City of Portland, Maine - Building or Use Permit Application

389 Congress Street, 04101 Tel: (207) 874-8703, Fax: (207) 874-8716

PERMIT ISSUED
MAY 13 2002
CITY OF PORTLAND

Permit No:	Issue Date:	CBL:
020345		447 A001001

Location of Construction:	Owner Name:	Owner Address:	Phone:
1 Bean Pot Cir	Heritage Acquistion Corp	4 Gatehall Dr Ste 110	
Business Name:	**Contractor Name:**	**Contractor Address:**	**Phone**
n/a	N.G. Bailey	2 Baily Drive Gray	2076573200
Lessee/Buyer's Name	**Phone:**	**Permit Type:**	**Zone:**
n/a	n/a	Additions - Commercial	I-M

Past Use:	Proposed Use:	Permit Fee:	Cost of Work:	CEO District:
Commercial / Baked Bean Processing Plant	Baked Bean Processing Plant / 1381 sq. ft. expansion for boiler room.	$1,073.00	$150,000.00	2

FIRE DEPT: ☑ Approved ☐ Denied

Signature:

INSPECTION: Use Group: F1 Type: 2C

5/6/02

Signature:

Proposed Project Description:
Build Boiler Room expansion 1381 sq. ft.

PEDESTRIAN ACTIVITIES DISTRICT (P.A.D.)

Action: ☐ Approved ☐ Approved w/Conditions ☐ Denied

Signature: Date:

Permit Taken By:	Date Applied For:
gg	04/10/2002

1. This permit application does not preclude the Applicant(s) from meeting applicable State and Federal Rules.
2. Building permits do not include plumbing, septic or electrical work.
3. Building permits are void if work is not started within six (6) months of the date of issuance. False information may invalidate a building permit and stop all work..

Zoning Approval

Special Zone or Reviews	Zoning Appeal	Historic Preservation
☐ Shoreland	☐ Variance	☑ Not in District or Landmark
☐ Wetland	☐ Miscellaneous	☐ Does Not Require Review
☐ Flood Zone	☐ Conditional Use	☐ Requires Review
☐ Subdivision	☐ Interpretation	☐ Approved
☐ Site Plan	☐ Approved	☐ Approved w/Conditions
Maj ☐ Minor ☐ MM ☐	☐ Denied	☐ Denied
Date: 4/12/02	Date:	Date:

CERTIFICATION

I hereby certify that I am the owner of record of the named property, or that the proposed work is authorized by the owner of record and that I have been authorized by the owner to make this application as his authorized agent and I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in the application is issued, I certify that the code official's authorized representative shall have the authority to enter all areas covered by such permit at any reasonable hour to enforce the provision of the code(s) applicable to such permit.

SIGNATURE OF APPLICANT | ADDRESS | DATE | PHONE

RESPONSIBLE PERSON IN CHARGE OF WORK, TITLE | DATE | PHONE

All Purpose Building Permit Application

If you or the property owner owes real estate or personal property taxes or user charges on any property within the City, payment arrangements must be made before permits of any kind are accepted.

Location/Address of Construction: Burnham + Morrill Co., One Bean Pot Circle, Portland, ME, 04103

Total Square Footage of Proposed Structure	Square Footage of Lot
1381 sqr ft.	13.0 Acres

Tax Assessor's Chart, Block & Lot (Chart# / Block# / Lot#)	Owner:	Telephone:
447 / A / 001	B+G Foods Inc.	772-8341 Ext 269

Lessee/Buyer's Name (If Applicable)	Applicant name, address & telephone:	Cost Of Work: $ / Fee: $
N/A	John Totman One Bean Pot Circle Portland ME 04103 772-8341 Ext. 269	Cost Of Work: $ 150,000 Fee: $ 1073-

Current use: Boiler Room

If the location is currently vacant, what was prior use: _____

Approximately how long has it been vacant: _____

Proposed use: Boiler Room

Project description:
Expansion of boiler room 2002 0018

Contractor's name, address & telephone: N.G. Bailey, 2 Bailey Dr, Gray, ME 04039 657-3200

Who should we contact when the permit is ready: John Totman

Mailing address: One Bean Pot Circle, Portland, ME 04103 772-8341 Ext. 269

We will contact you by phone when the permit is ready. You must come in and pick up the permit and review the requirements before starting any work, with a Plan Reviewer. A stop work order will be issued and a $100.00 fee if any work starts before the permit is picked up. **PHONE:** 772-8341 Ext. 269

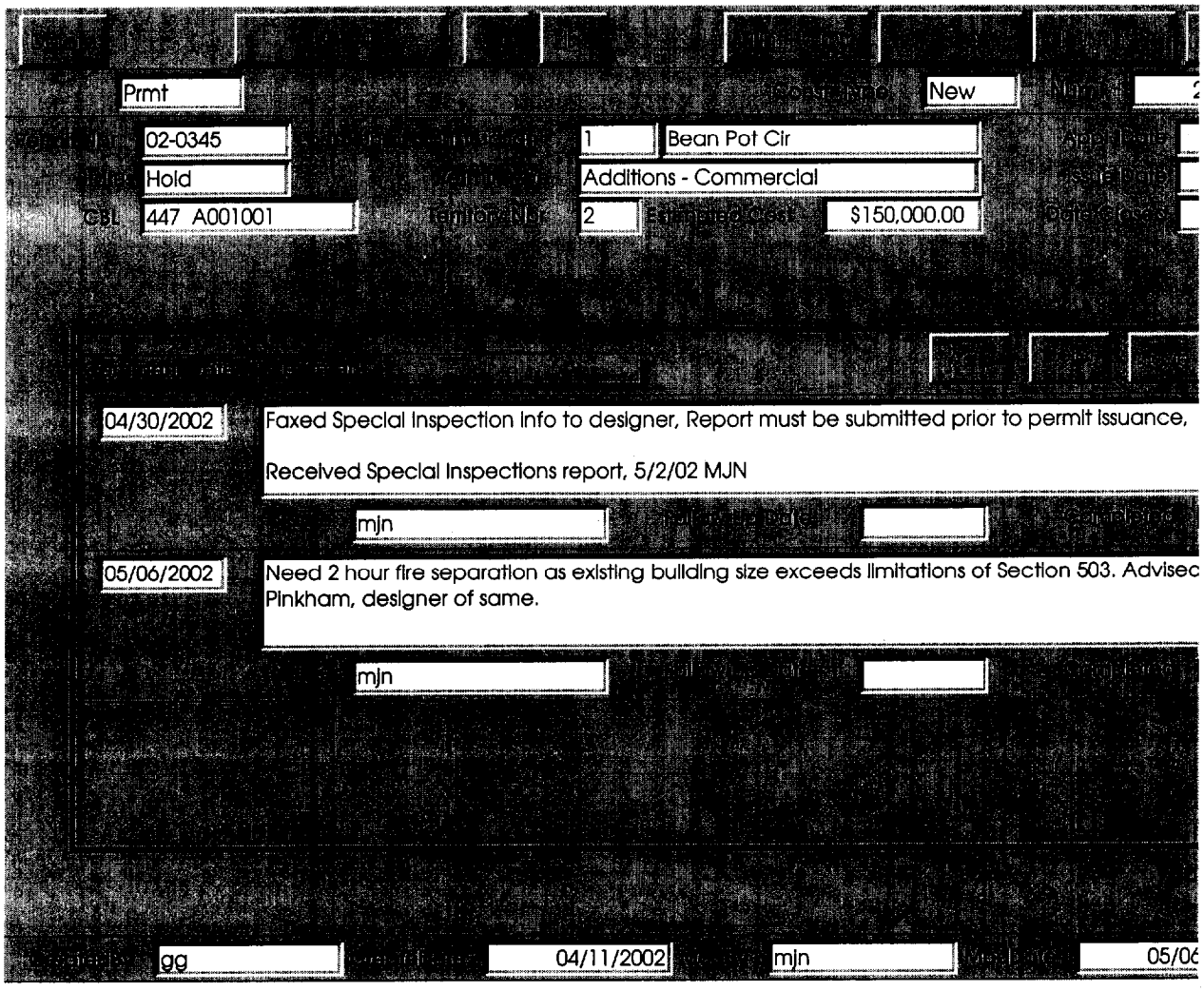
IF THE REQUIRED INFORMATION IS NOT INCLUDED IN THE SUBMISSIONS THE PERMIT WILL BE AUTOMATICALLY DENIED AT THE DISCRETION OF THE BUILDING/PLANNING DEPARTMENT, WE MAY REQUIRE ADDITIONAL INFORMATION IN ORDER TO APROVE THIS PERMIT.

I hereby certify that I am the Owner of record of the named property, or that the owner of record authorizes the proposed work and that I have been authorized by the owner to make this application as his/her authorized agent. I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in this application is issued, I certify that the Code Official's authorized representative shall have the authority to enter all areas covered by this permit at any reasonable hour to enforce the provisions of the codes applicable to this permit.

Signature of applicant: [signature] Date: 4-9-02

DEPT. OF BUILDING INSPECTION
CITY OF PORTLAND, ME
APR 10 2002
RECEIVED

This is NOT a permit, you may not commence ANY work until the permit is issued.
If you are in a Historic District you may be subject to additional permitting and fees with the Planning Department on the 4th floor of City Hall


Prmt
New
02-0345
1
Bean Pot Cir
Hold
Additions - Commercial
GBL
447 A001001
2
$150,000.00
04/30/2002
Faxed Special Inspection Info to designer, Report must be submitted prior to permit issuance,
Received Special Inspections report, 5/2/02 MJN
mjn
05/06/2002
Need 2 hour fire separation as existing building size exceeds limitations of Section 503. Advisec
Pinkham, designer of same.
mjn
gg
04/11/2002
mjn
05/06

S E A M

Structural Engineering Association of Maine

STATEMENT OF SPECIAL INSPECTIONS

PROJECT: Boiler Building

LOCATION: One Bean Pot Circle, Portland, ME

PERMIT APPLICANT: BURNHAM & MORRILL CO.

APPLICANT'S ADDRESS: One Bean Pot Circle, Portland,ME

STRUCTURAL ENGINEER OF RECORD. David K. Pinkham, P.E. (NAME) Pinkham & Greer Engineers (FIRM)

ARCHITECT OF RECORD: N/A (NAME) (FIRM)

This Statement of Special Inspections is submitted in accordance with Section 1705.0 of the 1999 BOCA National Building Code. It includes a listing of special inspections applicable to this project as well as the name of the Special Inspector, and the names of other agencies intended to be retained for conducting these inspections.

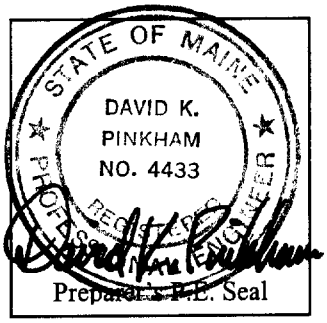
The Special Inspector shall keep records of all inspections listed herein, and shall furnish inspection reports to the Code Official and to the Registered Design Professional of Record. All discrepancies shall be brought to the immediate attention of the Contractor for correction. If the discrepancies are not corrected the discrepancies shall be brought to the attention of the Code Official and to the Registered Design Professional of Record. Interim reports shall be submitted to the Code Official and to the Registered Design Professional of Record monthly, unless more frequent submissions are requested by the Code Official.

Job site safety is solely the responsibility of the Contractor. Materials and activities to be inspected are not to include the Contractor's equipment and methods used to erect or install the materials listed.

Prepared By:

David K. Pinkham, P.E. (NAME)

David K. Pinkham 5.2.02 (SIGNATURE) (DATE)



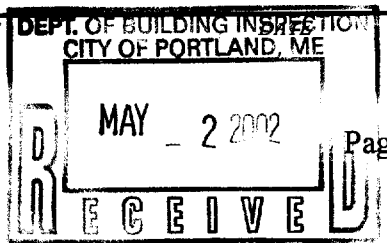
Preparer's P.E. Seal

Applicant's Authorization:

John Peters 5-2-02 (SIGNATURE) (DATE)

Building Code Official:

(SIGNATURE)



S E A M

Structural Engineering Association of Maine

LIST OF AGENTS

PROJECT: Boiler Building, Burnham & Morrill Co.

STRUCTURAL ENGINEER OF RECORD: David K. Pinkham, P.E. Pinkham & Greer Engineers, Inc.
NAME *FIRM*

170 U.S. Route One, Falmouth, ME 04105
ADDRESS

ARCHITECT OF RECORD: N/A
NAME *FIRM*

ADDRESS

Following is the List of Agents selected for performance of Special Inspections for this project.

	Name	Firm
1. Special Inspector	David K. Pinkham, P.E.	Pinkham & Greer Engineers
2. Inspector	Kenneth I. Marsh	Pinkham & Greer Engineers
3. Testing Laboratory	Quality Assurance Laboratories	80 Pleasant Ave. S. Portland, ME 04106 (207) 799-8911/799-7551

SCHEDULE OF SPECIAL INSPECTION SERVICES

PROJECT: BOILER BUILDING, BURNHAM & MORRILL CO.

MATERIAL/ACTIVITY	ITEM	SERVICE	APPLICABLE TO THIS PROJECT Y/N	EXTENT (All, Sample, Other, None)	COMMENTS	#	COMPLETED	#
1705.3 STEEL CONSTRUCTION	1.00							
Steel Fabrication		In-plant review Part A - Fabrication procedures	N					
		Part B - Procedures implementation Review conformance to Part A		SER to determine extent after completion of Part A				
		Review material certificates of compliance (Bolts, nuts, washers, structural steel, & weld filler material)	N					
		Review connections	N					
		Review welding of seismic-resisting system in Cat. "C" buildings	N					
Steel Erection		Review welder certification	Y	ALL		1		
		Review materials certificates of compliance (Bolts, nuts washers, & weld filler material)	N					
		Review primary steel connections						
		Moment connections	N			3		
		Shear connections	Y	Visual, ALL		3		
		Bracing connections	Y	Visual, ALL				
		Review welded Cat. "C" seismic connections	N					
		Review welded column splices	N					
		Review base metal testing for "t"> 1 1/2"	N					
		Review secondary steel connections						
		Girts	Y	Visual, ALL		1 or 3		
		Steel deck	Y	Visual, ALL		1 or 3		
		Lintels	N					
		Review installation of shear studs	N					
		Review Details / Steel Frame	Y	SAMPLE		1 or 3		

All Steel Construciton Special Inspections have been completed in accordance with BOCA Section 1705.3 Special Inspector _____ Date _____

SCHEDULE OF SPECIAL INSPECTION SERVICES

PROJECT: BOILER BUILDING, BURNHAM & MORRILL CO.

MATERIAL/ACTIVITY	ITEM	SERVICE	APPLICABLE TO THIS PROJECT Y/N	EXTENT (All, Sample, Other, None)	COMMENTS	#	COMPLETED	#
1705.3 STEEL CONSTRUCTION (Continued)	**1.00**							
Steel Joist & Joist Girder Fabrication		In-plant review Part A - Fabrication procedures	N	Unless excepted by 1705.2 review fabrication Q/A proceedures per 1705.2				
		Part B - Procedures implementation Review conformance to Part A		SER to determine extent after completion of Part A				
		Review material certificates of compliance (structural steel, & weld filler material)	N					
		Review connections	N					
Steel Joist & Joist Girder Erection		Review welder certification	Y	ALL		1		
		Review joist bearing connection	Y	Visual, ALL		1 or 3		
		Review joist bearing length	Y	Visual, ALL		1 or 3		
		Review joist bridging	Y	Visual, ALL		1 or 3		

All Steel Construction Special Inspections have been completed in accordance with BOCA Section 1705.3 Special Inspector _____ Date _____

SCHEDULE OF SPECIAL INSPECTION SERVICES

PROJECT: BOILER BUILDING, BURNHAM & MORRILL CO.

MATERIAL/ACTIVITY	ITEM	SERVICE	APPLICABLE TO THIS PROJECT Y/N	EXTENT (All, Sample, Other, None)	COMMENTS	#	COMPLETED	#
1705.4 CONCRETE CONSTRUCTION	2.00							
Concrete Materials		Review materials (ACI Chapter 3)	N					
		Review mix design (ACI Chapter 4 & 5)	Y	ALL		1		
		Review reinforcing certification & weldability (ASTM A760) if required	N					
Placing Reinforcement		Review condition & placement of reinforcing and prestressing steel (ACI 318 7.4-7.7)	N					
		Review welding of reinforcing in Cat "C" seismic-resisting systems	N					
Formwork		Review formwork (ACI 318 6.1)	N					
		Review form removal & reshoring (ACI 318 6.2)	N					
Concrete Operations		Review concrete strength tests (ACI 318 5.6)	N					
		Review mix proportions and technique (ACI 318 5.2, 5.3, 5.4, & 5.8)	N					
		Review concrete placement (ACI 318 5.9 & 5.10)	N					
		Review curing technique & temperature (ACI 318 5.11, 5.12, & 5.13)	N					
Prestressing Operations		Review application of prestressing force (ACI 318 18.18)	N					
		Review grouting of bonded prestressing tendons in Cat. "C" seismic-resisting systems	N					
Precast Manufacturing		In-plant review Part A- Fabrication procedures	N					
		Part B - Procedures implementation Review conformance to Part A	N					
Erection of Precast Concrete		Review erection of precast units	N					
		Review key reinforcement	N					
		Review key grouting	N					
		Review concrete topping	N					
		Review connections	N					

All Concrete Construction Special Inspections have been completed in accordance with BOCA Section 1705.4 Special Inspector _____ Date _____

PINKHAM & GREER
CONSULTING ENGINEERS, INC.

170 U.S. Route One
Falmouth, Maine 04105
Tel: 207.781.5242
Fax: 207.781.4245

May 8, 2002
File: 02302

Mr. Michael Nugent
Director of Inspection Services
CITY OF PORTLAND
389 Congress Street
Portland, ME 04101

RE: BURNHAM & MORRILL CO. BOILER BUILDING

Dear Mr. Nugent:

I have discussed the requirement for a two hour rated fire separation between Burnham & Morrill's (B&M) new boiler building and the existing buildings with Adam Bear of B&M. There are several openings through walls and at least three types of wall construction that will have to be addressed. Some of the walls are masonry and may already provide the two hour separation. The openings and other wall types will require modification.

Pinkham & Greer will evaluate the various conditions and develop a plan and details showing the existing conditions and necessary modifications. We will submit this to you for your review and make any revisions you feel are necessary.

Hopefully this letter will allow B&M to obtain a building permit for the boiler building with the understanding that the fire separation requirements will be met. Please call me if you have any questions.

Sincerely,
PINKHAM & GREER
David K. Pinkham
David K. Pinkham, P.E.

DKP/bt

170 U.S. Route One
Falmouth, Maine 04105
Tel: (207) 781-5242
Fax: (207) 781-4245

FAX MEMORANDUM

TO: Mr. Michael Nugent, City of Portland

FAX #: 874-8649

FROM: David K. Pinkham, P.E.

DATE: May 8, 2002

RE: B&M BOILER BUILDING

FILE: 02302

# of Pages (including this one): 2

The hard copy will be mailed to you.

PINKHAM & GREER
CONSULTING ENGINEERS, INC.

170 U.S. Route One
Falmouth, Maine 04105
Tel: 207.781.5242
Fax: 207.781.4245

May 8, 2002
File: 02302

Mr. Michael Nugent
Director of Inspection Services
CITY OF PORTLAND
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Portland, ME 04101

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Sincerely,
PINKHAM & GREER

David K. Pinkham

David K. Pinkham, P.E.

DKP/bt

14616-57

R 3-17-99

3:32

RELEASE DEED

The Pillsbury Company, a Delaware corporation with a principal place of business at 200 So. 6th Street, Minneapolis, County of Hennepin and State of Minnesota 55402, for consideration paid, releases to the Heritage Acquisition Corp., a Delaware corporation of Roseland, New Jersey, and whose mailing address is 426 Eagle Rock Avenue, Roseland, New Jersey 07068, all of The Pillsbury Company's right, title and interest in certain parcels of land, together with the fixtures and improvements thereon situated in the City of Portland, County of Cumberland, State of Maine, as more particularly described in Exhibit A, attached hereto and by this reference incorporated herein.

IN WITNESS WHEREOF, the Pillsbury Company has caused this instrument to be executed in its name by David Schmitt, Vice President, hereunto duly authorized, dated as of the 15th day of March, 1999

WITNESS:

THE PILLSBURY COMPANY

By: _____
Name: David Schmitt
Title: Vice President

STATE OF NEW YORK

COUNTY OF NEW YORK, SS.

March 15, 1999

Personally appeared the above named David Schmitt, the duly authorized Vice President of The Pillsbury Company and acknowledged the foregoing instrument to be his free act and deed in his said capacity and the free act and deed of The Pillsbury Company.

Before me,

_____
Notary Public

Print Name: Deborah R Grasich

Commission Expires: _____

EXHIBIT A

A certain parcel of land located on the easterly sideline of Interstate 295 and the southerly sideline of Sherwood Street, in the City of Portland, Cumberland County, Maine, bounded and described as follows:

Beginning at a 6-inch by 6-inch granite monument found inscribed "ECJ #356" marking the southerly sideline of Sherwood Street westerly sideline of said Canadian National Railroad and further described as the most northeasterly corner of described parcel;
THENCE, S 23-36-55 E, along said Railroad 316.33 feet to a 6-inch by 6-inch granite monument found inscribed "ECJ 356";
THENCE, S 23-36-55 E, along said Railroad, 526.90 feet, to a 4-inch by 4-inch granite monument with a drill hole;
THENCE, S 23-36-55 E, along said Railroad, 745 feet, more or less to the low water mark of the Casco Bay;
THENCE, in a general westerly direction along said low water line about 1,685 feet to a point on the easterly sideline of Interstate 295;
THENCE, N 2-23-50 W, along said sideline, 48 feet, more or less to a point;
THENCE, S 87-36-10 W, along said sideline, 30.00 feet to a point;
THENCE, N 2-23-50 W, along said sideline, 61.46 feet to a drill hole found, said drill hole lying on a tie course of S 73-07-39 W, 878.87 feet from the previous mentioned 4-inch by 4-inch granite monument with a drill hole;
THENCE, N 2-23-50 W, along said sideline, 237.87 feet to a 5/8 inch re-bar found capped "ECJ #509";
THENCE, N 89-44-34 E, along said sideline, 1.05 feet to a 5/8 inch re-bar found capped "ECJ #509";
THENCE, N 6-28-53 E, along said sideline, 122.88 feet to a 5/8 inch re-bar found capped "ECJ #509";
THENCE, N 0-24-30 E, along said sideline, 88.91 feet to a 4-inch by 4-inch granite monument found inscribed "ECJ #509";
THENCE, N 24-48-04 E, along said sideline, 60.29 feet to a 6-inch by 6-inch granite highway monument;
THENCE, N 31-29-16 W, along said sideline, 31.96 feet to a 4-inch by 4-inch granite monument inscribed "ECJ #509";
THENCE, N 3-26-49 W, along said sideline, 111.10 feet to a 5/8 inch re-bar found capped "ECJ #509";
THENCE, N 8-40-57 E, along said sideline, 89.05 feet to a 4-inch by 4-inch granite monument inscribed "ECJ #509";
THENCE, N 23-15-05 E, along said sideline, 66.28 feet to a 4-inch by 4-inch granite monument inscribed "ECJ #509";
THENCE, N 34-47-28 E, along said sideline, 96.68 feet to a 4-inch by 4-inch granite monument inscribed "ECJ #509";
THENCE, N 25-25-29 E, along said sideline, 160.02 feet to a 5/8 inch re-bar found capped "ECJ #509";

EXHIBIT A (contd.)

THENCE, N 23-37-03 E, along said sideline, 63.03 feet to a 4-inch by 4-inch granite monument inscribed "ECJ #509";
THENCE, N 39-23-04 E, along said sideline, 6.72 feet to a 6-inch by 6-inch granite monument found inscribed "ECJ #509";
THENCE, N 55-32-30 W, along said sideline, 11.67 feet to a 4-inch by 4-inch granite monument inscribed "ECJ #509";
THENCE, N 34-47-50 E, along said sideline, 117.04 feet to a 4-inch by 4-inch granite monument inscribed "ECJ #509";
THENCE, N 34-43-38 E, along said sideline, 15.02 feet to a 5/8 inch re-bar found capped "ECJ #509" marking the southerly sideline of Sherwood Street;
THENCE, S 55-12-08 E, along said Sherwood Street, 288.60 feet to the point of beginning.

Together with a constructive easement pursuant to 12 M.R.S.A. Section 1862 (6) for a thirty (30) year period commencing October 1, 1975 with respect to the structure shown on the Survey by OEST Associates recorded in the Cumberland County Registry of Deeds in Plan Book 199, Page 159 as lying partially below the low water mark of Casco Bay.

h 3-17-99

3:30

QUITCLAIM DEED WITH COVENANT

The Pillsbury Company, a Delaware corporation with a principal place of business at 200 So. 6th Street, Minneapolis, County of Hennepin and State of Minnesota 55402, for consideration paid, grants to the Heritage Acquisition Corp. , a Delaware corporation of Roseland, New Jersey, and whose mailing address is 426 Eagle Rock Avenue; Roseland, New Jersey 07068, with QUITCLAIM COVENANT, certain parcels of land, together with the fixtures and improvements thereon situated in the City of Portland, County of Cumberland, State of Maine, as more particularly described on Exhibit A, attached hereto and by this reference incorporated herein, including, without limitation, any and all appurtenances, easements and other improvements relating or pertaining thereto.

IN WITNESS WHEREOF, The Pillsbury Company has caused this instrument to be executed in its name by David Schmitt , Vice President hereunto duly authorized, dated as of the 15th day of March, 1999.

WITNESS:

THE PILLSBURY COMPANY

By: _____
Name: David Schmitt
Title: Vice President

STATE OF NEW YORK
COUNTY OF NEW YORK SS.: March 15, 1999

Personally appeared the above named David Schmitt, the duly authorized Vice President of The Pillsbury Company and acknowledged the foregoing instrument to be his free act and deed in his said capacity and the free act and deed of The Pillsbury Company.

Before me,

_____
Notary Public

Print Name: Elisabeth Falalcer

Commission Expires: Aug 17, 2000

MJG\G:\PillsburyCo\QuitclaimDeed3RDDraft.doc

EXHIBIT A

I. Ten parcels of land with the buildings thereon situated in the State of Maine, City of Portland, in the district formerly known as East Deering, and being more particularly bounded and described as follows:

Parcel One - So-called Merrill Lot

A parcel of land bounded northeasterly and easterly by land now or formerly of Mark Jordan; southerly by the channel of Back Bay; westerly by land now or formerly of George P. Derr; northerly and westerly by land now or formerly of Alberta Pettengill.

There is hereby excepted from Parcel One (a) that part which William W. Merrill conveyed to Ralph Kelley by deed of quit-claim bearing date of May 23, 1853, and recorded in the Cumberland County Registry of Deeds in Book 269, Page 137, and (b) that part of the same which William M. Merrill conveyed to Silas M. Adams by deeds bearing date of April 17, 1869, and December 11, 1860, and recorded in said Registry in Book 298, Page 425, and in Book 314, Page 463.

Parcel Two - So-called Jordan-Saunders Lot

A triangular parcel bounded and described as follows: beginning at a stake in the division line between land formerly of Sarah Jordan and Isabelle Jordan Saunders and land formerly of William Merrill, said stake being about eighty-eight (88) feet northerly of a street formerly known as Water Street on a line that passes through a stake five (5) feet northwest of the northwesterly corner of a house on said land of Merrill; thence from said first named stake north sixty-nine (69) degrees two (2) minutes east three hundred twenty-one (321) feet and thirty-five one hundredths (35/100) of a foot to a post in the westerly line of the right of way of Grand Trunk Railway Company of Canada; thence southerly by said westerly line of said right of way of said Railway Company five hundred sixty-eight (568) feet, more or less, to the northerly line of said land of Merrill; thence north forty (40) degrees thirty-seven (37) minutes west by said northerly line of Merrill's land five hundred ninety (590) feet, more or less, to point of beginning.

Parcel Three - So-called Derr Lot

A parcel bounded and described as follows, viz: beginning at a stake standing in the northerly side line of said Water Street, in the easterly corner of land formerly owned by Ephraim Sawyer; thence running south seventy-one (71) degrees east to a point where was situated an "Oak Stump" January 23, 1845, a distance of one (1) rod and eleven (11) links; thence south forty (40) degrees east five (5) rods and eight (8) links; thence north nine (9) degrees east twelve and one-half (12 1/2) rods to the line on which an old fence formerly stood; thence northwesterly by the line of said fence one hundred and thirty (130) feet to a point; thence on a straight line two hundred and thirty (230) feet, more or less, to the first bounds mentioned.

Also a certain lot or parcel of upland and flats on the southerly side of said Water Street, so-called, directly opposite the above described lot, bounded and described as follows: beginning on said southerly side of said Water Street at the northeasterly corner of land now or formerly of Ephraim Sawyer; thence southerly by said Sawyer land to the channel; thence easterly by said channel to land formerly of Burnham & Morrill Company; thence northerly by said Burnham & Morrill Company's land to said Water Street; thence westerly by said Water Street to said Sawyer land and point of beginning.

Excepting and reserving from Parcel Three that portion which was conveyed to Lemuel Dyer to Jacob P. Shattuck by his deed dated January 23, 1865, and recorded in said Registry in Book 770, Page 141.

Parcel Four - So-Called Pettengill Lot

A parcel of land bounded and described as follows: beginning on said Water Street where the said lot and the land now or formerly of L. W. Dyer intersects at a post firmly set in the ground; thence along said Water Street in a southeasterly direction about eighty feet (80') to a post where the said lot intersects with land formerly owned by William W. Merrill; thence in a northeasterly direction about 100 feet to a post and land formerly owned by Ralph Kelley; thence in a northwesterly direction about 120 feet to a post and land now or formerly of said L. W. Dyer; thence in a southwesterly direction along said Dyer land to the point of beginning.

Parcel Five - So-called Shattuck Lot

Two parcels of land bounded and described as follows: beginning on the southerly side of said Water Street at the northeasterly corner of land and flats formerly belonging to Isaac Sturdivant and the heirs of Isaac Ilsley; thence running southwesterly by said land and flats owned as aforesaid to low water mark; thence easterly by low water mark to land and flats formerly belonging to Lemuel Dyer; thence northeasterly by said Dyer land and flats to said Water Street; thence westerly by said Water Street to place of beginning; being about one hundred (100) feet on said Water Street and extending to low water mark, holding the width of one hundred (100) feet; subject to the rights and limitations set forth in the Colonial Ordinances, 1641-1646.

Also another certain lot or parcel of land with the buildings thereon, situated on the northerly side of said Water Street, bounded and described as follows: beginning at a post standing on the northerly side of said Water Street, south forty (40) degrees west, fourteen (14) feet and six (6) inches from the most southwesterly corner of the cellar wall of the house formerly on said premises; thence running north two (2) degrees east by land formerly of the heirs of James Lunt, one hundred (100) feet to a post at the southeasterly corner of land formerly of S. B. Brackett; thence north seven and one-half (7-1/2) degrees east by said Brackett's land, and by land formerly of the heirs of James Lunt, one hundred fifty-two (152) feet, more or less, to land formerly of Ralph Kelley; thence southeasterly by said Kelley's land, one hundred fifty-five (155) feet, more or less, to land conveyed by James Lunt to John Randall and William M. Merrill, afterwards owned by Lemuel Dyer; thence southwesterly by said last mentioned land to said Water Street; thence westerly by said Water Street to the place of beginning.

Parcel Six - So-called Dartmouth Real Estate Company Lot

A parcel of land with the buildings thereon bounded and described as follows: commencing at a stake standing in the northwesterly side line of a right of way granted to said Burnham & Morrill Company, which said stake is fifty-three and nine hundredths (53.09) feet on a course north 39 degrees 35 minutes west, from a concrete monument which stands on the southeasterly side line of said right of way and separating said right of way from other land of the said Burnham & Morrill Company; thence north 39 degrees 35 minutes west, three hundred forty-seven and sixty-eight hundredths (347.68) feet to a stake; thence at right angles on a course north 50 degrees 25 minutes east three hundred six and ninety-eight hundredths (306.98) feet to a stake; thence at right angles and running on a course of south 39 degrees 35 minutes east two hundred nine and fifty-five hundredths (209.55) feet to a stake; thence on a course of south 38 degrees 45 minutes east two hundred ninety-nine and eighteen hundredths (299.18) feet to a concrete monument standing in the line of the right of way of the Grand Trunk Railway of Canada; thence turning an interior angle of 71 degrees 11 minutes and running on a course south 70 degrees 04 minutes west one hundred sixty and sixty-eight hundredths (160.68) feet to a concrete monument at the easterly corner of the right of way of Burnham & Morrill Company; thence turning an angle of 90 degrees and running fifty (50) feet across the end of said right of way to a stake; thence in a course south 70 degrees 04 minutes west one hundred seventy-eight and fifty-two hundredths (178.52) feet to the point of beginning.

A plan of the above described premises entitled "The McLain Company Plan": is recorded in the Registry of Deeds for Cumberland County in Plan Book 15, Page 26.

Parcel Seven - So-called McCracken Lot

A parcel of land situated on the southwesterly side of Sherwood Street (formerly Winslow Street) in said Portland, and more particularly bounded and described as follows:

beginning at a point on the southwesterly line of Sherwood Street at the easterly corner of Lot No. 7 on Plan of Property belonging to the Lunt Heirs, made by John D. Bailey, C. E., dated May 15, 1857, and recorded in Cumberland County Registry of Deeds in Plan Book 3, Page 48; thence southwesterly along the southeasterly line of said Lot No. 7 one hundred thirty-two (132) feet to a point on the southwesterly line of Lot No. 6 as shown on said Plan; thence southeasterly along said southwesterly line of Lot No. 6 and Lot No. 5 on said Plan one hundred (100) feet to the westerly corner of Lot No. 4 on said Plan; thence northeasterly along the northwesterly line of Lot No. 4 one hundred thirty-two (132) feet to a point on the southwesterly line of said Sherwood Street; thence along the southwesterly line of Sherwood Street in a northwesterly direction one hundred (100) feet to the point of beginning.

Being Lots Nos. 5 and 6 as shown on said Plan of Property belonging to the Lunt Heirs, recorded in Plan Book 3, Page 48.

Parcel Eight - So-called Lunt Lot

A parcel of land consisting of lots numbered three (3) and four (4) on the southwesterly side of Sherwood Street, formerly called Winslow Street, as delineated upon a plan entitled "Plan of Property belonging to the Lunt Heirs" and duly recorded in Cumberland County Registry of Deeds, in Plan Book 3, Page 48, to which plan and record thereof reference is hereby made for a more particular description of the premises above described.

Parcel Nine - So-called Wells Lot

A parcel of land situated on the southwesterly side of Sherwood (formerly Winslow) Street in said Portland, bounded and described as follows: beginning on said southwesterly side of said Sherwood Street at the most easterly corner of lot number 3 and the most northerly corner of lot number 2 on a plan made by John D. Bailey dated May 15, 1857 for the James Lunt heirs recorded in Cumberland County Registry of Deeds in Plan Book 3, Page 48; thence running southeasterly by said Street 87 feet to a stake standing on the most westerly line of the location of the Grand Trunk Railroad; thence southerly on the line of said Grand Trunk Railroad location 251 feet, more or less, to a stake standing on the line of land now or formerly owned by Ralph Kelley; thence northwesterly by said Kelley land 300 feet, more or less, to the most southerly corner of lot number 3 on said plan; thence northeasterly 132 feet to the point of beginning; being lots number 1 and 2 on said plan.

Parcel Ten - So-called Eastern Dredging Company Lot

A parcel of land conveyed by Charles B.. Clarke, Trustee in Bankruptcy of Eastern Dredging Company, to Burnham & Morrill Company by deed dated May 22, 1916 and recorded in said Registry in Book 971, Page 270.

Excepting from the above parcels the land taken by the State of Maine by Notices of Layout and Taking dated August 12, 1953, recorded in the Cumberland County Registry of Deeds in Book 2146, Page 400; dated April 30, 1958, recorded in the Cumberland County Registry of Deeds in Book 2407, Page 418; dated September 3, 1985, recorded in the Cumberland County Registry of Deeds in Book 6897, Page 226; by deed to State of Maine dated July 17, 1957, recorded in the Cumberland County Registry of Deeds in Book 2369, Page 116 and by Receipt and Confirmation of Taking dated September 13, 1960, recorded in the Cumberland County Registry of Deeds in Book 2563, Page 202.

The above described parcels are, in whole or in part, conveyed subject to the conditions, obligations and restrictions to which reference is made in the following instruments:

1. Rights and easements granted to Central Maine Power Company and New England Telephone and Telegraph Company as set forth in an instrument from Burnham & Morrill Company dated July 13, 1948, recorded in the Cumberland County Registry of Deeds in Book 1989, Page 88.

2. Rights and easements granted to Central Maine Power Company and New England Telephone and Telegraph Company as set forth in an instrument from Burnham & Morrill Company dated July 5, 1951 and recorded in the Cumberland County Registry of Deeds in Book 2053, Page 394.

3. Rights and easements granted to central Maine Power Company and New England Telephone and Telegraph Company as set forth in an instrument from Burnham & Morrill Company dated July 1, 1959 and recorded in the Cumberland County Registry of Deeds in Book 2491, Page 45.

4. Rights and easements granted to Central Maine Power Company and New England Telephone and Telegraph Company as set forth in an instrument from George C. Seybolt et als as Trustees under Declaration of Trust dated May 31, 1966, dated March 4, 1969 and recorded in the Cumberland County Registry of Deeds in Book 3091, Page 18.

5. Rights and easements granted to Central Maine Power Company as set forth in an instrument from Burnham & Morrill Company dated October 31, 1986 and recorded in the Cumberland County Registry of Deeds in Book 7708, Page 219.

6. Terms and conditions as set forth in deed from City of Portland to Burnham & Morrill Company dated September 16, 1940 and recorded in the Cumberland County Registry of Deeds in Book 1622, Page 134, with reference to Water Street.

7. Terms and conditions of Maintenance of Service Road as set forth in deed from State of Maine Highway Commission to Burnham & Morrill Company dated August 7, 1957, recorded in the Cumberland County Registry of Deeds in Book 2368, Page 345.

8. Slope easements as set forth in Layout & Notice of Taking by State of Maine dated April 30, 1958 and recorded in the Cumberland County Registry of Deeds in Book 2407, Page 418.

9. Rights and easements as set forth in deed from Burnham & Morrill Company to City of Portland dated May 6, 1959 and recorded in the Cumberland County Registry of Deeds in Book 2471, Page 218.

10. Terms and conditions and rights and easements granted to Portland Water District as set forth in an instrument from George C. Seybolt et als, Trustees Under Declaration of Trust dated May 31, 1966, dated October 26, 1977 and recorded in the Cumberland County Registry of Deeds in Book 4124, Page 320.

11. Terms and conditions and rights and easements granted to Portland Water District as set forth in an instrument from George C. Seybolt et als, Trustees Under Declaration of Trust dated May 31, 1966, dated December 20, 1978 and recorded in the Cumberland County Registry of Deeds in Book 4360, Page 171.

12. Controlled Access rights taken by State of Maine in Notice of Layout & Taking dated September 3, 1985 and recorded in the Cumberland County Registry of Deeds in Book 6897, Page 226.

13. Rights of Canadian National Railroad with respect to tracks traversing the property.

14. Sewer easement granted to Sarah Jordan and Isabelle Jordan Saunders by Burnham & Morrill Company as set forth in an instrument dated April 18, 1912 and recorded in the Cumberland County Registry of Deeds in Book 891, Page 256.

Being the same premises conveyed to The Pillsbury Company from William Underwood Company by deed dated November 6, 1998 and recorded in the Cumberland County Registry of Deeds in Book 14595, Page 196.

\\AWORD1\mjgenti\PillsburyCo\QuitclaimDeedFINAL.doc (03/12/99 4:39 PM)

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City of Portland Site Plan Application

If you or the property owner owes real estate or personal property taxes or user charges on any property within the City, payment arrangements must be made before permits of any kind are accepted.

Location/Address of Construction: One Bean Pot Circle, Portland ME 04103		
Total Square Footage of Proposed Structure: 1381 sq ft.	Square Footage of Lot: 13.0 acres high tide / 23.5 low tide	
Tax Assessor's Chart, Block & Lot Chart# 447 Block# A Lot# 1	Property owner, mailing address: Heritage Acquisition Corp. 426 Edge Rock Ave Roseland, NJ 07068	Telephone: 973-401-6500
Consultant/Agent, mailing address, phone & contact person: John Lotman One Bean Pot Circle Portland, ME 04103	Applicant name, mailing address & telephone: (207) 772-8341 Ext 269	**Project name:** Boiler Room

Proposed Development (check all that applies) ___New Building X Building Addition ___Change of Use
___Residential ___Office ___Retail ___Manufacturing ___Warehouse/Distribution ___Parking lot
___Subdivision, amount of lots_____
___Other:_____

Major Development_____ $500.00 **Minor Development** X $400.00

Who billing will be sent to:
Mailing address: One Bean Pot Circle
State and Zip: Portland ME 04103
Contact person: John Lotman Phone: (207) 772-8341 Ext. 224

Nine (9) separate packets must include the following:

a. copy of application
b. cover letter stating the nature of the project
c. site plan containing the information found in the attached sample plans check list

All plans must be folded neatly and in packet form

Section 14-522 of the Zoning Ordinance outlines the process, copies are available at the counter at .25 per page, you may also visit the web site: ci.portland.me.us chapter 14

I hereby certify that I am the Owner of record of the named property, or that the owner of record authorizes the proposed work and that I have been authorized by the owner to make this application as his/her authorized agent. I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in this application is issued, I certify that the Code Official's authorized representative shall have the authority to enter all areas covered by this permit at any reasonable hour to enforce the provisions of the codes applicable to this permit.

Signature of applicant: John Lotman	**Date:** 1-15-02

FEB 5

This application is for site review ONLY, a building Permit application and associated fees will be required prior to construct

1/15/02

CITY OF PORTLAND, MAINE
DEVELOPMENT REVIEW APPLICATION
PLANNING DEPARTMENT PROCESSING FORM
DRC Copy

2002-0018
Application I. D. Number

2/5/02
Application Date

Boiler Room
Project Name/Description

Totman, John
Applicant

One Bean Pot Cirlce, Portland, ME 04103
Applicant's Mailing Address

John Totman
Consultant/Agent

Applicant Ph: (207) 772-8341 **Agent Fax:**
Applicant or Agent Daytime Telephone, Fax

1 - 1 Bean Pot Cir, Portland, Maine
Address of Proposed Site

447 A001001
Assessor's Reference: Chart-Block-Lot

Proposed Development (check all that apply): ☐ New Building ☑ Building Addition ☐ Change Of Use ☐ Residential ☐ Office ☐ Retail ☐ Manufacturing ☐ Warehouse/Distribution ☐ Parking Lot ☑ Other (specify) **expand boiler room**

1381 sq. ft.	**13. Acres**	
Proposed Building square Feet or # of Units	Acreage of Site	Zoning

Check Review Required:

☑ Site Plan (major/minor)	☐ Subdivision # of lots ____	☐ PAD Review	☐ 14-403 Streets Review
☐ Flood Hazard	☐ Shoreland	☐ HistoricPreservation	☐ DEP Local Certification
☐ Zoning Conditional Use (ZBA/PB)	☐ Zoning Variance		☐ Other ____

Fees Paid: Site Plan **$400.00** Subdivision ____ Engineer Review ____ Date **1/18/02**

DRC Approval Status:

Reviewer Jay Reynolds

☒ **Approved** ☐ **Approved w/Conditions** See Attached ☐ **Denied**

Approval Date 2/14/02 Approval Expiration ____ Extension to ____ ☐ Additional Sheets Attached

☐ Condition Compliance ____ signature ____ date ____

Performance Guarantee ☐ **Required*** ☒ **Not Required**

* No building permit may be issued until a performance guarantee has been submitted as indicated below

☐ Performance Guarantee Accepted	date	amount	expiration date
☐ Inspection Fee Paid	date	amount	
☐ Building Permit Issue	date		
☐ Performance Guarantee Reduced	date	remaining balance	signature
☐ Temporary Certificate of Occupancy	date	☐ Conditions (See Attached)	expiration date
☐ Final Inspection	date	signature	
☐ Certificate Of Occupancy	date		
☐ Performance Guarantee Released	date	signature	
☐ Defect Guarantee Submitted	submitted date	amount	expiration date
☐ Defect Guarantee Released	date	signature	

April 3, 2002

Burnham & Morrill / B&G Foods Inc.
One Bean Pot Circle
Portland, ME 04103

Proposed Building Cost: $150,000

Re: Expanding the existing boiler room.

To: City of Portland

Burnham and Morrill proposes to expand and existing boiler room building to make room for new boilers at the facility. The building expansion at approximately 1361 ft^2, will provide room for two new boilers. The addition to the building will be below the existing structures and will not be visible form I-295

All utility are currently present at the facility. A new electrical service will be installed from the main switch gear room. The proposed building will used as a boiler room and will require water to be supplied.

The current federal storm water permit # MER05A620 will not be affected by the proposed building. The current area is paved, and the new roof will have similar effects on the current storm water management plan.

Sincerely,

John E. Totman
Plant Engineer

CITY OF PORTLAND MAINE

389 Congress St., Rm 315
Portland, ME 04101
Tel. – 207-874-8704
Fax – 207-874-8716

TO: Inspector of Buildings City of Portland, Maine
Planning & Urban Development
Division of Housing & Community Services

FROM DESIGNER: David Pinkham, P.E.

Pinkham & Greer Consulting Engineers

DATE: 4-5-02

Job Name: Burnham & Morrill Co. Boiler Building

Address of Construction: One Bean Pot Circle, Portland, ME

THE BOCA NATIONAL BUILDING CODE/1999 FourteenthEDITION

Construction project was designed according to the building code criteria listed below:

Building Code and Year 1999 Use Group Classification(s) F1

Type of Construction 2C Bldg. Height 27' Bldg. Sq. Footage 2160

Seismic Zone N/A for 1999 Boca Group Class C

Roof Snow Load Per Sq. Ft. 45 Dead Load Per Sq. Ft. 20

Basic Wind Speed (mph) 85 Effective Velocity Pressure Per Sq. Ft. 18.5

Floor Live Load Per Sq. Ft. N/A only floor is slab on grade

Structure has full sprinkler system? Yes x No____ Alarm System? Yes x No____
Sprinkler & Alarm systems must be installed according to BOCA and NFPA Standards with approval from the Portland Fire Department.

Is structure being considered unlimited area building: Yes X No____

If mixed use, what subsection of 313 is being considered N/A

List Occupant loading for each room or space, designed into this Project. 2

(Designers Stamp & Signature)

STATE OF MAINE
DAVID K. PINKHAM
NO. 4433
REGISTERED PROFESSIONAL ENGINEER

4.9.02

PSH 6/07/2K

CITY OF PORTLAND
BUILDING CODE CERTIFICATE
389 Congress St., Rm 315
Portland, ME 04101

TO: Inspector of Buildings City of Portland, Maine
Department of Planning & Urban Development
Division of Housing & Community Service

FROM: David K. Pinkham, P.E.

RE: Certificate of Design

DATE: 4-5-02

These plans and/or specifications covering construction work on:

Burnham & Morrill Co. Boiler Building, drawings BD1,S1,S2,S3 and specification sections 03310,05120,05210, and 05300 only.

Have been designed and drawn up by the undersigned, a Maine registered architect/engineer according to the **BOCA National Building Code/1999 Fourteenth Edition**, and local amendments.

(SEAL) STATE OF MAINE DAVID K. PINKHAM NO. 4433 REGISTERED PROFESSIONAL ENGINEER

4.9.02

Signature David K. Pinkham

Title President

Firm Pinkham & Greer Consulting Engineers

Address 170 US Route One, Falmouth, ME

As per Maine State Law:

$50,000.00 or more in new construction, repair, expansion, addition, or modification for Building or Structures, shall be prepared by a registered design Professional.

PSH 6/20/2k

City of Portland, Maine

389 Congress St., Rm 315
Portland, ME 04101

ACCESSIBILITY CERTIFICATE

TO: Inspector of Buildings City of Portland, Maine
Department of Planning & Urban Development
Division of Housing & Community Services

FROM: David K. Pinkham, P.E.

RE: Certificate of Design, HANDICAP ACCESSIBILITY

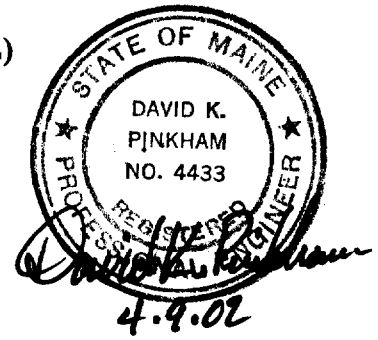
DATE: 4-5-02

These plans and/or specifications covering construction work on:

Burnahm &Morrill Co. Boiler Building, drawings BD1,S1,S2,S3 and specifiation sections 03310,05120,05210, and 05300 only.

Have been designed and drawn up by the undersigned, a Maine registered engineer/architect according to State Regulations as adopted by the State of Maine on Handicapped Accessibility.

(SEAL)



Signature David K. Pinkham

Title President

Firm Pinkham & Greer Consulting Engineers

Address 170 US Route One, Falmouth, ME

Planning & Urban Development

Joseph E. Gray Jr.
Director

CITY OF PORTLAND

February 25, 2002

Mr. John E. Totman
Burnham and Morrill Co.
One Beat Pot Circle
Portland, ME 04103

RE: B & M Plant, One Bean Pot Circle, Boiler Room Expansion Project
(Application #2002-0018, CBL 447-A--1)

Dear Mr. Totman,

On February 15, 2002 the Portland Planning Authority granted minor site plan approval with no conditions for the boiler room expansion project at the B & M Plant at One Bean Pot Circle.

The approval is based on the submitted site plan. If you need to make any modifications to the approved site plan, you must submit a revised site plan for staff review and approval.

Please note the following provisions and requirements for all site plan approvals:

1. The site plan approval will be deemed to have expired unless work in the development has commenced within one (1) year of the approval or within a time period agreed upon in writing by the City and the applicant. A one-year extension may be granted by this department if requested by the applicant in writing prior to the expiration date of the site plan.

2. Prior to construction, a pre-construction meeting shall be held at the project site with the contractor, development review coordinator, Public Work's representative and owner to review the construction schedule and critical aspects of the site work. At that time, the site/building contractor shall provide three (3) copies of a detailed construction schedule to the attending City representatives. It shall be the contractor's responsibility to arrange a mutually agreeable time for the pre-construction meeting.

3. If work will occur within the public right-of-way such as utilities, curb, sidewalk and driveway construction, a street opening permit(s) is required for your site. Please contact Carol Merritt at 874-8300, ext. 8822. (Only excavators licensed by the City of Portland are eligible.)

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CITY OF PORTLAND, MAINE
DEVELOPMENT REVIEW APPLICATION
PLANNING DEPARTMENT PROCESSING FORM
Planning Copy

2002-0018
Application I. D. Number

2/5/02
Application Date

Boiler Room
Project Name/Description

Totman, John
Applicant

One Bean Pot Cirlce, Portland, ME 04103
Applicant's Mailing Address

John Totman
Consultant/Agent 269

Applicant Ph: (207) 772-8341 **Agent Fax:**
Applicant or Agent Daytime Telephone, Fax

1 - 1 Bean Pot Cir, Portland, Maine
Address of Proposed Site

447 A001001
Assessor's Reference: Chart-Block-Lot

Proposed Development (check all that apply): ☐ New Building ☑ Building Addition ☐ Change Of Use ☐ Residential ☐ Office ☐ Retail ☐ Manufacturing ☐ Warehouse/Distribution ☐ Parking Lot ☑ Other (specify) **expand boiler room**

1381 sq. ft.	**13. Acres**	
Proposed Building square Feet or # of Units	Acreage of Site	Zoning

Check Review Required:

☑ Site Plan (major/minor) ☐ Subdivision # of lots ☐ PAD Review ☐ 14-403 Streets Review

☐ Flood Hazard ☐ Shoreland ☐ HistoricPreservation ☐ DEP Local Certification

☐ Zoning Conditional Use (ZBA/PB) ☐ Zoning Variance ☐ Other

Fees Paid: Site Plan **$400.00** Subdivision Engineer Review Date **1/18/02**

Planning Approval Status:

Reviewer Jonathan Spence

☒ **Approved** ☐ **Approved w/Conditions** See Attached ☐ **Denied**

Approval Date 2/14/02 Approval Expiration Extension to ☐ Additional Sheets Attached

☒ OK to Issue Building Permit Jonathan C. Spence signature 2/14/02 date

Performance Guarantee ☐ **Required*** ☒ **Not Required**

* No building permit may be issued until a performance guarantee has been submitted as indicated below

☐ Performance Guarantee Accepted	date	amount	expiration date
☐ Inspection Fee Paid	date	amount	
☐ Building Permit Issue	date		
☐ Performance Guarantee Reduced	date	remaining balance	signature
☐ Temporary Certificate of Occupancy	date	☐ Conditions (See Attached)	expiration date
☐ Final Inspection	date	signature	
☐ Certificate Of Occupancy	date		
☐ Performance Guarantee Released	date	signature	
☐ Defect Guarantee Submitted	submitted date	amount	expiration date
☐ Defect Guarantee Released	date	signature	

City of Portland Site Plan Application

If you or the property owner owes real estate or personal property taxes or user charges on any property within the City, payment arrangements must be made before permits of any kind are accepted.

Location/Address of Construction: One Bean Pot Circle, Portland ME 04103		
Total Square Footage of Proposed Structure: 1381 sq ft.	Square Footage of Lot: 13.0 acres high tide / 23.5 low tide	
Tax Assessor's Chart, Block & Lot Chart# 447 Block# A Lot# 1	Property owner, mailing address: Heritage Acquisition Corp. 426 Eagle Rock Ave Roseland, NJ 07068	Telephone: 973-401-6500
Consultant/Agent, mailing address, phone & contact person: John Lotman One Bean Pot Circle Portland, ME 04103	Applicant name, mailing address & telephone: (207) 772-8341 Ext 269	**Project name:** Boiler Room

Proposed Development (check all that applies) ___New Building X Building Addition ___Change of Use
___Residential ___Office ___Retail ___Manufacturing ___Warehouse/Distribution ___Parking lot
___Subdivision, amount of lots_____
___Other:_____

Major Development_____ $500.00 **Minor Development** X $400.00

Who billing will be sent to:
Mailing address: One Bean Pot Circle
State and Zip: Portland ME 04103 Contact person: John Lotman Phone: (207) 772-8341 Ext. 224

Nine (9) separate packets must include the following:

a. copy of application
b. cover letter stating the nature of the project
c. site plan containing the information found in the attached sample plans check list

All plans must be folded neatly and in packet form

Section 14-522 of the Zoning Ordinance outlines the process, copies are available at the counter at .25 per page, you may also visit the web site: ci.portland.me.us chapter 14

I hereby certify that I am the Owner of record of the named property, or that the owner of record authorizes the proposed work and that I have been authorized by the owner to make this application as his/her authorized agent. I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in this application is issued, I certify that the Code Official's authorized representative shall have the authority to enter all areas covered by this permit at any reasonable hour to enforce the provisions of the codes applicable to this permit.

Signature of applicant: John Lotman	**Date:** 1-15-02

This application is for site review ONLY, a building Permit application and associated fees will be required prior to construct

FEB 5

Site Plan Review
Written Statement

Owner: Burnham & Morrill / B&G Foods Inc.
One Bean Pot Circle
Portland, ME 04103

Proposed Building Cost: $75,000

1) Description:

The proposed project will expand the existing boiler room.

2) Total Land Area:

Total Land Area High Tide = 13.0 acres
Total Land Area Low Tide =23.5 acres
Total Floor Area = 1381 ft^2
Side Ground Cover = 17' x 60' = 1020 ft^2
Front Ground Cover = 19' x 19' = 361 ft^2
Total Ground Cover = 1381 ft^2

3) General Summary:

Burnham and Morrill proposes to expand and existing boiler room building to make room for new boilers at the facility. The building expansion at approximately 1361 ft^2, will provide room for two new boilers. The addition to the building will be below the existing structures and will not be visible form I-295

4) Solid Waste:

Plastic will be recycled by William Goodman & Sons, Scarborough, ME
Paper will be recycled by William Goodman & Sons, Scarborough, ME
Corrugated will be recycled by William Goodman & Sons, Scarborough, ME
Light Bulbs will be recycled by Safety Kleen, Leeds, ME

5) Off-Site Utilities:

All utility are currently present at the facility. A new electrical service will be installed from the main switch gear room. The proposed building will used as a boiler room and will require water but not floor drains.

6) Storm Water:

The current federal storm water permit # MER05A620 will not be affected by the proposed building. The current area is paved, and the new roof will have similar effects on the current storm water management plan.

7) Construction Plan:

1) The proposed building will be started Feb 15, 2002.
2) Excavation of area for foundation work will take place. Due to the area erosion should not be a factor, but if it becomes a problem erosion barriers can be implemented to control.
3) Construct roof and end walls.
4) Project completion by August 31, 2002

8) State and Federal Regulatory Approvals:

1) Federal Waste Discharge NPDES Permit # ME001741 will not be affected by the proposed building.

2) State of Maine Waste Discharge Permit # W000980-5R-C-R will not be affected by the proposed building.

9) Evidence of financial and technical capacity. (See Attachment)

10) Evidence of the applicant's title, right, or interest in the property, including without limited deeds lease, purchase option or any other documentation; (See Enclosed drawing)

11) There are currently no unusual natural areas, wildlife, and fisheries habitats, or archaeological sites located on or near the project site.

BOILER BUILDING

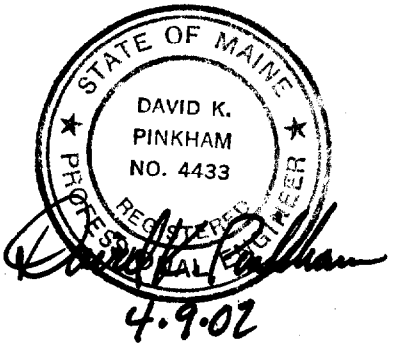
STRUCTURAL SPECIFICATIONS

BURNHAM & MORRILL CO.
PORTLAND, MAINE

April 5, 2002

Pinkham & Greer Consulting Engineers
170 US Route One
Falmouth, ME 04105

(207) 781-5242



SECTION 03310
CONCRETE WORK

PART 1 - GENERAL

1.01 DESCRIPTION OF WORK:

A. Provide all cast-in-place concrete work.

1.02 QUALITY ASSURANCE:

A. Codes and Standards: Comply with provisions of following codes, specifications, and standards, except where more stringent requirements are shown or specified.

- ACI 301 "Specifications for Structural Concrete for Buildings"
-
- ACI 318 "Building Code Requirements for Reinforced Concrete"
- Concrete Reinforcing Steel Institute, "Manual of Standard Practice"

B. Testing and Services by Contractor: Performed by an approved testing laboratory at the Contractor's expense:

- Material Evaluation Tests
- Concrete Mix Designs
- Tests not specifically indicated to be done at Owner's expense, including re-testing of rejected materials and installed work.

Furnish equipment, including buckets, shovels, and wheelbarrows, for proper sampling of concrete mix, molds for compression test specimens, and facilities for storing and curing specimens at the job site, and labor to assist technician performing field tests.

C. Materials and installed work may require testing and re-testing at any time during the progress of the work as directed by the Engineer. Allow free access to material stockpiles and facilities. These tests will be done by an independent approved laboratory at the Contractor's expense.

D. Testing by Owner: Field tests will be by the Engineer's representative or by an independent testing laboratory. Tests may be done for slump, air content, and concrete temperature, and compression test specimens will be taken. See Part 3 - Execution.

E. Field Testing for Small Placements: For placements of 15 cubic yards or less and for critical placements, variation from the ASTM requirement for sampling the middle portion of the batch for testing will be required. This is to prevent the first portion of a batch of defective concrete from being used in a small placement. Prior to small placements, meet with Engineer and determine an acceptable testing procedure based on the first portion of each batch.

1.03 SUBMITTALS:

A. Product Data: Submit manufacturer's product data with application and installation instructions for proprietary materials and items, including reinforcing accessories, admixtures, patching compounds, waterstops, joint systems, curing compounds, dry-shake finish materials, and others as requested by Engineer.

B. Shop Drawings; Reinforcement: Submit shop drawings for fabrication, bending, and placement of concrete reinforcement. Comply with ACI 315 "Manual of Standard Practice for Detailing Reinforced Concrete Structures," showing bar schedules, stirrup spacing, diagrams of bent bars, and arrangement of concrete reinforcement. Include special reinforcement required and openings through concrete structures.

C. Samples: Submit samples of materials as specified and as otherwise requested by Engineer, including names, sources, and descriptions.

D. Laboratory Test Reports and Mix Designs: Submit laboratory test reports for concrete materials and mix design test as specified.

E. Material Certificates: Provide materials certificates in lieu of materials laboratory test reports when permitted by Engineer. Material certificates shall be signed by manufacturer and Contractor, certifying that each material item complies with, or exceeds, specified requirements.

PART 2 - PRODUCTS

2.01 FORM MATERIALS:

A. Forms for Exposed Finish Concrete: Unless otherwise indicated, construct formwork for exposed concrete surfaces with plywood, metal, metal-framed plywood faced, or other acceptable panel-type materials, to provide continuous, straight, smooth, exposed surfaces. Furnish in largest practicable sizes to minimize number of joints and to conform to joint system shown on drawings. Provide form material with sufficient stiffness to withstand pressure of newly-placed concrete without bow or deflection.

B. Forms for Unexposed Finish Concrete: Form concrete surfaces which will be unexposed in finished structure with plywood, lumber, metal, or other acceptable material.

C. Form Coatings: Provide commercial formulation form-coating compounds that will not bond with, stain, nor adversely affect concrete surfaces, and will not impair subsequent treatments of concrete surfaces.

D. Form Ties: Factory-fabricated, adjustable-length, removable, or snap-off metal form ties with waterstops, designed to prevent form deflection, and to prevent spalling concrete surfaces upon removal.

Unless otherwise indicated, provide ties so portion remaining within concrete after removal is at least 1" inside concrete.

Unless otherwise shown, provide form ties, which will not leave holes larger than 1" diameter in concrete surface.

2.02 REINFORCING MATERIALS:

A. Reinforcing Bars: ANSI/ASTM A615, Grade 60, deformed.

B. Tolerances for fabrication are listed in a table at the end of this section.

C. Welded Wire Fabric (WWF): ANSI/ASTM A 185, welded steel wire fabric, flat sheets only.

D. Supports for Reinforcement: Provide supports for reinforcement, including bolsters, chairs, spacers, and other devices for spacing, supporting, and fastening reinforcing bars and welded wire fabric in place. Use wire bar type supports complying with CRSI recommendations, unless otherwise acceptable.

For slabs-on-grade, use continuous high chair upper supports with appropriate sand plates or horizontal runners which will not damage vapor barrier or where base material will not support chair legs. Do not use concrete bricks, concrete blocks, or plastic supports.

2.03 CONCRETE MATERIALS:

A. Portland Cement: ANSI/ASTM C 150, Type I or Type II, unless otherwise acceptable to Engineer.

Use one brand of cement throughout project, unless otherwise acceptable to Engineer.

B. Normal Weight Aggregates: ANSI/ASTM C 33, and as herein specified. Provide aggregates from a single source for exposed concrete.

Local aggregates not complying with ANSI/ASTM C33, but which have shown by special test or actual service to produce concrete of adequate strength and durability, may be used when acceptable to the Engineer.

C. Water: Potable.

D. Air-Entraining Admixture: ANSI/ASTM C 260.

E. Water-Reducing Admixture (use at Contractor's option): ANSI/ASTM C 494, Type A, and contain not more than 1% chloride ions.

F. High-Range Water-Reducing Admixture (Super Plasticizer) (use at Contractor's option except for slabs): ASTM C 494, Type F or Type G and contain not more than 1% chloride ions.

G. Water-Reducing, Accelerator Admixture: ASTM C 494, Type C or E.

H. Calcium chloride not permitted.

2.04 RELATED MATERIALS:

A. Non-Shrink Grout: CRD-C 621, Type D, non-metallic factory pre-mixed grout.

B. Moisture-Retaining Cover: One of the following, complying with ANSI/ASTM C 171:
For slabs: Standard Grade Wet Strength Curing Paper equivalent to Hydramat by Century Floors.
For other concrete: Polyethylene Film or Polyethylene-Coated Burlap

C. Liquid Membrane-Forming Curing Compound: Liquid type membrane-forming curing compound complying with ANSI/ASTM C 309, Type I, unless other type acceptable to Engineer. CURING COMPOUND IS NOT ACCEPTABLE FOR CURING FLOOR SLABS.

D. Bonding Compound: Acrylic emulsion, non-re-wettable type, equivalent to "Everbond" by L & M Chemicals, Inc.

E. Joint Sealant: Polyurethane-based one-part elastomeric sealant, complying with FS TT-S-00230, Class A, Type I (self-leveling) or Type II, as recommended by manufacturer for application shown.

F. Joint Primer/Sealer: As recommended by sealant manufacturer for joint surfaces to be primed or sealed.

G. Bond Breaker Tape: Polyethylene tape or other plastic tape as recommended by sealant manufacturer to be applied to sealant-contact surfaces where bond to substrate or joint filler must be avoided for proper performance of sealant. Provide self-adhesive tape where applicable.

H. Sealant Backer Rod: Compressible polyethylene foam, polyethylene jacketed polyurethane foam, butyl rubber foam, neoprene foam, or other flexible, permanent, durable, non-absorptive material as recommended by sealant manufacturer for compatibility with sealant.

I. Joint Filler: 1.5 LB density polyethylene foam with removable cap for sealant reservoir.

J. Bond Breaker for Joints: Equivalent to Thompsons Water Seal

2.05 PROPORTIONING AND DESIGN OF MIXES:

A. Prepare design mixes for each type and strength of concrete by either laboratory trial batch or field experience methods as specified in ACI 301. If trial batch method is used, use an independent testing facility acceptable to Engineer for preparing and reporting proposed mix designs. The testing facility shall not be the same as used for field quality control testing unless otherwise acceptable to Engineer.

B. Submit written reports to Engineer of each proposed mix for each class of concrete at least 15 days prior to start of work. Do not begin concrete production until mixes have been reviewed by Engineer. Include the following in mix design submittals:

- Identification of aggregate source of supply.
- Results of compliance tests for aggregates.
- Scale weights of each aggregate.

- Absorbed water in each aggregate.
- Brand, type, and amount of each cement and each admixture.
- Proportions of each material required per cubic yard.

C. Design mixes to provide normal weight concrete with the following properties:

- Min. 28-Day Compressive Strength: 4000 psi for slabs on grade
  3000 psi for other concrete
- Max. Water/Cement Ratio: 0.50
- Min. Cement Content: 564 lbs per cubic yard for all slabs
  470 lbs per cubic yard for other concrete
- Slump:
  Concrete for General Use: 4" ± 1"
  Concrete with HRWR Admixture: not more than 8"
- Max. Aggregate Size: 3/4"
- Air Content: 6% ± 1% by volume for exterior concrete
  2% maximum entrapped air only for interior slabs, no entrained air is permitted.

D. Adjustment to Concrete Mixes: Mix design adjustments may be requested by Contractor when characteristics of materials, job conditions, test results, weather, or other circumstances warrant, at no additional cost to Owner and as accepted by Engineer. Laboratory test data for revised mix design and strength results must be
submitted to and accepted by Engineer before using in work.

E. Admixtures: Comply with manufacturer's instructions for use of admixtures.

F. Use water-reducing admixture, high range water-reducing admixture, (HRWR) or accelerating admixture at Contractor's option. HRWR is not permitted in slabs.

2.06 CONCRETE MIXES:

A. Ready-Mix Concrete: Comply with requirements of ANSI/ASTM C 94, and as herein specified. Do not exceed total mixing and delivery time of 1-1/2 hours.

Water may be added for re-tempering provided maximum permissible slump and maximum water-cement ratio is not exceeded. Do not make additions without notifying the Engineer. Additional field tests and compressive test specimens may be required.

During hot weather, or under conditions contributing to rapid setting of concrete, a shorter mixing time than specified in ANSI/ASTM C 94 may be required.

When air temperature is between 85° F (30° C) and 90° F (32° C), reduce mixing and delivery time from 1-1/2 hours to 75 minutes and, when air temperature is above 90° F (32° C), reduce mixing and delivery time to 60 minutes.

PART 3 - EXECUTION

3.01 FORMS:

A. Design, erect, support, brace, and maintain formwork to support vertical and lateral loads that might be applied until such loads can be supported by concrete structure. Construct formwork so concrete members and structures are of correct size, shape, alignment, elevation, and position, and have correct finish.

B. Tolerances for formed surfaces are listed in a table at the end of this section.

C. Design formwork to be readily removable without impact, shock, or damage to cast-in-place concrete surfaces and adjacent materials.

D. Fabricate forms for easy removal without hammering or prying against concrete surfaces. Provide crush plates or wrecking plates where stripping may damage cast concrete surfaces. Provide top forms for inclined surfaces where slope is too steep to place concrete with bottom forms only. Kerf wood inserts for forming keyways, reglets, and recesses for easy removal.

E. Provide temporary openings where interior area of formwork is inaccessible for cleanout, for inspection before concrete placement, and for placement of concrete. Securely brace temporary openings and set tight-openings on forms at inconspicuous locations.

F. Chamfer exposed corners and edges as indicated, using wood, metal, PVC, or rubber chamfer strips fabricated to produce uniform smooth lines and tight edge joints.

G. Provisions for Other Trades: Provide openings in concrete formwork to accommodate work of other trades. Determine size and location of openings, recesses, and chases from trades providing such items. Accurately place and securely support items built into forms.

H. Cleaning and Tightening: Thoroughly clean forms and adjacent surfaces to receive concrete. Remove chips, wood, sawdust, dirt, or other debris just before concrete is placed. Retighten forms and bracing after concrete placement is required to eliminate mortar leaks and maintain proper alignment.

3.02 PLACING REINFORCEMENT:

A. Comply with Concrete Reinforcing Steel Institute's recommended practice for "Placing Reinforcing Bars" for details and methods of reinforcement placement and supports, and as herein specified.

B. Clean reinforcement of loose rust and mill scale, earth, ice, and other materials, which reduce or destroy bond with concrete.

C. Accurately position, support, and secure reinforcement against displacement by formwork, construction, or concrete placement operations. Locate and support reinforcing by metal chairs, runners, bolsters, spacers, and hangers, as required.

D. Tolerances for placing reinforcement are given in Table 3 at the end of this section.

E. Place reinforcement to obtain at least minimum coverages for concrete protection. Arrange, space, and securely tie bars and bar supports to hold reinforcement in position during concrete placement operations. Set wire ties so ends are directed into concrete, not toward exposed concrete surfaces.

F. Install welded wire fabric in lengths as long as practicable. Lap adjoining pieces at least one full mesh and lace splices with wire. Offset end laps in adjacent widths to prevent continuous laps in either direction.

G. Support reinforcement for slabs-on-grade with continuous high chairs placed 3' o.c.

3.03 JOINTS IN WALLS:

A. Construction Joints: Locate and install construction joints, which are not shown on drawings, so as not to impair strength and appearance of the structure, as acceptable to Engineer.

B. Provide keyways at least 1-1/2" deep in construction joints in walls and slabs and between walls and footings unless shown otherwise on the drawings.

C. Place construction joints perpendicular to the main reinforcement. Continue reinforcement across construction joints.

D. Control joints: Locate control joints in walls 30' o.c. max. at mid point in wall between column piers.

E. Sealant Installation: Comply with manufacturer's printed instructions.

3.04 JOINTS IN SLABS-ON-GRADE:

A. Construction joints in slabs: Required construction joint locations are shown on the Drawings. Other joint locations maybe construction or control joints as required by placement sequencing and operations. Continue reinforcement across construction joints.

B. Apply bond breaker material to face of previous placement concrete at construction joints.

C. Control joints in slabs: Locate as shown on the Drawings. Continue reinforcement across joints.

D. Sawcut control joints as soon as possible after finishing, without raveling joints using soft-cut saws. Sawcut joints at column lines first. Install a new skid for soft-cut saws when a new saw blade is installed.

3.05 INSTALLATION OF EMBEDDED ITEMS:

A. General: Set and build into work anchorage devices and other embedded items required for other work that is attached to, or supported by, cast-in-place concrete. Use setting drawings, diagrams, instructions, and directions provided by suppliers of these items.

B. Edge Forms and Screed Strips for Slabs: Set edge forms or bulkheads and intermediate screed strips for slabs to obtain required elevations and contours in finished slab surface. Provide and secure units sufficiently strong to support types of screed strips by use of strike-off templates or accepted compacting type screeds.

3.06 PREPARATION OF FORM SURFACES:

A. Coat contact surfaces of forms with a form-coating compound before reinforcement is placed.

Thin form-coating compounds only with thinning agent of type, and in amount, and under conditions of form-coating compound manufacturer's directions. Do not allow excess form coating material to accumulate in forms or to come into contact with concrete surfaces against which fresh concrete will be placed. Apply in compliance with manufacturer's instructions.

Coat steel forms with a non-staining, rust-preventative form oil or otherwise protect against rusting. Rust- stained steel formwork is not acceptable.

3.07 CONCRETE PLACEMENT:

A. General: Comply with ACI 304, and these specifications. Before placing concrete, inspect and complete formwork installation, reinforcing steel, and items to be embedded or cast in. Notify other crafts to permit installation of their work; cooperate with other trades in setting such work. Moisten wood forms immediately before placing concrete where form coatings are not used. Coordinate the installation of joint materials and moisture barriers with placement of forms and reinforcing steel.

B. Deposit concrete continuously or in layers of such thickness that no concrete will be placed on concrete which has hardened sufficiently to cause the formation of seams or planes of weakness. If a section cannot be placed continuously, provide construction joints as herein specified. Deposit concrete as nearly as practicable to its final location to avoid segregation.

C. Placing Concrete in Forms: Deposit concrete in forms in horizontal layers not deeper than 24" and in a manner to avoid inclined construction joints. Where placement consists of several layers, place each layer while preceding layer is still plastic to avoid cold joints.

Do not exceed 5' free fall of concrete without approval of Engineer.

D. Consolidate placed concrete by mechanical vibrating equipment supplemented by hand-spading, rodding, or tamping. Use equipment and procedures for consolidation of concrete in accordance with ACI recommended practices.

E. Do not use vibrators to transport concrete inside forms. Insert and withdraw vibrators vertically at uniformly spaced locations not farther than visible effectiveness of machine. Place vibrators to rapidly penetrate placed layer and at least 6" into preceding layer. Do not insert vibrators into lower layers of concrete that have begun to set. At each insertion, limit duration of vibrator to time necessary to consolidate concrete and complete embedment of reinforcement and other embedded items without causing segregation of mix.

F. Remove all frost from slab base. Do not place concrete if temperature difference between base material and air is more than 20° F.

G. Maintain reinforcing in proper position during concrete placement operations.

H. Cold Weather Placing: Comply with ACI 306. Protect concrete work from physical damage or reduced strength, which could be caused by frost, freezing actions, or low temperatures. When air

temperature has fallen to or is expected to fall below 40° F (4° C), obtain a concrete mixture temperature of not less than 50° F (10° C), and not more than 80° F (27° C) at point of placement.

Do not use frozen materials or materials containing ice or snow. Do not place concrete on frozen subgrade or on subgrade containing frozen materials.

Do not use calcium chloride, salt, or other materials containing antifreeze agents or chemical accelerators, unless otherwise accepted in mix designs.

I. Hot Weather Placing: Comply with ACI 305 when hot weather conditions exist that would impair quality and strength of concrete.

Maintain concrete temperature at time of placement below 90° F (32° C). Mixing water may be chilled or chopped ice may be used to control temperature provided water equivalent of ice is calculated to total amount of mixing.

Cover reinforcing steel with water-soaked burlap if it becomes too hot so that steel temperature will not exceed the ambient air temperature immediately before embedment in concrete.

Wet forms thoroughly before placing concrete.

Use water-reducing retarding admixture (Type D) when required by high temperatures, low humidity, or other adverse placing conditions.

3.08 FINISH OF FORMED SURFACES:

A. Rough Form Finish: For formed concrete surfaces not exposed-to-view in the finish work or by other construction, unless otherwise indicated. This is the concrete surface having texture imparted by form facing material used, with tie holes and defective areas repaired and patched and fins and other projections exceeding 1/4" in height rubbed down or chipped off.

B. Smooth Form Finish: For formed concrete surfaces exposed-to-view or that are to be covered with a coating material applied directly to concrete, or a covering material applied directly to concrete, such as waterproofing, damp-proofing, painting, or other similar system. This is as-cast concrete surface obtained with selected form facing material, arranged orderly and symmetrically with a minimum of seams. Repair and patch defective areas with fins or other projections completely removed and smoothed.

C. Related Unformed Surfaces: At tops of walls, horizontal offsets surfaces occurring adjacent to formed surfaces, strike-off smooth and finish with a texture matching adjacent formed surfaces. Continue final surface treatment of formed surfaces uniformly across adjacent unformed surfaces, unless otherwise indicated.

3.09 MONOLITHIC SLAB FINISHES:

A. Float Finish: Apply float finish to all monolithic slab surfaces. After screeding, consolidating, and leveling concrete slabs, do not work surface until ready for floating. Begin floating when surface water has disappeared or when concrete has stiffened sufficiently to permit operation of power-driven floats, or both. Consolidate surface with power-driven floats, or by hand-floating if area is

small or inaccessible to power units. Cut down high spots and fill low spots. Uniformly slope surfaces to drains. Immediately after leveling, re-float surface to a uniform, smooth granular texture.

B. Trowel Finish: Apply a smooth, tight trowel finish to monolithic slab surfaces to be exposed-to-view, and slab surfaces to be covered with resilient flooring, paint or other thin film finish coating system. A burnished finish is not desired.

Use operations to produce a concrete surface free of trowel marks, uniform in texture and appearance, and with a surface plane tolerance not exceeding 1/8" in 10' when tested with a 10' straightedge. Grind smooth surface defects which would telegraph through applied floor covering system.

C. Non-Slip Broom Finish: Apply non-slip broom finish to exterior concrete platforms, steps and ramps, and elsewhere as indicated.

Immediately after trowel finishing, slightly roughen concrete surface by brooming with fiber bristle broom perpendicular to main traffic route. Coordinate required final finish with Architect before application.

3.10 CONCRETE CURING AND PROTECTION:

A. General: comply with ACI 308. Protect freshly placed concrete from premature drying and excessive cold or hot temperatures. Start curing as soon as free water has disappeared from concrete surface after placing and finishing. Continue curing for at least 7 days in accordance with ACI procedures. Avoid rapid drying at end of final curing period.

B. Curing Methods: Perform curing of concrete **except for slabs on grade or structural slabs** by moisture curing, by moisture-retaining cover curing, by curing compound and/or combinations of methods.

1. Provide moisture curing by the following methods:

- Keep concrete surface continuously wet by covering with water.

- Continuous water-fog spray.

- Covering concrete surface with specified absorptive cover, thoroughly saturating cover with water and keeping continuously wet. Place absorptive cover to provide coverage of concrete surfaces and edges, with 4" lap over adjacent absorptive covers.

2. Provide moisture-retaining cover curing as follows:

Cover concrete surfaces with moisture-retaining cover for curing concrete, placed in widest practicable width, with sides and ends lapped at least 4" and sealed by waterproof tape or adhesive. Immediately repair any holes or tears during curing period using cover material and waterproof tape.

3. Curing compound methods are not acceptable for slabs-on-grade or structural slabs. For surfaces where this method is acceptable, follow manufacturer's instructions for application methods and rate of application.

C. Curing Formed Surfaces: Cure formed concrete surfaces, including undersides of beams, supported slabs, and other similar surfaces by moist curing with forms in place for full curing period or until forms are removed. If forms are removed, continue curing by methods specified above, as applicable.

D. Curing Unformed Surfaces: Cure unformed surfaces, other than slabs, using curing methods specified above.

E. Curing Slabs-on-Grade and Structural Slabs:

Provide 7 day wet cure by covering slab with dry, wet strength curing paper placed in widest practical widths, with all seams lapped 4" and sealed with waterproof tape. Immediately repair any holes or tears during curing period using cover material and waterproof tape. Do not add additional water for curing. Limit construction traffic to foot traffic for 3 days. Do not allow lifts on floor during curing period. Do not apply curing compound.

3.11 REMOVAL OF FORMS:

A. Formwork not supporting weight of concrete, such as sides of beams, walls, columns, and similar parts of the work, may be removed after cumulatively curing at not less than 50° F (10° C) for 24 hours after placing concrete, provided concrete is sufficiently hard to not be damaged by form removal operations, and provided curing and protection operations are maintained.

B. Formwork supporting weight of concrete, such as beam soffits, joints, slabs, and other structural elements, may not be removed in less than 14 days and until concrete has attained design minimum compressive strength at 28 days. Determine potential compressive strength of in-place concrete by testing field-cured specimens representative of concrete location or members.

3.12 MISCELLANEOUS CONCRETE ITEMS:

A. Filling-In: Fill-in holes and openings left in concrete structures for passage of work by other trades, unless otherwise shown or directed, after work of other trades is in place. Mix, place, and cure concrete as herein specified, to blend with in-place construction. Provide other miscellaneous concrete filling shown or required to complete work.

3.13 CONCRETE SURFACE REPAIRS:

A. Patching Defective Areas: Repair and patch defective areas with cement mortar immediately after removal of forms, when acceptable to Engineer.

Cut out honeycomb, rock pockets, voids over 1/4" in any dimension, and holes left by tie rods and bolts, down to solid concrete, but in no case to a depth of less than 1". Make edges of cuts perpendicular to the concrete surface. Thoroughly clean, dampen with water, and brush coat the area to be patched with specified bonding agent. Place patching mortar after bonding compound has dried.

B. For exposed-to-view surfaces, blend white Portland cement and standard Portland cement so that, when dry, patching mortar will match color surrounding. Provide test areas at inconspicuous location to verify mixture and color match before proceeding with patching. Compact mortar in place and strike off slightly higher than surrounding surface.

C. Repair of Formed Surfaces: Remove and replace concrete having defective surfaces if defects cannot be repaired to satisfaction of Engineer. Surface defects, as such, include color and texture irregularities, cracks, spalls, air bubbles, honeycomb, rock pockets, fins and other projections on surface, and stains and other discolorations that cannot be removed by cleaning. Flush out form tie holes, fill with dry pack mortar, or precast cement cone plugs secured in place with bonding agent.

D. Repair concealed formed surfaces, where possible, that contain defects that affect the durability of concrete. If defects cannot be repaired, remove and replace concrete.

E. Repair of Unformed Surfaces: Test unformed surfaces, such as monolithic slabs, for smoothness and verify surface plane to tolerances specified for each surface and finish. Correct low and high areas as herein specified. Test unformed surfaces sloped to drain for trueness of slope, in addition to smoothness, using a template having required slope.

Repair finished, unformed surfaces that contain defects, which affect durability of concrete. Surface defects, as such, include crazing, cracks in excess of 0.01" wide or which penetrate to reinforcement or completely through non-reinforced sections regardless of width, spalling, pop-outs, honeycomb, rock pockets, and other objectionable conditions.

Correct high areas in unformed surfaces by grinding, after concrete has cured at least 14 days.

Correct low areas in unformed surfaces during, or immediately after, completion of surface finishing operations by cutting out low areas and replacing with fresh concrete. Finish repaired areas to blend into adjacent concrete. Proprietary patching compounds may be used when acceptable to Engineer.

F. Repair defective areas, except random cracks and single holes not exceeding 1" diameter, by cutting out and replacing with fresh concrete. Remove defective areas to sound concrete with clean, square cuts and expose reinforcing steel with at least 3/4" clearance all around. Dampen concrete surfaces in contact with patching concrete and apply bonding compound. Mix patching concrete of same materials to provide concrete of same type or class as original concrete. Place, compact, and finish to blend with adjacent finished concrete. Cure in same manner as adjacent concrete.

G. Repair isolated random cracks and single holes not over 1" in diameter by dry-pack method. Groove top of cracks and cut-out holes to sound concrete and clean of dust, dirt, and loose particles. Dampen cleaned concrete surfaces and apply bonding compound. Mix dry-pack, consisting of one part Portland cement to 2-1/2 parts fine aggregate passing a No. 16 mesh sieve, using only enough water as required for handling and placing. Place dry pack after bonding compound has dried.

Compact dry-pack mixture in place and finish to match adjacent concrete. Keep patched area continuously moist for not less than 72 hours.

Use epoxy-based mortar for structural repairs, where directed by Engineer.

H. Repair cracks in floor slabs, which form due to shrinkage occurring prior to cutting of control joints using epoxy injection.

I. Repair methods not specified above may be used, subject to acceptance of Engineer.

3.14 QUALITY CONTROL TESTING DURING CONSTRUCTION:

A. The Owner will employ a testing laboratory to perform field tests and to submit test reports.

B. Sampling and testing for quality control during placement of concrete may include the following, as directed by Engineer. Tests will be made for each concrete load and for each set of compression test specimens or as often as required to ensure compliance with the specifications.

- Sampling Fresh Concrete: ASTM C 172, except modified for slump to comply with ASTM C 94.
- Slump: ASTM C 143, at point of discharge.
- Air Content: ASTM C 173, volumetric method for lightweight or normal weight concrete; ASTM C 231 pressure for normal weight concrete.
- Concrete Temperature: Test hourly when air temperature is 40° F (4° C) and below, and when 80° F (27° C) and above.
- Compression Test Specimen: ASTM C 31; one set of 3 standard cylinders for each compressive strength test unless field-cured cylinders are required. Mold and store cylinders for laboratory cured test specimens except when field-cure test specimens are required.
- Compressive Strength Tests: ASTM C 39; 1 specimen tested at 7 days, 2 specimens tested at 28 days.

When frequency of testing will provide less than 5 strength tests for a given class of concrete, conduct testing from at least 5 randomly selected batches or from each batch if fewer than 5 are used.

When strength of field-cured cylinders is less than 85% of companion laboratory-cured cylinders, evaluate current operations and provide corrective procedures for protecting and curing the in-place concrete.

Concrete is satisfactory if averages of sets of three consecutive strength test results equal or exceed specified compressive strength, and no individual strength test result falls below specified compressive strength by more than 500 psi.

C. Test results will be reported in writing to Engineer and Contractor on same day that tests are made. Reports of compressive strength tests shall contain the project identification name and, date of concrete placement, name of concrete testing service, concrete type and class, location of concrete batch in structure, design compressive strength at 28 days, concrete mix proportions and materials, compressive breaking strength, and type of break for both 7-day tests and 28-day tests.

D. Additional Tests: The testing service will make additional tests of in-place concrete when test results indicate that specified concrete strengths and other characteristics have not been attained in

the structure, as directed by Engineer. Testing service may conduct tests to determine adequacy of concrete by cored cylinders complying with ASTM C 42, or by other methods as stated in ACI 301. Contractor shall pay for such tests conducted, and any other additional testing as may be required, when unacceptable concrete is verified.

3.15 DEFECTIVE WORK ACCEPTANCE AND REMEDIES:

A. Defective Work: Any work which fails to comply with the requirements of these specifications or does not comply with the acceptance requirements of Chapters 17 and 18 of ACI 301.

B. Remedies: Work, which may be repaired to comply with these specifications using approved repair methods may be accepted. All repairs are at the Contractor's expense.

C. Remove and replace work, which cannot be repaired or strengthened with, approved methods. Removal and replacement are at the Contractor's expense.

D. Inadequate Concrete Strength: If test results show inadequate concrete strength, the following may be required at the Contractor's expense:

- Additional curing of areas with inadequate concrete.
- Modifications to mix designs for remaining work.
- Changes in member size or reinforcing for remaining work.

TABLE 1

TOLERANCES FOR FORMED SURFACES

1. Variation from plumb:

   A. In the lines and surfaces of columns, piers, walls, and in arrises:
   In any 10-foot length.....1/4 in.
   Maximum for the entire length.....1 in.

   B. For exposed corner columns, control-joint grooves, and other conspicuous lines:
   In any 20-foot length.....1/4 in.
   Maximum for the entire length.....1/2 in.

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2. Variation from the level or from the grades specified in the contract documents:

   A. In slab soffits, ceilings, beam soffits, and in arrises, measured before removal of supporting shores
   In any 10-foot length.....1/4 in.
   In any bay or in any 20-foot length.....3/8 in.
   Maximum for the entire length.....3/4 in.

   B. In exposed lintels, sills, parapets, horizontal grooves, and other conspicuous lines:
   In any bay or in 20-foot length.....1/4 in.
   Maximum for the entire length.....1/2 in.

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3. Variation of the linear building line from established position in plan and related position of columns, walls, and partitions:
   In any bay.....1/2 in.
   In any 20-foot length.....1/2 in.
   Maximum for the entire length..... 1 in.

TABLE 1 (cont.)

TOLERANCES FOR FORMED SURFACES

4. Variation in the sizes and location of sleeves, floor openings, and wall openings.....±1/4 in.

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5. Variation in cross-sectional dimensions of columns and beams and in the thickness of slabs and walls:
   Minus.....1/4 in.
   Plus.....1/2 in.

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6. Footings*
   A. Variations in dimensions in plan:
      Minus.....1/2 in.
      Plus..... 2 in.
   B. Misplacement or eccentricity:
      2 percent of the footing width in the direction of misplacement but not more than..... 2 in.
   C. Thickness:
      Decrease in specified thickness..... 5 %
      Increase in specified thickness.....No limit

---

7. Variation in steps:
   A. In a flight of stairs:
      Rise..... ±1/16 in.
      Tread.....± 1/8 in.

---

*Tolerances apply to concrete dimensions only, not to positioning of vertical reinforcing steel, dowels, or embedded items.

TABLE 2

TOLERANCES FOR FABRICATING REINFORCEMENT

Sheared length ..... ± 1 in.
Depth of truss bars .....+ 0 - 1/2 in.
Overall dimensions of stirrups, ties, & spirals ... ± 1/2 in.
All other bends ..... ± 1 in.

TABLE 3

TOLERANCES FOR PLACING REINFORCEMENT

Clear distance to formed surfaces..... ± 1/4 in.
Minimum spacing between bars..... - 1/4 in.
Top bars in slabs and beams:
Members 8 inches deep or less.....± 1/4 in.
Members more than 8 inches
but not over 2 feet deep..... ± 1/2 in.
Members more than 2 feet deep.....1 in.
Crosswise of members....spaced evenly within 2 in.
Lengthwise of members..... ± 2 in.

* END OF SECTION 03310 *

SECTION 05120

STRUCTURAL STEEL

PART 1 - GENERAL

1.01 DESCRIPTION OF WORK:

A. Provide all structural steel as shown on the Drawings.

Structural steel: Work defined in AISC "Code of Standard Practice".

1.02 QUALITY ASSURANCE:

A. Comply with the following, except as otherwise indicated.

AISC "Code of Standard Practice for Steel Buildings and Bridges".

AISC "Specifications for the Design, Fabrication, and Erection of Structural Steel for Buildings", including "Commentary" of Supplements thereto as issued.

AISC "Specifications for Structural Joints using ASTM A 325 or A 490 Bolts" approved by the Research Council on Structural Connections (RCSC) of the Engineering Foundation.

AWS D1.1 "Structural Welding Code".

ASTM A6 "General Requirements for Delivery of Rolled Steel Plates, Shapes, Sheet Piling and Bars for Structural Use".

B. Fabricator Qualifications: 10 years experience in fabrication of building types listed in AISC Category I.

C. Qualifications for Welding Work: Qualify welding processes and welding operators in accordance with AWS "Standard Qualification Procedure".

Provide certification that welders to be employed in work have satisfactorily passed AWS qualification tests.

If recertification of welders is required, retesting will be Contractor's responsibility.

1.03 SUBMITTALS:

A. Shop Drawings: Submit shop drawings including complete details and schedules for fabrication and assembly of structural steel members procedures and diagrams.

Provide setting drawings, templates, and directions for installation of anchor bolts and other anchorages to be installed by others.

B. Welder Qualifications: Submit certification required in paragraph 1.02 C.

C. Welding Procedures: Submit written welding procedures for all field welding prior to start of any field welding.

1.04 DELIVERY, STORAGE AND HANDLING:

A. Deliver anchor bolts and anchorage devices, which are to be embedded in cast-in-place concrete or masonry, in ample time to not delay work.

B. Store materials to permit easy access for inspection and identification. Keep steel members off ground. Protect steel members and packaged materials from deterioration.

Do not store materials on structure in a manner that might cause distortion or damage to members or supporting structures. Repair or replace damaged materials or structures as directed.

PART 2 - PRODUCTS

2.01 MATERIALS:

A. Metal Surfaces, General: For fabrication of work which will be exposed to view, use only materials which are smooth and free of surface blemishes including pitting, seam marks, rolled trade names and roughness. Remove such blemishes by grinding, or by welding and grinding, prior to cleaning, treating and application of surface finishes.

B. Wide Flange Shapes: ASTM A992, Grade 50.

C. Other Structural Steel Shapes, Plates and Bars: ASTM A 36.

D. Structural Steel Tubes: ASTM A500, Grade B.

E. Anchor Bolts: ASTM A 36.

F. High-Strength Threaded Fasteners: Heavy hexagon structural bolts, heavy hexagon nuts, and hardened washers, ASTM A 325.

G. Electrodes for Welding: Comply with AWS Code.

H. Structural Steel Primer Paint: Equal to Tnemec 10-99 or 55-888.

I. Non-metallic Shrinkage-Resistant Grout: Pre-mixed, non-metallic, non-corrosive, non-staining complying with CRD C621.

2.02 FABRICATION:

A. Shop Fabrication and Assembly: Shop fabricate and assemble to greatest extent possible, in accordance with AISC Specifications and as indicated on final shop drawings. Provide camber in structural members where indicated.

Properly mark and match-mark materials for field assembly. Fabricate for delivery sequence which will expedite erection and minimize field handling of materials.

Where finishing is required, complete assembly, including welding of units, before start of finishing operations. Provide finish surfaces of members exposed in final structure free of markings, burrs, and other defects.

B. Connections: Weld or bolt shop connections, as indicated.

Bolt field connections, except where welded connections or other connections are indicated.

C. High-Strength Bolted Construction: Install in accordance with AISC "Specifications for Structural Joints using ASTM A325 or A 490 Bolts".

D. Welded Construction: Comply with AWS Code for procedures, appearance and quality of welds, and methods used in correcting welding work.

E. Holes: Provide holes required for securing other work to structural steel framing, and for passage of other work through steel framing members, as shown on final shop drawings.

Provide threaded nuts welded to framing, and other specialty items as indicated to receive other work.

Cut, drill, or punch holes perpendicular to metal surfaces. Do not flame cut holes or enlarge holes by burning. Drill holes in bearing plates.

2.03 SHOP PAINTING:

A. General: Shop paint structural steel, except those members or portions of members to be embedded in concrete or mortar. Paint embedded steel which is partially exposed on exposed portions and initial 2" of embedded areas only.

Do not paint surfaces which are to be welded.

B. Surface Preparation: Clean in accordance with Steel Structures Painting Council (SSPC) as follows:

SP-1 Solvent Cleaning: Oil and grease removal.

SP-2 Hand Tool Cleaning or SP 3 Power Tool Cleaning: for steel to be enclosed or protected.

SP-6 Commercial Blast Cleaning: for exposed steel or steel subject to prolonged jobsite exposure.

C. Painting: Apply primer in accordance with manufacturer's instructions to provide dry film thickness of not less than 2.0 mils. Use painting methods which result in full coverage of joints, corners, edges and exposed surfaces.

PART 3 - EXECUTION

3.01 ERECTION:

A. Check elevations of concrete and masonry bearing surfaces, and locations of anchor bolts and similar devices, before erection work proceeds, and report discrepancies to Engineer. Do not proceed with erection until corrections have been made, or until compensating adjustments to structural steel work have been agreed upon with Engineer.

B. Temporary Shoring and Bracing: Provide temporary shoring and bracing members with connections of sufficient strength to bear imposed loads. Remove temporary members and connections when permanent members are in place and final connections are made. Provide temporary guy lines to achieve proper alignment of structures as erection proceeds.

C. Temporary Planking: Provide temporary planking and working platforms as necessary.

D. Anchor Bolts: Furnish anchor bolts and other connectors required for securing structural steel to foundations and other in-place work.

Furnish templates and other devices as necessary for presetting bolts and other anchors to accurate locations.

E. Setting Bases and Bearing Plates: Clean concrete and masonry bearing surfaces of bond-reducing materials and roughen to improve bond to surfaces. Clean bottom surface of base and bearing plates.

Set loose and attached base plates and bearing plates for structural members on wedges or other adjusting devices, or set loose leveling plates in grout. Set leveling plates so that additional shimming or adjustment is not required to plumb supported members.

F. Tighten anchor bolts after supported members have been positioned and plumed. Do not remove wedges or shims, but if protruding, cut off flush with edge of base or bearing plate prior to packing with grout.

G. Pack grout solidly between bearing surfaces and bases or plates to ensure that no voids remain. Finish exposed surfaces, protect installed materials, and allow to cure.

For proprietary grout materials, comply with manufacturer's instructions.

H. Field Assembly: Set structural frames accurately to lines and elevations indicated. Align and adjust various members forming a part of complete frame or structure before permanently fastening. Clean bearing surfaces and other surfaces which will be in permanent contact before assembly. Perform necessary adjustments to compensate for discrepancies in elevations and alignment.

Level and plumb individual members of structure within specified AISC tolerances.

Splice members only where indicated and accepted on shop drawings.

I. Comply with AISC Specifications for bearing, adequacy of temporary connections, alignment, and removal of paint on surfaces adjacent to field welds.

Do not enlarge unfair holes in members by burning or by use of drift pins, except in secondary bracing members. Ream holes that must be enlarged to admit bolts.

J. Gas Cutting: Do not use gas cutting torches in field for correcting fabrication errors in primary structural framing.

K. Touch-up Painting: Immediately after erection, clean field welds, bolted connections, and abraded areas of shop paint. Apply paint to exposed areas using same material as used for shop painting to provide minimum dry film thickness of 2.0 mils.

3.02 QUALITY CONTROL:

A. Owner may engage an independent testing and inspection agency to inspect high-strength bolted connections and field welded connections and to perform tests and prepare test reports.

Testing agency shall conduct and interpret tests and state in each report whether test specimens comply with requirements, and specifically state any deviations therefrom.

Provide access for testing agency to places where structural steel work is being fabricated or produced so that required inspection and testing can be accomplished.

Testing agency may inspect structural steel at plant before shipment; however, Engineer reserves right, at any time before final acceptance, to reject material not complying with specified requirements.

B. Correct deficiencies in structural steel work which inspections and laboratory test reports have indicated to be not in compliance with requirements. Perform additional tests, at Contractor's expense, as necessary to reconfirm any noncompliance of original work, and as necessary to show compliance of corrected work.

C. Shop Welding: Inspect and test during fabrication of structural steel assemblies, as follows:

Certify welders and conduct inspections and tests as required. Record types and locations of defects found in work. Record work required and performed to correct deficiencies.

Perform visual inspection of all welds.

D. Testing and Inspection of Field Welding by Contractor: Inspect and test during erection of structural steel as follows:

Certify welders and conduct inspections and tests as required. Record types and locations of defects found in work. Record work required and performed to correct deficiencies.

Perform visual inspection of all welds.

E. Testing and inspection of Field Welding by Owner: Perform visual inspection of all welds.

Test 20% of all field welds using liquid penetrant, magnetic particle, radiographic or ultrasonic methods. If 20% of these tests show deficiencies, Engineer may require additional tests at no cost to the Owner.

*** END OF SECTION 05120 ***

SECTION 05210

STEEL JOISTS

PART 1 - GENERAL

1.01 DESCRIPTION OF WORK:

A. Provide KCS Series steel joists and bridging as shown on drawings.

1.02 QUALITY ASSURANCE:

A. Provide joists fabricated in compliance with SJI "Standard Specifications, Load Tables and Weight Tables: Steel Joists and Joist Girders".

B. Qualification of Field Welding: Qualify welding processes and welding operators in accordance with the AWS "Standard Qualification Procedure".

Joists welded in place are subject to inspection and testing. Expense of removing and replacing any portion of steel joists for testing purposes will be born by Owner if welds are found to be satisfactory. Remove and replace work found to be defective and provide new acceptable work.

1.03 SUBMITTALS:

A. Product Data: Manufacturer's specifications and installation instructions for each type of joist and accessories. Include manufacturer's certification that joists comply with SJI "Specifications".

B. Shop Drawings: Submit detailed drawings showing layout of joist units, special connections, jointing and accessories. Include mark, number, type, location and spacing of joists and bridging.

1.04 DELIVERY, STORAGE AND HANDLING:

A. Deliver, store and handle steel joists as recommended in SJI "Specifications". Handle and store joists in a manner to avoid deforming members and to avoid excessive stresses.

PART 2 - PRODUCTS

2.01 MATERIALS:

A. Steel: Comply with SJI "Specifications".

B. Unfinished Threaded Fasteners: ASTM A 307, Grade A, regular hexagon type, low carbon steel.

C. Steel Prime Paint: Comply with SJI "Specifications", except asphalt type paint not permitted.

2.02 FABRICATION:

A. General: Fabricate steel joists in accordance with SJI "Specifications".

B. Bridging: Provide diagonal or horizontal type bridging for "open web" joists, complying with SJI "Specifications".

Provide bridging anchors for ends of bridging lines terminating at walls or beams.

C. End Anchorage: Provide end anchorages to secure joists to adjacent construction, complying with SJI "Specifications", unless otherwise indicated. Do not extend bottom chords of joists at walls.

D. Shop Painting: Remove loose scale, heavy rust, andother foreign materials from fabricated joists and accessories before application of shop paint, per SSPC- SP 3.63.

Apply one shop coat of primer paint to steel joists and accessories, by spray, dipping, or other method to provide a continuous dry paint film thickness of not less than 1.5 mil.

PART 3 - EXECUTION

3.01 INSPECTION:

A. Erector must examine areas and conditions under which steel joists are to be installed and notify Contractor and Engineer in writing of conditions detrimental to proper and timely completion of work. Do not proceed with work until unsatisfactory conditions have been corrected in a manner acceptable to Erector.

3.02 ERECTION:

A. Place and secure steel joists in accordance with SJI "Specifications", final shop drawings, and as herein specified.

B. Placing Joists: Do not start placement of steel joists until supporting work is in place and secured. Place joists on supporting work, adjust and align in accurate locations and spacing before permanently fastening.

Provide temporary bridging, connections, and anchors to ensure lateral stability during construction.

C. Bridging: Install bridging simultaneously with joist erection, before construction loads are applied. Anchor ends of bridging lines at top and bottom chords where terminating at walls or beams.

D. Field weld joists to bearing plates in accordance with SJI "Specifications" for type of joists used. Coordinate welding sequence and procedure with placing of joists.

E. Touch-Up Painting: After joist installation, paint field bolt heads and nuts, and welded areas, abraded or rusty surfaces on joists and steel supporting members. Wire brush surfaces and clean with solvent before painting. Use same type of paint as used for shop painting.

* END OF SECTION *

SECTION 05300

METAL DECKING

PART 1 - GENERAL

1.01 DESCRIPTION OF WORK:

A. Type B metal roof deck.

1.02 QUALITY ASSURANCE:

A. Codes and Standards: Comply with provisions of the following codes and standards, except as otherwise indicated or specified:

1. AISI "Specification for the Design of Cold-Formed Steel Structural Members".
2. AWS "Structural Welding Code".
3. SDI "Design Manual for Floor Decks and Roof Decks", "Specifications for Roof Deck".

B. Qualification of Field Welding: Qualify welding processes and welding operators in accordance with AWS "Standard Qualification Procedure".

Welded decking in place is subject to inspection and testing. Expense of removing and replacing portions of decking for testing purposes will be borne by Owner if welds are found to be satisfactory. Remove work found to be defective and replace with new acceptable work.

C. Underwriters' Label: Provide UL labeled roof deck units.

1.03 PERFORMANCE REQUIREMENTS:

A. Uplift Loading: Install and anchor roof deck units to resist gross uplift loading of 30 lbs. per sq. ft.

1.04 SUBMITTALS:

A. Product Data: Submit manufacturer's specifications and installation instructions for each type of decking and accessories. Include manufacturer's certification as may be required to show compliance with these specifications.

B. Shop Drawings: Submit detailed drawings showing layout and types of deck panels, anchorage details, and conditions requiring closure panels, supplementary framing, cant strips, cut openings, special jointing or other accessories.

PART 2 - PRODUCTS

2.01 MATERIALS:

A. Steel for Painted Metal Deck Units: ASTM A 611, Grade C.

B. Miscellaneous Steel Shapes: ASTM A 36.

C. Sheet Metal Accessories: ASTM A 526, commercial quality, galvanized.

2.02 FABRICATION:

A. General: Form deck units in length to span 3 or more supports, with flush, telescoped or nested 2" laps at ends and nested side laps.

B. Roof Deck Units: 1 ½" deep, Type B, wide rib, 22 ga., painted.

C. Metal Closure Strips: Fabricate metal closure strips, for cell raceways and openings between decking and other construction, of not less than 0.045" min. (18 gage) sheet steel. Form to provide tight-fitting closures at open ends of cells or flutes and sides of decking.

PART 3 - EXECUTION

3.01 INSPECTION:

A. Installer must examine areas and conditions under which metal decking is to be installed and notify Contractor and Engineer in writing of conditions detrimental to proper and timely completion of work. Do not proceed with work until unsatisfactory conditions have been corrected in a manner acceptable to Installer.

3.02 INSTALLATION:

A. General: Install deck units and accessories in accordance with manufacturer's recommendations and final shop drawings, and as specified herein.

Place deck units on supporting steel framework and adjust to final position with ends accurately aligned and bearing on supporting members before being permanently fastened. Do not stretch or contract side lap interlock.

Place deck units in straight alignment for entire length of run of cells and with close alignment between cells at end of abutting units.

Place deck units flat and square, secured to adjacent framing without warp or excessive deflection.

3.03 FASTENING DECK UNITS:

A. Comply with AWS requirements and procedures for manual shielded metal arc welding, appearance and quality of welds, and methods used in correcting welding work.

B. Roof Deck Units:

Fasten roof deck units to steel supporting members using 5/8" diameter puddle welds in pattern shown on drawings.

Side Laps: Fasten using #10 hex head screws as shown on drawings.

C. Cutting and Fitting: Cut and neatly fit deck units and accessories around other work projecting through or adjacent to the decking.

D. Reinforcement at Openings: Provide additional metal reinforcement and closure pieces as required for strength, continuity of decking and support of other work shown.

E. Closure Strips: Provide metal closure strips at open uncovered ends and edges of roof decking, and in voids between decking and other construction. Weld into position to provide a complete decking installation.

F. Touch-Up Painting: After decking installation, wire brush, clean and paint scarred areas, welds and rust spots on top and bottom surfaces of decking units and supporting steel members.

* END OF SECTION 05300 *

GENERAL NOTES

1. RECORD OWNER OF PROPERTY: The Pillsbury Company of Minneapolis, Minnesota by deed recorded in the Cumberland County Registry of Deeds in Book 11988 Page 283 with a Corrective Quitclaim deed recorded in book 14385 page 198.
   Area of parcel = 12.6 acres above high tide
   Area of parcel = 23.5 acres above low tide
2. Improvements shown on property were located by a combination of aerial photography dated Nov. 1988 and standard radial topography. Utilities shown on this plan are located approximately, utilities are based on physical locations and/or maps provided by the client and from respective utility companies.
3. All bearings are based on the Maine State Grid Coordinate System NAD83 (West Zone).
4. Property is located within zones C, A2 and A4. Zone C is defined as an area of minimal flooding. Zone A2 is defined as an area of 100-year flood based on flood elevation 9'. Zone A4 is defined as an area of 100-year flood based on flood elevation 11'. Firm panel #230051 0007 b, with effective date of July 17, 1986. See locations on plan.
5. Property is located within the Moderate Impact Industrial Zone (Im). Dimensional requirements:
   - Minimum Street Frontage: 80'
   - Minimum Lot Size: None
   - Maximum Building Height: 75'
   - Minimum Side yards: Each structure shall be set back 1' from each side property line for each 1' of building height, up to 25', except that the minimum side yard shall be 35' when the side property abuts a Residential zone.
   - Minimum Rear yards: Each structure shall be set back 1' from each side property line for each 1' of building height, up to 25', except that the minimum side yard shall be 35' when the side property abuts a Residential zone.
   - Minimum Front yards: Each structure shall be set back 1' from each side property line for each 1' of building height, up to 25'.
6. Topographic/instrument survey performed by Oest Associates Inc., February 1988. Elevations are based on NGVD 1929 (Mean Sea Level). Bench Mark found MDOT Disk 30689M Elev.=32.87'. Located at Station 157+0 and 88ft left in the sidewalk of Tukey's Bridge on the North end.
7. Restrictive easements with approximate locations shown on plan:

Commitments (Typ.):	Easements Of Record:	Book/Page:
No. (9)	Colonial Ordinance	Title 12 MRSA
No. (11)	CMP-NET&T	1989/88
No. (13)	CMP-NET&T	2053/364
No. (15)	CMP-NET&T	2491/45
No. (17)	CMP-NET&T	3091/18
No. (20)	CMP	7708/219
No. (22)	Service road	2388/345
No. (23)	Sewer Easement	2471/218
No. (24)	Sewer Easement	4124/320
No. (21)	Sewer Easement	4380/171
No. (19)	Sewer Easement	881/256

8. Beneficial Easements with approximate locations shown on plan:

Commitments (Typ.):	Description of Record:	Book/Page:
No. (18)	Water St. Discontinuance	1822/134

   A. Water Street Discontinued July 8, 1969. On record at the City of Portland Engineers Office, Volume 78 – Page 137.
   B. Water Street Discontinued February 17, 1841. On record at the City of Portland Engineers Office, Volume 83 – Page 371.

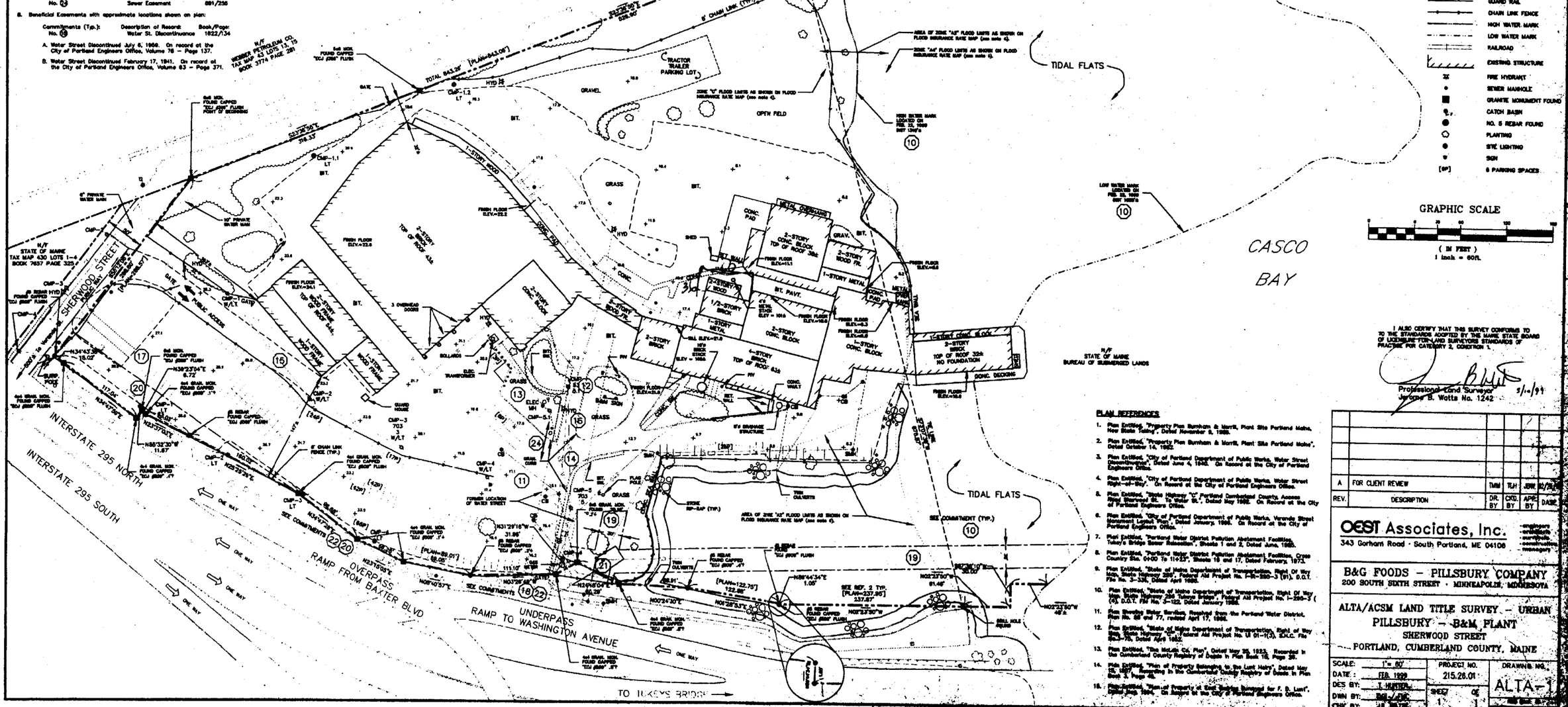
SURVEYORS CERTIFICATION

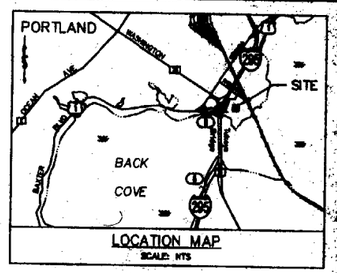
The undersigned, being a Professional Land Surveyor of Maine, No. 1242, hereby certifies to: Heritage Acquisition Corp., a Delaware corporation; B&G Food, INC., a Delaware corporation, Lehman Commercial Paper Inc., a New York banking corporation, as Administrative Agent, its successors and assigns; and Chicago Title Insurance Company this print of survey is a true and accurate survey based on survey made on the premises on February 19, 1988 and correctly represents the boundaries and area of the following described real estate:

A certain parcel of land located on the easterly sideline of Interstate 295 and the southerly sideline of Sherwood Street, in the City of Portland, Cumberland County, Maine, bounded and described as follows:

Beginning at a 6-inch by 6-inch granite monument found inscribed "ECJ #356" marking the southerly sideline of Sherwood Street westerly sideline of said Canadian National Railroad and further described as the most northeasterly corner of described parcel;
THENCE, S 23-36-55 E, along said Railroad 316.33 feet to a 6-inch by 6-inch granite monument found inscribed "ECJ 356";
THENCE, S 23-36-55 E, along said Railroad, 526.90 feet, to a 4-inch by 4-inch granite monument with a drill hole;
THENCE, S 23-36-55 E, along said Railroad, 745 feet, more or less to the low water mark of the Casco Bay;
THENCE, in a general westerly direction along said low water line about 1,685 feet to a point on the easterly sideline of Interstate 295;
THENCE, N 2-23-50 W, along said sideline, 45 feet, more or less to a point;
THENCE, N 87-36-10 W, along said sideline, 30.00 feet to a point;
THENCE, N 2-23-50 W, along said sideline, 61.46 feet to a drill hole found, said drill hole lying on a tie course of S 73-07-28 W, 878.87 feet from the previous mentioned 4-inch by 4-inch granite monument with a drill hole;
THENCE, N 2-23-50 W, along said sideline, 237.87 feet to a 5/8 inch re-bar found capped "ECJ #509";
THENCE, N 86-44-34 E, along said sideline, 1.05 feet to a 5/8 inch re-bar found capped "ECJ #509";
THENCE, N 2-28-53 E, along said sideline, 122.86 feet to a 5/8 inch re-bar found capped "ECJ #509";
THENCE, N 0-24-30 E, along said sideline, 88.91 feet to a 4-inch by 4-inch granite monument found inscribed "ECJ #509";
THENCE, N 24-48-04 E, along said sideline, 80.29 feet to a 6-inch by 6-inch granite highway monument;
THENCE, N 31-29-16 W, along said sideline, 31.96 feet to a 4-inch by 4-inch granite monument inscribed "ECJ #509";
THENCE, N 3-28-49 W, along said sideline, 111.10 feet to a 5/8 inch re-bar found capped "ECJ #509";
THENCE, N 8-40-57 E, along said sideline, 89.05 feet to a 4-inch by 4-inch granite monument inscribed "ECJ #509";
THENCE, N 23-15-05 E, along said sideline, 66.28 feet to a 4-inch by 4-inch granite monument inscribed "ECJ #509";
THENCE, N 34-47-28 E, along said sideline, 96.88 feet to a 4-inch by 4-inch granite monument inscribed "ECJ #509";
THENCE, N 25-25-29 E, along said sideline, 160.02 feet to a 5/8 inch re-bar found capped "ECJ #509";
THENCE, N 23-37-03 E, along said sideline, 63.03 feet to a 4-inch by 4-inch granite monument inscribed "ECJ #509";
THENCE, N 30-23-04 E, along said sideline, 8.72 feet to a 6-inch by 6-inch granite monument found inscribed "ECJ #509";
THENCE, N 85-32-30 W, along said sideline, 11.87 feet to a 4-inch by 4-inch granite monument inscribed "ECJ #509";
THENCE, N 34-47-50 E, along said sideline, 117.04 feet to a 4-inch by 4-inch granite monument inscribed "ECJ #509";
THENCE, N 34-43-38 E, along said sideline, 15.02 feet to a 5/8 inch re-bar found capped "ECJ #509" marking the southerly sideline of Sherwood Street;
THENCE, S 55-12-58 E, along said Sherwood Street, 288.00 feet to the point of beginning.
The above described parcel contains 23.5 acres, more or less.

and that the property described herein is the same as the property described in Chicago Title Insurance Company commitment No. 10887749 dated December 16, 1988; and the location of all buildings and other improvements and structures situated thereon; the location and name of all public and private streets or ways located thereon or adjacent thereto; the location, and dimensions of all recorded easements, rights-of-way and party walls visible thereon; the location of applicable building setback, side yard and rear yard lines required by local ordinances and regulations; that there are no building encroachments onto or from said premises unless shown hereon. The undersigned further certifies that this survey was made in accordance with "Minimum Standard Detail Requirements for ALTA/ACSM Land Title Surveys," jointly established and adopted by ALTA and ACSM in 1992 and meets the accuracy requirements of a Urban Class Survey, as defined thereto, and including Items 2,3, 7(a), 7(b)(1) and 7(c), 8-11 and 13 in Table A contained thereto. Said described property is located within an area having a zone designation of "Moderate Impact Industrial Zone" by the City of Portland, ME., on Flood Insurance Rate Map No. 7 of 7, with a date of identification of July 17, 1981, for Community No. 230051-0007 B, in Portland, Maine which is the current Flood Insurance Rate Map for the community in which said premises is situated; the property has direct access to Sherwood Street, being a dedicated public street.





LEGEND

- PROPERTY LINE
- EDGE OF PAVEMENT
- GAS MAIN
- W WATER MAIN
- S SANITARY SEWER
- SD STORM DRAIN
- OE OVERHEAD ELECTRIC
- UE UNDERGROUND ELECTRIC
- EASEMENT
- GUARD RAIL
- CHAIN LINK FENCE
- HIGH WATER MARK
- LOW WATER MARK
- RAILROAD
- EXISTING STRUCTURE
- FIRE HYDRANT
- SEWER MANHOLE
- GRANITE MONUMENT FOUND
- CATCH BASIN
- NO. 5 REBAR FOUND
- PLANTING
- STR. LIGHTING
- SIGN
- [6P] 6 PARKING SPACES

GRAPHIC SCALE
(IN FEET)
1 inch = 60ft.

I ALSO CERTIFY THAT THIS SURVEY CONFORMS TO TO THE STANDARDS ADOPTED BY THE MAINE STATE BOARD OF LICENSURE FOR LAND SURVEYORS STANDARDS OF PRACTICE FOR CATEGORY 2, CONDITION 1.

Professional Land Surveyor
Jerome B. Watts No. 1242
5/1-/89

PLAN REFERENCES

1. Plan Entitled, "Property Plan Burnham & Morrill, Plant Site Portland Maine, New State Taking", Dated November 5, 1985.
2. Plan Entitled, "Property Plan Burnham & Morrill, Plant Site Portland Maine", Dated October 14, 1982.
3. Plan Entitled, "City of Portland Department of Public Works, Water Street Reconstruction", Sheet 4, Dated June 4, 1990. On Record at the City of Portland Engineers Office.
4. Plan Entitled, "City of Portland Department of Public Works, Water Street Plan, Sheet 4", On Record at the City of Portland Engineers Office.
5. Plan Entitled, "State Highway "U" Portland Cumberland County Access Road, Sheet No. 2", Dated May 1982. On Record at the City of Portland Engineers Office.
6. Plan Entitled, "City of Portland Department of Public Works, Veranda Street Improvement Layout Plan", Dated January, 1968. On Record at the City of Portland Engineers Office.
7. Plan Entitled, "Portland Water District Pollution Abatement Facilities, Tukey's Bridge Sewer Relocation", Sheets 1 and 2, Dated June, 1980.
8. Plan Entitled, "Portland Water District Pollution Abatement Facilities, Cross Country Sta. 0+00 To 117+53", Sheets 16 and 17, Dated February, 1973.
9. Plan Entitled, "State of Maine Department of Transportation, Right Of Way Plan, Route Interstate 295", Federal Aid Project No. I-BR-295-3 (91), D.O.T. File No. 3-33E, Dated April 1985.
10. Plan Entitled, "State of Maine Department of Transportation, Right of Way Plan, Route Interstate 295, Tukey's Bridge", Federal Aid Project No. I-295-3 ( ), D.O.T. File No. 3-152, Dated January 1986.
11. Plan Showing Water Services Received from the Portland Water District, Plan No. 68 and 77, revised April 17, 1990.
12. Plan Entitled, "State of Maine Department of Transportation, Right of Way Plan, Route Highway "U", Federal Aid Project No. U 01-1(7), S.H.C. File No. 78, Dated April 1982.
13. Plan Entitled, "The McLain Co. Plan", Dated May 26, 1923, Recorded in the Cumberland County Registry of Deeds in Plan Book 18, Page 28.
14. Plan Entitled, "Plan of Property Belonging to the Lent Heirs", Dated May, 1924, Recording in the Cumberland County Registry of Deeds in Plan Book 19, Page 28.
15. Plan Entitled, "Plan of Property of Ernest Thurlow Belonging to F. D. Lent", Dated June 1924, On Record in the City of Portland Engineers Office.

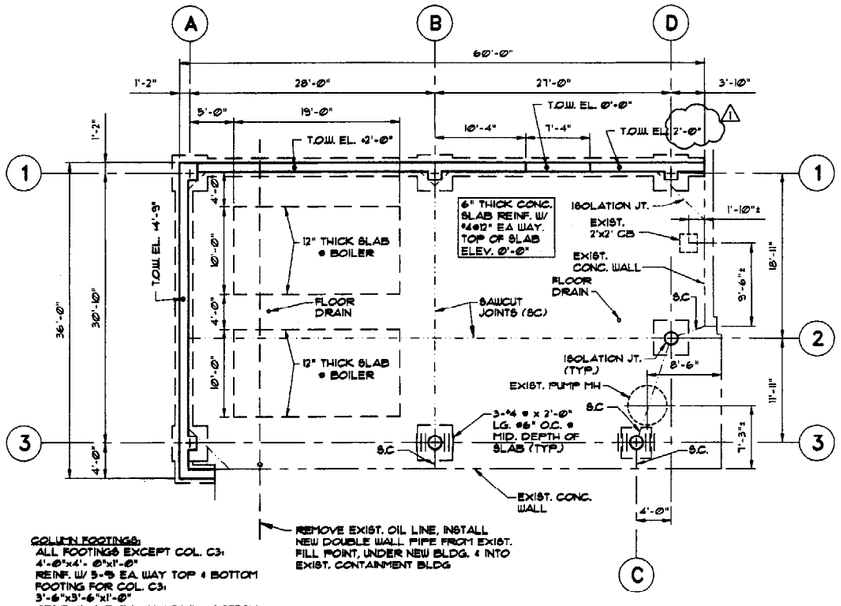
A	FOR CLIENT REVIEW	TMM	TLH	JBW	02/28/89
REV.	DESCRIPTION	DR. BY	CKD. BY	APP. BY	DATE

OEST Associates, Inc.
343 Gorham Road · South Portland, ME 04106
engineers · architects · construction management

B&G FOODS – PILLSBURY COMPANY
200 SOUTH SIXTH STREET · MINNEAPOLIS, MINNESOTA

ALTA/ACSM LAND TITLE SURVEY – URBAN
PILLSBURY – B&M PLANT
SHERWOOD STREET
PORTLAND, CUMBERLAND COUNTY, MAINE

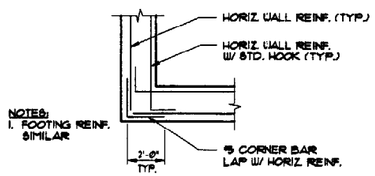
SCALE:	1"= 60'	PROJECT NO.	DRAWING NO.:
DATE:	FEB. 1989	215.26.01	ALTA-1
DES BY:	T. HUNTER		
DWN BY:	BGL / DE	SHEET	OF
CHK BY:	JBW/TMM	1	1



COLUMN FOOTINGS:
ALL FOOTINGS EXCEPT COL. C3:
4'-0"x4'-0"x1'-0"
REINF. W/ 5-#5 E.A. WAY TOP & BOTTOM
FOOTING FOR COL. C3:
3'-6"x3'-6"x1'-0"
REINF. W/ 4-#5 E.A. WAY TOP & BOTTOM

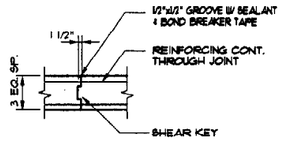
REMOVE EXIST. OIL LINE, INSTALL NEW DOUBLE WALL PIPE FROM EXIST. FILL POINT, UNDER NEW BLDG. & INTO EXIST. CONTAINMENT BLDG

FOUNDATION PLAN
1/8" = 1'-0"



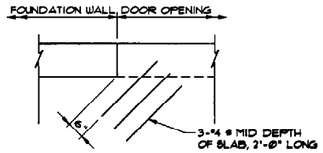
NOTES:
1. FOOTING REINF. SIMILAR

TYP. CORNER REINFORCING
N.T.S.

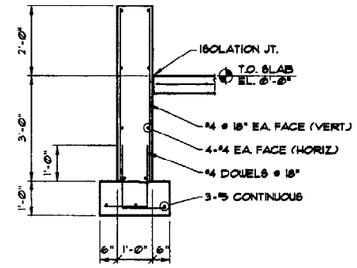


CONSTRUCTION JOINT

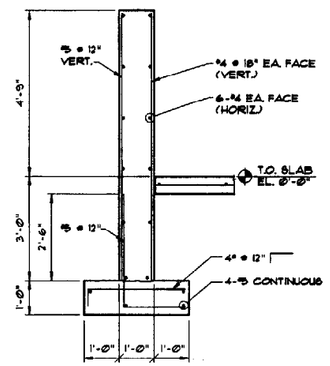
TYP. WALL JOINT
N.T.S.



ADD'L SLAB REINF. @ DOORS AND RE-ENTRANT CORNERS
N.T.S.



FOUND. WALL ALONG COL. LINE (1)
1/2"=1'-0"



FOUND. WALL ALONG COL. LINE (A)
1/2"=1'-0"

FOUNDATION NOTES:

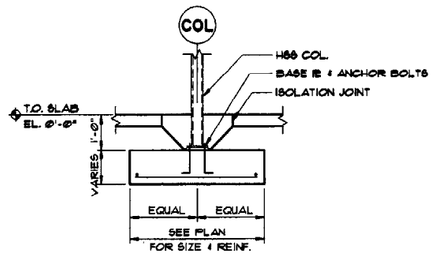
1. DESIGN BEARING CAPACITY:
   2.5 KSF FOOTINGS ON CRUSHED STONE FILL BEARING ON LEDGE
2. PLACE FOOTINGS ON EXPOSED LEDGE SURFACE WHERE POSSIBLE. STRIP SOIL COVER FROM LEDGE AND CLEAN ALL LOOSE MATERIAL FROM LEDGE SURFACE BEFORE CONSTRUCTING FOOTINGS.
3. WHERE FOOTINGS DO NOT BEAR DIRECTLY ON LEDGE PROVIDE MIN. 4'-0" OF SOIL COVER ABOVE BOTTOM OF FOOTING. PLACE FOOTINGS ON CRUSHED STONE FILL BEARING ON LEDGE. NOTIFY ENGINEER IF UNSUITABLE MATERIALS ARE ENCOUNTERED AT FOOTING SUBGRADE.
4. UNDER FLOOR SLABS, REMOVE TOPSOIL AND UNSUITABLE MATERIALS. FILL OVER- EXCAVATED AREAS WITH COMPACTED GRAVEL OR SAND. PROVIDE MIN. 18" OF COMPACTED GRAVEL UNDER SLAB.
5. UNDER SLABS (AND FOOTINGS IF REQUIRED) COMPACT MATERIAL TO 95% OF MAX. DENSITY DETERMINED BY ASTM D1557, MODIFIED.
6. PROVIDE MATERIAL MEETING THE FOLLOWING GRADATIONS BY WEIGHT:

GRAVEL:	SIEVE SIZE	% PASSING
	3"	100
	1/4"	30-70
	NO.40	0-30
	NO. 200	0-5

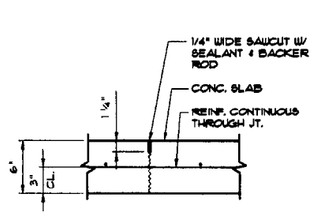
7. BACKFILL BOTH SIDES OF FOUNDATION WALLS AT THE SAME TIME EXCEPT FOR WALLS DESIGNED AS PERMANENT RETAINING WALLS. DO NOT BACKFILL RETAINING WALLS UNTIL 7 DAYS AFTER PLACING CONCRETE.

CONCRETE NOTES:

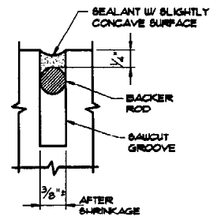
1. DESIGN CODE: ACI 318-99
2. MINIMUM 28 DAY COMPRESSIVE STRENGTH:
   4000 PSI FOR SLABS ON GRADE
   3000 PSI FOR OTHER CONCRETE
3. REINFORCEMENT: GRADE 60, ASTM A615
4. MINIMUM CONCRETE COVER:
   3" FOR CONCRETE CAST AGAINST SOIL.
   2" FOR OTHER CONCRETE UNLESS SHOWN OTHERWISE.
5. PROVIDE CONTROL JOINTS OR CONSTRUCTION JOINTS IN FOUNDATION WALLS AT 30' O.C. MAXIMUM SPACING.
6. SPLICE LENGTHS (UNLESS SHOWN OTHERWISE):
   HORIZONTAL BARS IN WALLS:
   #4 3'-1"
   #5 3'-11"
   OTHER BARS:
   #4 2'-5"
   #5 3'-0"
   #6 3'-7"
7. COORDINATE PENETRATIONS THROUGH CONCRETE WITH MECHANICAL AND ELECTRICAL REQUIREMENTS.



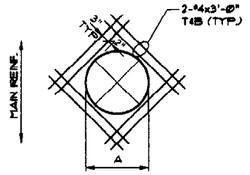
TYP. INTERIOR COLUMN FOOTING
N.T.S.



SAWCUT CONTROL JOINT DETAIL
1 1/2"=1'-0"

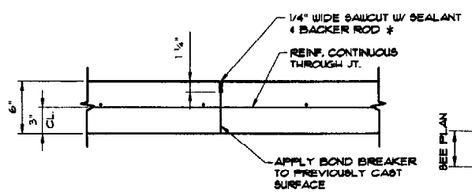


SAWCUT JOINT DETAIL
1"=1'



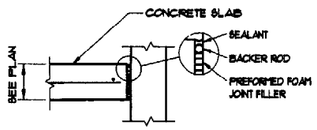
ROUND OPENING
"A"= LESS THAN 1'-6"ø

ADD'L REINFORCING @ OPENINGS IN REINFORCED CONCRETE
N.T.S.



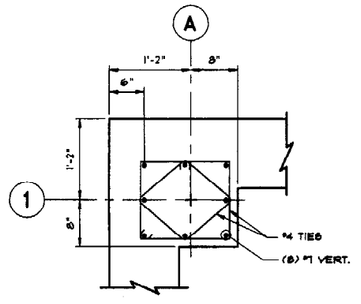
* MAKE SAWCUT AFTER SHRINKAGE CRACK FORMS AT SLAB SURFACE. FOLLOW CRACK WITH SAWCUT

CONSTRUCTION JOINT DETAIL
1 1/2"=1'-0"

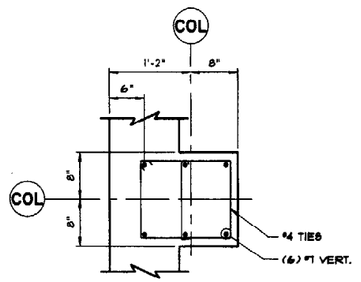


ISOLATION JOINT

TYP. SLAB JOINTS
N.T.S.

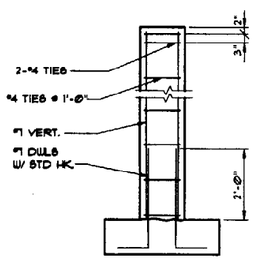


CORNER PIER



PIER ON STRAIGHT WALL

COLUMN PIER PLANS
1"=1'-0"



NOTES:
1. WALL AND FTG. REINF. NOT SHOWN. REINFORCING FOR ALL COLUMN PIERS IS SIMILAR USING #7 VERT. BARS AND #4 TIES AT 1'-0".
2. SEE PIER DETAILS FOR REINFORCING DIMENSIONS/ CONFIGURATION.

SECTION @ TYP. EXTERIOR COLUMN PIER
NTS

REV.	DATE	DESCRIPTION
1	4/4/02	CHANGED EL. No.

BURNHAM & MORRILL CO.
PORTLAND, MAINE

BOILER BUILDING PROJECT

CONCRETE PLAN & DETAILS

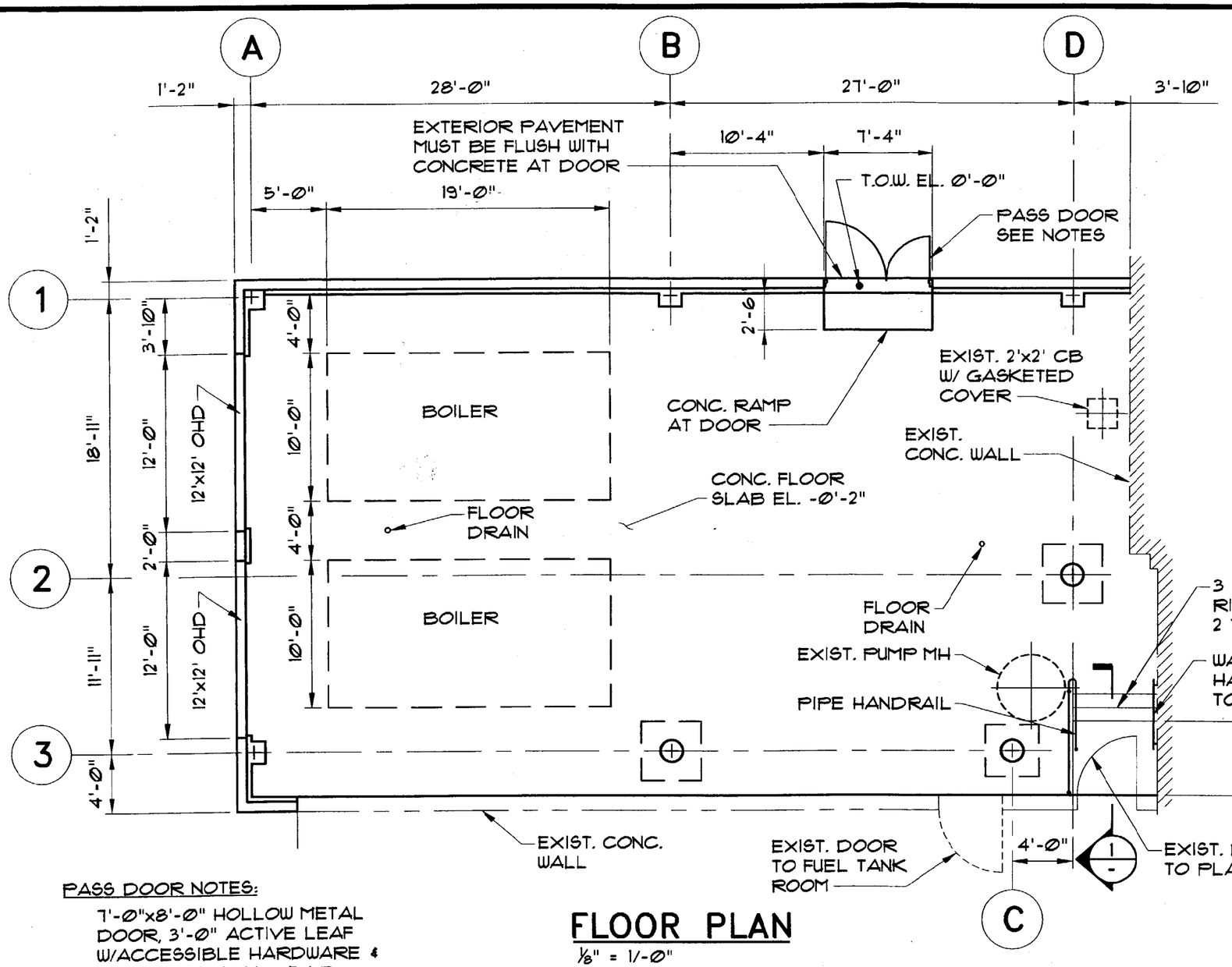
CONSULTING ENGINEERS, INC. BELFAST, MAINE

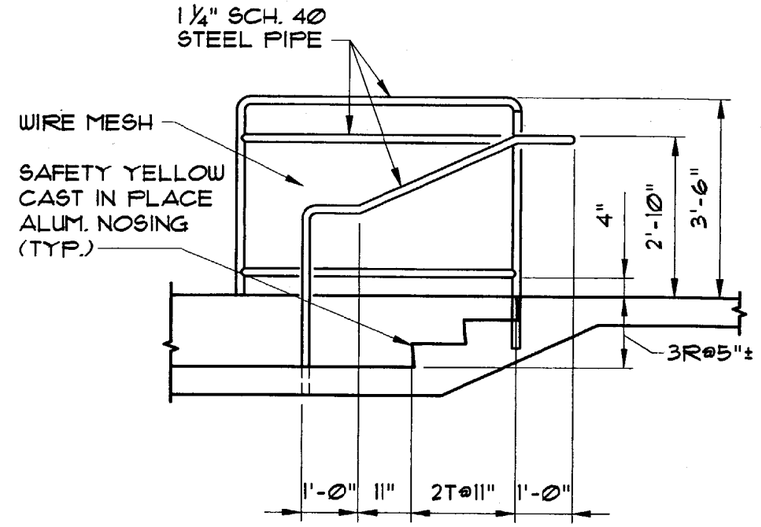
STATE OF MAINE * DAVID K. PINKHAM NO. 4433 REGISTERED PROFESSIONAL ENGINEER *
4-9-02

SCALE:	AS NOTED	DRN BY:	RJS
DATE:	MARCH, 2002	DESG BY:	DKP
PROJECT:	02302	CHK BY:	DKP

S1

CAD FILE: 02302S02 FILE SCALE: 1"=6" PLOT DATE: 3/15/02





SECTION

3/8"=1'-0"

PASS DOOR NOTES:

7'-0"x8'-0" HOLLOW METAL DOOR, 3'-0" ACTIVE LEAF W/ACCESSIBLE HARDWARE & CLOSER WITH MAX. 5 LB. PULL. 4'-0" INACTIVE LEAF MAX. ½" HIGH THRESHOLD

FLOOR PLAN

⅛" = 1/-0"

STATE OF MAINE
DAVID K. PINKHAM
NO. 4433
REGISTERED PROFESSIONAL ENGINEER

4.10.02

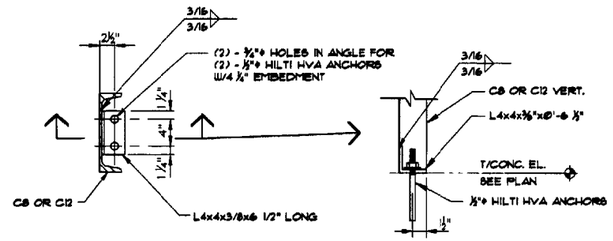
BURNHAM & MORRILL CO.
PORTLAND, MAINE

BOILER BLDG PROJECT
ACCESSIBILITY INFORMATION

SCALE: 1/8"=1'-0"
DATE: APRIL, 2002
DESG BY: DKP
PROJECT: 02302

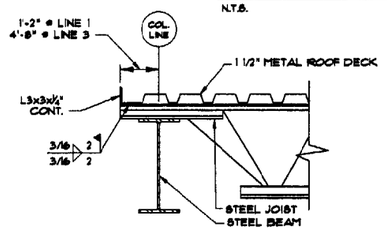
BD1

CAD FILE: 02302-BD1 FILE SCALE: 1"=1'



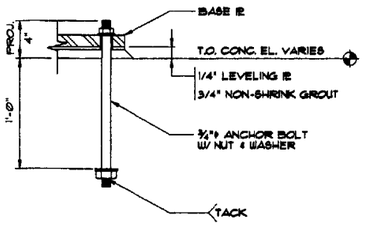
TYPICAL VERTICAL CHANNEL TO CONCRETE CONNECTION

N.T.S.



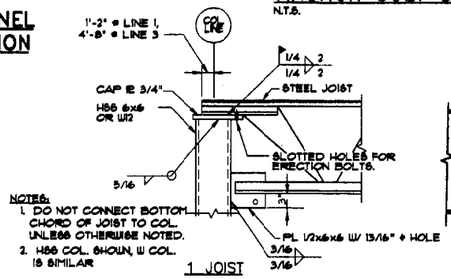
TYP. ROOF DECK ⊙ JOIST ⊙ PERIMETER BEAM

N.T.S.



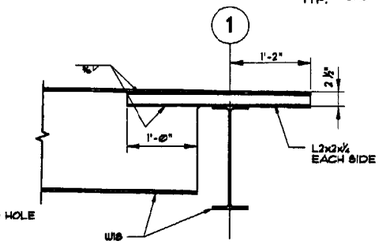
ANCHOR BOLT DETAIL

N.T.S.



NOTES:
1. DO NOT CONNECT BOTTOM CHORD OF JOIST TO COL. UNLESS OTHERWISE NOTED.
2. HSS COL. SHOWN, W COL. IS SIMILAR

1 JOIST

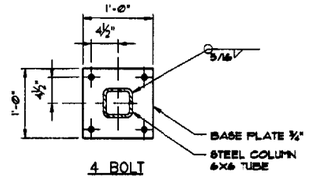


SECTION 4 / 82
1"=1'-0"

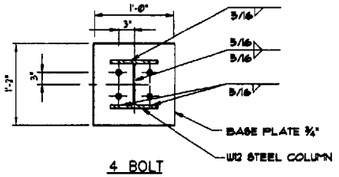
NOTE:
FRAMING ELEVATION COL. LINE D ON 82 SIMILAR

K-SERIES JOIST TO COLUMN ⊙ ROOF

N.T.S.



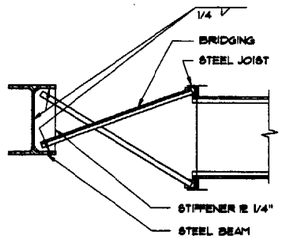
4 BOLT



4 BOLT

BASE PLATE DETAILS

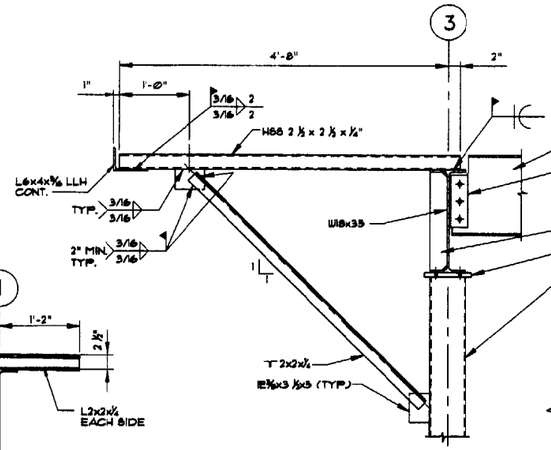
N.T.S. NOTE: CENTER COLUMN ON ℄



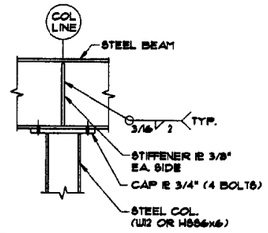
NOTE:
DETAIL APPLIES TO HORIZONTAL ¢ DIAGONAL BRIDGING.

TYP. JOIST BRIDGING TO STEEL BEAM

N.T.S.

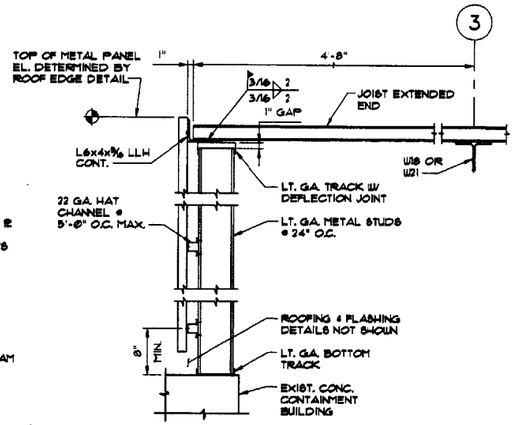


SECTION 1 / 82
1"=1'-0"

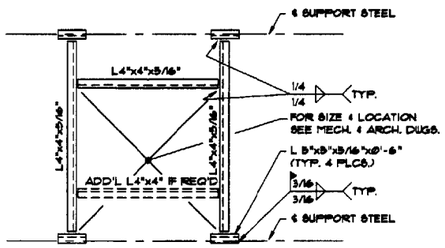


BEAM FRAMING OVER COLUMN

N.T.S.

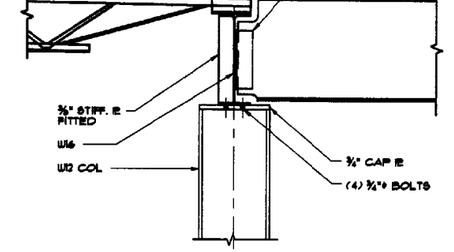


SECTION 3 / 82
1"=1'-0"

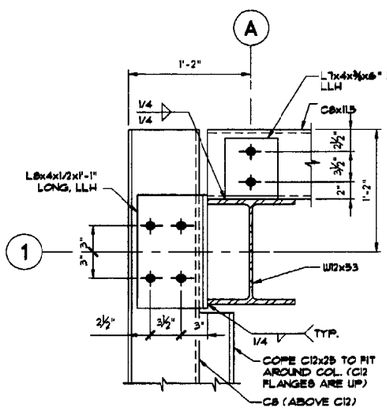


METAL DECK SUPPORT AT OPENINGS

N.T.S.



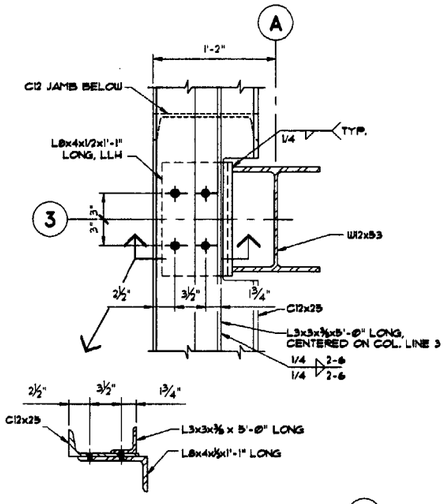
SECTION 5 / 82
1"=1'-0"



GIRT CONN. ⊙ COL (A1)

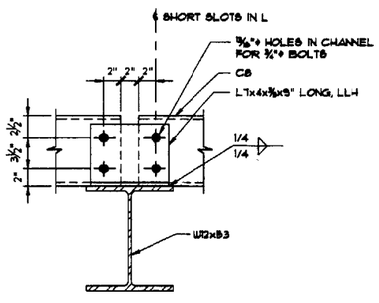
1 1/2"=1'-0"

(C8 GIRT CONN. ¢ COLUMN A3 IS SIMILAR)



C12 GIRT CONN. ⊙ COL (A3)

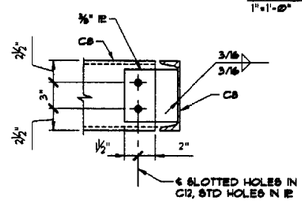
1 1/2"=1'-0"



GIRT CONN. ⊙ COL (B1)

1 1/2"=1'-0"

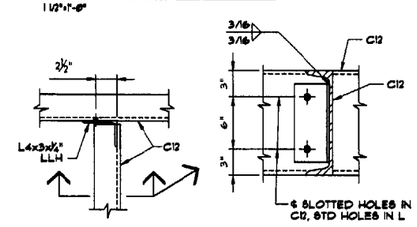
(CONN. ¢ COLUMN D1 IS SIMILAR W/ 4 BOLTS)



NOTE: CONN. FOR C8 SHOWN, C12 IS SIMILAR

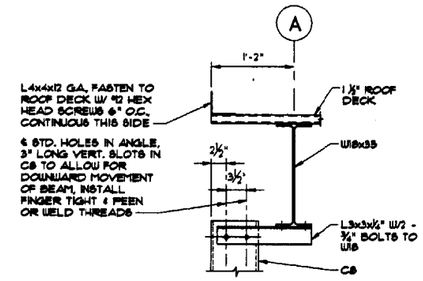
GIRT TO VERTICAL CHANNEL CONNECTION

1 1/2"=1'-0"



JAMB TO HEAD CONNECTION

1 1/2"=1'-0"



SECTION 2 / 82
1"=1'-0"

REV.	DATE	DESCRIPTION

BURNHAM & MORRILL CO.
PORTLAND, MAINE

BOILER BUILDING PROJECT

STRUCTURAL STEEL SECTIONS & DETAILS

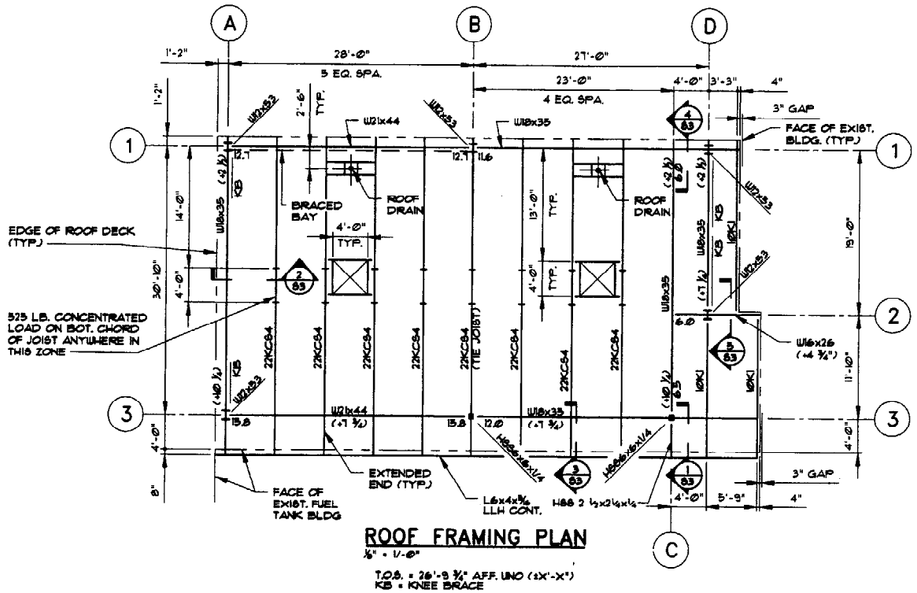
CHARLES MAHONEY, INC. PLANNERS ENGINEERS PORTLAND, MAINE

STATE OF MAINE
DAVID K. PINKHAM
NO. 4433
REGISTERED PROFESSIONAL ENGINEER
4-9-02

SCALE:	AS NOTED	DRN BY:	RJS
DATE:	MARCH, 2002	DESG BY:	DKP
PROJECT:	02302	CHK BY:	DKP

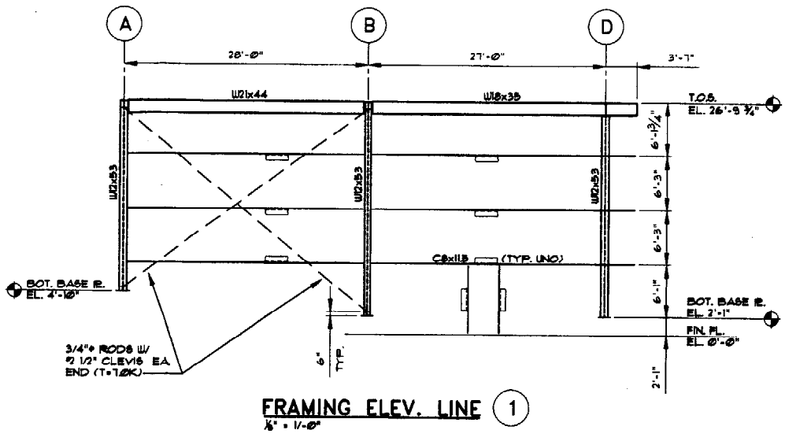
S3

CAD FILE: 02302S02 FILE SCALE: 1"=1" PLOT DATE: 3/13/02



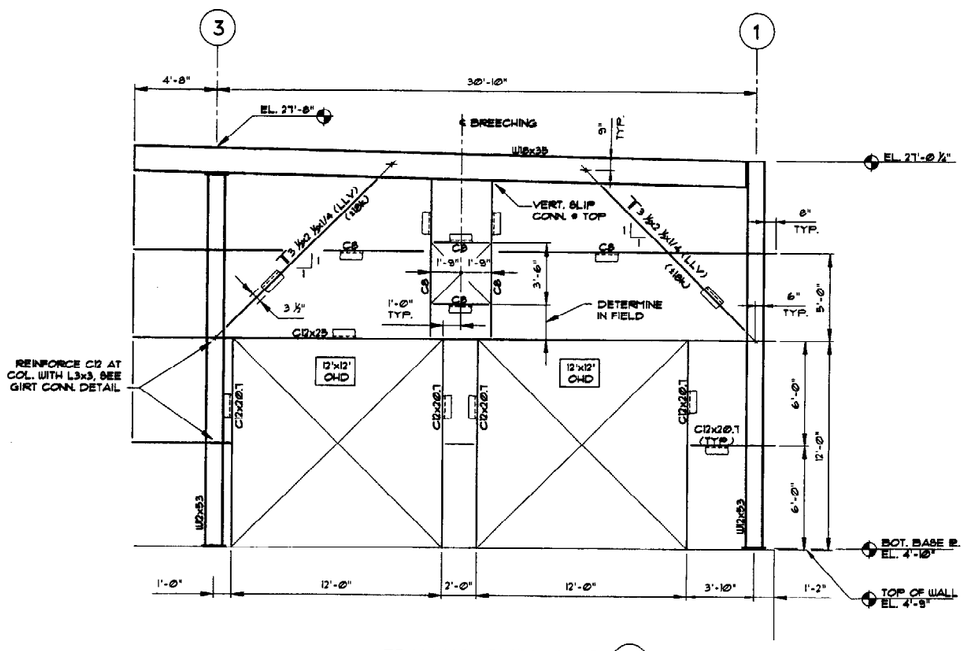
ROOF FRAMING PLAN

⅛" = 1'-Ø"



FRAMING ELEV. LINE (1)

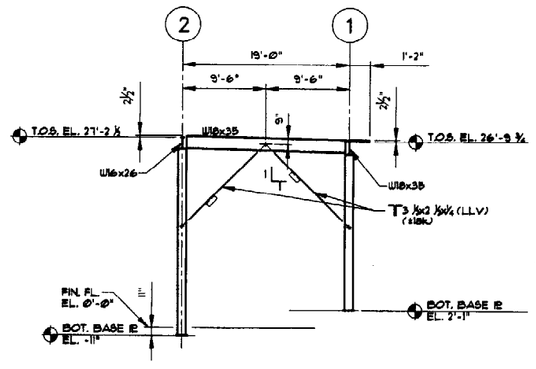
⅛" = 1'-Ø"



FRAMING ELEV. LINE (A)

¼" = 1'-Ø"

CS = C8x11.5 U.N.O.



FRAMING ELEV. LINE (D)

⅛" = 1'-Ø"

GENERAL STRUCTURAL NOTES:

1. DESIGN CODE: 1999 BOCA NATIONAL BUILDING CODE.
2. ROOF DESIGN LOADS:

   GROUND SNOW LOAD : 60 PSF
   FLAT ROOF SNOW LOAD: 45 PSF
   SNOW EXPOSURE FACTOR CE = 1.0
   SNOW IMPORTANCE FACTOR I = 1.0
   ROOF THERMAL FACTOR CT = 1.0
   MECHANICAL/ELECTRICAL LOAD: 10 PSF
3. WIND LOADS:
   BASIC WIND SPEED: 85 MPH
   WIND LOAD IMPORTANCE FACTOR 1.00
   EXPOSURE C
   INTERNAL PRESSURE COEFFICIENT GCPI = +/- Ø.25
   WIND PRESSURE ON LATERAL LOAD SYSTEM: 10 PSF NORTH/SOUTH
   32 PSF EAST/WEST
4. SEISMIC LOADS:
   SEISMIC HAZARD EXPOSURE GROUP I
   AV = Ø.10
   AA = Ø.10
   SEISMIC PERFORMANCE CATEGORY C
   RESPONSE MODIFICATION FACTOR: R = 3
   DEFLECTION AMPLIFICATION FACTOR: CD = 3
5. U.N.O. = UNLESS NOTED OTHERWISE.
6. CONTRACTOR SHALL BRING TO THE ATTENTION OF THE ENGINEER ANY CONDITIONS DIFFERENT FROM THOSE SHOWN ON THE DRAWINGS AND SHALL BRING TO THE ATTENTION OF THE ENGINEER ANY CONDITIONS THAT PREVENT CONTRACTOR'S COMPLETION OF THE WORK AS SHOWN ON THE DRAWINGS.
7. ROOF MUST PROVIDE DIAPHRAGM ACTION.
   ROOF DECK WELD TO SUPPORTING MEMBERS USING 36/4 PATTERN. PROVIDE 2 #Ø HEX HEAD SCREWS PER SPAN FOR SIDELAP CONNECTIONS.
8. IF SINGLE PLATE SHEAR CONNECTIONS ARE USED AT TUBE COLUMNS, BEAM WEBS, OR COLUMN WEBS, LIMIT MAXIMUM PLATE THICKNESS TO 1/4" TUBE WALL OR WEB THICKNESS.

STRUCTURAL STEEL:

1. DESIGN SPECIFICATION: AISC ALLOWABLE STRESS DESIGN SPECIFICATION FOR BUILDINGS, 1989.
2. CONSTRUCTION IS AISC TYPE 2. FRAMES ARE NOT FULLY SELF-SUPPORTING AND REQUIRE SUPPORT FROM OTHER STRUCTURAL ELEMENTS.

   THESE ELEMENTS INCLUDE:

   HORIZONTAL METAL ROOF DIAPHRAGMS

   TEMPORARY SUPPORT FOR THE STEEL FRAME MUST BE PROVIDED UNTIL THESE ELEMENTS ARE COMPLETE AND CONNECTED TO THE STEEL FRAME. THE STRUCTURAL ENGINEER OF RECORD HAS NOT DESIGNED AND IS NOT RESPONSIBLE FOR TEMPORARY SUPPORT DURING ERECTION.
3. STRUCTURAL STEEL: ASTM A 992 FOR WIDE FLANGE SHAPES.
   ASTM A500 GRADE B FOR TUBES
   ASTM A36 FOR ALL OTHER SHAPES AND PLATES.
4. CONNECTIONS: FIELD BOLTS, ASTM A325N BOLTS
   ANCHOR BOLTS: MIN. YIELD 36 KSI
   WELDING: E70 ELECTRODES.
5. DESIGN AND DETAIL SIMPLE SHEAR CONNECTIONS USING "ALLOWABLE STRESS DESIGN FOR STRUCTURAL STEEL BUILDINGS, VOLUME II, 1992" BY AISC.
6. CONNECTIONS WITH BOLTS SUBJECT TO SHEAR ONLY ARE DESIGNATED AS SNUG-TIGHT CONNECTIONS REQUIRING WRENCH TIGHTENING ONLY. BOLTS SUBJECT TO TENSION OR SHEAR AND TENSION MUST BE FULLY TIGHTENED. PROVIDE TENSION CONTROL BOLTS FOR FULLY TIGHTENED CONNECTIONS.
7. WHERE SHOWN END REACTIONS AND MEMBER FORCES ARE IN KIPS AND KIP-FT. WHERE BEAM REACTIONS ARE NOT SHOWN, DESIGN AND DETAIL CONNECTIONS FOR ONE-HALF OF THE ALLOWABLE LOAD CARRYING CAPACITY OF THE BEAM BUT NOT LESS THAN 6 KIPS SERVICE LOAD.
8. PROVIDE MINIMUM OF 2 BOLTS FOR CONNECTIONS TO BRACING UNLESS SHOWN OTHERWISE.

REV.	DATE	DESCRIPTION

BURNHAM & MORRILL CO.
PORTLAND, MAINE

BOILER BUILDING PROJECT

ROOF FRAMING PLAN & FRAMING ELEVATIONS

STATE OF MAINE — DAVID K. PINGHAM NO. 4433 — REGISTERED PROFESSIONAL ENGINEER
4.9.02

SCALE:	AS NOTED	DRN BY:	RJS
DATE:	MARCH, 2002	DESG BY:	DKP
PROJECT:	02302	CHK BY:	DKP

S2

CAD FILE: 02302S01 FILE SCALE: 1"=6' PLOT DATE: 3/13/02

BUILDING PERMIT INSPECTION PROCEDURES
Please call 874-8703 or 874-8693 to schedule your inspections as agreed upon

Permits expire in 6 months, if the project is not started or ceases for 6 months.

The Owner or their designee is required to notify the inspections office for the following inspections and provide adequate notice. Notice must be called in 48-72 hours in advance in order to schedule an inspection:

By initializing at each inspection time, you are agreeing that you understand the inspection procedure and additional fees from a "Stop Work Order" and "Stop Work Order Release" will be incurred if the procedure is not followed as stated below.

✓____ **Pre-construction Meeting**: Must be scheduled with your inspection team upon receipt of this permit. Jay Reynolds, Development Review Coordinator at 874-8632 must also be contacted at this time, before **any** site work begins on any project other than single family additions or alterations.

✓____ **Footing/Building Location Inspection:** Prior to pouring concrete

✓____ **Re-Bar Schedule Inspection:** Prior to pouring concrete

✓____ **Foundation Inspection:** Prior to placing ANY backfill

✓____ **Framing/Rough Plumbing/Electrical:** Prior to any insulating or drywalling

✓____ **Final/Certificate of Occupancy:** Prior to any occupancy of the structure or use. NOTE: There is a $75.00 fee per inspection at this point.

Certificate of Occupancy is not required for certain projects. Your inspector can advise you if your project requires a Certificate of Occupancy. All projects DO require a final inspection

_____ **If any of the inspections do not occur, the project cannot go on to the next phase, REGARDLESS OF THE NOTICE OR CIRCUMSTANCES.**

_____ **CERIFICATE OF OCCUPANICES MUST BE ISSUED AND PAID FOR, BEFORE THE SPACE MAY BE OCCUPIED**

[signature] 5/15/02
Signature of applicant/designee Date

[signature] 5/15/02
Signature of Inspections Official Date

CBL: 447 A001 Building Permit #: 02 0345