

Job 57435	Truss P1057901	Truss Type HINGED COMMON	Qty 1	Ply 1	Keiser Ind. 212 U-1040
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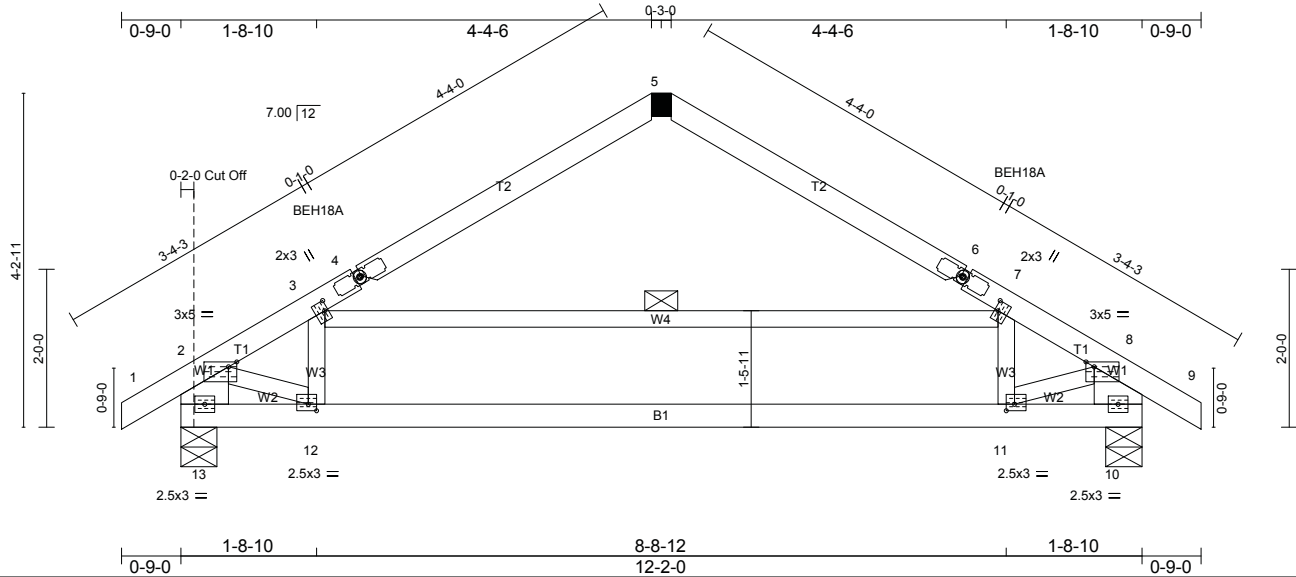


Plate Offsets (X,Y): [2:0-1-4,Edge], [3:0-1-8,0-0-8], [4:0-0-5,0-0-0], [6:0-0-5,0-0-0], [7:0-1-8,0-0-8], [8:0-1-4,Edge], [11:0-1-4,0-1-0], [12:0-1-4,0-1-0]

<b>LOADING</b> (psf)	<b>SPACING</b>	<b>CSI</b>	<b>DEFL</b>	<b>PLATES</b>	<b>GRIP</b>
TCLL 46.2 (Ground Snow=60.0)	1-4-0 Plates Increase 1.15 Lumber Increase 1.15 Rep Stress Incr YES Code IBC2003/TPI2002	TC 0.43 BC 0.73 WB 0.37 (Matrix)	in (loc) l/defl L/d Vert(LL) -0.11 11-12 >999 240 Vert(TL) -0.20 11-12 >711 180 Horz(TL) 0.01 10 n/a n/a	MT20 MII18	197/144 141/138
TCDL 7.0 BCLL 10.0 BCDL 10.0				Weight: 45 lb	FT = 0%

**LUMBER**  
TOP CHORD 2 X 4 SPF No.2  
BOT CHORD 2 X 4 SPF No.2  
WEBS 2 X 3 SPF Stud \*Except\*  
W1: 2 X 8 SPF No.2

**BRACING**  
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.  
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.  
WEBS 1 Row at midpt 3-7

**REACTIONS** (lb/size) 13=514/0-5-8, 10=514/0-5-8  
Max Horz 13=112(LC 7)  
Max Uplift 13=-202(LC 8), 10=-201(LC 9)  
Max Grav 13=645(LC 13), 10=642(LC 14)

**FORCES** (lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=0/70, 2-3=-586/151, 3-4=-356/135, 4-5=-267/144, 5-6=-269/145, 6-7=-356/135, 7-8=-586/151, 8-9=0/70, 8-10=-597/180, 2-13=-597/180  
BOT CHORD 12-13=-158/155, 11-12=-109/510, 10-11=-115/52  
WEBS 3-12=-281/172, 7-11=-279/170, 3-7=-328/108, 8-11=-136/611, 2-12=-136/611

**REQUIRED FIELD JOINT CONNECTIONS** - Maximum Compression (lb)/ Maximum Tension (lb)/ Maximum Shear (lb)/ Maximum Moment (lb-in)  
5=203/146/168/0

- NOTES**
- 1) Wind: ASCE 7-02: 110mph; TCCL=4.2psf; BCDL=6.0psf; h=30ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - 2) TCLL: ASCE 7-02; Pg=60.0 psf (ground snow); Ps=46.2 psf (roof snow); Category II; Exp C; Partially Exp.; Ct=1.1
  - 3) Roof design snow load has been reduced to account for slope.
  - 4) Unbalanced snow loads have been considered for this design.
  - 5) This truss has been designed for greater of min roof live load of 17.0 psf or 2.00 times flat roof load of 46.2 psf on overhangs non-concurrent with other live loads.
  - 6) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
  - 7) All plates are MT20 plates unless otherwise indicated.
  - 8) See BEH18 DETAILS for plate placement.
  - 9) Provisions must be made to prevent lateral movement of hinged member(s) during transportation.
  - 10) All additional member connections shall be provided by others for forces as indicated.
  - 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 202 lb uplift at joint 13 and 201 lb uplift at joint 10.
  - 12) This truss has been designed to meet the 2003 IBC Section 2308.10.7.1; 2003 IRC R802.10.2
  - 13) Take precaution to keep the chords in plane, any bending or twisting of the hinge plate must be repaired before the building is put into service.
  - 14) If shown, field installed members are an integral part of this design. To ensure proper performance, all field installed members must be installed prior to applying any loading to the truss.



11/3/10

PFS Corporation  
Northeast Region  
APPROVED  
H Raup - 3  
2/27/15  
Approval limited to  
Factory Built Portion

**WARNING - Verify design parameters and READ NOTES**

This building component has only been designed for the loads noted on this drawing. Construction and lifting forces have not been considered. The builder is responsible for lifting methods and system design. Builder responsibilities are defined under TPI1. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult BCSI 1-06 from the Wood Truss Council of America and Truss Plate Institute Recommendation available from WTCA, 6300 Enterprise LN, Madison, WI 53719. J:\support\MitekSupp\templates\ufp.tpe© copyright 2010 by: Universal Forest Products, Inc.

