.0038	.0014	.0002	.0311		.2550
W250X38.5		.82	56400	68400	27.65	.0013	.0019	.0004	.0009	.0057	.0000	2.2206	.1217
013.7160m			389	472		6 Piece(s)							
W10x26.0	2202769	.81	57500	71000	26.50	.0720	.8210	.0104	.0127	.1790	.1160	.0350	.2235
050' 00.00"	A992-01		336	490		.0230	.0000	.0050	.0001	.0001	.0272		.2588
W250X38.5		.82	58600	71200	28.40	.0001	.0007	.0008	.0001	.0075	.0000	3.1448	.1271
015.2400m			404	491		6 Piece(s)							
W10x26.0	1202596 ✓	.83	58700	71100	27.90	.0710	.8650	.0122	.0145	.2350	.1270	.0330	.2310
050' 00.00"	A992-01		405	490		.0240	.0010	.0051	.0001	.0009	.0305		.2763
W250X38.5		.83	58700	70900	27.20	.0036	.0014	.0007	.0002	.0061	.0000	3.4483	.1313
015.2400m			405	489		6 Piece(s)							
W12x22	2205695	.81	55900	68900	27.26	.0740	.8960	.0097	.0095	.1960	.1030	.0350	.2394
040' 00.00"	A992-01		385	475		.0340	.0000	.0053	.0016	.0004	.0293		.2779
W310X32.7		.82	57400	69800	28.25	.0016	.0012	.0008	.0002	.0064	.0000	2.9317	.1338
012.1920m			396	481		8 Piece(s)							

5 Heat(s) for this MTR.

Elongation based on 8" (20.32cm) gauge length.
CI = 26.01Cu+3.88Ni+1.20Cr+1.49Si+17.28P-(7.29Cu*Ni)-(9.10Ni*P)-33.39(Cu*Cu)
Pcm = C+(Si/30)+(Mn/20)+(Cu/20)+(Ni/60)+(Cr/20)+(Mo/15)+(V/10)+5B

CE1 = C+(Mn/6)+((Cr+Mo+V)/5)+((Ni+Cu)/15)
CE2 = C+((Mn+Si)/6)+((Cr+Mo+V+Cb)/5)+((Ni+Cu)/15)

I hereby certify that the contents of this report are accurate and correct. All test results and operations performed by the material manufacturer are in compliance with material specifications, and when designated by the Purchaser, meet applicable specifications.

Bruce A. Work
Metallurgist
Bruce A Work

(State of South Carolina
(County of Berkeley
Sworn and subscribed before me
13 day of May, 02
Clara L Davis

Commission Expires October 5, 2009

**END

PO 10251 - 5'4071

100 pcs.= 9800
HEAT# P5416
DATE SHIPPED 5/29/2002
SHIPMENT# 0206435
SIZE: AN 3 X 3 X 1/4
ORDER ITEM# 121774/ 01
SOLD TO: AMERICAN STEEL and ALUM -

CERTIFIED TEST REPORT

NUCOR
BAR MILL - AUBURN
NUCOR STEEL AUBURN, INC.
P.O. BOX 2008
AUBURN, NY 13021

CUST. P.O. M0513740
PART #:
GRADE : ABS GR A ASTM A36-00A
SPEC MILS22698C AMD#1&2 A CL-U
SUPP. REQ:
SHIP TO: AMERICAN STEEL - P
FAX # 2077720359

CHEMICAL ANALYSIS %

C	MN	SI	P	S	CU	NI	CR	MO	SN	V	CB	TI	B	N2	O2
.130	.630	.170	.013	.027	.230	.1100	.170	.033	.015	.002	.0030	.0018	.0011	XXX	.001

MECHANICAL RESULTS

YIELD	TENSILE	GAUGE	%	BEND	%
K.S.I.	K.S.I.	LENGTH	ELONG	PIN. DIA	R.A.
47.70	67.10	8	25.0	.0	.0
46.60	66.20	8	35.0	.0	.0
MPa	MPa	GAUGE	%	BEND	
		LENGTH	ELONG	PIN. DIA	R.A.

CHARPY IMPACT TEST

TEMP. F	FT./LB.	%SHEAR	LAT.EXP.MILS

I CERTIFY THESE RESULTS TO BE CORRECT AS CONTAINED IN THE RECORDS OF THE COMPANY.

JIM BIERNAT, METALLURGIST
STATE OF NEW YORK SS.
COUNTY OF CAYUGA

Grain Size	Reduction Ratio	As Rolled Hardness	D.I.	C.E.	C.I.
XXX	XXX	XXX	XXX	XXX	XXX

Jim Biernat
(print)

AFTER BEING DULY SWORN BY ME, DECLARES THAT THESE RESULTS ARE CORRECT AS CONTAINED IN THE RECORDS OF NUCOR STEEL AUBURN, INC

(sign)

SUBSCRIBED AND SHOWN BEFORE ME

JOMINY END-QUENCH HARDENABILITY RESULTS (HRC)

J1	J2	J3	J4	J5	J6	J7	J8	J9	J10	J11	J12

J13	J14	J15	J16	J18	J20	J22	J24	J26	J28	J30	J32

ALL MANUFACTURING PROCESSES FOR THIS STEEL INCLUDING MELTING FROM SCRAP AND HOT ROLLING HAVE BEEN PERFORMED IN THE U.S.A. NO WELD REPAIR PERFORMED, STEEL NOT EXPOSED TO MERCURY OR ANY LIQUID ALLOY WHICH IS LIQUID AT AMBIENT TEMPERATURES.

THIS _____ DAY OF _____

L.S. _____

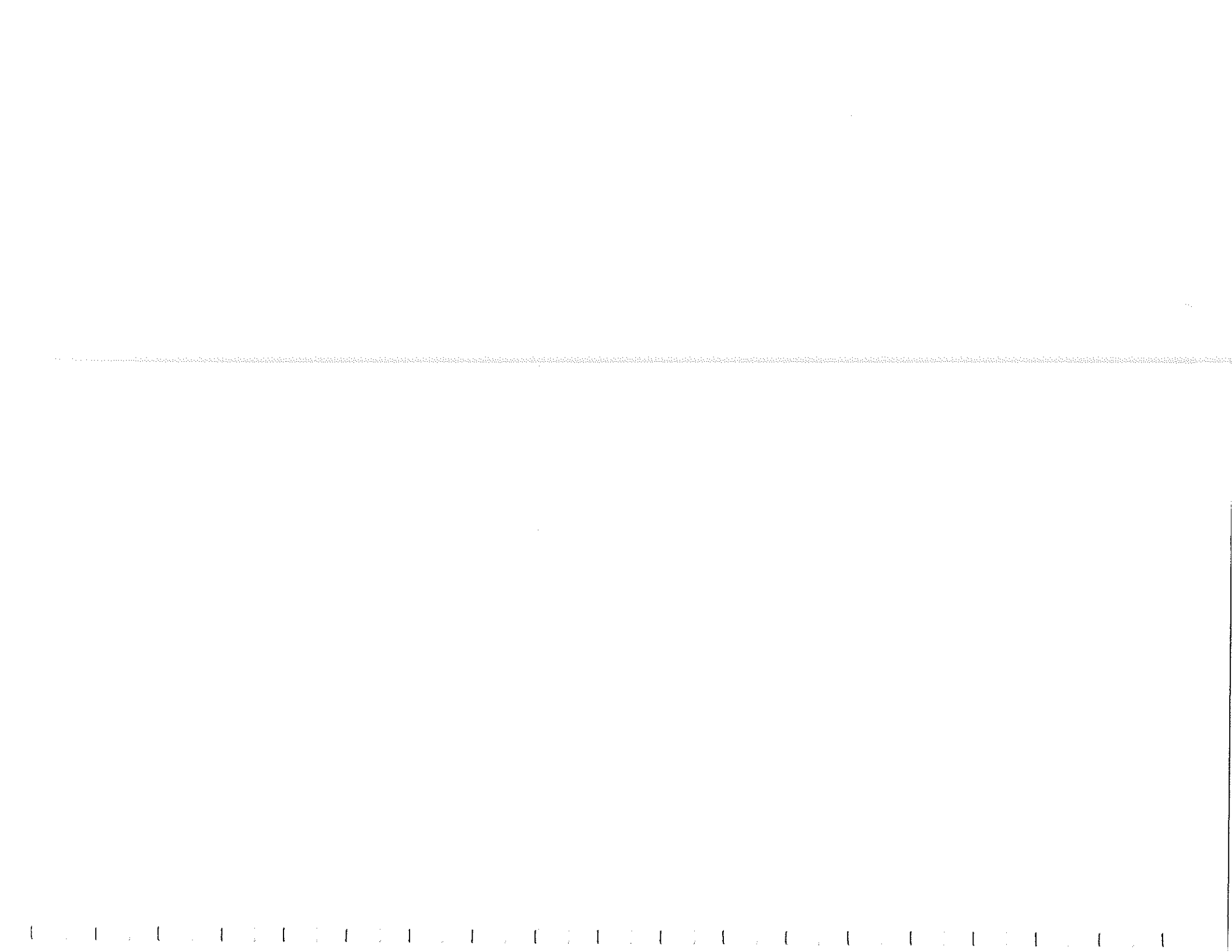
THIS CERTIFICATE IS NOTARIZED ONLY WHEN REQUESTED

EJ/06095
105000

CUSTOMER SPECIAL INSTRUCTIONS:

SHIP BY
AMERICAN STEEL & ALUMINUM CORP.
YOUR P.O. NO. 10251
OUR P.O. NO. 05-03318
CUSTOMER Ms Brady

End of Report



BECKER
structural engineers, inc.

Special Inspections Report

H.P. Hood, Inc.
Bottle Conveyor Bridge & Facility

349 Park Avenue
Portland, ME

March 12, 2003

Prepared for:

H.P. Hood, Inc.
349 Park Avenue
Portland, ME 04102

In conjunction with:

The City of Portland
389 Congress Street
City Hall Room 315
Portland, ME 04101

19 Commercial Street, Portland, ME 04101-4701 ■ Tel. 207-879-1838 ■ Fax 207-879-1822

H.P. Hood, Inc.
Bottle Conveyor Bridge & Facility
349 Park Avenue
Portland, ME

March 12, 2003

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1000 Statement of Special Inspections

Becker Structural Engineers, Inc.

STATEMENT OF SPECIAL INSPECTIONS

PROJECT: HP Hood Bottle Conveyor Bridge & Facility

LOCATION: 349 Park Ave, Portland, Maine 04102

PERMIT APPLICANT: Centerline Construction

APPLICANT'S ADDRESS: P O BOX 1264, Portland, ME 04104

STRUCTURAL ENGINEER OF RECORD: Becker Structural Engineers, Inc.

ARCHITECT OF RECORD: CWS Architects

This Statement of Special Inspections is submitted in accordance with Section 1705.0 of the 1999 BOCA National Building Code. It includes a listing of special inspections applicable to this project as well as the name of the Special Inspector, and the names of other agencies intended to be retained for conducting these inspections.

The Special Inspector shall keep records of all inspections listed herein, and shall furnish inspection reports to the Code Official and to the Registered Design Professional of Record. All discrepancies shall be brought to the immediate attention of the Contractor for correction. If the discrepancies are not corrected the discrepancies shall be brought to the attention of the Code Official and the Registered Design Professional of Record. Interim reports shall be submitted to the Code Official and to the Registered Design Professional of Record monthly, unless more frequent submissions are requested by the Code Official.

Job site safety is solely the responsibility of the Contractor. Materials and activities to be inspected are not to include the Contractor's equipment and methods used to erect or install the materials listed.

Prepared By:

Paul B. Becker, P.E.
NAME

8/20/02
SIGNATURE DATE

STATE OF MAINE
PAUL B. BECKER 6554
LICENSED PROFESSIONAL ENGINEER

Preparer's P.E. Seal

Applicant's Authorization:

8/21/02
SIGNATURE DATE

Building Code Official:

SIGNATURE DATE

Becker Structural Engineers, Inc.

LIST OF AGENTS

PROJECT: HP Hood Bottle Conveyor Bridge & Facility

STRUCTURAL ENGINEER OF RECORD: Becker Structural Engineers, Inc.
Firm
19 Commercial Street, Portland, ME 04101
Address

ARCHITECT OF RECORD: CWS Architects
Firm
434 Cumberland Ave, Portland, ME 04102
Address

Following is the List of Agents selected for performance of Special Inspections for this project:

		Name	Firm	Abbreviation
1.	Special Inspector	Paul B. Becker, P.E.	Becker Structural Engineers, Inc.	BSE
2.	Special Inspector	Dan S. Burne, E.I.	Becker Structural Engineers, Inc.	BSE
3.	Testing Laboratory	Roger Domingo	S.W. Cole, Inc.	SWC
4.	Geotechnical Engineer	Paul F. Kohler, P.E.	S.W. Cole, Inc.	SWC

Becker Structural Engineers, Inc.

FINAL REPORT OF SPECIAL INSPECTIONS

PROJECT: HP Hood Bottle Conveyor Bridge & Facility

LOCATION: 349 Park Ave., Portland, ME 04104

PERMIT APPLICANT: Centerline Construction

APPLICANT'S ADDRESS: PO Box 1264, Portland, ME 04104

STRUCTURAL ENGINEER OF RECORD: Paul B. Becker, P.E. Becker Structural Engineers, Inc.

	Name	Firm
ARCHITECT OF RECORD:	Guy Labrecque, A.I.A.	CWS Architects
	Name	Firm
GENERAL CONTRACTOR:	Richard Miller	Centerline Construction

To the best of my information, knowledge, and belief, the Special Inspections required for this project. and described in the Statement of Special Inspections submitted for the project, have been completed.

The following discrepancies that were outstanding since the last interim report, No.__ dated _____, have been corrected:

N/A

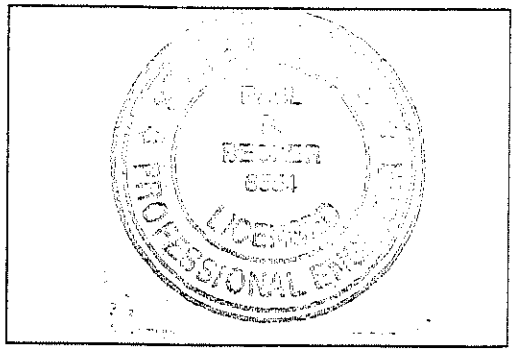
(Use additional sheets, if necessary)

Interim reports submitted to this final report and numbered__ to __, form a basis for, and are to be considered an integral part of this final report.

Submitted By:
SPECIAL INSPECTOR

Paul B. Becker, P.E.
NAME

SIGNATURE DATE 3.12.03



Special Inspector's P.E. Seal

Summary of Services (Exhibit A)

SCHEDULE OF SPECIAL INSPECTION SERVICES

PROJECT: Bottle Conveyor Bridge & Facility, HP Hood Inc., Portland, ME.

MATERIAL/ACTIVITY	ITEM	SERVICE	APPLICABLE TO THIS PROJECT Y/N	EXTENT (All,Sample, Other, None)	COMMENTS	AGENT	DATE	REV.
1705.2 Inspection of Fabricators	1.00							
		Fabrication Procedures	Y	As Required by following sections	See Wood, Steel	BSE	3/3/05	
		Procedure Implementation	Y	As Required by following sections	& concrete Sections	BSE	3/3/05	

All Steel Construction Special Inspections have been completed in accordance with BOCA Section 1705.12 Special Inspector [signature] Date 3/12/05

SCHEDULE OF SPECIAL INSPECTION SERVICES

PROJECT: Bottle Conveyor Bridge & Facility, HP Hood Inc., Portland, ME

MATERIAL/ACTIVITY	ITEM	SERVICE	APPLICABLE TO THIS PROJECT Y/N	EXTENT (All,Sample, Other, None)	COMMENTS	AGENT	DATE	REV.
1705.3 STEEL CONSTRUCTION	2.00							
Steel Fabrication		In-plant review Part A - Fabrication procedures/QA						
		1. AISC Category 1	Y	Provide AISC Certification	AISC / SCSE/IC	BSE	3/3/03	
		2. AWS Quality Assurance	Y	Provide Welder Certification		BSE	3/5/03	
		Part B - Procedures implementation Review conformance to Part A						
		Review material certificates						
		1. Bolts, Nuts, Washers	Y	Sample	AISC ASD A3.4	BSE	3/3/03	
		2. Structural Steel	Y	Sample	AISC A6 or A568	BSE	3/3/03	
		3. Weld Filler Material	Y	Sample	AISC ASD A3.6	BSE	3/3/03	
		Review connections						
		1. Shop Bolted	N				—	
		2. Shop Welded	Y	ALL @ CONVEYOR BRIDGE	IN SHOP (VISUAL)	SWC		
		3. Connection Design Calcs	N		Design By BSE		—	
		4. Shop Welder Certs	Y	ALL		BSE	3/3/03	
		Review welding of seismic-resisting system in Cat. "C" buildings	Y	FABRICATOR'S IN-HOUSE QA AS ALTERNATE ALL @ CONVEYOR BRIDGE FRAME	AWS D1.1 SECTION 6	FAB. ~~SWC~~	3/3/03	
Steel Erection		Review materials certs of compliance						
		1. Bolts, Nuts, Washers	Y	ALL		BSE	3/3/03	
		2. Structural Steel	Y	ALL		BSE	3/3/03	
		3. Weld Filler Material	Y	ALL		BSE	3/3/03	
		Review primary steel connections						
		Moment connections	Y	ALL	ULTRASONIC	SWC	1/17/03	
		Shear connections						
		1. Field Bolted	Y	ALL		SWC	1/17/03	
		2. Field Welded	Y	ALL		SWC	1/17/03	
		Bracing connections						
		Review welded Cat. "C" seismic connections	Y	ALL		SWC	1/8/03	
		Review welded column splices	Y	ALL		SWC	N/A	
		Review base metal testing for "t" > 1 1/2"	Y	ALL		SWC	N/A	
		Review secondary steel connections	N				—	
		1. Girts	Y	ALL		BSE	1/3/03	
		2. Lintels	N				—	
		3. Steel Deck	Y	ALL		BSE	1/3/03	
		Lintels/Relieving Angles	N				—	
		Review installation of shear studs	N				—	
		Review Details/Steel Frame	Y	Sample		BSE	1/3/03	

All Steel Construction Special Inspections have been completed in accordance with BOCA Section 1705.3 Special Inspector _____ Date 3/12/03

SCHEDULE OF SPECIAL INSPECTION SERVICES

PROJECT:Bottle Conveyor Bridge & Facility, HP Hood Inc., Portland, ME.

MATERIAL/ACTIVITY	ITEM	SERVICE	APPLICABLE TO THIS PROJECT Y/N	EXTENT (All,Sample, Other, None)	COMMENTS	AGENT	DATE	REV.
1705.3 STEEL CONSTRUCTION (Continued)	2.00							
Steel Joist & Joist Girder		In-plant review			CANAM STEEL LISTED AS MEMBER ON SJI WEBSITE		3/11/03	
		Part A - Fabrication procedures	Y	SUBMIT SJI CERTIFICATION OF PLANT		BSE		
		Part B - Procedures implementation					—	
		Review conformance to Part A	N					
		Review material certificates of compliance						
		1. Structural Steel	N				—	
		2. Weld Material	N				—	
		Review connections	N				—	
		Review welder certification	N				—	
Steel Joist/Joist Girder Erection		Review joist bearing connections	Y	ALL		BSE	1/5/03	
		Review joist bearing length	Y	ALL		BSE	1/5/03	
		Review joist bridging	Y	ALL		BSE	1/5/03	
Steel Roof Deck		Review Gage Thicknesses	Y	ALL		BSE	1/5/03	
		Review Welding	Y	ALL		SWC	1/17/03	

All Steel Construction Special Inspections have been completed in accordance with BOCA Section 1705.3 Special Inspector _____ Date 3/12/03

SCHEDULE OF SPECIAL INSPECTION SERVICES

PROJECT: Bottle Conveyor & Facility, HP Hood Inc., Portland, ME.

MATERIAL/ACTIVITY	ITEM	SERVICE	APPLICABLE TO THIS PROJECT Y/N	EXTENT (All,Sample, Other, None)	COMMENTS	AGENT	DATE	REV.
1705.4 CONCRETE CONSTR.	3.00							
Concrete Materials		Review materials (ACI Chapter 3)						
		1. Cement	Y	ALL	ASTM C150	BSE	11/8/02	
		2. Normal WT aggregates	Y	ALL	ASTM C33	BSE	11/8/02	
		3. Air Entraining admix	Y	ALL	ASTM C260	BSE	11/8/02	
		4. Normal range water reducing admix	Y	ALL	ASTM C494	BSE	11/8/02	
		5. Hi-Range water reducing admix	Y	ALL	ASTM C494	BSE	11/8/02	
		6. Accel Admix	Y	ALL	ASTM C494 Type A	BSE	11/8/02	
		7. Moisture Barrier	Y	ALL	8 mil	BSE	N/A	
		8. Curing Products	Y	ALL		BSE	N/A	
		9. Preformed expansion Joint	Y	ALL		BSE	N/A	
		Review mix design	Y	ALL	ACI Chapter 4	BSE	11/8/02	
		Review reinforcing certification & weld-ability if required	Y	ALL	No Welding Certificate Only	BSE	3/12/03	
Placing Reinforcement		Review condition & placement of rein-forcing and prestressing steel	Y	Sample	ACI 318 7.4-7.7	BSE		
		Review welding of reinforcing in Cat "C" seismic-resisting systems	N	Not Applicable			—	
		Review Embedded items, bolts, plates, etc.	Y	Sample		BSE		
Formwork								
Formwork		Review form removal & reshoring	N	Contractor Means & Methods	ACI 318.6.2		—	
Concrete Operations		Field Sampling & Testing of Concrete	Y	Every 50 yards or each separate placement Sample Air, Temp, Slump ASTM C172, C231				
		Review concrete strength tests	Y		ACI 318 5.6	BSE	10/02	
		Review mix proportions and technique	Y		ACI 318 5.2, 5.3, 5.4, & 5.8	SWC	10/02	
		Review concrete placement	Y		ACI 318 5.9 & 5.10	SWC	10/02	
		Review curing technique & temperature	Y		ACI 318 5.11, 5.12, & 5.13	SWC	10/02	
Prestressing Operations		Review application of prestressing force	N	Not applicable			—	
		Review grouting of bonded prestressing tendons in Cat.Cseismic-resisting systems	N	Not applicable			—	
Precast Manufacturing		In-plant review Part A - Fabrication procedures	N				—	
		Part B - Procedures implementation	N				—	
		Review conformance to Part A	N				—	
Erection of Precast Concrete		Review erection of precast units	N				—	
		Review key reinforcement	N				—	
		Review key grouting	N				—	
		Review concrete topping	N				—	
		Review connections	N				—	

All Concrete Construction Special Inspections have been completed in accordance with BOCA Section 1705.4 Special Inspector _____ Date 3/12/03

SCHEDULE OF SPECIAL INSPECTION SERVICES

PROJECT: Bottle Conveyor Bridge & Facility, HP Hood Inc., Portland, ME.

MATERIAL/ACTIVITY	ITEM	SERVICE	APPLICABLE TO THIS PROJECT Y/N	EXTENT (All,Sample, Other, None)	COMMENTS	AGENT	DATE	REV.
1705.5 MASONRY CONSTR	4.00							
Materials		Review materials certification					–	
		Masonry units	N				–	
		Reinforcing steel	N				–	
		Review grout materials & mix design	N				–	
		Review mortar materials & mix design	N				–	
		Review strength determination					–	
		Unit strength method	N				–	
		Review unit strengths & grout, mortar mixes	N				–	
		Prism strength method	N				–	
		Review pre-construction test results	N				–	
		Field tests during construction	N				–	
		Grout testing					–	
		Determine compressive strength	N				–	
		Mortar testing					–	
		Field test compressive strength ASTM C780 (Req'd only if property reqs of ASTM C270 are used)	N				–	
		Review mortar mix proportions & mixing	N		ACI 530.1;2.3.2.5		–	
General Masonry Work		Review general installation of mortar	Y	Sample	ACI 530.1;4.2.2	BSE	10/2/02	
		Review general installation of mortar grout, masonry units.	Y	Sample	ACI 530.1;2.3.3.3,4.3.3	BSE	10/2/02	
		Review installation of horiz., vert., & joint reinforcing (incl. Location, sizes, splices, & positioning devices)	Y	Sample	ACI 530, CH. 8	BSE	10/2/02	
		Review hot/cold weather procedures	Y	Sample	ACI 530; 1;2.3.2.2,2.3.2.3	BSE	NA	
		Review installation of anchorage devices	Y	Sample	ACI 530; 4.2, 5.14	BSE	10/2/02	
		Review installation of lintels	Y	Sample		BSE	NA	
		Review welding of reinf., grouting, consolidation and reconsolidation for seismic Cat. "C" buildings	N				–	

All Masonry Construction Special Inspections have been completed in accordance with BOCA Section 1705.5 Special Inspector [signature] Date 5/12/05

SCHEDULE OF SPECIAL INSPECTION SERVICES

PROJECT: Bottle Conveyor Bridge & Facility, HP Hood Inc., Portland, ME.

MATERIAL/ACTIVITY	ITEM	SERVICE	APPLICABLE TO THIS PROJECT Y/N	EXTENT (All,Sample, Other, None)	COMMENTS	AGENT	DATE	REV.
1705.7 PREPARED FILL	6.00							
Site Preparation		Review site preparation prior to prepared fill placement.	Y		Building only	SWC	8/31/02	
During Fill Placement		Review compliance to soils report				SWC		
		Material	Y	Sample	Building only	SWC	10/10/02	
		Lift thickness	Y	Sample	Building only	SWC	10/10/02	
Evaluation of in-Place Density		Review in-place dry density for compli-ance with soils report	Y	Sample	Building only	SWC	N/A	

All Prepared Fill Special Inspections have been completed in accordance with BOCA Section 1705.7 Special Inspector _____ Date 3/12/03

20-Aug-02

SCHEDULE OF SPECIAL INSPECTION SERVICES

PROJECT: Bottle Conveyor Bridge & Facility, HP Hood Inc., Portland ME.

MATERIAL/ACTIVITY	ITEM	SERVICE	APPLICABLE TO THIS PROJECT Y/N	EXTENT (All,Sample, Other, None)	COMMENTS	AGENT	DATE	REV.
1705.12 SPECIAL CASES	8.00							
Fireproofing		Fireproofing	N				–	
		Review Product Data Sheets	N				–	
		Review In-Place Density	N				–	
		Review In-Place Thickness	N				–	
		Review In-Place Bond	N				–	
		Review scope of installation for	N				–	
		conformance with design documents	N				–	

All Steel Construction Special Inspections have been completed in accordance with BOCA Section 1705.12 Special Inspector _____ Date 5/16/05

20-Aug-02

1001 Disclaimers and Qualifications

The program of Structural/Special Tests and Inspections does not relieve the Contractor or its subcontractors of their responsibilities and obligations for quality control of the work, for any design work which is included in the scope of services, and for full compliance with the requirements of the Construction Documents. Furthermore, the detection of, or the failure to detect, deficiencies or defects in work during testing and inspection conducted pursuant to the Program does not relieve the Contractor or its subcontractors of their responsibility to correct all deficiencies or defects, whether detected or undetected, in all parts of work, and to otherwise comply with all requirements of the Construction Documents. Additional disclaimers and/or qualifications may be included in the Owner-Special Inspection agreement.

02200 Earthwork
02200.1 Testing Reports

S.W.COLE ENGINEERING, INC.

• Geotechnical Engineering • Field & Lab Testing • Scientific & Environmental Consulting

DAILY CONSTRUCTION REPORT

Project: H.P. HOOD Project No.: 02-0153.1

Client: CENTERLINE CONST Date: 8/31/02

Client's Rep.: DICK MILLER / SEAN

Weather: OVERCAST Temp. Range: 50's

Arrived at Site at: 9:30

Work in Progress: AREAS 1 & 2 EXCAVATED. FINISHING EXCAVATION IN AREA 3.

Work Performed by SWC Rep.: OBSERVATIONS OF FOUNDATION SUBGRADE
AREA 1 : 2 to 4/F to G LINE ; AREA 2 : 4/D to E LINE
AREA 3 : 4/C to D LINE

General Observations, Discussions, Etc.: AREA 1 : USED SMOOTH-EDGED BUCKET TO EXCAVATE TO BOTTOM OF EXISTING FOOTINGS. SOIL WAS GRAY FINE SANDY SILT W/SOME CLAY, SHELLS, SOME PIECES OF BRICK. PENETROMETER = 2.0 KSF. SOME AREAS OF PONDED WATER 1 TO 3 INCHES. SUMP AND PUMP INSTALLED - NOT OPERATING.

AREAS 2 AND 3 : USED SMOOTH-EDGED BUCKET TO EXCAVATE TO BOTTOM OF EXISTING FOOTINGS. ENCOUNTERED SIMILAR SOIL CONDITIONS, SOME AREAS MORE GRAVELLY SAND. NO PONDED WATER OBSERVED.

Recommendations to Contractor/Owner's Rep.: AREA 1 OK FOR PLACEMENT OF GEOTEXTILE FABRIC AND CRUSHED STONE AFTER USING SUMP AND PUMP TO DEWATER. AREAS 2 & 3 OK FOR PLACEMENT OF GEOTEXTILE FABRIC AND CRUSHED STONE AFTER REMOVAL OF 1 TO 4 INCHES OF DISTURBED/LOOSE SOIL IN A FEW AREAS (DISCUSSED ON SITE).

Left Site at: 10:30 ± SWC Rep.: Paul F Hobbins

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286 Portland Road, Gray, ME 04039, Tel (207) 657-2866, Fax (207) 657-2840, (E) infogray@swcole.com, (I) www.swcole.com

Other offices in Augusta, Bangor and Caribou, Maine & in Somersworth, New Hampshire

1.2

S. W. COLE ENGINEERING, INC.

REPORT OF GRADATION
ASTM C-117, C-136

Project No. 20153.1
Date 10/10/2002

Project H.P.HOOD

Client CENTERLINE CONSTRUCTION

Sample No. 1, GRAVEL ONSITE, GROVEVILLE PIT

Sieve Size	Percent Passing		MDOT TYPE B Specifications %
4 "	100.0		100
3 "	100.0		
2 "	100.0		
1 1/2 "	98.0		
1 "	96.9		
3/4 "	95.7		
1/2 "	94.0	*	35-75
1/4 "	91.8	*	25-60
# 4	90.5		
# 10	85.6		
# 20	70.6		
# 40	46.1	*	0-25
# 60	23.9		
# 100	9.9		
# 200	2.8		0-5.0

* Does not meet project specifications

MOISTURE - DENSITY TEST

ASTM D-1557 Method A
Maximum Dry Density : 118.4 pcf
Optimum Moisture Content : 10.1 %

02300 Pile Foundations
02300.1 Monitoring Reports

S.W.COLE ENGINEERING, INC. • Geotechnical Engineering • Field & Lab Testing • Scientific & Environmental Consulting

DAILY CONSTRUCTION REPORT

Project: H.P. Hood Proposed Building Addition Project No: 02-153.1

Client: Centerline Construction Date: 9-6-2002

Client's Rep.: Dick Miller

Weather: Warm and humid Temp. Range: 75-85

Arrived at Site at: 7:00 AM

Work in Progress: Pile Driving by H.B. Flemming

Work Performed by SWC Rep.: Pile driving monitoring

General Observations, Discussions, Etc.: The pile driving operations began on the eight-pile group that will support the proposed silo (Pile No's 1 thru 8). The pile is being driven by H.B. Flemming with a 2,300 lb drop hammer that cannot drive pile longer than thirty feet. The depth to bedrock in this area appears to be about 35 feet. Three piles (No's 3, 6, and 7) were driven to set criteria, and one pile (No 6) was started and will need to be spliced prior to further driving. Note: The set criteria used is 4 blows per inch for 6 consecutive inches, or 8 blows for any half inch, or a cessation of movement. This criteria was provided by the pile driving contractor.

Based on conversation with Centerline Construction, it appears that the original building is not square, and three of the pile (No's 3, 7, and 8) would have struck existing footers. Two of the pile locations (No's 7 and 8) were relocated by the representative from Centerline Construction based on verbal instructions he had received from the project structural engineers (Becker). All three of the pile struck an obstruction after four feet of penetration which caused them to move from their intended locations, and driving was continued to set criteria. The attached pile driving summary sheet will show the approximate locations of these pile.

Left Site at: 3:30 PM SWC Rep.: ARS

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DAILY CONSTRUCTION REPORT

Project: H.P. Hood Proposed Building Addition Project No.: 02-153.1

Client: Centerline Construction Date: 9-9-2002

Client's Rep.: Dick Miller

Weather: Warm and humid Temp. Range: 75-90

Arrived at Site at: 7:00 AM

Work in Progress: Pile Driving by H.B. Flemming

Work Performed by SWC Rep.: Pile driving monitoring

General Observations, Discussions, Etc.: The pile driving operations were completed on the pile group supporting the silo (Pile No's 1 thru 8). H.B. Flemming provided measurements for cut-off elevation. Cut-off elevation for pile No's 1 thru 8 is reportedly 97.2 feet.

Two pile (Pile No's 11 and 12) were started on PC-4B, which will support the west end of the proposed bridge structure. These two piles were driven to within three feet of grade and halted to allow for the welding of splices. There appears to be a discrepancy between the written plans and the verbal instructions that Centerline Construction has received from the project structural engineers (Becker) regarding the cut-off elevations at PC-4B. The plans show the cut-off elevation being 93.5 feet, but based on verbal instructions between the project structural engineer and Centerline Construction the bottom of this pile cap should start at the same elevation as the bottom of the existing footers. If the pile cap started at this elevation, the pile cut-off elevation would be closer to 92.4 feet. We understand that Centerline Construction is to resolve this issue by 9/10/02.

Left Site at: 3:30 PM SWC Rep.: ARS

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DAILY CONSTRUCTION REPORT

Project: H.P. Hood Proposed Building Addition Project No.: 02-153.1

Client: Centerline Construction Date: 9-10-2002

Client's Rep.: Dick Miller

Weather: Warm and humid Temp. Range: 75-90

Arrived at Site at: 7:00 AM

Work in Progress: Pile Driving by H.B. Flemming

Work Performed by SWC Rep.: Pile driving monitoring

General Observations, Discussions, Etc.: The pile driving operations were completed on PC-4B. Centerline Construction provided measurements for cut-off elevation. The final cut-off elevation was determined to be 92.5', which is about eight inches above the level of the existing footers.

Pile driving operations on PC-4A, the pile group that is adjacent to the H.P. Hood Dairy building and will provide the easterly support for the bridge structure, are scheduled for September 25th. This pile group includes 4 piles (Pile No's 13 thru 16). This pile group is in a high truck traffic area and has to be coordinated with the H.P. Hood operations.

Left Site at: 12:30 PM SWC Rep.: ARS

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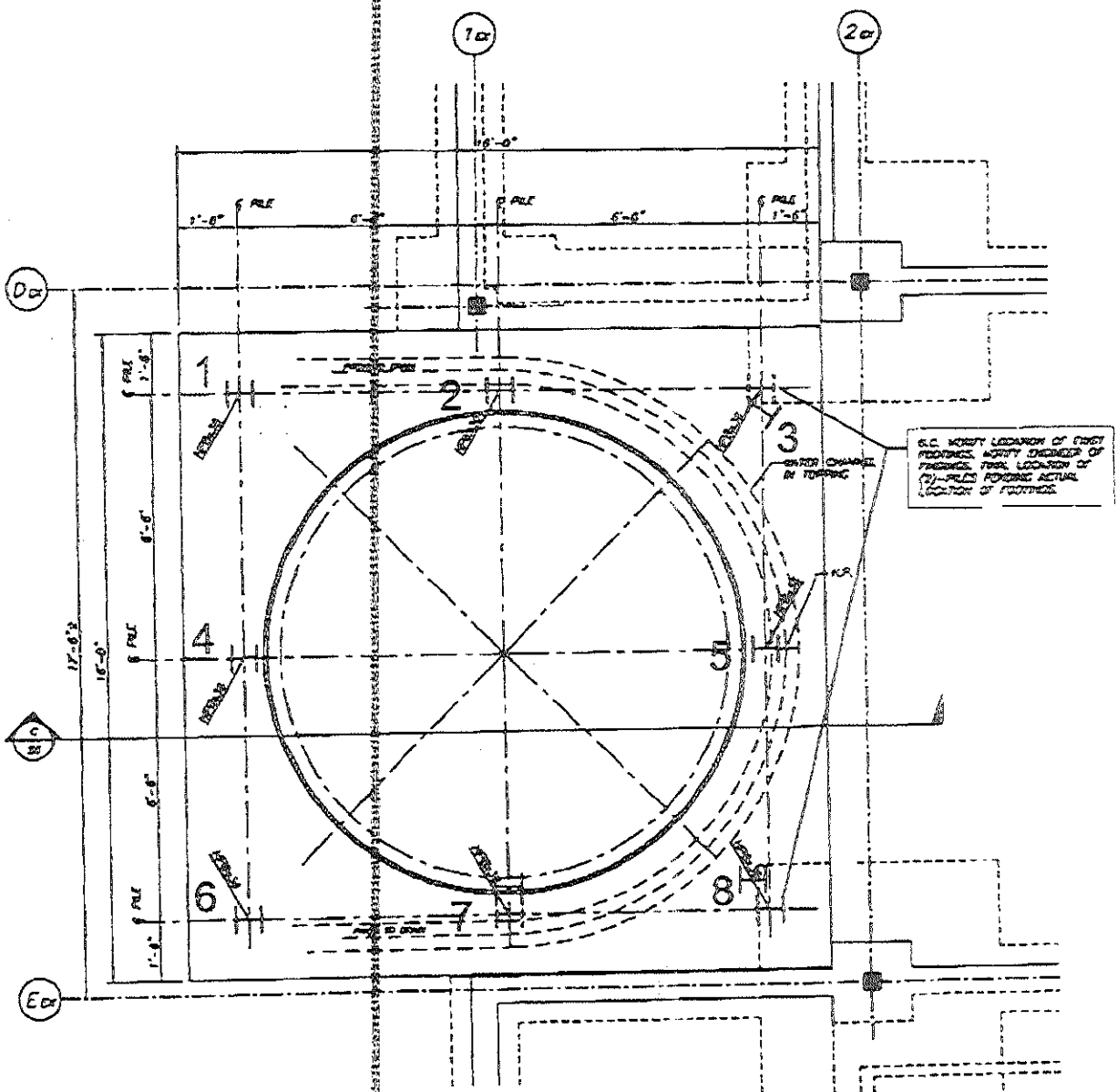

PC-8 @ CARVEL BLDG

LEGEND

6 Pile Number

NOTE :

Base plan provided by
Becker Structural Engineers, Inc.

S.W.COLE ENGINEERING, INC.

CENTERLINE CONSTRUCTION

PILE SUMMARY

Proposed H.P. Hood Building Expansion
Park Avenue
Portland, Maine

Job No. 02-0153.1 S
Date : 09/12/02

Scale 1/4" = 1'
Sheet 1

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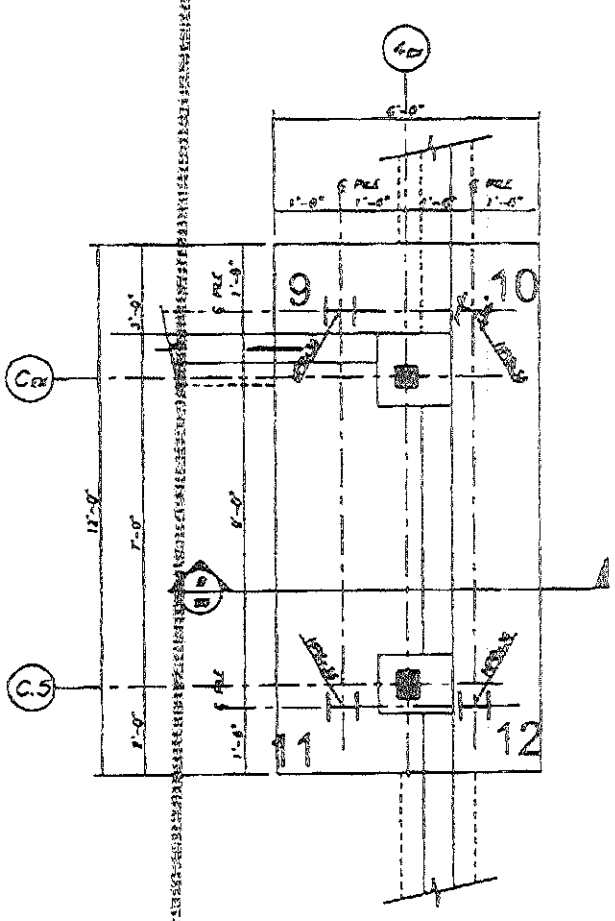

PC-4B @ CARVEL BLDG

LEGEND

6 Pile Number

NOTE :

Base plan provided by
Becker Structural Engineers, Inc.

S.W.COLE ENGINEERING, INC.

CENTERLINE CONSTRUCTION

PILE SUMMARY

Proposed H.P. Hood Building Expansion
Park Avenue
Portland, Maine

Job No. 02-0153.1 S
Date : 09/12/02

Scale 1/4" = 1'
Sheet 2

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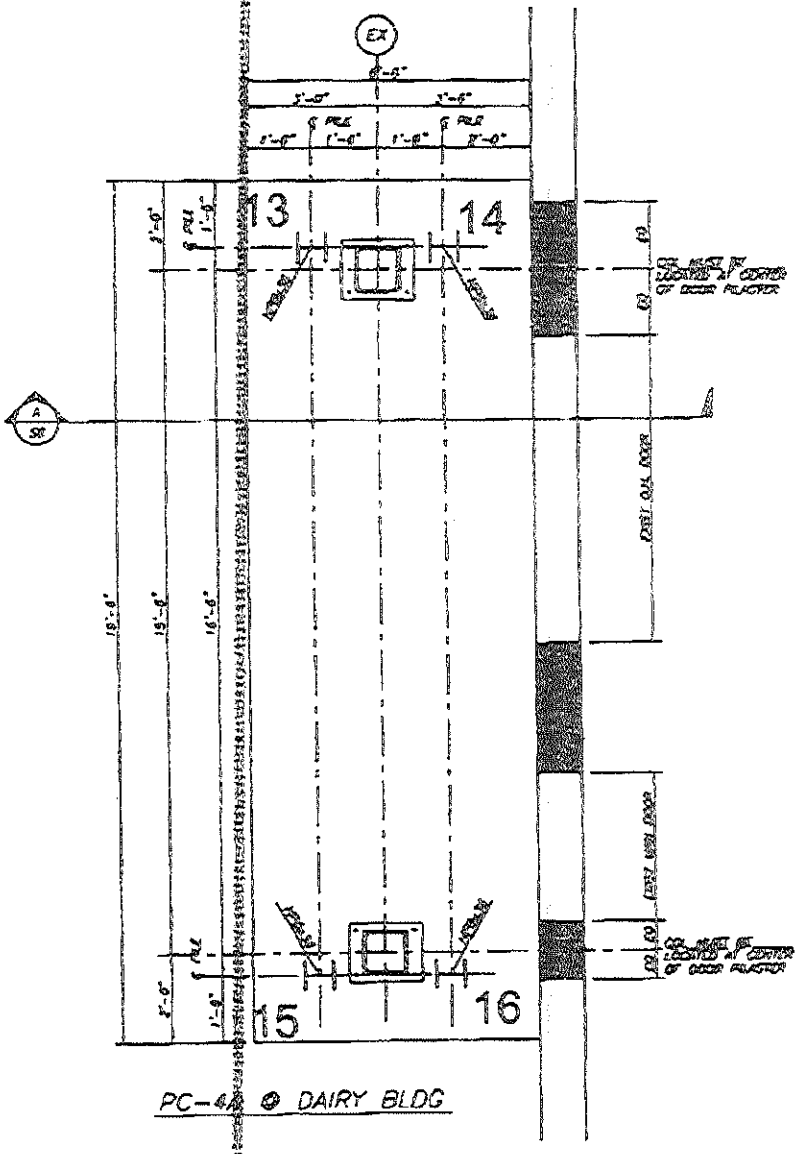

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LEGEND

6 Pile Number

NOTE :

Base plan provided by
Becker Structural Engineers, Inc.

S.W.COLE ENGINEERING, INC.

CENTERLINE CONSTRUCTION

PILE SUMMARY

Proposed H.P. Hood Building Expansion
Park Avenue
Portland, Maine

Job No. 02-0153.1 S
Date : 09/12/02

Scale 1/4" = 1'
Sheet 3

S.W.COLE ENGINEERING, INC.

PILE DRIVING SUMMARY

S.W.C.E #: 02-0153.1

Client: Centerline Construction
Project: H.P. Hood building expansion, Park Avenue Portland, Maine
General Contractor: Centerline Construction (H.B. Fleming, Pile Contractor)
Pile Hammer: 2,300 lb drop hammer
Rated Energy: 13,800 lb-ft @ 6' stroke
Pile Type: HP 8x36 ASTM A572 Grade 50
Design Capacity: 40 ton in compression
Reference Elev: FFE = 100 feet (Architectural Datum)

Pile #	Total Driven Length (feet)	Cut-Off Elevation (feet)	Total Cut-off Length (feet)	As-Built Section Lengths I	As-Built Section Lengths II	Splice Reference Elevation (feet)	Tip Reference Elevation (feet)	Pay Length (feet)	Final Set: Last 2 Inches (blows/inch)		Date started	Date finished	Remarks
1	40.2	97.2	5.8	30.0	4.4	92.8	62.8	34.4	5	6/.25"	9/9/02	9/9/02	
2	40.4	97.2	4.8	30.1	5.5	91.7	61.6	35.6	5	4/.75"	9/9/02	9/9/02	
3	45.3	97.2	10.4	29.8	5.1	92.1	62.3	34.9	6	8/.5"	9/6/02	9/6/02	Heave = 0.18", Pile rotated ±30°, Pile off design location by ±6"
4	40.3	97.2	5.9	29.9	4.5	92.7	62.8	34.4	4	6/.25"	9/6/02	9/9/02	
5	40.6	97.2	5.6	30.1	4.9	92.3	62.2	35.0	3	5/.25"	9/9/02	9/9/02	
6	[illegible]	97.2	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	4	5/.125"	9/6/02	9/6/02	Heave = 0.05"
7	40.2	97.2	4.6	28.8	6.8	90.4	61.6	35.6	4	5/.125"	9/6/02	9/6/02	Pile off design location by ±9"
8	39.5	97.2	3.8	30.1	5.6	91.6	61.5	35.7	2	6/.125"	9/9/02	9/9/02	Pile off design location by ±9"
9	30.5	92.5	1.2	29.3	—	—	63.2	29.3	3	5/.25"	9/10/02	9/10/02	
10	30.5	92.5	1.1	29.4	—	—	63.1	29.4	6	5/.25"	9/10/02	9/10/02	Pile rotated ±20°
11	30.2	92.5	0.3	29.9	—	—	62.8	29.9	4	4/.25"	9/9/02	9/10/02	
12	30.3	92.5	0.8	29.5	—	—	63.0	29.5	5	8	9/9/02	9/10/02	
13													To be driven 9/25/02
14													To be driven 9/25/02
15													To be driven 9/25/02
16													To be driven 9/25/02

Total Pay Length: 399.6 feet (as of 9-10-2002)

S.W.COLE ENGINEERING, INC. • Geotechnical Engineering • Field & Lab Testing • Scientific & Environmental Consulting

DAILY CONSTRUCTION REPORT

Project: H.P. Hood Proposed Building Addition Project No.: 02-0153.1

Client: Centerline Construction Date: 9-25-2002

Client's Rep.: Dick Miller

Weather: Sunny Temp. Range: 65-75

Arrived at Site at: 7:00 AM

Work in Progress: Pile Driving by H.B. Flemming

Work Performed by SWC Rep.: Pile driving monitoring

General Observations, Discussions, Etc.: The pile driving operations were started and completed today on the pile group adjoining the existing H.P. HOOD building (Pile No's 13 thru 16). The piles were cut-off at ground level and will need to be cut-off at their final cut-off elevation at a future date. The present temporary cut-off elevations were estimated as follows by measuring from existing site features: 94.4 feet for pile No's 13 and 14 and 95.5 feet for piles 15 and 16. Each pile was driven to refusal at a depth of about 12 feet from the ground surface. Pile No's 13 and 14 were shifted from their design locations by Sean Boyles. Pile No. 14 was moved approximately 16" (away from the building) and approximately 7" south towards the center of the pile cap (see sheet 4). The location was changed so that the pile driving equipment would not strike the building during the driving operations. Pile No. 13 was moved approximately 7" to the south (see sheet 4) to clear an underground storm water drainpipe.

Before the pile driving operations could begin, some roof structure had to be removed from the existing building. A small 3± foot awning that extended over the bay doors down the length of the H.P. Hood building overhung pile No's 14 and 16. The awning was cut free and removed over the construction area.

A representative from Elite Inspection was on site to test pile-splicing welds; however, no splicing was needed. Originally, H.B. Flemming had planned on driving 7-foot pile sections and welding additional short sections because of overhead power lines. However, they decided that they would be able to drive longer piling sections if the power was turned off on the overhead lines.

Left Site at: 1:30 PM SWC Rep.: ARS

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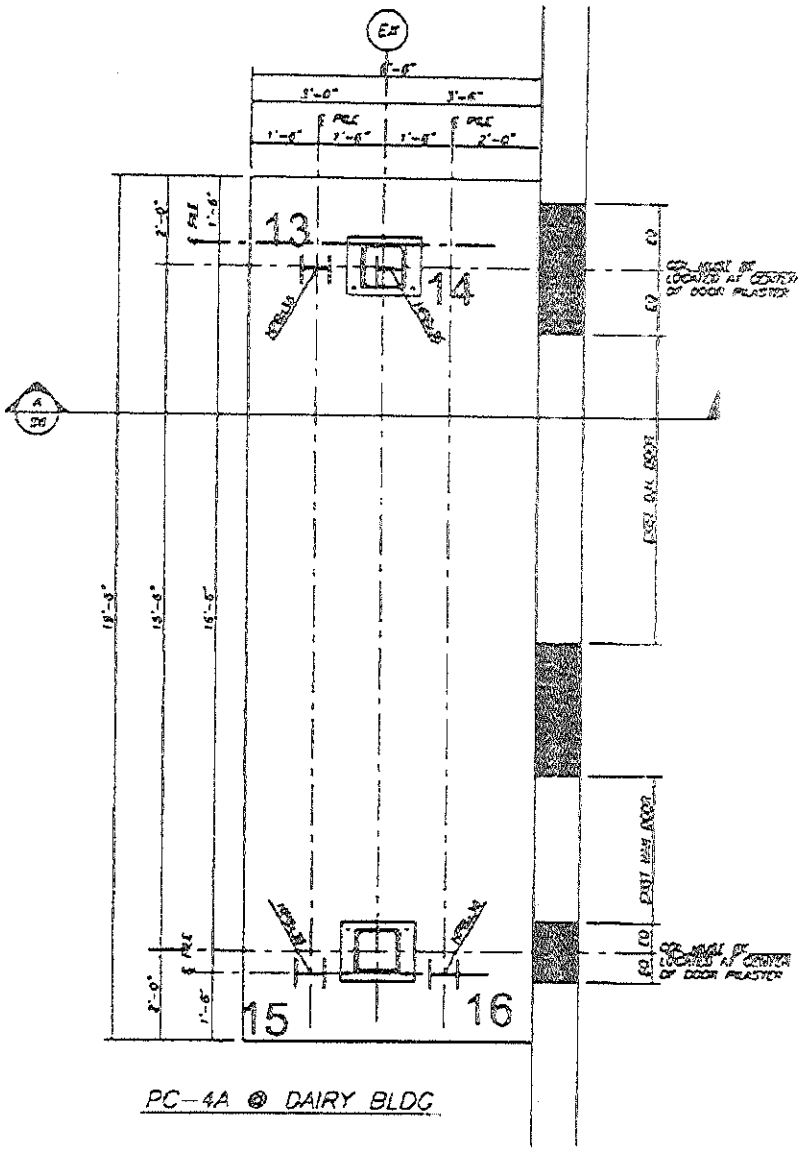

LEGEND

6 Pile Number

NOTE :

Base plan provided by
Becker Structural Engineers, Inc.

S.W.COLE ENGINEERING, INC.

CENTERLINE CONSTRUCTION

PILE SUMMARY

Proposed H.P. Hood Building Expansion
Park Avenue
Portland, Maine

Job No. 02-0153.1 S
Date : 09/27/02

Scale 1/4" = 1'
Sheet 3

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S.W.COLE ENGINEERING, INC.

PILE DRIVING SUMMARY

Client: Centerline Construction
Project: H.P. Hood building expansion, Park Avenue Portland, Maine
General Contractor: Centerline Construction (H.B. Fleming, Pile Contractor)
Pile Hammer: 2,300 lb drop hammer
Rated Energy:. 13,800 lb-ft @ 6' stroke
Pile Type: HP 8x36 ASTM A572 Grade 50
Design Capacity: 40 ton in compression
Reference Elev: FFE = 100 feet (Architectural Datum)

S.W.C.E.# 02-0153.1

Pile #	Total Driven Length (feet)	Cut-Off Elevation (feet)	Total Cut-off Length (feet)	As-Built Section Lengths I	As-Built Section Lengths II	Splice Reference Elevation (feet)	Tip Reference Elevation (feet)	Pay Length (feet)	Final Set Last 2 Inches (blows/inch)		Date started	Date finished	Remarks
1	40.2	97.2	5.8	30.0	4.4	92.8	62.8	34.4	5	6/.25"	9/9/02	9/9/02	
2	40.4	97.2	4.8	30.1	5.5	91.7	61.6	35.6	5	4/.75"	9/9/02	9/9/02	
3	45.3	97.2	10.4	29.8	5.1	92.1	62.3	34.9	6	8/.5"	9/6/02	9/6/02	Heave = 0.18", Pile rotated ±30°. Pile off design location by ±6"
4	40.3	97.2	5.9	29.9	4.5	92.7	62.8	34.4	4	6/.25"	9/6/02	9/9/02	
5	40.6	97.2	5.6	30.1	4.9	92.3	62.2	35.0	3	5/.25"	9/9/02	9/9/02	
6	45.3	97.2	9.4	30.0	5.9	91.3	61.3	35.9	4	5/.125"	9/6/02	9/6/02	Heave = 0.05"
7	40.2	97.2	4.6	28.8	6.8	90.4	61.6	35.6	4	5/.125"	9/6/02	9/6/02	Pile off design location by ±9"
8	39.5	97.2	3.8	30.1	5.6	91.6	61.5	35.7	2	6/.125"	9/9/02	9/9/02	Pile off design location by ±9"
9	30.5	92.5	1.2	29.3	—	—	63.2	29.3	3	5/.25"	9/10/02	9/10/02	
10	30.5	92.5	1.1	29.4	—	—	63.1	29.4	6	5/.25"	9/10/02	9/10/02	Pile rotated ±20°
11	30.2	92.5	0.3	29.9	—	—	62.6	29.9	4	4/.25"	9/9/02	9/10/02	
12	30.3	92.5	0.8	29.5	—	—	63.0	29.5	5	8	9/9/02	9/10/02	
13	18.4	94.4	6.3	12.1	—	—	82.3	12.1	2/.5"	4/0"	9/25/02	9/25/02	Pile off design location by ±7" an additional ±2.5' to be cut-off later
14	30.2	94.4	18.2	12.0	—	—	82.4	12.0	2	5/.125"	9/25/02	9/25/02	Pile off design location by ±17", Heave = 0" an additional ±2.5' to be cut-off later
15	18.4	95.5	6.1	12.3	—	—	83.2	12.3	4/.5"	4/.25"	9/25/02	9/25/02	an additional ±3.5' to be cut-off later
16	30.2	95.5	18.2	12.0	—	—	83.5	12.0	4/.5	4/.125"	9/25/02	9/25/02	an additional ±3.5' to be cut-off later

Total Pay Length: 448.0 feet

S.W.COLE ENGINEERING, INC.

• Geotechnical Engineering • Field & Lab Testing • Scientific & Environmental Consulting

DAILY CONSTRUCTION REPORT

Project: H.P. Hood Proposed Building Addition Project No.: 02-0153.1

Client: Centerline Construction Date: 10-29-2002

Client's Rep.: Dick Miller

Weather: Overcast Temp. Range: 40-50

Arrived at Site at: 3:30 AM

Work in Progress: Casing installation by Northern Test Borings Inc.

Work Performed by SWC Rep.: Casing installation observations

General Observations, Discussions, Etc.: Casing was driven at the H.P. Hood site to give additional support to the existing pile group. The Casing was driven 17.5" from the exterior wall of the H.P. Hood building and 1.75 inches north of the pile centerline, see sheet 1. Several attempts were made to drive the casing in-line with the other two piles (Pile No's 13 and 14) but an obstruction at a depth of 8 feet shifted the pile North 1.75 inches.

Three sections of casing were driven, two 5-foot sections and one 2-foot section. The pieces are threaded together. The two foot section is above the final cut-off elevation and is to be removed and returned to Northern Test Borings Inc. The hole was washed out with water to remove debris and the casing was covered to prevent debris from falling in to the hole.

The casing was driven to bedrock at a depth of 11.6 feet with a 140-pound hammer dropped 30 inches. Then a 3± foot socket was bored into the bedrock with a roller cone to a depth of 14.8'. We understand this socket will receive a piece of reinforcing steel in order to tie the casing to the bedrock. The following measurements were taken in the field.

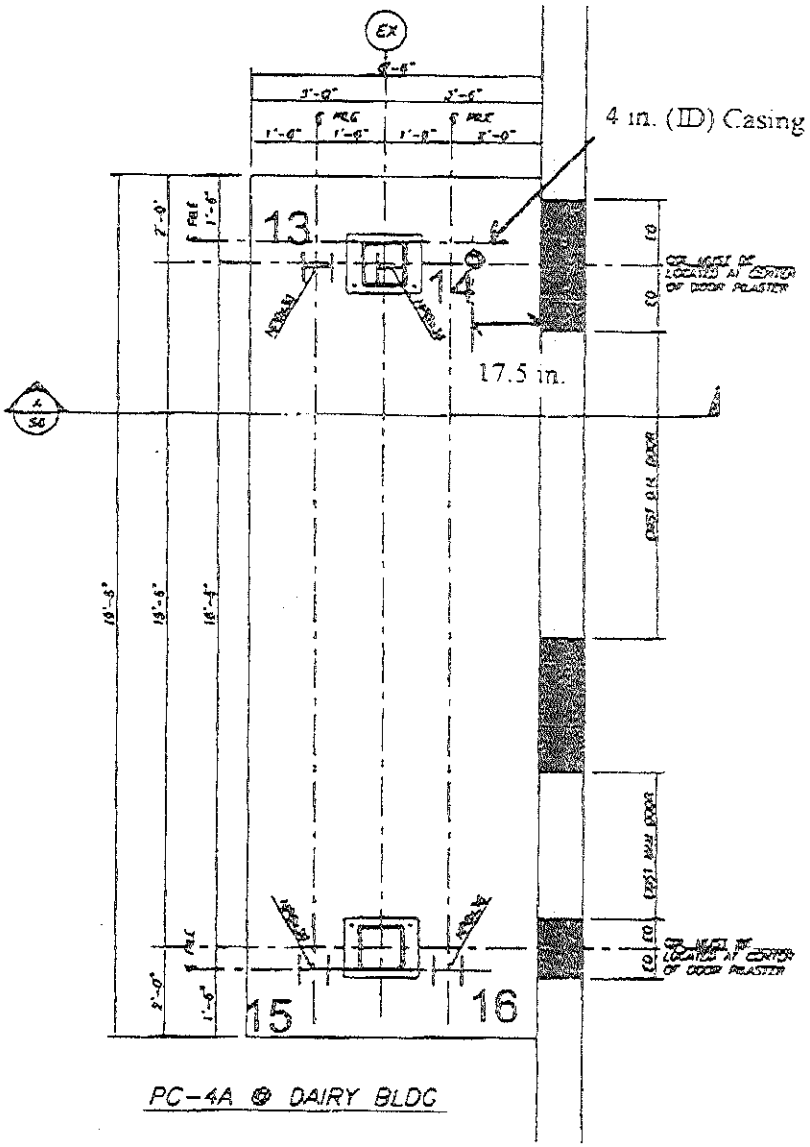
Outside Diameter of Casing	4.5 inches
Inside Diameter of Casing	4.0 inches
Diameter of bedrock socket (Outside Diameter of Roller Cone)	$3\ ^{7}/_{8}$ inches
Elevation of Casing Tip	82.8 feet
Elevation of Rock Socket Tip	79.6 feet
Thread Splice Elevation	87.5 feet
Final Blow Count	2/1" 15/0"

Reference Elevation Note: Finished floor of bottle plant = 100 feet

Left Site at: 4:45 PM SWC Rep.: ARS

GRAY, ME OFFICE
286 Portland Road, Gray, ME 04039, Tel (207) 657-2866, Fax (207) 657-2840, (E) infogray@swcole.com, (I) www.swcole.com

Other offices in Augusta, Bangor and Caribou, Maine & in Somersworth, New Hampshire



PC-4A @ DAIRY BLDG

LEGEND

6 Pile Number

NOTE :

Base plan provided by
Becker Structural Engineers, Inc.

S.W.COLE ENGINEERING, INC.

CENTERLINE CONSTRUCTION

PILE SUMMARY

Proposed H.P. Hood Building Expansion
Park Avenue
Portland, Maine

Job No. 02-0153.1 S
Date : 09/27/02

Scale 1/4" = 1'
Sheet 1

03300 Cast-in-Place Concrete
03300.1 Inspection Reports

BECKER
structural engineers

Date: September 20, 2002
Time: 8:30 A.M.
Temp: mid 60's
Weather: Cloudy

Project: **H.P. Hood Bottle Conveyor Bridge & Facility**

Inspection Report - Cast-in-place Concrete

Location: Mat Footing: Col. Line "G"-Col. Line "F"

Satisfactory	Unsatisfactory	Not Completed	N/A	
X				Reinforcement Size
X				Quanity
X				Condition
X				Placement
X				Embed/Anchors
X				Lap Splices
			X	Reinf. Weld
			X	Hot Weather
			X	Cold Weather

Satisfactory	Unsatisfactory	Not Completed	N/A	Additional Items:

Notes: I reviewed reinforcement for one of the large mat footings. Footing reinforcement conformed to project documents and ACI standards.

Signed: Daniel S. Burne, E.I. Date: 9/20/02

WO 893

Becker Structural Engineers
19 Commerical Street
Portland, Maine 04101
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