Job	Truss	Truss Type	Qty	Ply	Keiser Ind 212
57435	P1057901	HINGED COMMON	1	1	U-1040

Universal Forest Products Inc Grand Rapids MI 49525, Corey Daubert — 7.240 e Jun 18 2010 MiTek Industries Inc. Wed Nov 03 08:59:59 2010 Page 1 of 1

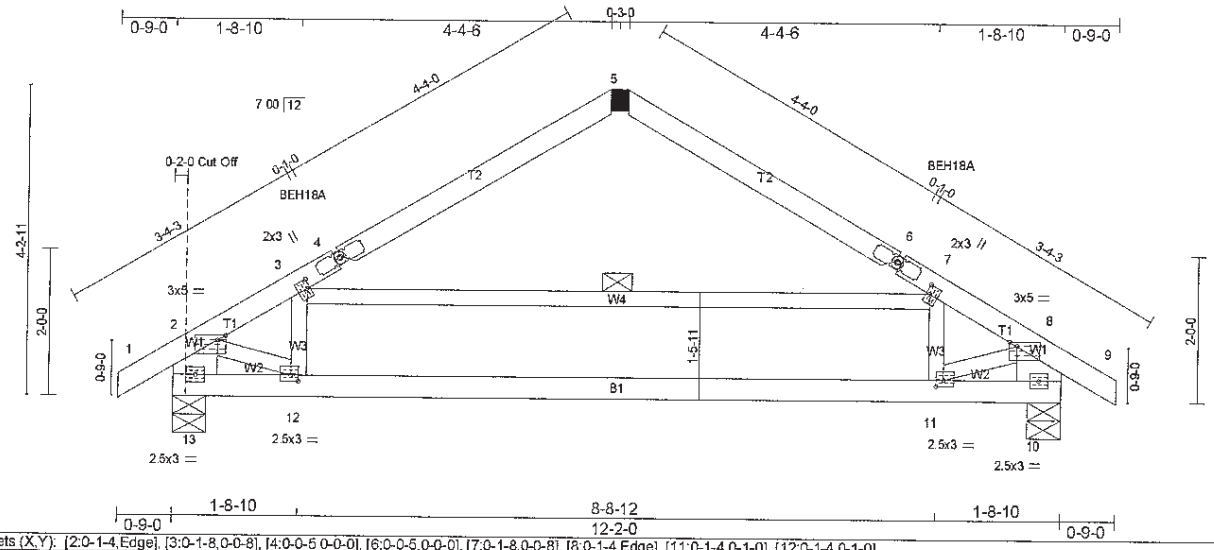

Plate Offsets (X,Y): [2:0-1-4,Edge], [3:0-1-8,0-0-8], [4:0-0-5,0-0-0], [6:0-0-5,0-0-0], [7:0-1-8,0-0-8], [8:0-1-4,Edge], [11:0-1-4,0-1-0], [12:0-1-4,0-1-0]

LOADING (psf)		SPACING	1-4-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	46.2	Plates Increase	1.15	TC	0.43	Vert(LL)	-0.11	11-12	>999	240	MT20	197/144
(Ground Snow=60.0)		Lumber Increase	1.15	BC	0.73	Vert(TL)	-0.20	11-12	>711	180	MI18	141/138
TCDL	7.0	Rep Stress Incr	YES	WB	0.37	Horz(TL)	0.01	10	n/a	n/a		
BCLL	10.0	Code IBC2003/TPI2002		(Matrix)							Weight: 45 lb	FT = 0%
BCDL	10.0											

LUMBER
TOP CHORD 2 X 4 SPF No.2
BOT CHORD 2 X 4 SPF No.2
WEBS 2 X 3 SPF Stud "Except"
W1: 2 X 8 SPF No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 3-7

REACTIONS (lb/size) 13=514/0-5-8, 10=514/0-5-8
Max Horz 13=112(LC 7)
Max Uplift13=-202(LC 8), 10=-201(LC 9)
Max Grav 13=645(LC 13) 10=642(LC 14)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/70, 2-3=-586/151, 3-4=-356/135, 4-5=-267/144, 5-6=-269/145, 6-7=-356/135, 7-8=-586/151, 8-9=0/70, 8-10=-597/180, 2-13=-597/180
BOT CHORD 12-13=-158/155, 11-12=-109/510, 10-11=-115/52
WEBS 3-12=-281/172, 7-11=-279/170, 3-7=-328/108, 8-11=-136/611, 2-12=-136/611

REQUIRED FIELD JOINT CONNECTIONS - Maximum Compression (lb)/ Maximum Tension (lb)/ Maximum Shear (lb)/ Maximum Moment (lb-in)
5=203/146/168/0

NOTES
1) Wind: ASCE 7-02; 110mph; TCDL=4.2psf; BCDL=6.0psf; h=30ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-02; Pg=60.0 psf (ground snow); Ps=46.2 psf (roof snow); Category II; Exp C; Partially Exp.; Ct=1.1
3) Roof design snow load has been reduced to account for slope.
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 17.0 psf or 2.00 times flat roof load of 46.2 psf on overhangs non-concurrent with other live loads.
6) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
7) All plates are MT20 plates unless otherwise indicated
8) See BEH18 DETAILS for plate placement.
9) Provisions must be made to prevent lateral movement of hinged member(s) during transportation
10) All additional member connections shall be provided by others for forces as indicated.
11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 202 lb uplift at joint 13 and 201 lb uplift at joint 10
12) This truss has been designed to meet the 2003 IBC Section 2308.10.7.1; 2003 IRC R802.10.2
13) Take precaution to keep the chords in plane, any bending or twisting of the hinge plate must be repaired before the building is put into service.
14) If shown, field installed members are an integral part of this design. To ensure proper performance, all field installed members must be installed prior to applying any loading to the truss

STATE OF CONNECTICUT
KEVIN W FREEMAN
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PROFESSIONAL ENGINEER

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Northeast Region
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Approval limited to
Factory Built Portion

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CIVIL
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E-Signed by Kevin Freeman
VERIFY authenticity with ApproveIt

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FREEMAN
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11-3-2010
11/3/10

WARNING - *Verify design parameters and READ NOTES*

This building component has only been designed for the loads noted on this drawing. Construction and lifting [illegible] considered. The builder is responsible for lifting methods and system design. Builder responsibilities are defined under TPI1. This design is based only [illegible] shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult BCSI 1-06 from the Wood Truss Council of America and Truss Plate Institute Recommendation available from WTCA, 6300 Enterprise LN, Madison, WI 53719. J:\support\MitekSupp\templates\ufp.tpe

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S5-Hinge and Joint Connection Detail3