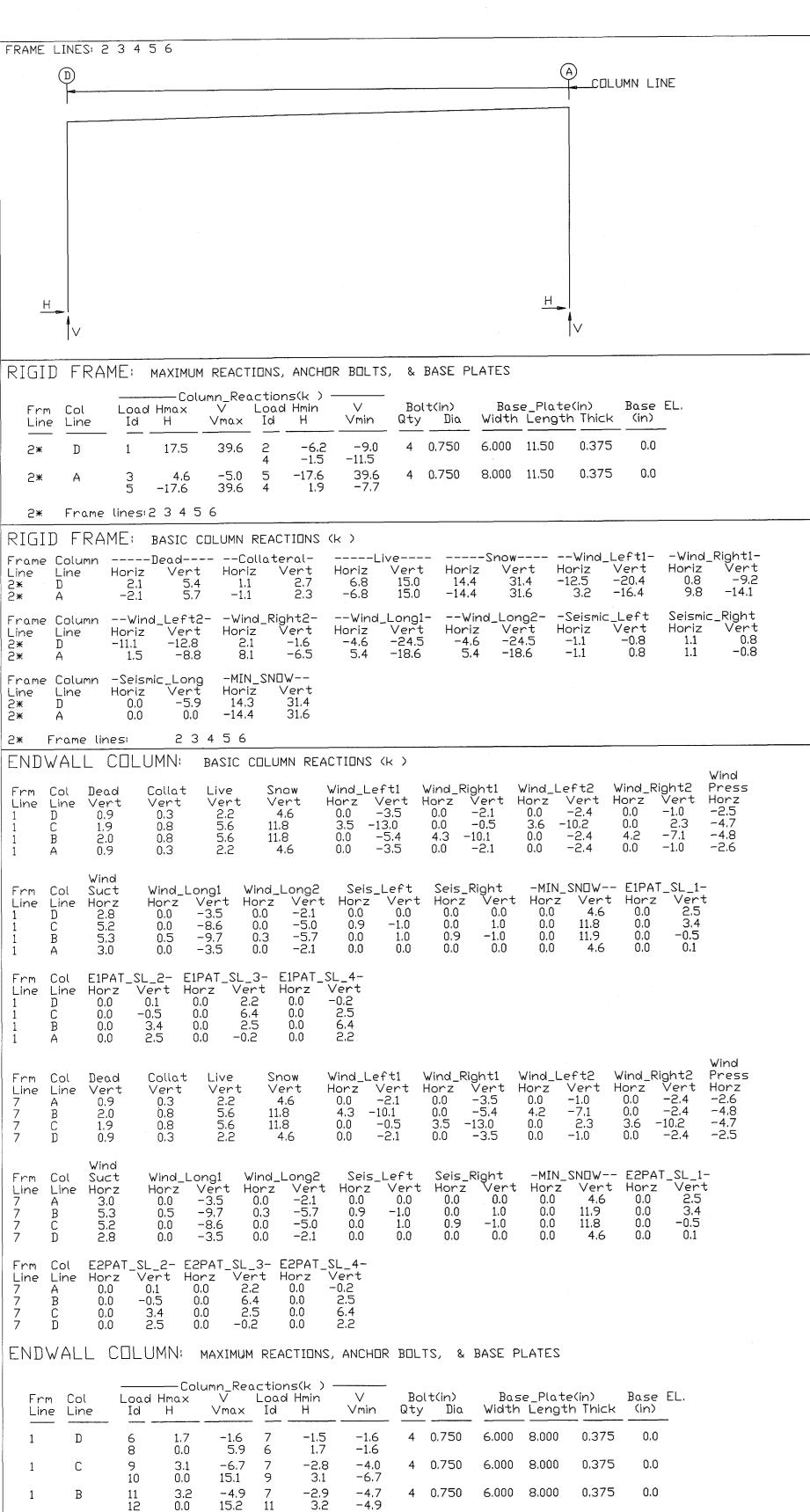




Dia= 3/4"	Dia= 3/4"	Dia= 3/4"	Dia= 3/4"	Dia= 1/2"	Dia= 1/2"	Dia = 3/4''
				. 3" .	. 3" .	6" 1'-2"
- - - % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % % %	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\					
				m 0 7 1/2"	\ \tag{8} \ \tag{9} \ \tag{2} \ 1/2"	
			<u> </u>	2 3/4"	2 3/4"	
1 1/2" 1 1/2" 8"	1 1/2" 1 1/2" 8"	1 1/2" 1 1/2" 8"	1 1/2" 1 1/2" 8"	EW/	EW/	SW
1 1/2 1 1/2	-EW	SW SW	/Sw	SW	SW 2"	9" 3"
See Plan	1'-0"	See Plan	See Plan	See Plan	See Plan	
DETAIL A	DETAIL B	DETAIL C	DETAIL D	DETAIL E Base EL, 101'-0"	DETAIL F Base EL. 103'-4"	DETAIL G

					•
REV.	DESCRIPTI	[O N:	DATE	DRAFT	ENG.
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⚠	INITIAL DRAWING: RELEASED FOR CONSTRUCT	TION	CURRENT	REVISI]N: 0
PACI	KAGE STEEL SYSTEMS, INC. B	Biskup Construction Inc.			
				1 //	//

PACKAG	E STEEL SYSTEMS, INC.	Biskup Constr	ruction Inc.				
PROJECT	320 P Street LLC	ANCHOR BOLT PLAN	ANCHOR BOLT PLAN & DETAILS				
ID	1805-076	DESIGN: ZRM	DESIGN CHECK: ZRM	/ PACKAGE			
PROJECT	314 Presumpscott St.	DRAFT: TMZ	DRAFT CHECK: TMZ				
ADDRESS	Portland, ME 04103	DATE: 5/30/18	DRAWING: ABLT-1				



ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm	Col	Load	Hmax	V	Load	Hmin	\vee		t(in)		e_Plate(Base EL,
Line	Line	Id	Н	Vmax	Id	Н	Vmin	Qty	Dia	Width	Length	Thick	(in)
1	D	6 8	1.7 0.0	-1.6 5.9	7 6	-1.5 1.7	-1.6 -1.6	4	0.750	6,000	8,000	0.375	0.0
1	С	9 10	3.1 0.0	-6.7 15.1	7 9	-2.8 3.1	-4.0 -6.7	4	0.750	6.000	8.000	0.375	0.0
1	В	11 12	3.2 0.0	-4.9 15.2	7 11	-2.9 3.2	-4.7 -4.9	4	0.750	6,000	8.000	0.375	0.0
1	Α	6 13	1.8 0.0	-1.6 6.0	7 6	-1.6 1.8	−1.6 −1.6	4	0.750	6.000	8.000	0.375	0.0
7	Α	6 14	1.8 0.0	-1.6 6.0	7 6	-1.6 1.8	−1.6 −1.6	4	0.750	6.000	8.000	0.375	0.0
7	В	9 15	3.2 0.0	-4.9 15.2	7 9	-2,9 3,2	-4.7 -4.9	4	0.750	6,000	8.000	0.375	0.0
7	С	11 16	3.1 0.0	-6.7 15.1	7 11	-2.8 3.1	-4.0 -6.7	4	0.750	6.000	8.000	0.375	0.0
7	D	6 17	1.7 0.0	-1.6 5.9	7 6	-1.5 1.7	-1.6 -1.6	4	0.750	6.000	8.000	0.375	0.0

NOTES FOR REACTIONS

The following Design Data is per Package Steel Systems, Inc.'s standard design practices and established procedures and recommendations of the following Organizations and/or Specifications.

American Institute of Steel Construction (AISC 2005) American Welding Society Structural Welding Code (AWS D1.1) North American United States (NAUS07)

- 1. For maximum reactions tables, all loading conditions are examined and only the maximum/minimum horizontal or vertical reactions along with the corresponding horizontal or vertical for those
- load IDs are reported. 2. Positive reactions are shown in the sketch. Foundation loads are in the opposite directions.
- 3. Bracing reactions are in the plane of the brace with the horizontal pointing away from the braced bay. The vertical reaction can be downward or upward.
- 4. Reactions given are based on the design data below. Reactions are not furnished for loads not
- 5. The endwall column reaction tables include wind and seismic reactions from the endwall bracing. Horizontal reactions shown in the tables, except for "Wind Press" and "Wind Suct" are in the plane of the wall, perpendicular to the web of the columns.
- 6. The rigid frame reaction tables include the vertical component of the wind and seismic reaction from the sidewall bracing. The horizontal component of the sidewall bracing from wind and seism is in the plane of the wall, perpendicular to the web of the columns and should be combined wit the appropriate basic column reactions to determine the maximum reactions for foundation design
- 7. Foundation construction and design is not the responsibility of Package Steel Systems, Inc. The embedment of the anchor bolts in concrete is the responsibility of the foundation designer.
- 8. Suggested anchor rod diameter, quantity, minimum projection and placement are shown. All anchor rods are assumed to be ASTM F1544 Grade 36 or equal. Anchor rods (not by PSS) shall be set to
- 9. Column base plates are designed not to exceed a bearing pressure of 1050 pounds per sq. inch (0.35f'c where f'c= 3000 psi) unless noted otherwise.
- 10. Basic design wind pressure is furnished. For components and cladding not specifically designed and/or furnished by PSS, the design pressures and suctions shall be increased based on tributary area and location. Confirmation of the design loads and adequacy to resist such loads shall be the responsibility of a licensed design professional by others.

Building Reactions are based on the following information:

a tolerance of +-1/8" in both elevation and location.

Governina Code: Building Risk Category: II - Normal Occupancy Classification: Mixed, Group B Business & F-1 Moderate Hazard Factory Industrial

Rairaind 2156:	
Width (ft.) Length (ft.) Back Side Eave Height (ft.) Front Side Eave Height (ft.) Back Side Roof Slope Front Side Roof Slope	60'-0" 150'-0" 22'-9" 24'-0" 0.25:12

Roof Dead, Collateral, & Live Loads:						
Dead Load Collateral Load Live Load Live Load Reduction Taken	5.00 3.00 20.00 No	psf psf psf				
Wind Loads:						

Serviceability Wind 76 mph(10yrMRI) Wind Exposure C Building Enclosure (O/C/P) Closed Internal Pressure Coeff. (GCpi) ±0.18	Building Enclosure (🛮/C/P)	
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Acronyms:
AUXx = Auxiliary Load - Case x
C= Closed
CL= Collateral Load
DL= Dead Load
FxUNB_LL= Unbalanced Live Load for Frame IDx
LL= Max. of (Live or Snow)
LLR= Live Load Unbalanced
LnWndL= Longitudinal Wind Load - Left
LnWndR= Longitudinal Wind Load - Right
mph= miles per hour

HP	1- Miles per Hour
Local 12345678910112131415167	ading Conditions are as follows: Dead+Collateral+Snow+Slide_Snow 0.6Dead+0.6Wind_Left1 0.6Dead+0.6Wind_Right1 0.6Dead+0.6Wind_Long2L Dead+Collateral+MIN_SNOW 0.6Dead+0.6Wind_Suction+0.6Wind_Long1L 0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L 0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L 0.6Dead+0.6Wind_Eft1+0.6Wind_Suction Dead+Collateral+Snow/2+E1PAT_SL_1 0.6Dead+0.6Wind_Right1+0.6Wind_Suction Dead+Collateral+Snow+1.0E1PAT_SL_2 0.6Dead+0.6Wind_Right1+0.6Wind_Suction Dead+Collateral+Snow/2+E1PAT_SL_4 Dead+Collateral+Snow/2+E2PAT_SL_1 Dead+Collateral+Snow/2+E2PAT_SL_1 Dead+Collateral+Snow/2+E2PAT_SL_3 Dead+Collateral+Snow+1.0E2PAT_SL_1 Dead+Collateral+Snow+1.0E2PAT_SL_1

Ground Snow (Pg)

Flat Roof Snow (Pf)

Mimimum Uniform Snow

Snow Exposure Factor (Ce)

Snow Thermal Factor (Ct)

Snow Importance Factor (Is) Sloped Roof Factor (Cs)	1.00 1.00
Seismic Loads:	
Seismic Importance (Ie) Seismic Design Category (A/B/C/D) Site Class-Type Seismic Response Coeff. (Sds) Seismic Response Coeff. (Sd1) Response Modification (MF) Response Modification (BF) Design Base Shear (V) = Longit. Design Base Shear (V) = Transv. Analysis Procedure: Equivalent Later	1.00 B D 0.259 0.126 3.50 5.00 11.36 kips 12.70 kips al Force

Auxiliary Load(s): ______ (3) 250# Unit Heaters Hung From Roof Purlins

BF= Braced Frame MF= Moment Frame P= Partially Enclosed psf= pounds per square foot SEIS= Seismic WLx= Wind Left - Case x WP= Wind Pressure WRx= Wind Right - Case x WS= Wind Suction

WIND BENT REACTIONS

			all Line	Col Line	Wind Horz	± Read N(k) Vert	Seismic	c(k) Vert	Bol Qty	t(in) Dia	Base. Width	_Plate(in) Length	
Н	Н	 F_SW F_SW	A A	4 5	3.8 3.8	7.4 7.4	3,3 3,3	6.5 6.5	4 4	0.750 0.750	8.000	14.000 14.000	0.375 0.375
1	V												

BUILDING BRA	CING REACTIONS

		Col	±	React	tions(k	<)	Panel	.r
		Line						Note
	1		4.3	4.7	0.9	1.0		(a)
	A 7 D	B,C	4.3 7.4			1.0 3.9		ζū
(a)Wir	nd ber	nt in b	ay					

ANCH	OR BOL	T SL	JMMAR	Υ
Qty	Locate	Dia (in)	Туре	Proj (in)
O 36 O 32 O 40 O 8	Jamb Endwall Frame WindCol	1/2" 3/4" 3/4" 3/4"	A307 A307 A307 A307	1.50 2.00 2.00 2.00

General Notes

Design Responsibility:

Package Steel Systems, Inc. (PSS) is responsible only for the structural design of the Metal Building System it sells to the Builder. Neither PSS nor PSS's Engineer is the Design Professional or the Engineer of Record for the Construction Project. PSS is not responsible for the design of any components or materials manufactured or supplied by others or their interaction and connection to the Metal Building System unless such design responsibility is specifically required by the Order Documents.

Close Proximity Structures:

PSS is not responsible for loads (Seismic, Snow, etc.) imposed by, field modifications needed on, or structures in close proximity to this structure. It is the Builder's responsibility to verify that close proximity structures, together with their foundations, are capable of resisting all additional loads that may result from this structure.

Bracing

Metal building brace rods and cables work in pairs to balance the forces caused by initial tensioning. Care must be taken when tightening brace rods or cables so as not to cause accidental damage or misalignment of building components. All rods/cables must be installed loose and then tightened sequentially and equally to maintain proper alignment of components. When properly tightened, rods and cables should not exhibit excessive sag. For long or large rod bracing it may be necessary to support the rod at mid-bay by suspending it from a purlin at the appropriate elevation.

A qualified professional engineer must design bracing for seismic or wind loading of suspended objects that are not part of the PSS structure. The design must meet code requirements and safely deliver the lateral loads to one of the PSS primary bracing systems. In addition, the bracing must be designed and erected in a manner that will not impose torsional or minor axis loads, or cause local failures in any PSS structural components. No material may be cut, drilled, or otherwise removed from any part of this building without the written consent of PSS. The engineer CANNOT rely on the roof deck to act as a diaphragm. PSS accepts no responsibility for the design and installation of bracing for objects that are not furnished or specified by PSS.

Field Work:

60.00 psf

42.00 psf

42.00 psf 1.00 1.00

All local, state, and federal safety regulations are to be strictly followed. Temporary supports or bracing required for the building erection is the responsibility of the erector to determine, furnish and install. It is the responsibility of the Builder/Contractor to obtain appropriate approvals and necessary permits from city, county, state, or federal agencies, as required.

PSS provides complete components to erect all projects with minimal modifications. However, minor fieldwork of structural, secondary, panel, and trim items may be necessary to ensure proper fit. Such work is considered a normal part of metal building erection. Back charges for minor fieldwork will not be honored

Welds shall be made only by operators certified by the standard qualifications procedure of the American Welding Society for the type of weld required. All field welds to be done using E70XX electrodes and in accordance with the American Welding Society Structural Welding Code.

A325 Bolt Tightening Requirements

All high strength bolts are A325-N unless specifically noted otherwise. Structural bolts shall be tightened by the TURN-OF-THE-NUT method in accordance with the ninth-edition AISC "Specifications for Structural Joints using ASTM A325 or A490 Bolts" per section 8D.1. A325 bolts may be installed without washers when tightened by the TURN-OF-THE-NUT method. All high strength bolts, except as noted otherwise, are subject to direct tension and may require inspection as defined by AISC/RCSC "Specifications for Structural joints using ASTM A325 or A490 Bolts and the applicable building code or standard. It is the responsibility of the erector to assure proper tightness.

PSS accepts no responsibility for the consequences of any additions or alterations to this structure. Modifications to this structure must be performed under the supervision of a qualified licensed professional engineer who accepts responsibility for the adequacy and consequences of the additions or alterations.

The primary and secondary framing of this building may have been designed to support additional collateral loads (These loads may include sprinkler systems, mechanical equipment, ducts, ceilings, etc.). Care must be exercised however, to prevent local overstress of light gauge secondary members supporting concentrated loads.

Masonry & Concrete:

PSS accepts no responsibility for the design of masonry walls, concrete walls, Reviewed for Code Compli Permitting and Inspections Department foundations, mezzanine slabs, and floor slabs. Also, the attachment to masonry Approved with Condition or concrete is not designed or supplied by PSS (Masonry anchor sizes, spacing, and quantity, unless specifically stated will be designed and supplied by others). The engineer responsible for the design of the masonry and concrete is also responsible for ensuring that the design (including wall base details) is compatible with the deflection criteria for this building. Eave purlins and rake channels are not designed to support lateral loads from masonry or other walls not by PSS. Values given for bends and anchor bolt total lengths are suggested lengths only. It is the responsibilty of the foundation engineer to determine these values since they are a function of concrete strength as well as other factors,

Base plates are designed assuming concrete has a minimum strength of 3000 psi at 28 days unless otherwise noted.

Jamb foundations should be designed for a shear of 2 kips unless other wise stated.

Independent Mezzanines:

Independent mezzanines must be designed by a qualified professional engineer to meet all code requirements. The engineer must also ensure that proper isolation from the PSS building has been provided to avoid contact with PEMB structure due to differential movement. PSS accepts no responsibility for the design of independent mezzanines,

Panelsi

Dil Canning is an inherent characteristic of cold rolled roof and wall panels. It is the result of several factors that include, but are not limited to, induced stresses in the base material, fabrication methods, installation procedures, and post installation thermal forces. Dil Canning does not affect the structural integrity or overall performance of the metal panels. Dil Canning is an aesthetic issue only and is not grounds for rejection of the panels.

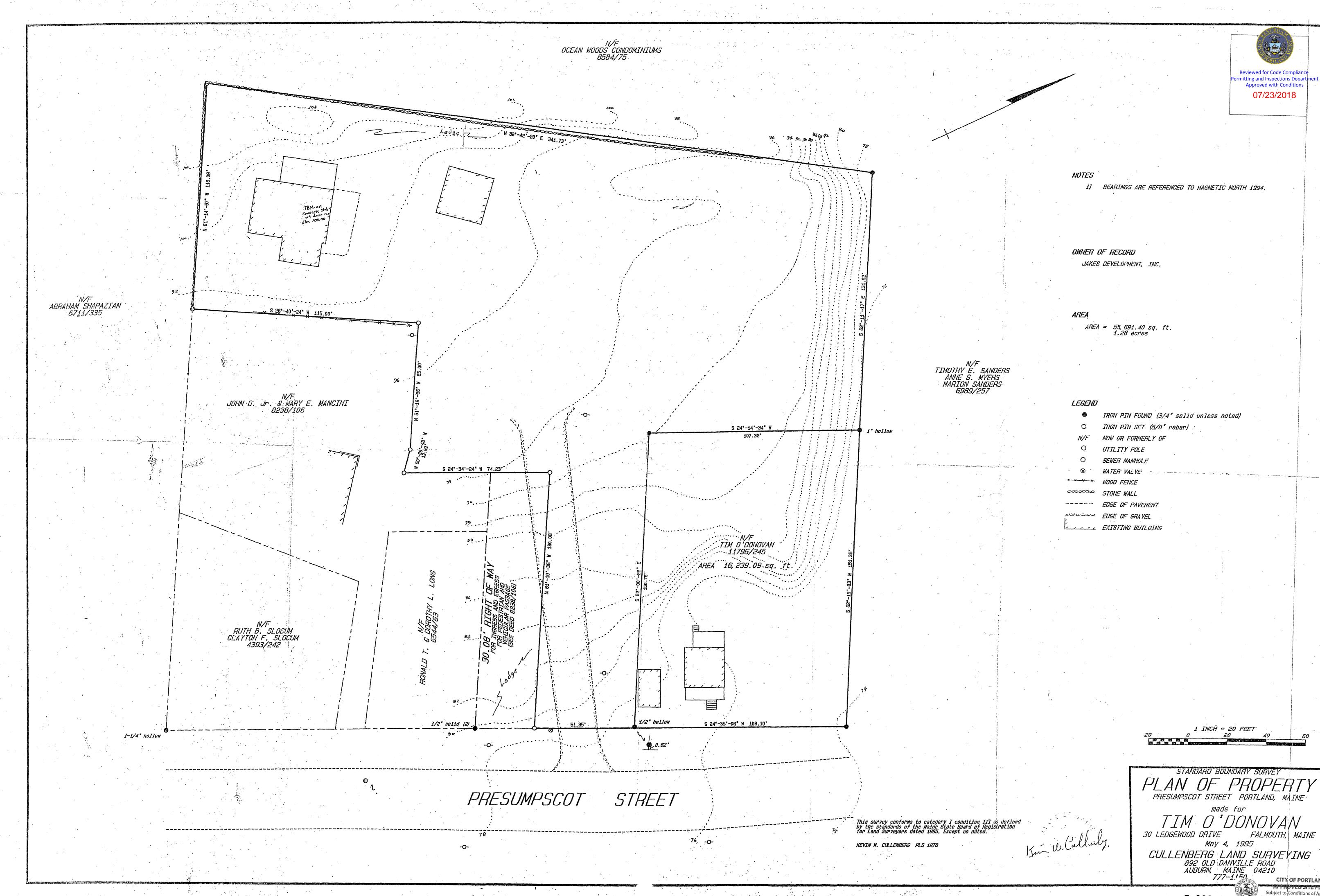
Parapets:

Buildings with parapet walls and internal gutters must be furnished with rainwater overflow mechanisms (such as scuppers) to prevent the accumulation of water in the event of a gutter blockage. It is the responsibility of the Builder to make sure that the scuppers are of the appropriate size, quantity, location, and design to prevent water accumulation on the roof. Failure to do so can result in building collapse. PSS accepts no responsibility for the design and installation of overflow

MATERIALS:	ASTM DESIGNATIONS:	YIELD STRENGTH:
Structural Steel Plate (Built-up Sections)	A529 Grades 50 & 55 A572 Grades 50 & 55 A1011 HSLAS Grades 50 & 55	50 ksi 50 ksi 50 ksi
Hot Rolled Mill Shapes (WF, Channels, Angles)	A36 A572 Grades 50 A992	36 ksi 50 ksi 50 ksi min.
Round Struct, Tubing - Pipe Shaped Struct, Tubing- Tube	A500 Grade B A500 Grade B	42 ksi 46 ksi
	A653 (SS) Grade 50 Class 1, 2, 3 A653 (HSLAS) Grade 50, Types A or B	
Roof and Wall Sheets	A653/A792 SS Grade 50 Class 1 or 2 (AZ55 Coating)	50 ksi
	A755/A792 SS Grade 50 Class 1 or 2 (AZ50 Coating)	50 ksi
Brace Rods	A529	50 ksi
Brace Angles	A36	36 ksi
Structural Cables (Cable Bracing)	A475 7-wire EHS Grade	
Cable Hardware	A536 Grade 65-45-12	45 ksi
Bolts	A307 Grade A SAE-J429 Grade 2 A325 Type 1	60 ksi (tensile strength 120 ksi, 105 ksi
Nuts	A563 Grade A SAE-J995 Grade 2 A563 Grade C, D or DH (A325)	
Washers (Hardened) Washers (Plain)	F436 Type 1 F844	
Anchor Bolts	A307 unless otherwise noted	

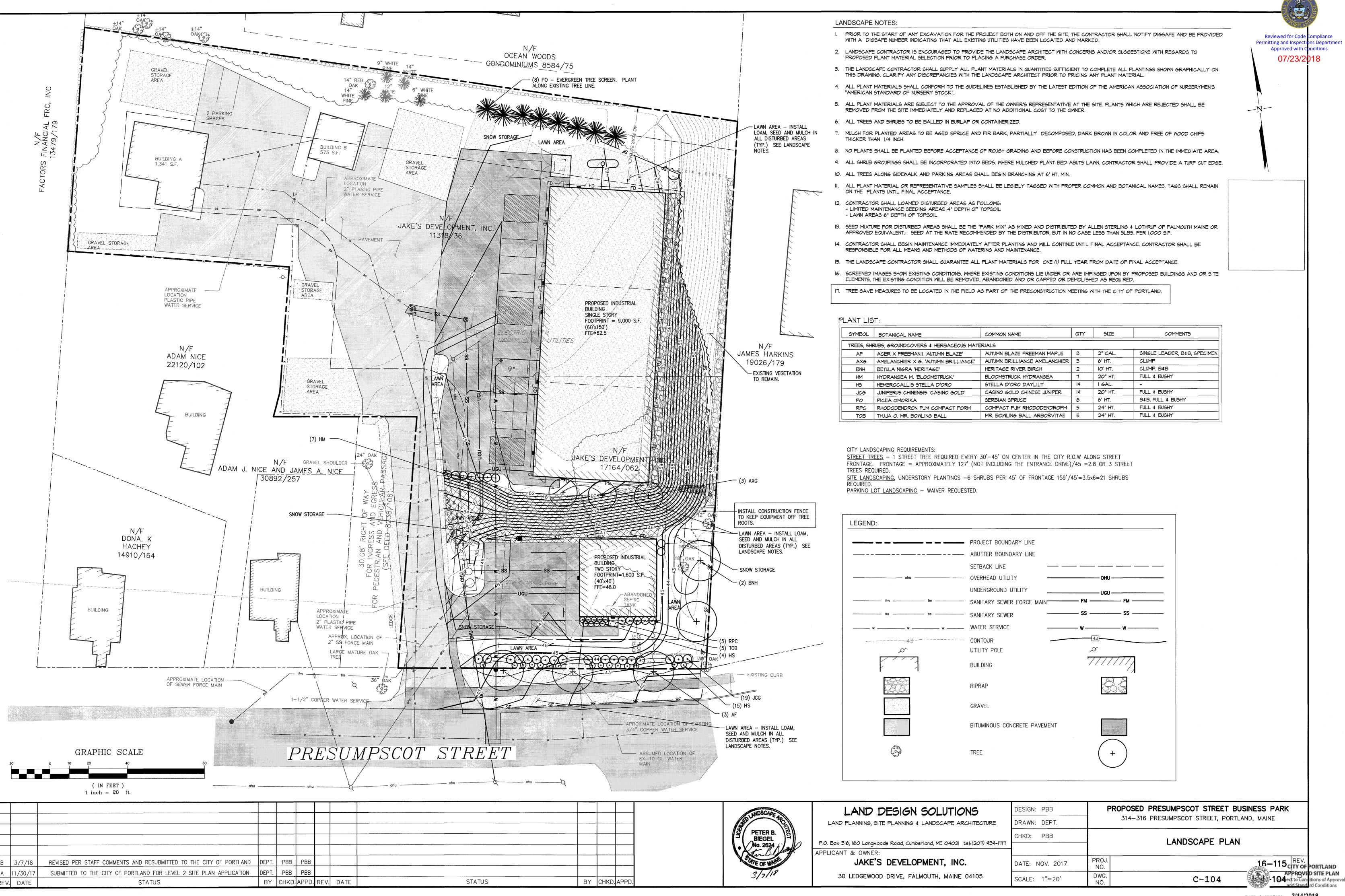


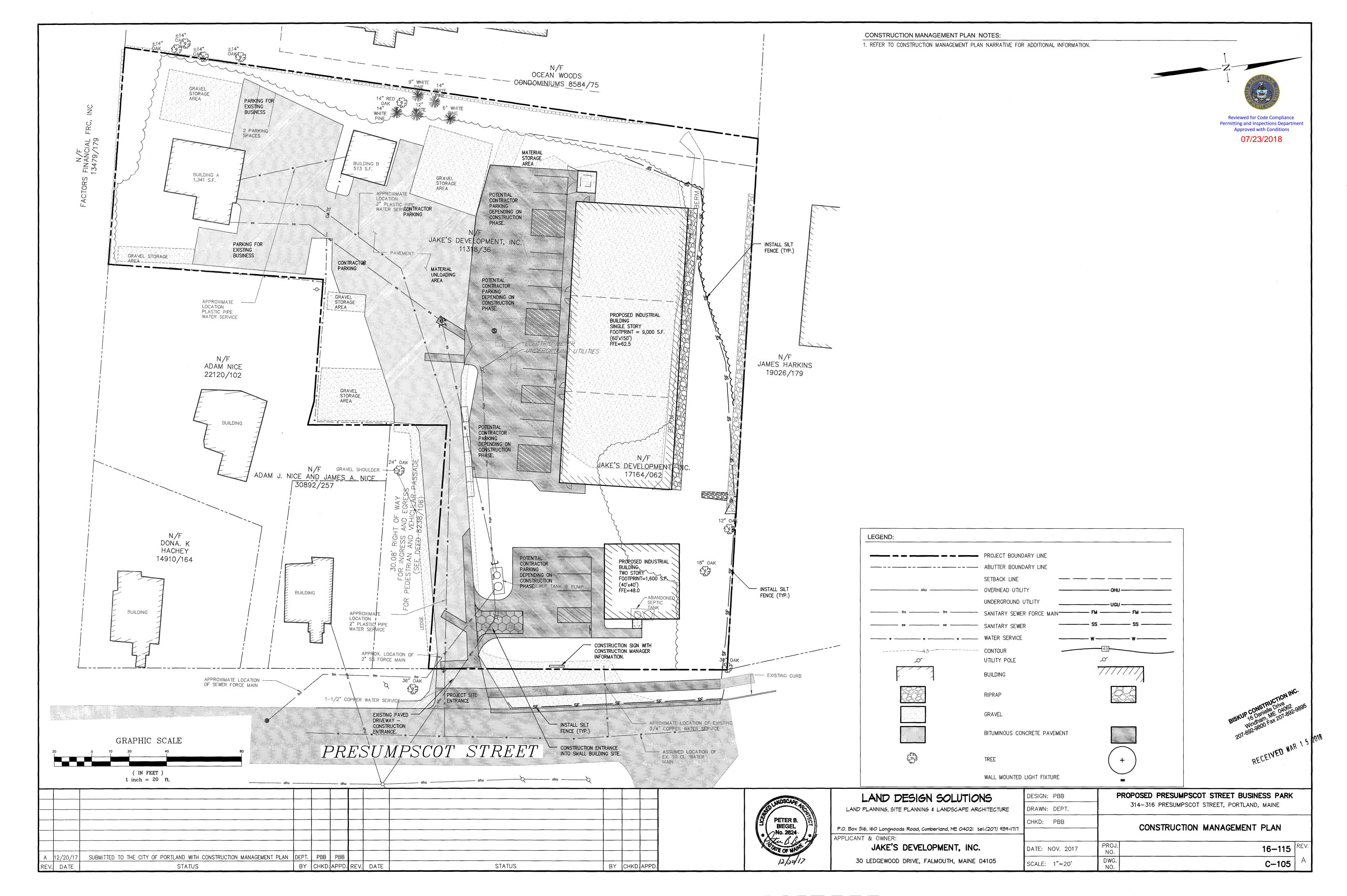
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PROJE	CT	320 P Street LLC	ANCHOR BOLT REACT	TIONS & NOTES			140.55
ID		1805-076	DESIGN: ZRM	DESIGN CHEC	K: ZRM	/ PAC	KAGE
PROJE	CT	314 Presumpscott St.	DRAFT: TMZ	DRAFT CHECK	: TMZ		_//
ADDRE	222	Portland, ME 04103	DATF: 5/30/18	TRAWING: ARI	T-2	1/ /	



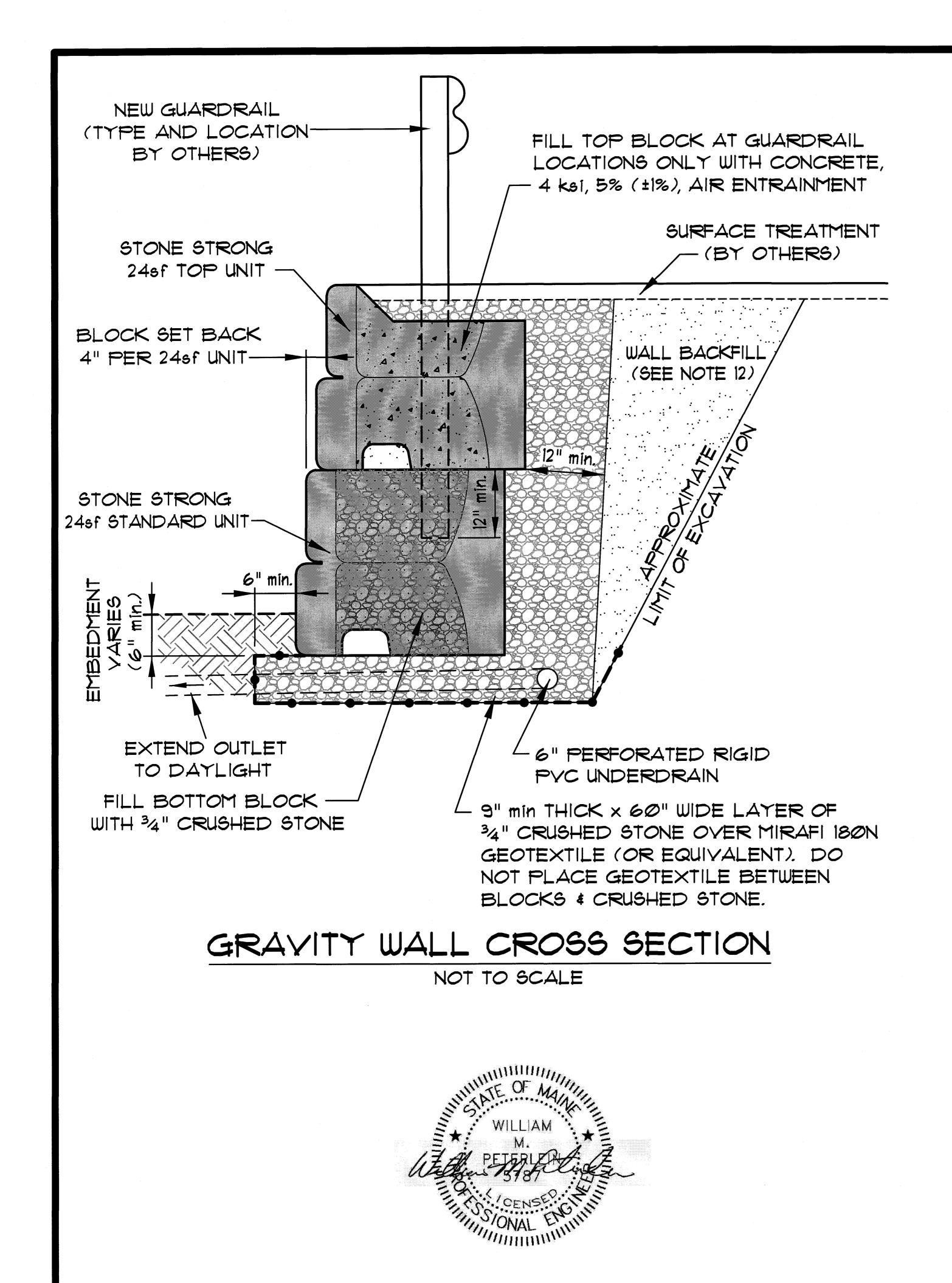
C-099.1 Subject to

DATE of APPROVAL 3/14/2018





WALL NOTES



GENERAL NOTES

- 1) CROSS SECTION HEIGHT IS BASED UPON A PLAN ENTITLED "GRADING, DRAINAGE AND EROSION CONTROL PLAN", REVISION DATED NOVEMBER 30, 2017, PREPARED BY LAND DESIGN SOLUTIONS.
- 2) IT IS THE RESPONSIBILITY OF THE OWNER, CONTRACTOR OR THEIR RESPECTIVE REPRESENTATIVES TO ENSURE THAT CONSTRUCTION OF THE WALL AND MATERIALS USED IN THE CONSTRUCTION OF THE WALL ARE IN ACCORDANCE WITH THESE SPECIFICATIONS AND/OR THE CONTRACT SPECIFICATIONS WHICH EVER ARE MORE STRINGENT
- 3) SUMMIT GEOENGINEERING SERVICES (SGS) ACCEPTS NO RESPONSIBILITY NOR LIABILITY IN THE DETERMINATION OF THE ADEQUACY OF SITE MATERIALS AND WALL LAYOUT
- 4) PRIOR TO THE START OF CONSTRUCTION THE CONTRACTOR SHALL VERIFY THAT THE WALL HEIGHT DOES NOT EXCEED 6' FROM BOTTOM OF WALL TO TOP OF WALL. ANY DISCREPANCY SHALL BE BROUGHT TO THE ATTENTION OF SGS PRIOR TO THE START OF CONSTRUCTION.
- 5) THE FOLLOWING PARAMETERS WERE USED IN THE DESIGN ARE BASED ON A GEOTECHNICAL INVESTIGATION PERFORMED BY SGS: A) SLOPE AT TOP: 4°
 - B) MAXIMUM SLOPE AT BASE = LEVEL
 - C) GROUNDWATER CONTROLLED TO BELOW BASE OF WALL
 - D) MAXIMUM CONTACT PRESSURE AT WALL BASE IS LESS THAN 1,000 psf
 - E) RETAINED (EXISTING FILL) uw = 130 pcf, phi = 32°
- F) FOUNDATION SOIL UW = 120 pcf, phi = 30° IF ACTUAL CONDITIONS VARY FROM THOSE LISTED ABOVE, SGS SHALL BE NOTIFIED IMMEDIATELY.
- TO CONFIRM WITH THE JURISDICTIONAL AUTHORITY PRIOR TO CONSTRUCTION OF THE WALL WHETHER A CONSTRUCTION CERTIFICATION IS REQUIRED. A CONSTRUCTION CERTIFICATION WILL REQUIRE ON-SITE INSPECTIONS, MATERIAL TESTING, PHOTOGRAPHIC DOCUMENTATION AND OTHER QUALITY CONTROL MEASURES AND AS-BUILT DOCUMENTATION. SGS CAN PROVIDE CERTIFICATION, IF REQUIRED, UNDER A SEPARATE CONTRACT.
- 1) THE OWNER IS RESPONSIBLE TO RETAIN THE SERVICES OF A QUALIFIED CONSTRUCTION MATERIALS FIRM TO PERFORM COMPACTION TESTS ON WALL BACKFILL TO CONFIRM THAT THE MINIMUM COMPACTION REQUIREMENTS ARE MET.

WALL INSTALLATION

- 8) THE STONE STRONG WALL SYSTEM SHALL BE INSTALLED IN ACCORDANCE WITH THE STONE STRONG MANUALS (www.stonestrong.com).
- 9) FOUNDATION EXCAYATION SHALL EXTEND TO COMPETENT SOIL. ALL EXISTING TOPSOIL, LOOSE MATERIAL, ORGANIC SOIL AND OTHER SOFT OR UNSTABLE FOUNDATION SOILS SHALL BE REMOVED FROM THE AREA TO BE OCCUPIED BY THE WALL AND ENGINEER.
- 10) UPON COMPLETION OF THE EXCAYATION, THE WALL BASE SUBGRADE SHALL BE PROOF ROLLED BY MAKING A MINIMUM OF 6 PASSES USING A LARGE VIBRATORY PLATE COMPACTOR.
- 11) INSTALL A 9" (MINIMUM THICK) LAYER OF COMPACTED 34" CRUSHED STONE ON TOP OF THE GEOTEXTILE LAYER FOR BLOCK WALL BLOCKS.
- 12) INSTALL THE BASE COURSE OF BLOCKS ON A PREPARED FOUNDATION LEVELING PAD. ENSURE THAT THE BASE COURSE IS LEVEL SIDE TO SIDE AND PLUMB. ADJUST BLOCKS AS REQUIRED TO PROVIDE A STRAIGHT AND LEVEL BASE COURSE. PLACE AND BACKFILL ONLY ONE COURSE OF BLOCKS AT A TIME. DO NOT STACK BLOCKS PRIOR TO BACKFILLING.
- 95% OF ASTM DI557. FIELD DENSITY TESTS SHALL BE PERFORMED AT A MINIMUM RATE OF 1 TEST PER EVERY OTHER LIFT FOR EVERY 100' OF WALL (3 TESTS PER EVERY OTHER LIFT, MINIMUM)
- BACKFILL, A TEMPORARY SOIL BERM SHALL BE CONSTRUCTED NEAR THE CREST OF THE GRAVITY STRUCTURES TO PREVENT SURFACE WATER RUNOFF FROM OVERTOPPING THE WALLS.

MATERIAL SPECIFICATIONS

15) 34" CRUSHED DRAINAGE STONE SHALL BE CLEAN ANGULAR CRUSHED STONE MEETING THE FOLLOWING GRADATION AS DETERMINED IN ACCORDANCE WITH ASTM D422

₩ TEE.	
EVE SIZE	PERCENT PASSIN
1"	100
1/4"	9Ø - 1ØØ
3/8"	2 0 - 55
No. 4	0 -10
No 8	0 - 5

16) WALL BACKFILL SHALL MEET THE FOLLOWING GRADATION SPECIFICATIONS (MDOT 703.06 TYPE D)

> SIEVE SIZE PERCENT PASSING 35 - 80 25 - 65 0 - 30

17) THE MAXIMUM PARTICLE SIZE SHALL BE LIMITED TO 6".

18) BLOCKS SHALL BE 3 s.f., 6 s.f., 9 s.f. AND 24 s.f. "STONE STRONG", MANUFACTURED BY PRECAST CONCRETE

19) GEOTEXTILE SHALL CONSIST OF 180N OR APPROVED EQUIVALENT.

20) UNDERDRAIN PIPE SHALL CONSIST OF 4" PERFORATED SDR 35 PVC OR APPROVED EQUIVALENT.



CITY OF PORTLAND APPROVEDESTE PLAN

3/14 2018 **DATE of APPROVAL**

2017-286

THE FOLLOWING PLAN FOR CONTROLLING SEDIMENTATION AND EROSION IN THIS PROJECT IS BASED ON CONSERVATION PRACTICES FOUND IN THE MAINE EROSION 4 SEDIMENT CONTROL BMPS MANUAL, MAINE DEPARTMENT OF ENVIRONMENTAL PROTECTION, MARCH 2003, OR LATEST EDITION. THE CONTRACTOR WHO IMPLEMENTS THIS PLAN SHALL BE FAMILIAR WITH THIS PUBLICATION AND ADHERE TO IT AND THE PRACTICES PRESENTED HEREIN.

REFERENCE IS MADE TO THE GRADING AND DRAINAGE PLANS (C3.0) WITHIN THE PLAN SET, SHOWING THE LOCATIONS AND TYPES OF PROPOSED MEASURES TO BE

GENERAL EROSION AND SEDIMENTATION CONTROL PRACTICES

THE FOLLOWING IS A LIST OF GENERAL EROSION CONTROL PRACTICES THAT WILL BE USED TO PREVENT EROSION AND SEDIMENTATION BEFORE, DURING AND AFTER THE CONSTRUCTION OF THIS PROJECT. IN ADDITION, SPECIAL CARE SHALL BE USED AT ALL TIMES TO: 1) LIMIT DISTURBANCE AND, HENCE, EROSION

2) CORRECT ANY EROSION PROBLEMS IMMEDIATELY 3) REGULARLY MONITOR THE IMPLEMENTED PRACTICES, ESPECIALLY AFTER EVERY RAINFALL

4) REVEGETATE DISTURBED AREAS AS SOON AS POSSIBLE AFTER CONSTRUCTION 5) CONFORM TO ALL REQUIREMENTS/STANDARDS OF THE SITE'S MAINE DEP EROSION & SEDIMENT CONTROL BMP MANUAL.

SILT FENCE AND/OR EROSION CONTROL MIX SEDIMENT BARRIERS
SILT FENCE AND/OR EROSION CONTROL MIX SEDIMENT BARRIERS WILL BE INSTALLED ALONG THE DOWN GRADIENT SIDE OF THE PROPOSED GROUND DISTURBANCE AREAS PRIOR TO ANY CONSTRUCTION ACTIVITIES.

CATCH BASIN PROTECTION WILL BE INSTALLED AT THE FIRST DOWNGRADIENT CATCH BASIN IN STREET ADJACENT TO ANY CONSTRUCTION ACTIVITIES.

CONSTRUCTION PHASE

THE FOLLOWING GENERAL PRACTICES WILL BE IMPLEMENTED TO PREVENT EROSION DURING CONSTRUCTION ON THIS PROJECT:

- I. ONLY THOSE AREAS UNDER ACTIVE CONSTRUCTION WILL BE CLEARED AND LEFT IN AN UNTREATED OR UNVEGETATED CONDITION. ONCE CONSTRUCTION OF AN AREA IS COMPLETE, FINAL GRADING, LOAMING AND SEEDING SHALL OCCUR IMMEDIATELY (REFER TO "POST CONSTRUCTION REVEGETATION" SECTION). IF DURING FINAL GRADING, LOAMING AND SEEDING CAN NOT OCCUR IMMEDIATELY, IT SHALL BE DONE PRIOR TO ANY STORM EVENT AND WITHIN 15 DAYS OF COMPLETING CONSTRUCTION IN THE AREA. IF FINAL GRADING, LOAMING AND SEEDING CANNOT OCCUR WITHIN 7 DAYS, OR IF THE AREA IS NOT UNDER ACTIVE CONSTRUCTION FOR A PERIOD LONGER THAN 7 DAYS, SEE ITEM NO. 4 BELOW.
- 2. PRIOR TO THE START OF CONSTRUCTION IN A SPECIFIC AREA, SILT FENCING SHALL BE INSTALLED ON DOWNGRADIENT PORTIONS OF THE SITE AS LOCATED ON THE PLANS TO PROTECT AGAINST ANY CONSTRUCTION RELATED EROSION.
- 3. TOPSOIL WILL BE STOCKPILED WHEN NECESSARY IN AREAS WHICH HAVE MINIMUM POTENTIAL FOR EROSION AND WILL BE KEPT AS FAR AS POSSIBLE FROM EXISTING DRAINAGE AREAS AND WETLANDS. ALL STOCKPILES EXPECTED TO REMAIN LONGER THAN 7 DAYS SHALL BE:
- A. TREATED WITH ANCHORED MULCH (WITHIN 5 DAYS OF THE LAST DEPOSIT OF STOCKPILED SOIL).
- B. SEEDED WITH CONSERVATION MIX AND MULCHED IMMEDIATELY.
- STOCKPILES SHALL BE EITHER PLACED UPHILL OF AN EXISTING SEDIMENT BARRIER ON THE SITE OR ENCIRCLED BY A HAY BALE OR SILT FENCE BARRIER THE FIRST DAY THAT STOCKPILING COMMENCES.
- 4. ALL DISTURBED AREAS EXPECTED TO REMAIN LONGER THAN 7 DAYS SHALL BE:
- A. TREATED WITH STRAW AT A RATE OF 70-90 LBS. PER 1000 SQUARE FEET FROM 4/16 TO 10/1, OR AT A RATE OF 150-200 LBS. PER 1000 SQUARE FEET FROM 10/1 TO 4/15.
- B. SEEDED WITH CONSERVATION MIX OF PERENNIAL RYE GRASS (1.0 LBS/1000 SQ.FT.) AND MULCHED IMMEDIATELY. FROM 10/1 TO 4/15, FOLLOW THE SEEDING RATES AS OUTLINED BELOW IN SUB-SECTION 4.D. OF THE "POST CONSTRUCTION REVEGETATION" SECTION.
- C. MONITORED EVERY TWO WEEKS UNTIL SEEDING CAN OCCUR AND REMULCHED AS NEEDED TO PROTECT SLOPES.
- 5. ALL GRADING WILL BE HELD TO A MAXIMUM 3:1 SLOPE WHERE PRACTICAL. GREATER SLOPES MAY BE USED WHERE THE BANKS ARE PROTECTED WITH SOFT ARMOUR MATTING, EROSION CONTROL MATTING, OR RIPRAP. ALL SLOPES WILL BE STABILIZED WITH PERMANENT SEEDING IMMEDIATELY AFTER FINAL GRADING IS COMPLETE. (IT IS UNDERSTOOD THAT IMMEDIATELY MEANS WITHIN 5 DAYS OF THE COMPLETION OF WORK. SEE POST-CONSTRUCTION REVEGETATION FOR SEEDING SPECIFICATION.)
- 6. CONSTRUCTION TRAFFIC WILL BE DIRECTED OVER THE EXISTING SITE ENTRANCE. THE ROAD SHALL BE SWEPT DAILY SHOULD SEDIMENT BE TRACKED ONTO

1. ALL DEWATERING DISCHARGE LOCATIONS SHALL BE LOCATED ON RELATIVELY FLAT GROUND AT LEAST 75' FROM STREAMS AND 25' FROM WETLANDS. THE CONTRACTOR SHALL UTILIZE DIRTBAGS, EROSION CONTROL MIX BERMS, OR SIMILAR METHODS FOR FILTRATION OF DEWATERING AND SHALL CONFORM TO THE MAINE EROSION AND SEDIMENT CONTROL BMPS G-1, G-2, AND G-3.

POST CONSTRUCTION REVEGETATION

THE FOLLOWING GENERAL PRACTICES WILL BE IMPLEMENTED TO PREVENT EROSION AS SOON AS AN AREA IS READY TO UNDERGO FINAL GRADING: I. A MINIMUM OF 6" OF LOAM WILL BE SPREAD OVER DISTURBED AREAS AND GRADED TO A UNIFORM DEPTH AND NATURAL APPEARANCE.

- 2. LAWN AREAS: "ESTATE GREEN" GRASS SEED BY ALLEN, STERLING \$ LOTHROP (FALMOUTH, MAINE), OR APPROVED EQUAL.
- 3. MULCH SHALL BE HAY OR STRAW MULCHES THAT ARE DRY AND FREE FROM UNDESIRABLE SEEDS AND COURSE MATERIALS.
- A. APPLICATION RATE MUST BE 2 BALES (70-90 LBS.) PER 1,000 SQUARE FEET OR 1.5 TO 2 TONS (90-100 BALES) PER ACRE TO COVER 75 TO 90% OF THE GROUND SURFACE.
- B. DRIVE OVER WITH TRACKED CONSTRUCTION EQUIPMENT ON GRADES OF 5% AND LESS. C. BLANKET WITH TACKED PHOTODEGRADABLE/BIODEGRADABLE NETTING ON GRADES GREATER THAN 5%.
- 4. HYDRO-MULCH SHALL CONSIST OF A MIXTURE OF ASPHALT, WOOD FIBRE OR PAPER FIBRE AND WATER, WHICH IS SPRAYED OVER A SEEDED AREA. HYDRO-MULCH SHALL NOT BE USED BETWEEN 10/1 AND 4/15.
- 5. CONSTRUCTION SHALL BE PLANNED TO ELIMINATE THE NEED FOR SEEDING BETWEEN OCTOBER 1ST AND APRIL 15TH. SHOULD SEEDING BE NECESSARY BETWEEN THESE DATES, THE FOLLOWING PROCEDURE SHALL BE FOLLOWED:
- A. ONLY UNFROZEN LOAM SHALL BE USED.
- B. LOAMING, SEEDING AND MULCHING WILL NOT BE DONE OVER SNOW OR ICE COVER. IF SNOW EXISTS, IT MUST BE REMOVED PRIOR TO PLACEMENT OF
- C. WHERE PERMANENT SEEDING IS NECESSARY, ANNUAL WINTER RYE (1.2 LBS/1000 S.F.) SHALL BE SOWN INSTEAD OF THE PREVIOUSLY NOTED SEEDING
- D. WHERE TEMPORARY SEEDING IS REQUIRED, ANNUAL WINTER RYE (2.5 LBS/1000 S.F.) SHALL BE SOWN INSTEAD OF THE PREVIOUSLY NOTED SEEDING
- E. FERTILIZING, SEEDING AND MULCHING SHALL BE DONE ON LOAM THE DAY THE LOAM IS SPREAD.
- F. HAY MULCH SHALL BE SECURED WITH PHOTODEGRADABLE/BIODEGRADABLE NETTING. TRACKING BY MACHINERY ALONE WILL NOT SUFFICE. WINTER MULCHING RATES, AS SPECIFIED ABOVE IN SUBSECTION 5.A. OF THE "CONSTRUCTION PHASE" SECTION, SHOULD BE APPLIED DURING THIS PERIOD.
- 5. FOLLOWING FINAL SEEDING, THE SITE WILL BE INSPECTED EVERY 30 DAYS UNTIL 80% COVER HAS BEEN ESTABLISHED. RESEEDING WILL BE CARRIED OUT BY THE CONTRACTOR WITHIN TO DAYS OF NOTIFICATION BY THE DESIGN PROFESSIONAL THAT THE EXISTING CATCH IS INADEQUATE.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR INSTALLING, MONITORING, MAINTAINING, REPAIRING, REPLACING AND REMOVING ALL OF THE EROSION AND SEDIMENTATION CONTROLS OR APPOINTING A QUALIFIED SUBCONTRACTOR TO DO SO.

- MAINTENANCE MEASURES WILL BE APPLIED AS NEEDED DURING THE ENTIRE CONSTRUCTION CYCLE. IMMEDIATELY FOLLOWING ANY SIGNIFICANT RAINFALL, AND AT LEAST ONCE A WEEK, A VISUAL INSPECTION WILL BE MADE OF ALL EROSION AND SEDIMENTATION CONTROLS AS FOLLOWS:
- 1. SILT FENCE SHALL BE INSPECTED AND REPAIRED. SEDIMENT TRAPPED BEHIND THESE BARRIERS SHALL BE EXCAVATED WHEN IT REACHES A DEPTH OF 6" AND REDISTRIBUTED TO AREAS UNDERGOING FINAL GRADING.
- 2. CONSTRUCTION ENTRANCE SHALL BE VISUALLY INSPECTED AND REPAIRED AS NEEDED. ANY AREAS SUBJECT TO RUTTING SHALL BE STABILIZED IMMEDIATELY. IF THE VOIDS OF THE CONSTRUCTION ENTRANCE BECOME FILLED WITH MUD, MORE CRUSHED STONE SHALL BE ADDED AS NEEDED. THE PUBLIC ROADWAY SHALL BE SWEPT SHOULD MUD BE DEPOSITED/TRACKED ONTO THEM.

STANDARDS FOR STABLIZING SITES FOR THE WINTER

THE FOLLOWING STANDARDS AND METHODOLOGIES SHALL BE USED FOR STABILIZING THE SITE DURING THE WINTER CONSTRUCTION PERIOD

- I. STANDARD FOR THE TIMELY STABILIZATION OF DISTURBED SLOPES (ANY AREA HAVING A GRADE GREATER THAN 25%) THE CONTRACTOR WILL SEED AND MULCH ALL SLOPES TO BE VEGETATED BY SEPTEMBER 15TH. IF THE CONTRACTOR FAILS TO STABILIZE ANY SLOPE TO BE VEGETATED BY SEPTEMBER 15TH, THEN THE CONTRACTOR WILL TAKE ONE OF THE FOLLOWING ACTIONS TO STABILIZE THE SLOPE FOR LATE FALL AND WINTER.
- A. STABILIZE THE SOIL WITH TEMPORARY VEGETATION AND EROSION CONTROL MATS BY OCTOBER 1ST THE CONTRACTOR WILL SEED THE DISTURBED SLOPE WITH WINTER RYE AT A RATE OF 3 POUNDS PER 1000 SQUARE FEET AND THEN INSTALL EROSION CONTROL MATS OR ANCHORED HAY MULCH OVER THE SEEDING. THE CONTRACTOR WILL MONITOR GROWTH OF THE RYE OVER THE NEXT 30 DAYS.
- B. STABILIZE THE SLOPE WITH WOOD-WASTE COMPOST THE CONTRACTOR WILL PLACE A SIX-INCH LAYER OF WOOD-WASTE COMPOST ON THE SLOPE BY NOVEMBER 15TH. THE CONTRACTOR WILL NOT USE WOOD-WASTE COMPOST TO STABILIZE SLOPES HAVING GRADES GREATER THAN 50% (2H:IV) OR HAVING GROUNDWATER SEEPS ON THE SLOPE FACE.

- C. STABILIZE THE SLOPE WITH STONE RIPRAP THE CONTRACTOR WILL PLACE A LAYER OF STONE RIPRAP ON THE SLOPE BY NOVEMBER 15TH. THE DEVELOPMENT'S OWNER WILL HIRE A REGISTERED PROFESSIONAL ENGINEER TO DETERMINE THE STONE SIZE NEEDED FOR STABILITY ON THE SLOPE AND TO DESIGN A FILTER LAYER FOR UNDERNEATH THE RIPRAP.
- 2. STANDARD FOR THE TIMELY STABILIZATION OF DISTURBED SOILS BY SEPTEMBER 15TH THE CONTRACTOR WILL SEED AND MULCH ALL DISTURBED SOILS ON THE SITE. IF THE CONTRACTOR FAILS TO STABILIZE THESE SOILS BY THIS DATE, THEN THE CONTRACTOR WILL TAKE ON OF THE FOLLOWING ACTIONS TO STABILIZE THE SOIL FOR LATE FALL AND WINTER.
- A. STABILIZE THE SOIL WITH TEMPORARY VEGETATION BY OCTOBER 1ST THE CONTRACTOR WILL SEED THE DISTURBED SOIL WITH WINTER RYE AT A SEEDING RATE OF 3 POUNDS PER 1000 SQUARE FEET, LIGHTLY MULCH THE SEEDED SOIL WITH HAY OR STRAW AT 75 POUNDS PER 1000 SQUARE FEET, AND ANCHOR THE MULCH WITH PLASTIC NETTING. THE CONTRACTOR WILL MONITOR GROWTH OF THE RYE OVER THE NEXT 30 DAYS. IF THE RYE FAILS TO GROW AT LEAST 75% OF THE DISTURBED SOIL BEFORE NOVEMBER, 1, THEN THE CONTRACTOR WILL MULCH THE AREA FOR OVER-WINTER PROTECTION AS DESCRIBED IN ITEM III OF THIS STANDARD.
- B. STABILIZE THE SOIL WITH SOD THE CONTRACTOR WILL STABILIZE THE DISTURBED SOIL WITH PROPERLY INSTALLED SOD BY OCTOBER 1ST. PROPER INSTALLATION INCLUDES THE CONTRACTOR PINNING THE SOD ONTO THE SOIL WITH WIRE PINS, ROLLING THE SOD TO GUARANTEE CONTACT BETWEEN THE SOD AND UNDERLYING SOIL, AND WATERING THE SOD TO PROMOTE ROOT GROWTH INTO THE DISTURBED SOIL.
- C. STABILIZE THE SOIL WITH MULCH BY NOVEMBER 15TH THE CONTRACTOR WILL MULCH THE DISTURBED SOIL BY SPREADING HAY OR STRAW AT A RATE OF AT LEAST 150 POUNDS PER 1000 SQUARE FEET ON THE AREA SO THAT NO SOIL IS VISIBLE THROUGH THE MULCH. IMMEDIATELY AFTER APPLYING THE MULCH, THE CONTRACTOR WILL ANCHOR THE MULCH WITH NETTING OR OTHER METHOD TO PREVENT WIND FROM MOVING THE MULCH OFF THE

EROSION CONTROL REMOVAL

AN AREA IS CONSIDERED STABLE IF IT IS PAVED OR IF 80% GROWTH OF PLANTED SEEDS IS ESTABLISHED. ONCE AN AREA IS CONSIDERED STABLE, THE EROSION CONTROL MEASURES CAN BE REMOVED AS FOLLOWS:

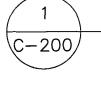
SILT FENCE SHALL BE DISPOSED OF LEGALLY AND PROPERLY OFF-SITE. ALL SEDIMENT TRAPPED BEHIND THESE CONTROLS SHALL BE DISTRIBUTED TO AN

AREA UNDERGOING FINAL GRADING OR REMOVED AND RELOCATED OFF-SITE. CATCH BASIN INLET PROTECTION SHALL BE REMOVED FOLLOWING PERMANENT STABILIZZTION OF UPGRADIENT AREAS. SEDIEMENT SHALL BE REMOVED

FROM TEH SACK AND LEGALLY DISPOSED. SEDIMENT SHALL NOT BE WASHED INTO THE CATCH BASIN.

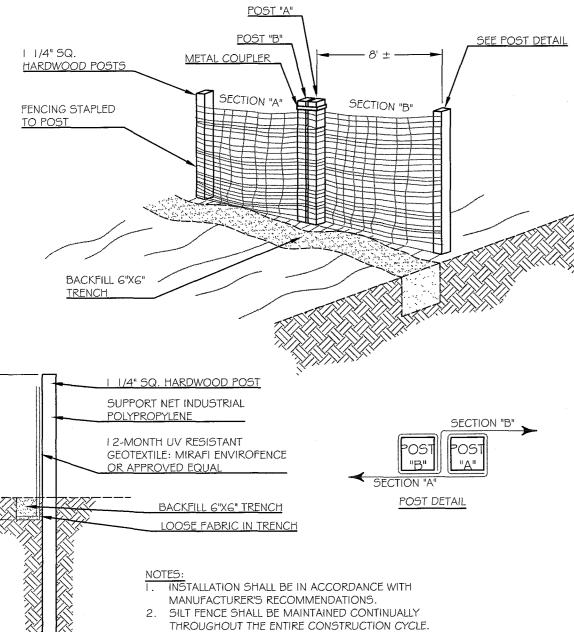
ONCE ALL THE TRAPPED SEDIMENTS HAVE BEEN REMOVED FROM THE TEMPORARY SEDIMENTATION DEVICES THE DISTURBED AREAS MUST BE REGRADED IN AN AESTHETIC MANNER TO CONFORM TO THE SURROUNDING TOPOGRAPHY. ONCE GRADED THESE DISTURBED AREAS MUST BE LOAMED (IF NECESSARY), FERTILIZED, SEEDED AND MULCHED IN ACCORDANCE WITH THE RATES PREVIOUSLY STATED.

THE ABOVE EROSION CONTROLS MUST BE REMOVED WITHIN 30 DAYS OF FINAL STABILIZATION OF THE SITE. CONFORMANCE WITH THIS PLAN AND FOLLOWING THESE PRACTICES WILL RESULT IN A PROJECT THAT COMPLIES WITH THE STATE REGULATIONS AND THE STANDARDS OF THE NATURAL RESOURCES PROTECTION ACT, AND WILL PROTECT WATER QUALITY IN AREAS DOWNSTREAM FROM THE PROJECT.



NOT TO SCALE

EROSION AND SEDIMENTATION CONTROL NOTES



PREFABRICATED SILT FENCE

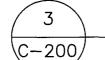
NOT TO SCALE

(C-200*/*

DIVERSION RIDGE REQUIRED WHERE GRADE EXCEEDS 2% 2% OR GREATER STRAW BALES, SANDBAGS. OR CONTINUOUS BERM OF QUIVALENT HEIGHT SECTION A-A SPILLWAY SUPPLY WATER TO WASH USE SANDBAGS, STRAW WHEELS IF NECESSARY BALES OR OTHER APPROVED METHODS TO CHANNELIZE RUNOFF TO BASIN AS REQUIRED 2-3" (50-75 mm) COURSE AGGREGATE MIN. 6" (150mm) THICK

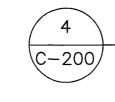
THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION THAT WILL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC RIGHT-OF-WAYS. THIS MAY REQUIRE TOP DRESSING, REPAIR AND/OR CLEAN OUT OF ANY MEASURES USED TO TRAP SEDIMENT

WHEN NECESSARY, WHEELS SHALL BE CLEANED PRIOR TO ENTRANCE ONTO PUBLIC RIGHT-OF-WAY. 3. WHEN WASHING IS REQUIRED, IT SHALL BE DONE ON AN AREA STABILIZED WITH CRUSHED STONE THAT DRAINS ONTO AN APPROVED SEDIMENT TRAP OR SEDIMENT BASIN.



STABILIZED CONSTRUCTION ENTRANCE

NOT TO SCALE



STRAW MATTING AND TRM EROSION CONTROL

USE STAPLES RECOMMENDED

BY MANUFACTURER

INSTALLATION INSTRUCTIONS:

TURF REINFORCEMENT MAT (TRM) MATERIAL SHALL BE ENKAMAT 7020, OR APPROVED EQUAL.

3.2. OVER TOP THE 5" OF LOAM, UNROLL MAT IN THE DIRECTION OF WATER FLOW.

BRIDGE DEPRESSIONS IN THE SURFACE AND ALLOW EROSION UNDERNEATH.

9. OVERLAP ADJACENT EDGES OF MAT BY THREE (3) INCHES (MIN.) AND STAKE.

12. STAKE OVERLAPS LONGITUDINALLY AT THREE (3) TO FIVE (5) FOOT INTERVALS.

10. USE WOOD STAKES OR STAPLES FOR PINNING MAT TO THE GROUND SURFACE, PER

2. EROSION CONTROL BLANKET (ECB) SHALL BE BIONET S75BN SINGLE NET STRAW BLANKET BY NORTH

4. MAT SHOULD LIE FLAT. DO NOT STRETCH MAT OVER GROUND. STRETCHING MAY CAUSE MAT TO

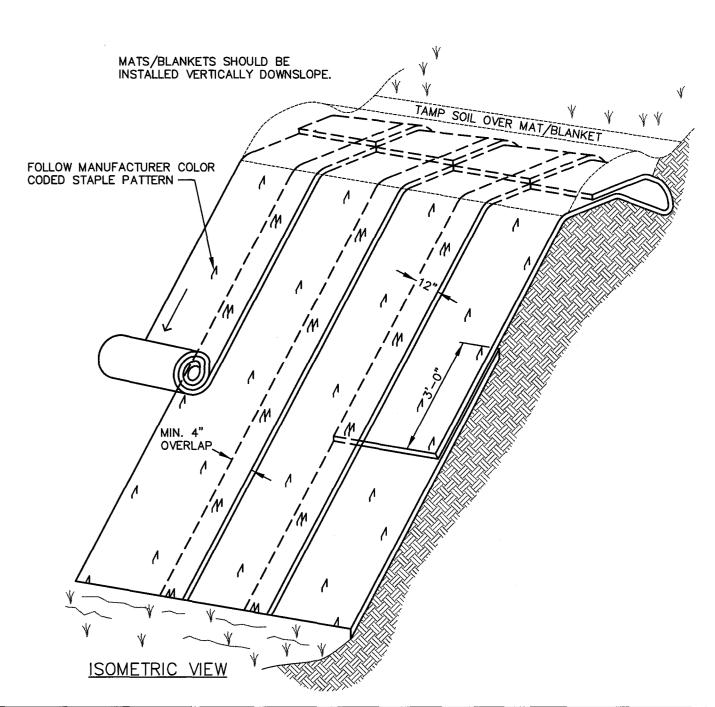
5. BURY TRANSVERSE TERMINAL ENDS OF MAT TO SECURE AND PREVENT EROSIVE FLOW UNDERNEATH

7. BACKFILL AND COMPACT TRENCHES AND CHECK SLOTS AFTER STAKING THE MAT IN BOTTOM OF

8. OVERLAP ROLL ENDS BY THREE (3) FEET (MIN.) WITH UPSLOPE MAT ON TOP TO PREVENT UPLIFT OF MAT END BY WATER FLOW. IF INSTALLING IN THE DIRECTION OF A CONCENTRATED WATER FLOW,

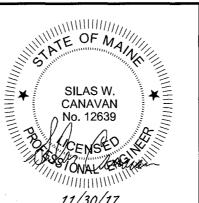
II. IN ALL TRANSVERSE TERMINAL TRENCHES AND CHECK SLOTS, STAKE EACH MAT AT ITS CENTER AND





SCALE: N.T.S.

									J		
Α	11/30/17	SUBMITTED TO THE CITY OF PORTLAND FOR LEVEL 2 SITE PLAN APPLICATION	DEPT.	SWC	PBB						
	. DATE	STATUS	BY	CHKD	.APPD	REV.	DATE	STATUS	BY	CHKD.	APPD.



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AMERICAN GREEN OR APPROVED EQUAL.

3.1. APPLY 5" OF LOAM ONTO THE GROUND SURFACE

START NEW ROLLS IN A TRANSVERSE DITCH.

MANUFACTURER'S RECOMMENDATIONS.

6. SECURE MAT SNUGLY INTO ALL TRANSVERSE CHECK SLOTS.

OVERLAP EDGES BEFORE BACKFILLING AND COMPACTING.

3. FOR TRM INSTALLATION ONLY:

LAND DESIGN SOLUTIONS LAND PLANNING, SITE PLANNING & LANDSCAPE ARCHITECTURE

30 LEDGEWOOD DRIVE, FALMOUTH, MAINE 04105

P.O. Box 316, 160 Longwoods Road, Cumberland, ME 04021 tel:(207) 939-1717 JAKE'S DEVELOPMENT, INC.

PROPOSED PRESUMPSCOT STREET BUSINESS PARK DESIGN: PBB 314-316 PRESUMPSCOT STREET, PORTLAND, MAINE DRAWN: DEPT. CHKD: PBB **EROSION AND SEDIMENTATION CONTROL** NOTES AND DETAILS

DATE: NOV. 2017 NO. DWG. C-200

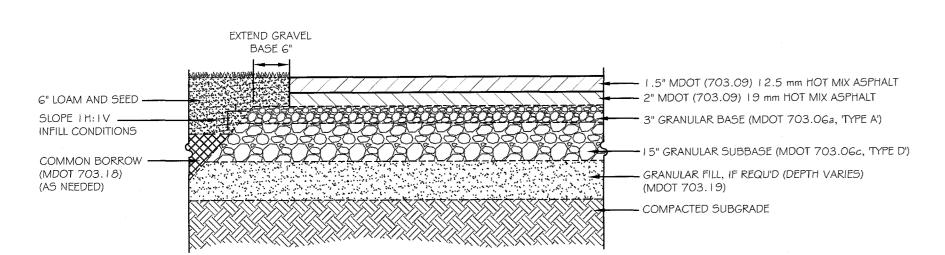
Christian Roadman

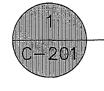
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Approved with Conditions

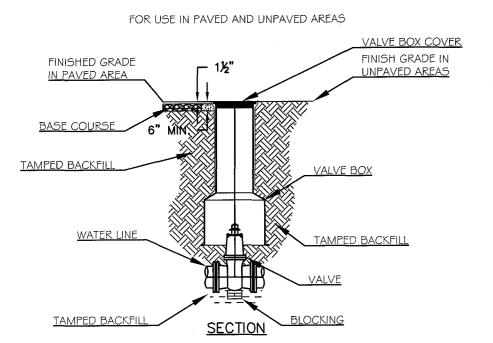
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BITUMINOUS CONCRETE PAVEMENT SECTION

NOT TO SCALE



NOTES:

I. ALL WORK TO CONFORM TO THE PORTLAND WATER DISTRICT STANDARDS AND REQUIREMENTS.

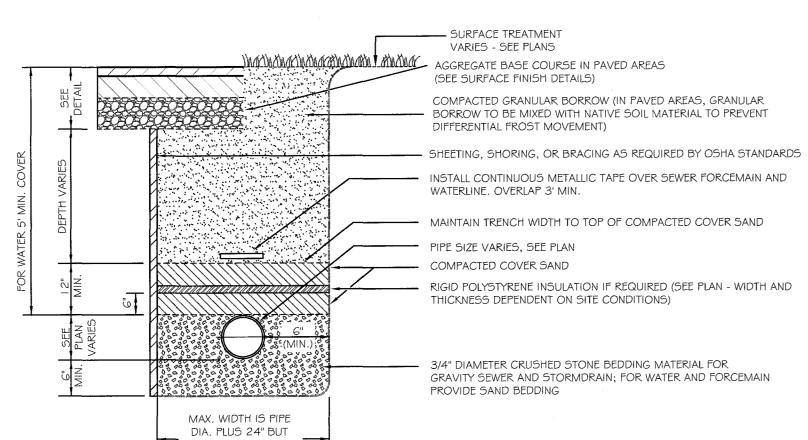
2. D.I.P. MAY BE USED FOR VALVE BOX EXTENSIONS.

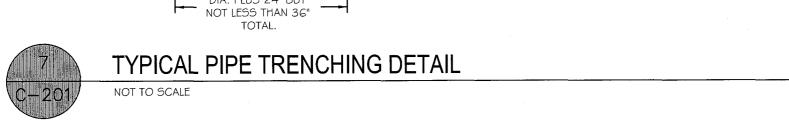
3. VALVE BOX SHOULD NOT CONTACT WATER MAIN OR VALVE.

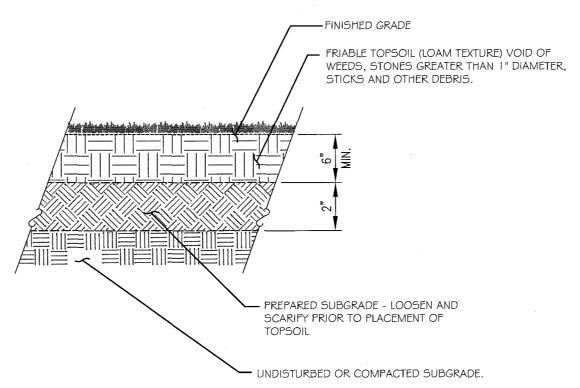
VALVE AND VALVE BOX INSTALLATION DETAIL

NOT TO SCALE

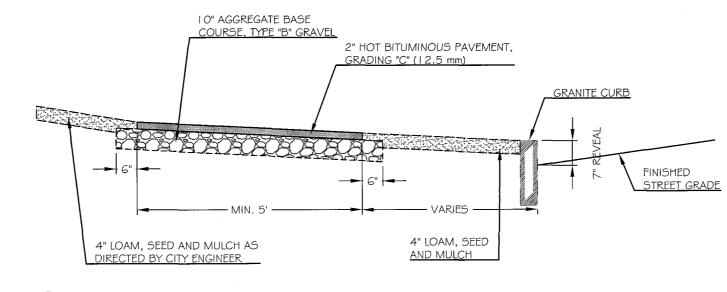
4. CONCRETE PROTECTOR RING SHALL BE USED IN ALL UNPAVED AREAS.



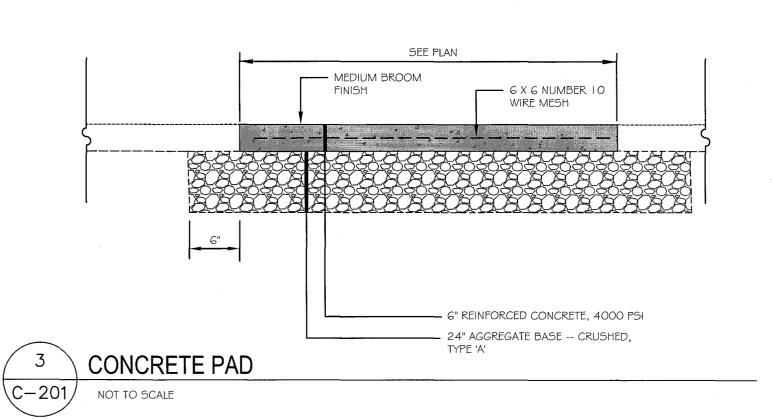


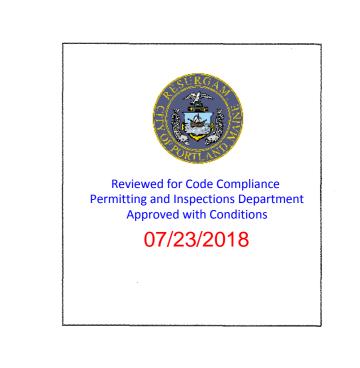






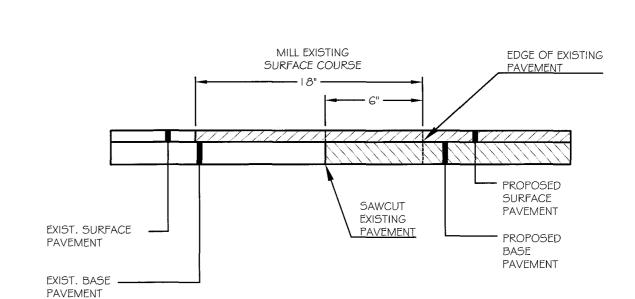




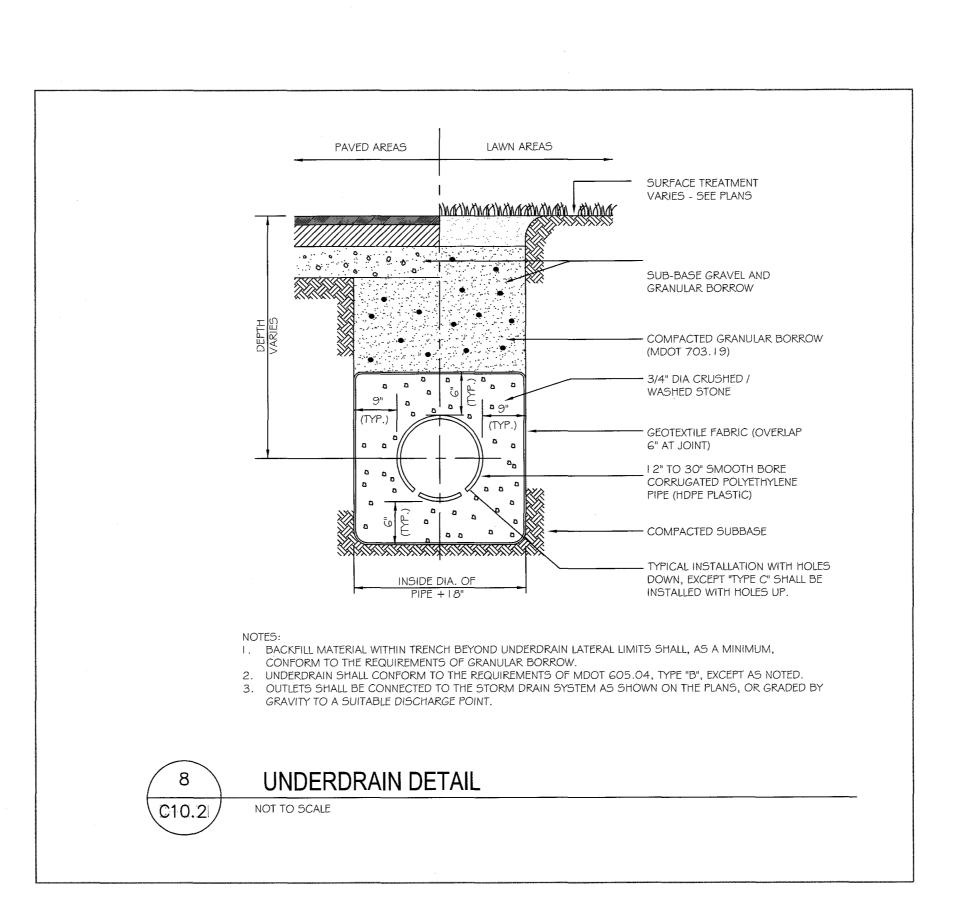


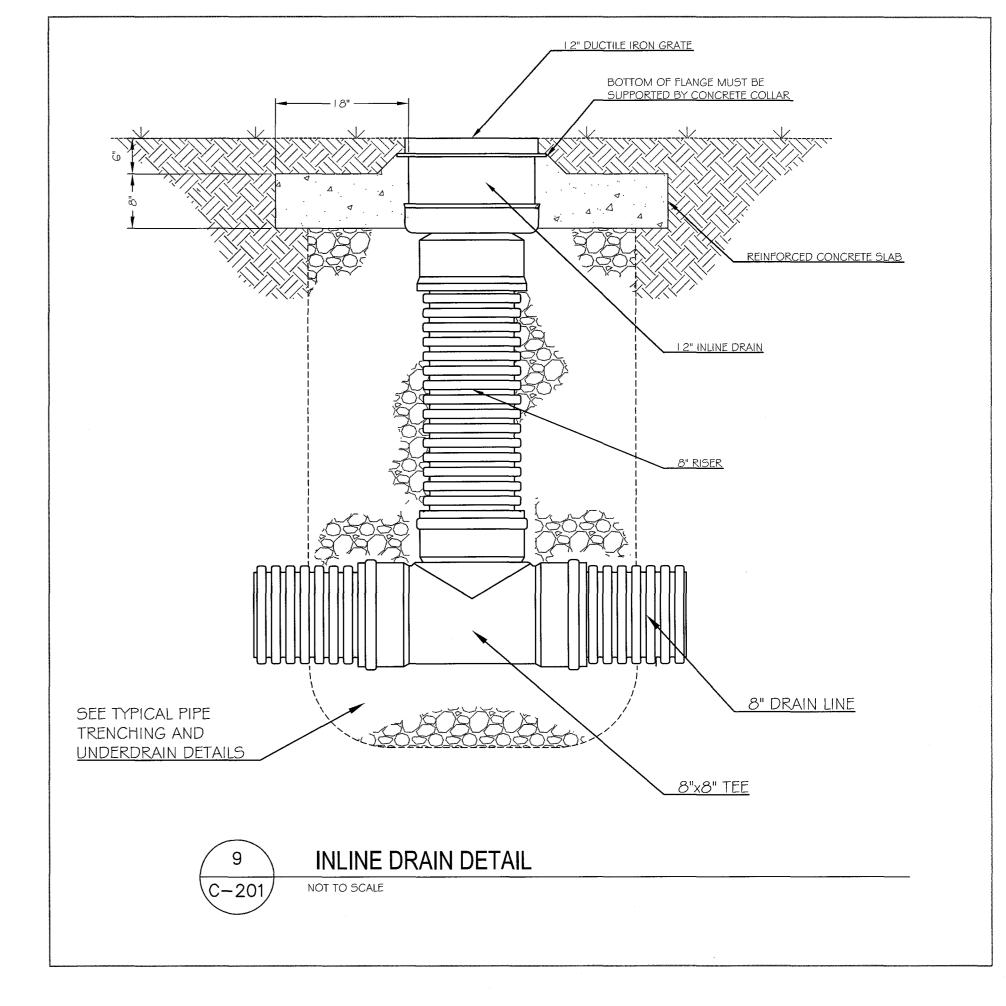
PLANNER Christian Roadman

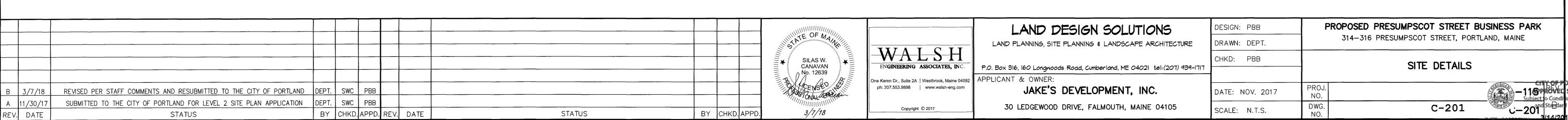
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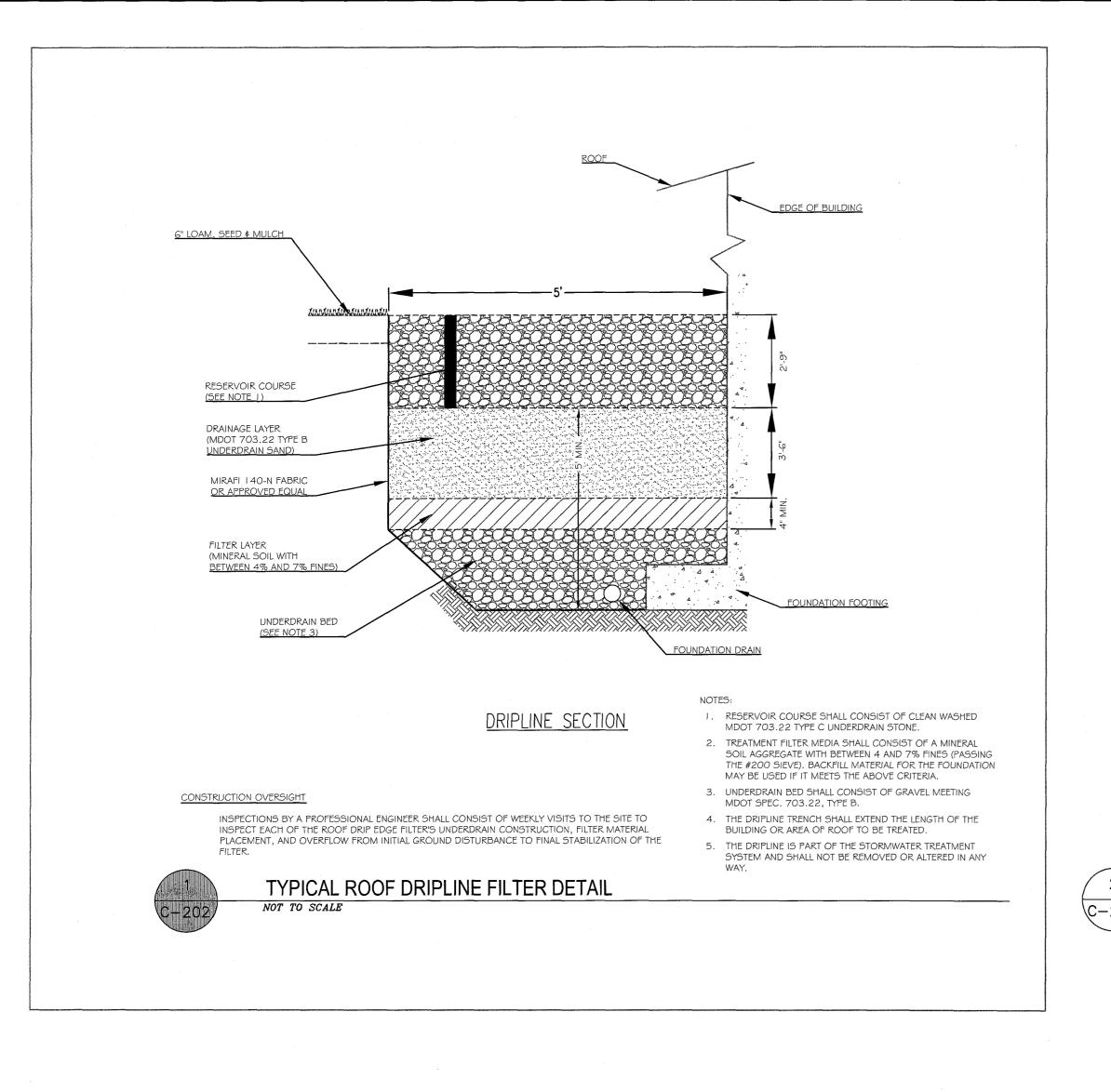


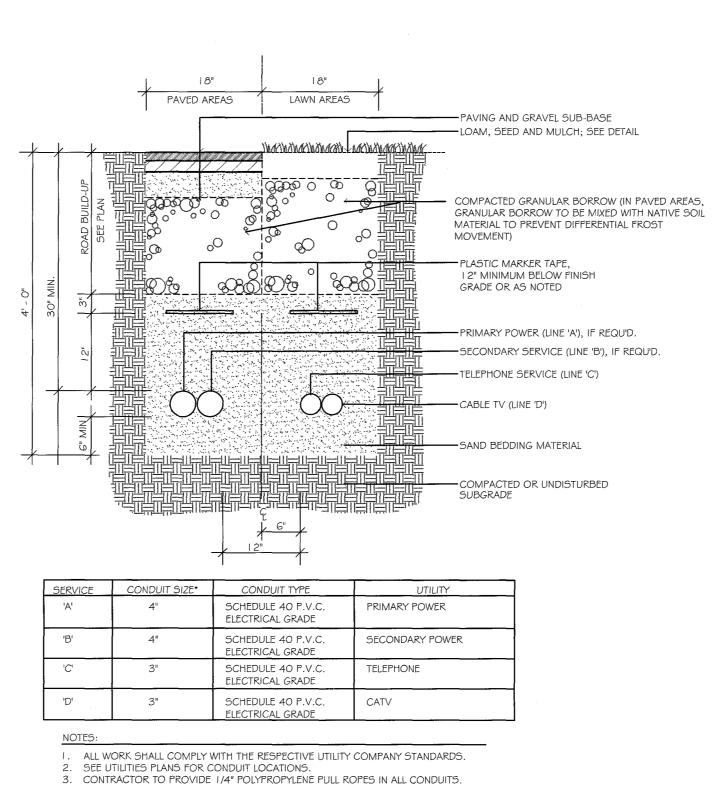








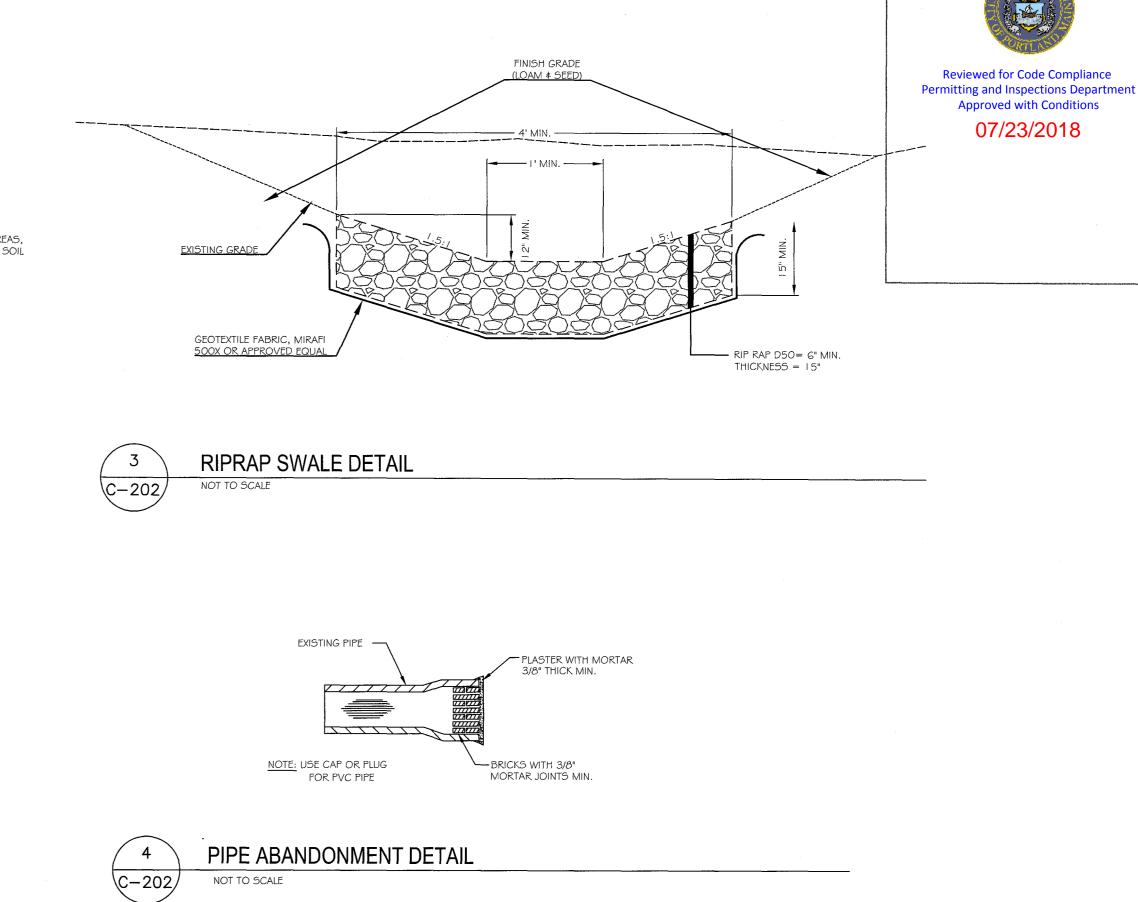


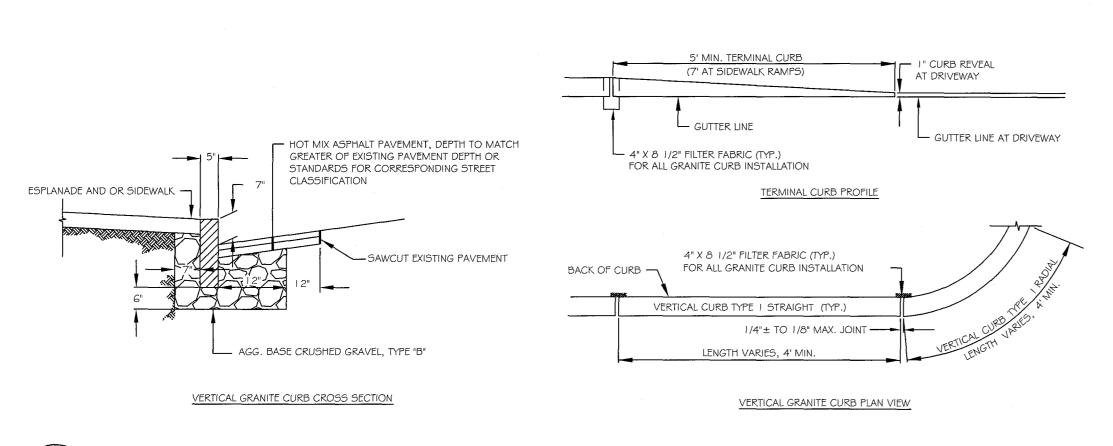


4. CONTRACTOR SHALL VERIFY CONDUIT SIZE WITH APPLICABLE UTILITY COMPANIES.

NOT TO SCALE

UNDERGROUND UTILITY TRENCH SECTION (MULITPLE CONDUITS)





GRANITE CURB INSTALLATION IN EXISTING STREETS DETAIL

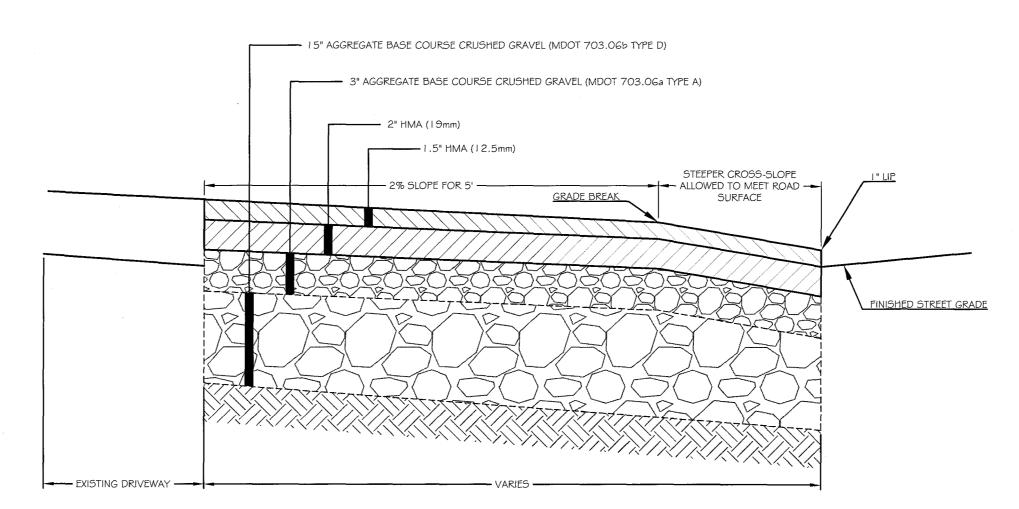
DEPT. SWC PBB

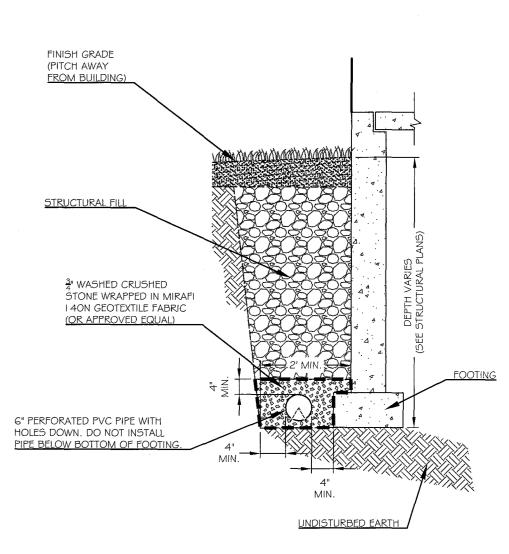
BY CHKD.APPD. REV. DATE

REVISED PER STAFF COMMENTS AND RESUBMITTED TO THE CITY OF PORTLAND DEPT. SWC PBB

SUBMITTED TO THE CITY OF PORTLAND FOR LEVEL 2 SITE PLAN APPLICATION

STATUS





6 BITUMINOUS DRIVEWAY APRON DETAIL
NOT TO SCALE

BY CHKD.APPE

STATUS

7 FOUNDATION DRAIN DETAIL
NOT TO SCALE

PROJ. NO.

DWG. NO.

SILAS W.
CANAVAN
No. 12639

ONAL

3/7/18

ENGINEERING ASSOCIATES, INC.

One Karen Dr., Suite 2A | Westbrook, Maine 04092 ph: 207.553.9898 | www.walsh-eng.com

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LAND DESIGN SOLUTIONS

LAND PLANNING, SITE PLANNING & LANDSCAPE ARCHITECTURE

P.O. Box 316, 160 Longwoods Road, Cumberland, ME 04021 tel:(207) 939-1717

APPLICANT & OWNER:

JAKE'S DEVELOPMENT, INC.

30 LEDGEWOOD DRIVE, FALMOUTH, MAINE 04105

DESIGN: PBB

DRAWN: DEPT.

CHKD: PBB

DATE: NOV. 2017

SCALE: N.T.S.

PROPOSED PRESUMPSCOT STREET BUSINESS PARK
314-316 PRESUMPSCOT STREET, PORTLAND, MAINE

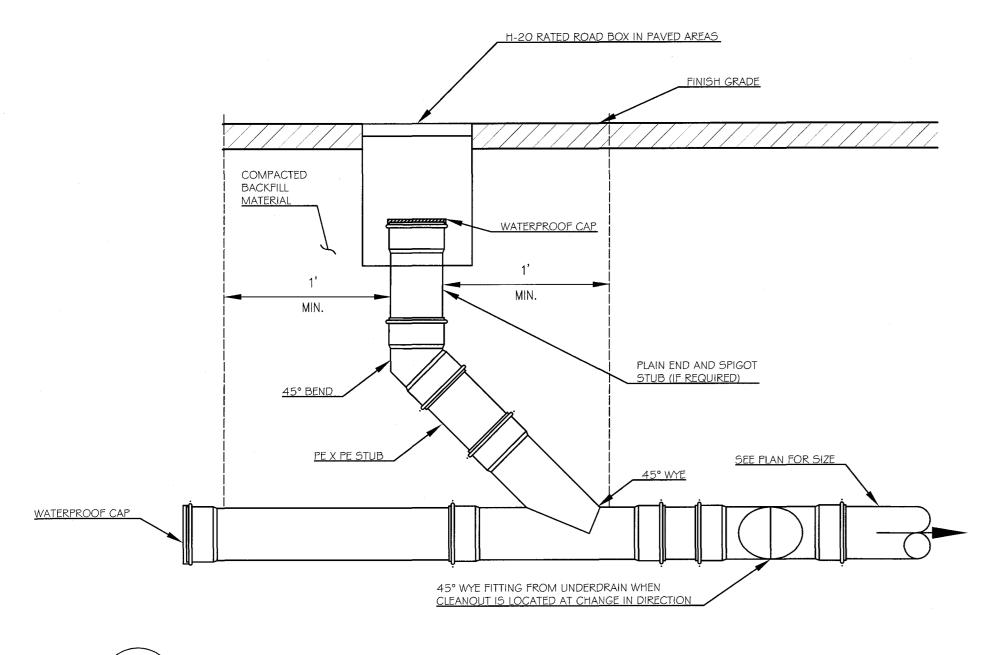
SITE DETAILS

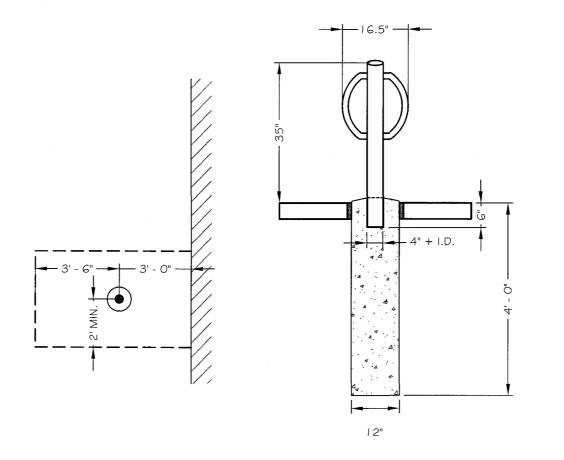
C-202

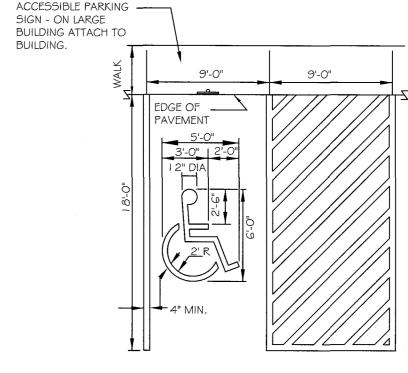
Subject to Conditions of Condi

PLANNER Christian Roadman









NOTE:

1. ALL ACCESSIBLE PARKING SPACE SIGNS SHALL BE MUTCD R7-8. 'VAN ACCESSIBLE' PLAQUES (MUTCD R7-8P) SHALL BE PROVIDED FOR ALL SPACES WITH AN 8' WIDE (OR WIDER) AISLE. BOTTOM OF SIGNS SHALL BE MIN. 5' ABOVE GRADE. 2. PAINT ALL PAVEMENT STRIPES AND LINES 4 INCHES WIDE (TYP.) . ALL ACCESSIBLE PARKING SPACES SHALL MEET MOST RECENT ADA STANDARDS FOR ACCESSIBLE DESIGN

C-203

ACCESSIBLE PARKING STALL DETAIL NOT TO SCALE

NOT TO SCALE

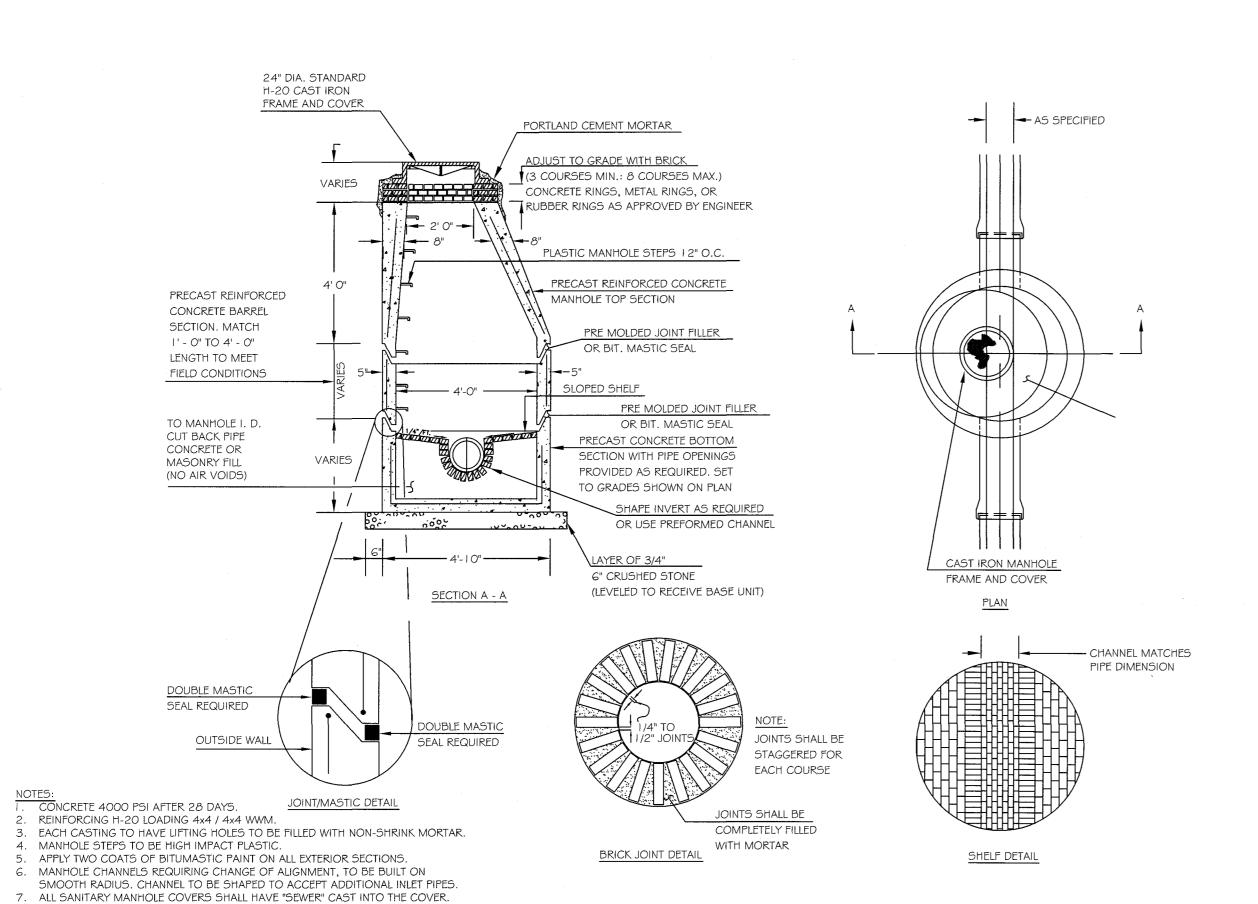
TYPICAL SANITARY MANHOLE

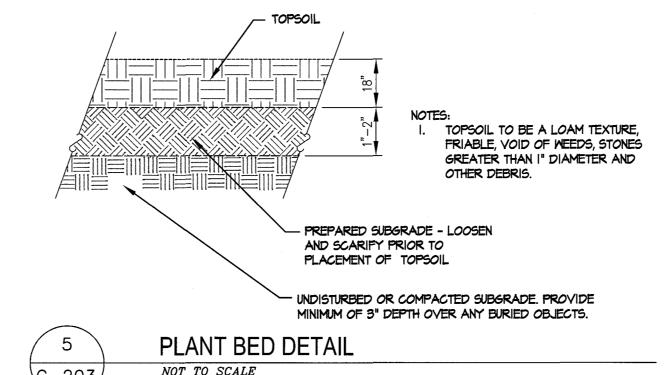
C-203/ NOT TO SCALE

SEWER/STORMDRAIN CLEANOUT

BIKE HITCH MOUNTING DETAIL

NOT TO SCALE

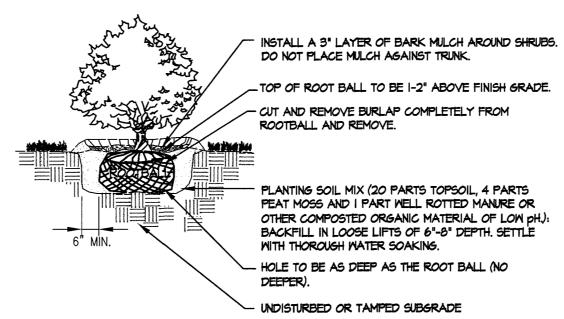




I. CONSTRUCT TEMPORARY PLANT SAUCIER FROM MULCH TO ASSIST WITH WATER RETENTION. SAUCER NOT REQUIRED WHEN SHRUB OCCURS IN A PLANT

 WHEN PLANTING CONTAINER GROWN PLANTS-REMOVE THE BALL FROM THE CONTAINER AND LOOSEN THE OUTSIDE LAYER OF THE ROOT SYSTEM BY SCORING WITH A CLEAN KNIFE. DIVIDE ALL CIRCLING ROOTS.

3. SHRUBS TO BE WATERED IMMEDIATELY AFTER PLANTING.



(C-203)

SHRUB PLANTING DETAIL NOT TO SCALE

I. STAKE ONLY TREES WITH SANDY OR CLAYEY ROOTBALLS. WHEN REQUIRED STAKING SHALL CONFORM TO THE LATEST GUIDELINES FROM THE INTERNATIONAL SOCIETY OF ARBORICULTURE. 2. DO NOT OUT MAIN LEADER. - PLACE ROOT BALL SO TOP OF ROOTBALL IS I" ABOVE "FINISHED GRADE" <u>DO NOT COVER TRUNK</u> FLARE WITH SOIL. - PLACE 3" SHREDDED BARK MULCH AROUND TREE. KEEP MULCH 8" AWAY FROM TRUNK. CONSTRUCT A TEMPORARY 3" HIGH EARTH SAUCER BEYOND EDGE OF ROOTBALL FOR WATER RETENTION. -CUT & REMOVE WIRE BASKET COMPLETELY FROM ROOTBALL. CUT AND REMOVE BURLAP FROM TOP 3/4 OF ROOTBALL. FOLD REMAINING 1/4 DOWN INTO PLANTING PIT, REMOVE IF POSSIBLE. PLANTING SOIL MIX (20 PARTS TOPSOIL, 4 PARTS PEAT MOSS AND I PART WELL ROTTED MANURE OR 2' MIN. OTHER COMPOSTED ORGANIC MATERIAL OF LOW ph.): BACKFILL IN LOOSE LIFTS OF 6"-8" DEPTH. SETTLE WITH THOROUGH WATERING

(C-203)

TREE PLANTING DETAIL NOT TO SCALE

									 		_
Α	11/30/17	SUBMITTED TO THE CITY OF PORTLAND FOR LEVEL 2 SIT	FE PLAN APPLICATION	DEPT.	SWC	PBB					
REV.	DATE	STATUS		BY	CHKD.	APPD.	REV.	DATE		STATUS	

ATE OF MAIN SILAS W. CANAVAN _O No. 12639 CENSTO DE CONSTRUCTION DE CONS 11/30/17

WALSH ne Karen Dr., Suite 2A | Westbrook, Maine 04092 | APPLICANT & OWNER: ph: 207.553.9898 www.walsh-eng.com Copyright © 2017

LAND DESIGN SOLUTIONS LAND PLANNING, SITE PLANNING & LANDSCAPE ARCHITECTURE P.O. Box 316, 160 Longwoods Road, Cumberland, ME 04021 tel:(207) 939-1717

JAKE'S DEVELOPMENT, INC.

30 LEDGEWOOD DRIVE, FALMOUTH, MAINE 04105

DESIGN: PBB DRAWN: DEPT. CHKD: PBB PROJ. DATE: NOV. 2017 NO. DWG. SCALE: N.T.S.

- PLACE ROOTBALL ON UNEXCAVATED OR TAMPED

PROPOSED PRESUMPSCOT STREET BUSINESS PARK 314-316 PRESUMPSCOT STREET, PORTLAND, MAINE

SITE DETAILS

C-203

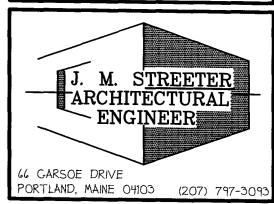
PLANNER Christian Roadman





FAX. (201) 892-9895

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JILDING T, LLC STREE IAINE PROPOSED BY 320 P STREE PRESUMPSCOT PORTLAND, N

REVISIONS DATE DESCRIPTION

DATE: 5/30/2018 SCALE: 1/8" = 1'-0" DESIGNER: JS CHECKED BY: JB

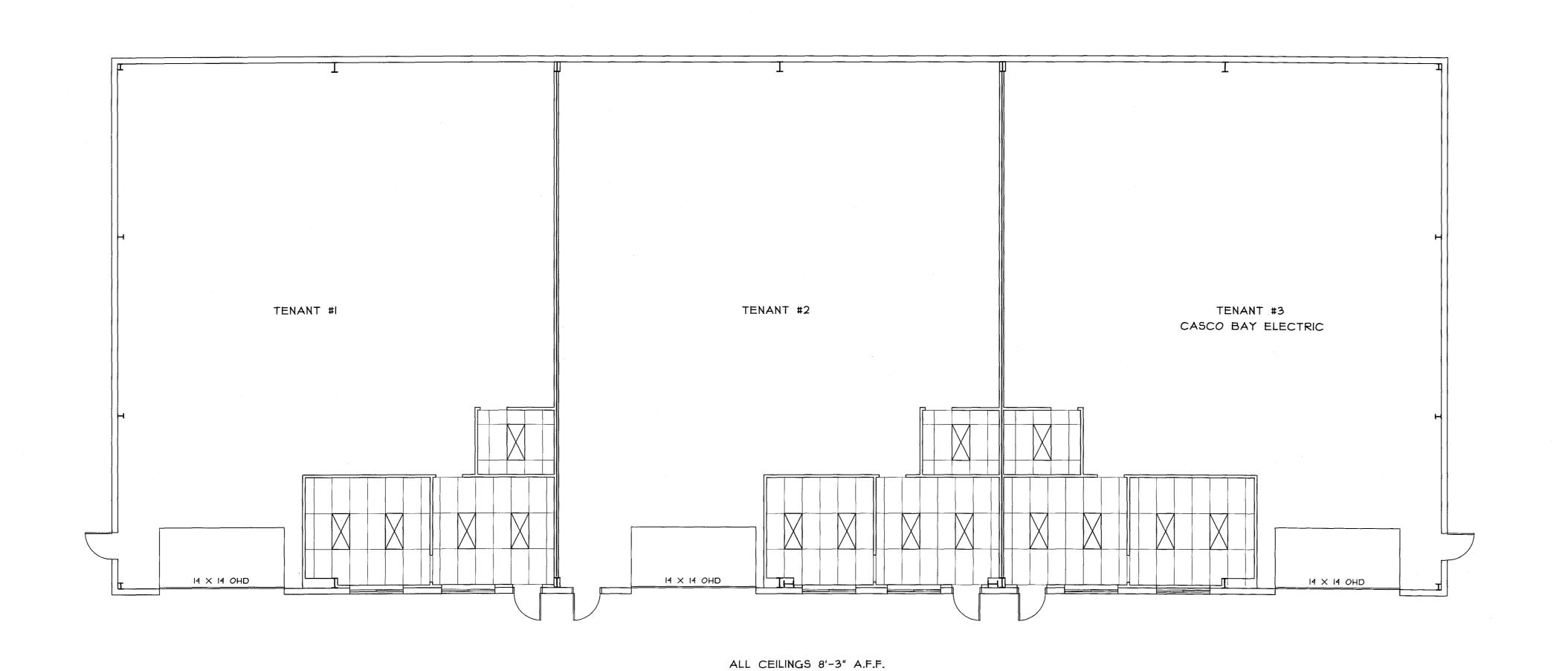
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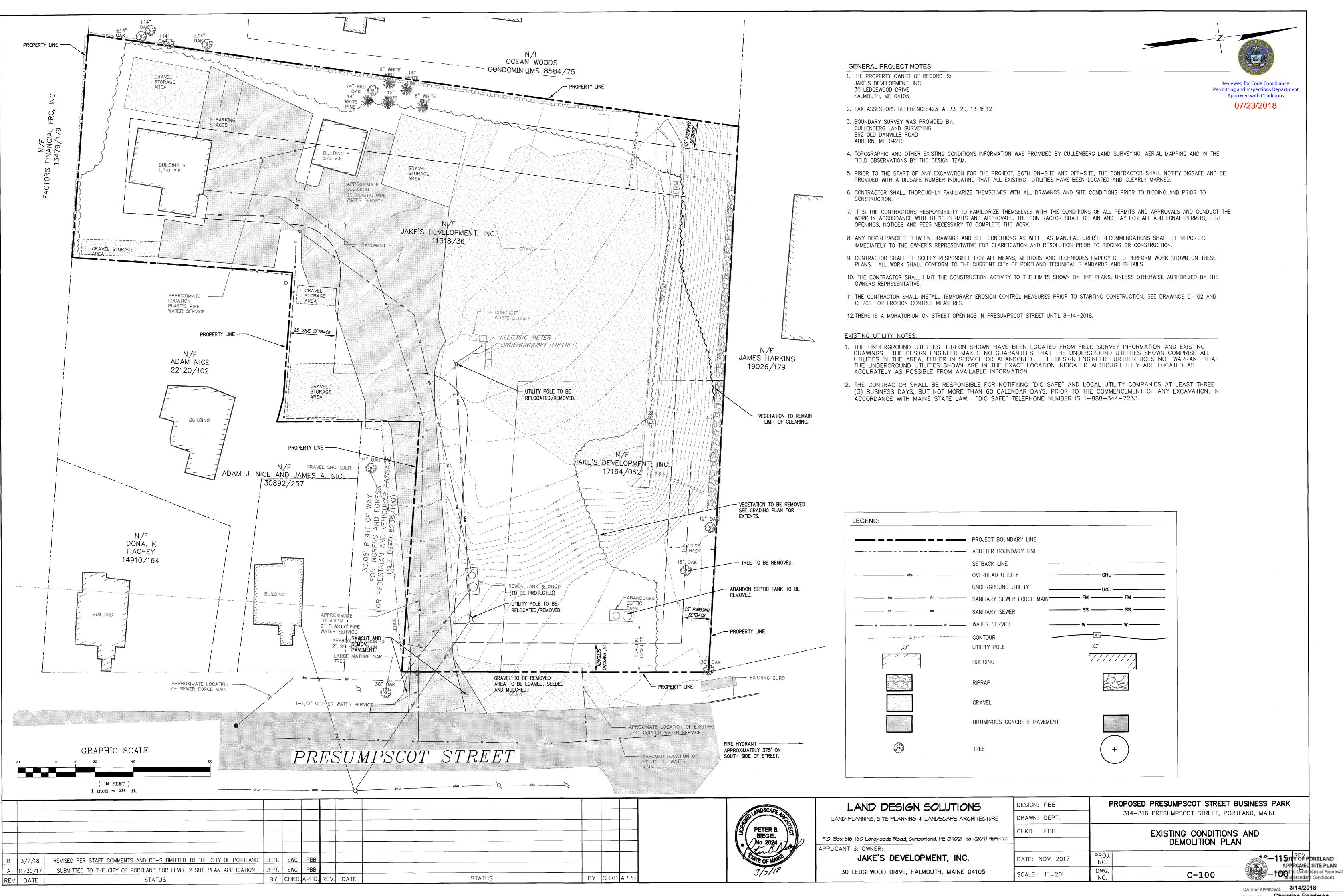
BISKUP CONSTRUCTION, INC. SHEET TITLE

CEILING PLAN

SHEET NUMBER

SHEET 5 OF 5







BISKUP CONSTRUCTION, INC.

I6 DANIELLE DRIVE

WINDHAM, MAINE 04062

TEL. (201) 892-9800

FAX. (201) 892-9895

WWW.BISKUPCONSTRUCTION.COM

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PROPOSED BUILDING 320 P STREET, LLC 314 PRESUMPSCOTT STREET PORTLAND, MAINE

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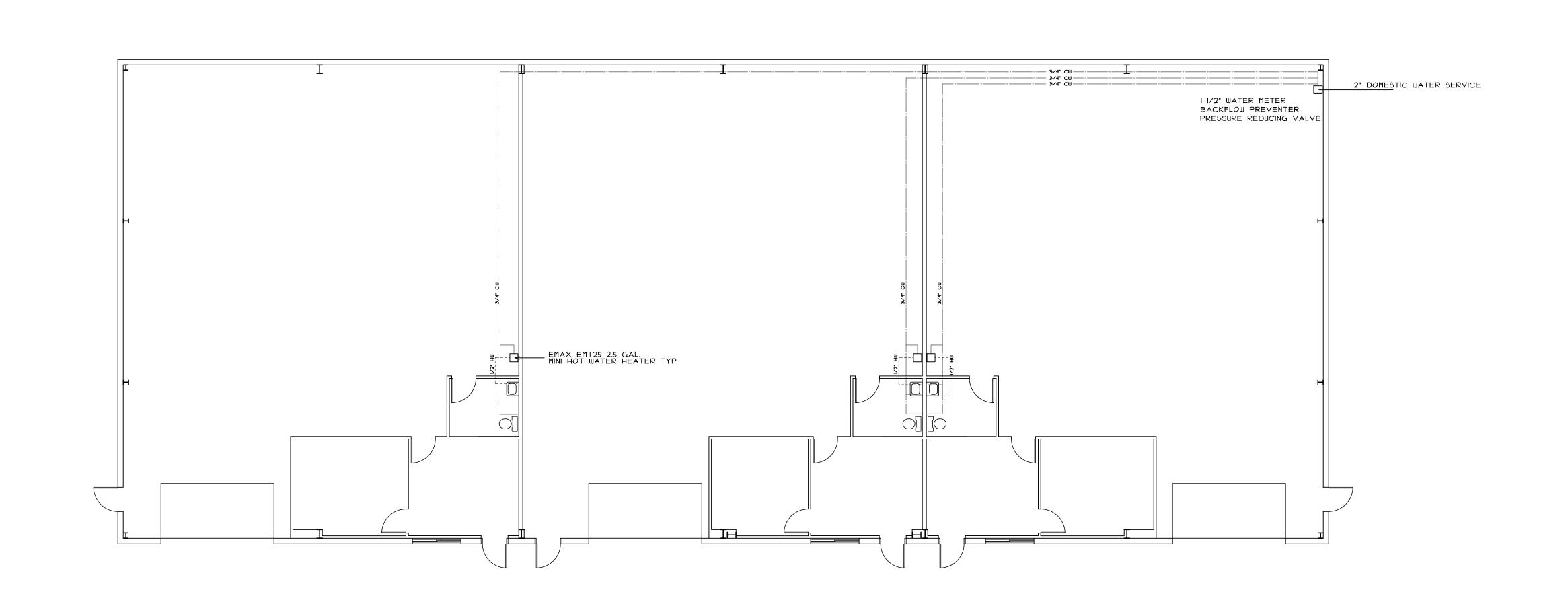
SHEET TITLE

DOMESTIC WATER PLAN

SHEET NUMBER

P-2

SHEET 2 OF 2







STAMP

PROPOS 320 P 9 PRESUMF PORTL,

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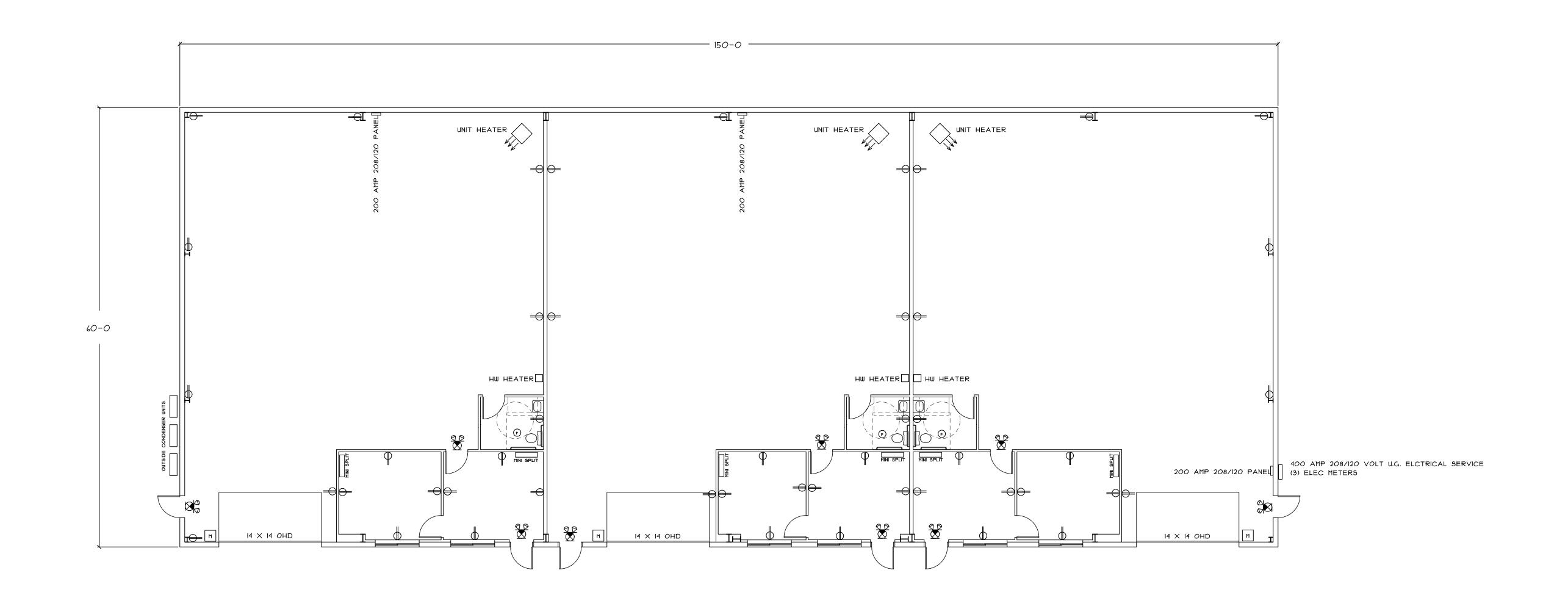
SHEET TITLE

ELECTRICAL PLAN

SHEET NUMBER

E-1

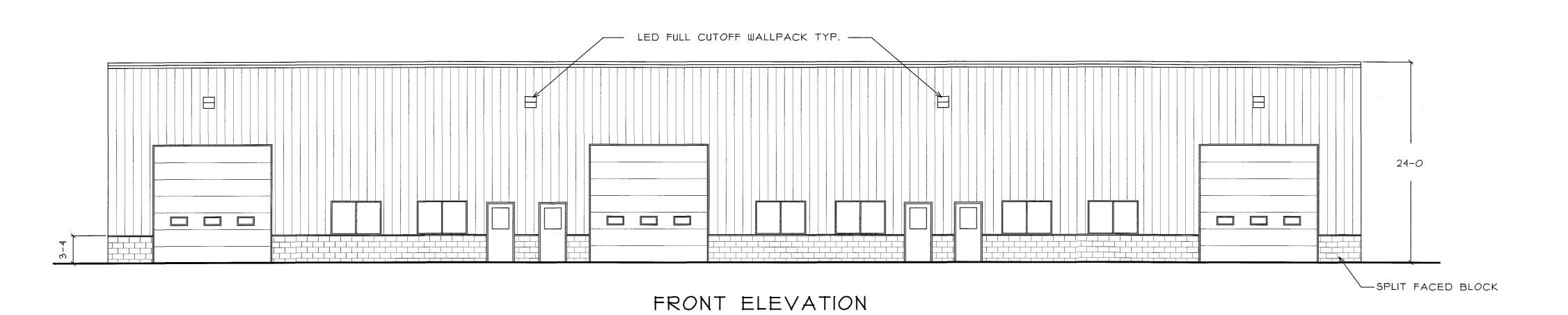
SHEET | OF 2

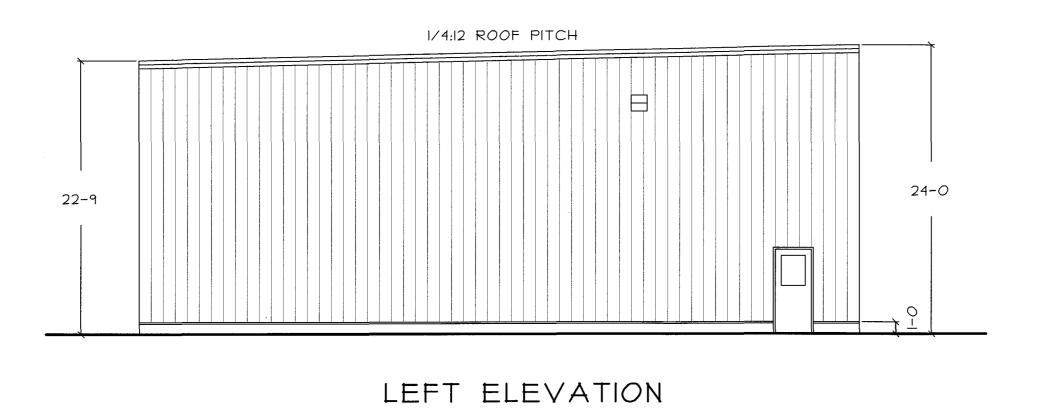


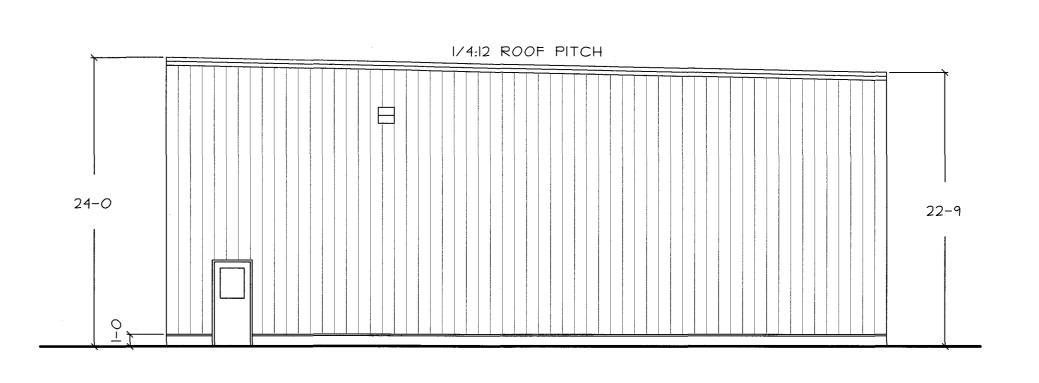
- → DUPLEX RECEPTACLE
- OHD DOOR OPERATOR

 EXHAUST FAN
- EXIT/EMERGENCY LIGHT

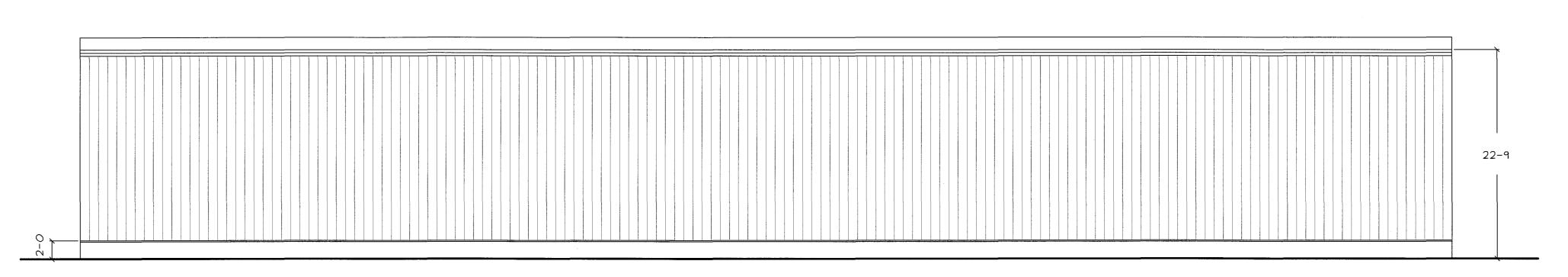








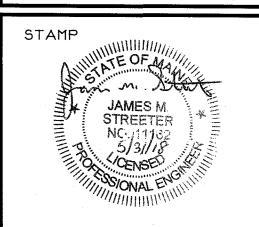
RIGHT ELEVATION

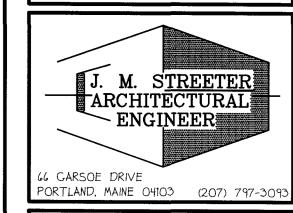


REAR ELEVATION

BISKUP CONSTRUCTION, INC.
16 DANIELLE DRIVE
WINDHAM, MAINE 04062
TEL. (201) 892-9800
FAX. (201) 892-9895

WWW.BISKUPCONSTRUCTION.COM





PROPOSED BUILDING 320 P STREET, LLC 314 PRESUMPSCOT STREET PORTLAND, MAINE

REVISIONS	
DATE DESCRIPTION	N
10 - 10 - 10 - 10 - 10 - 10 - 10 - 10 -	

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SHEET TITLE

ELEVATIONS

SHEET NUMBER

A-3

SHEET 3 OF 5



Reviewed for Code Compliance Permitting and Inspections Departme Approved with Conditions 07/23/2018

BUILDING CODES: MUBEC, IBC 2015, NFPA I (2009), NFPA IO, NFPA I3, NFPA IOI (2006), IECC 2009

GENERAL NOTES

THIS BUILDING DOES NOT HAVE A SPRINKLER SYSTEM, BUT WILL HAVE A FIRE ALARM SYSTEM

HANDICAPPED SIGNAGE SHALL BE MOUNTED AT 5'-O" A.F.F. AT ALL FUTURE BATHROOMS AND EXITS

THE BUILDING IS A PRE-ENGINEERED METAL BUILDING BY PACKAGE STEEL SYSTEMS, INC.

ALL DOORS SHALL BE 3'-O" WIDE AND HAVE HANDICAPPED LEVER TYPE HARDWARE

SEPARATE PERMITS ARE REQUIRED FOR THE ELECTRICAL, PLUMBING, AND HVAC WORK

OF SUTTON, MA .. BUILDING MANUFACTURER TO PROVIDE STRUCTURAL DESIGN,

FOUNDATION DESIGN TO BE BY ASSOCIATED DESIGN PROFESSIONALS

5 LB ABC FIRE EXTINGUISHERS SHALL BE MOUNTED AT EVERY EXIT

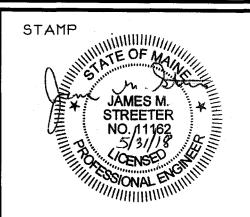
THE BUILDING WILL BE HEATED BY GAS FIRED REZNOR HEATING UNITS

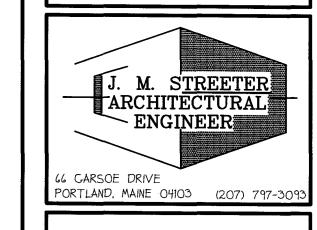
BUILDING ENVELOPE DETAILS, AND LETTER OF CERTIFICATION.

BISKUP CONSTRUCTION, INC. 16 DANIELLE DRIVE WINDHAM, MAINE 04062 TEL. (201) 892-9800

WWW.BISKUPCONSTRUCTION.COM

FAX. (201) 892-9895





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	R	EVISIONS
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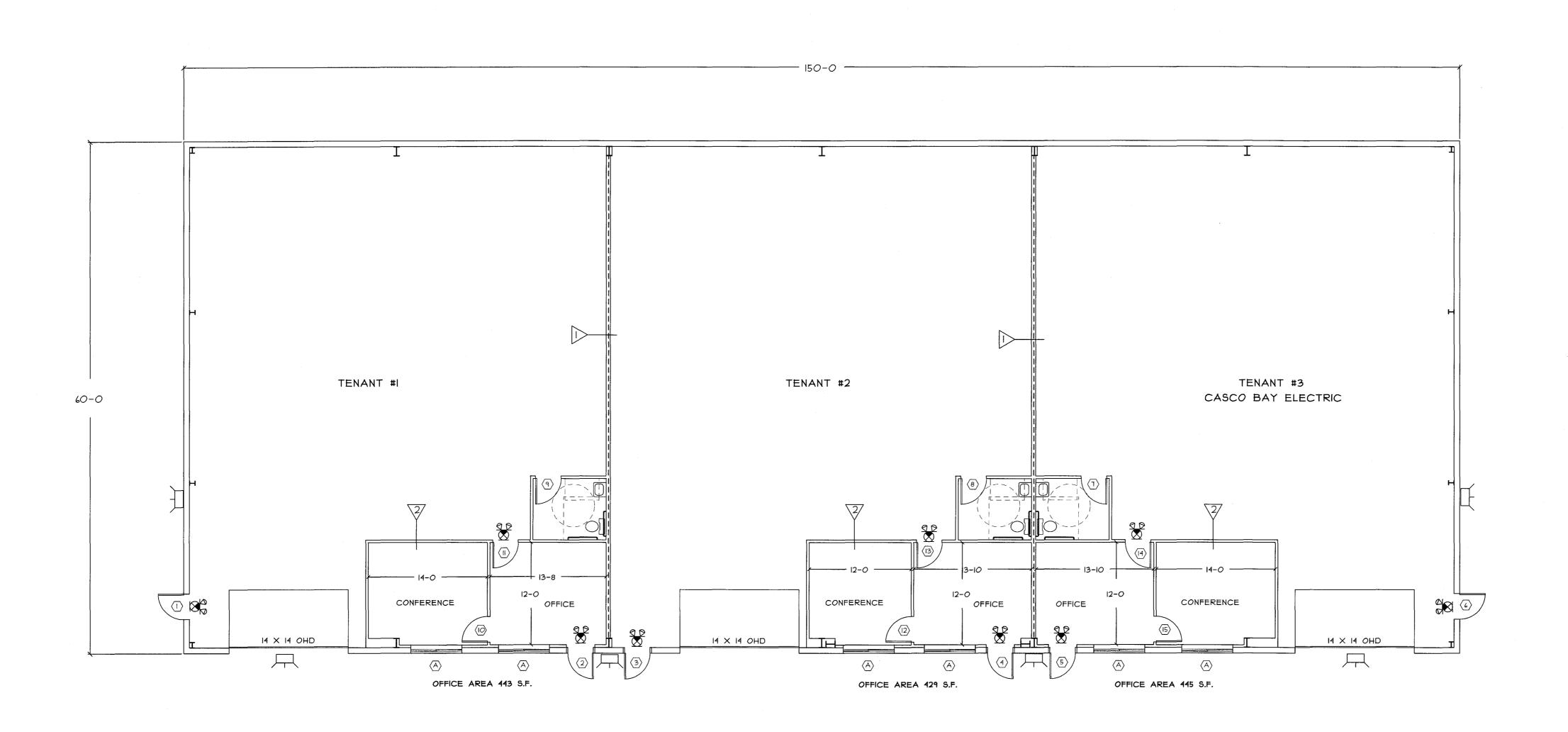
SHEET TITLE

FLOOR PLAN

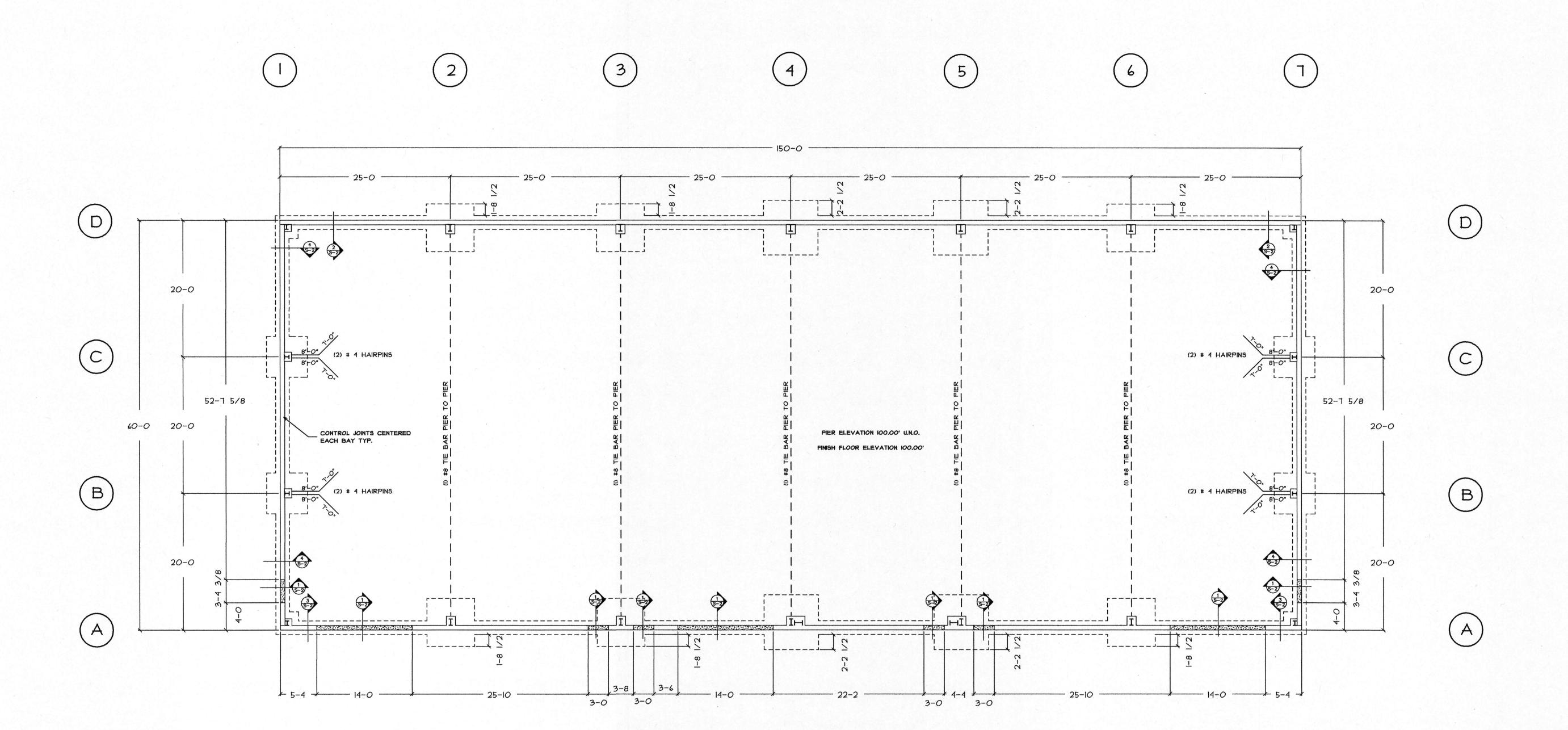
SHEET NUMBER

A-1

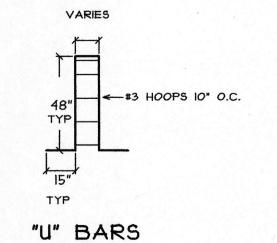
SHEET I OF 5







		FOC	DTING	# PIER SCHEDULE	
LOCATION	FOOTING SIZE	FOOTING REBAR	PIER SIZE	PIER REBAR	ANCHOR BOLTS
IA, ID, TA, TD	3'-4" × 3'-4" × l'-0"	3 #5 E.W BOTT.	12" × 12"	2 #5 "U" BARS, I5" × 48" × 10"	3/4" × 18" A 301 HEADED
IB, IC, 7B, 7C	6'-0" × 6'-0" × 1'-0"	6 #5 E.W BOTT.	16" × 12"	2 #5 "U" BARS, I5" X 48" X I2"	3/4" X 18" A 301 HEADED
2A, 3A, 6A, 2D, 3D, 6D	7'-0" × 7'-0" × 1'-2"	8 #5 E.W BOTT.	16" × 16"	2 #5 "U" BARS, I5" $ imes$ 48" $ imes$ 12", 2 #5 "U" BARS, I5" $ imes$ 48" $ imes$ 16"	3/4" X 18" A 521 THREADED ROD WITH NUT
4D, 5D	8'-0" × 8'-0" × 1'-2"	9 #5 E.W. BOTT.	16" × 16"	2 #5 "U" BARS, I5" $ imes$ 48" $ imes$ 12", $ imes$ 2 #5 "U" BARS, I5" $ imes$ 48" $ imes$ 16"	3/4" X 18" A 521 THREADED ROD WITH NUT
4A, 5A	8'-0" × 8'-0" × 1'-4"	II #5 E.W. BOTT.	32" × 16"	6 #5 "U" BARS, 15" × 48" × 16"	3/4" X 18" A 521 THREADED ROD WITH NUT



GENERAL NOTES:

ALL FOOTINGS TO BEAR ON 6" OF 3/4" STONE ATOP UNDISTURBED SOIL, WITH AN ALLOWABLE NET, BEARING CAPACITY OF 3,000 PSF.

MINIMUM COMPRESSIVE STRENGTH OF CONCRETE, 28 DAYS = 3,000 PSI.
MINIMUM YIELD STRENGTH OF REINF. STEEL = 60,000 PSI.

REINF. STEEL SHALL BE FABRICATED AND INSTALLED IN ACCORDANCE WITH THE CRSI MANUAL OF STANDARD PRACTICE, LATEST EDITION

CONCRETE WORKMANSHIP SHALL CONFORM TO THE REQUIREMENTS OF

ACI 318 AND "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" ACI 301, LATEST EDITION

BACKFILL BOTH SIDES OF ALL WALLS SIMULTANEOUSLY

FOUNDATION DESIGN IS BASED ON THE GEOTECHNICAL REPORT BY SUMMIT GEOENGINEERING DATED II/II/2016. THIS DESIGN PROFESSIONAL SHALL NOT BE RESPONSIBLE FOR SETTLEMENT OF THE BUILDING, OR DISCREPANCIES OF ON SITE CONDITIONS. A GEOTECHNICAL ENGINEER SHALL BE CONTACTED IF ANY UNSUITABLE MATERIALS ARE ENCOUNTERED.

BISKUP CONSTRUCTION, INC.
16 DANIELLE DRIVE

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WINDHAM, MAINE 04062 TEL. (201) 892-9800 FAX. (201) 892-9895

ASSOCIATED DESIGN
ASSOCIATED DESIGN
PARTNERS INC.

80 Leighton Road
Falmouth, Maine 04105
Fax: (207) 878-1751
Fax: (207) 878-1788
E-Mail: adplitudepengineering.com



SED BUILDING
STREET, LLC
PSCOT STREET
AND, MAINE

32C 314 PRE

REVISIONS

DATE DESCRIPTION

DATE: 6/6/2018

SCALE: 1/8" = 1'-0"

DESIGNER: AW

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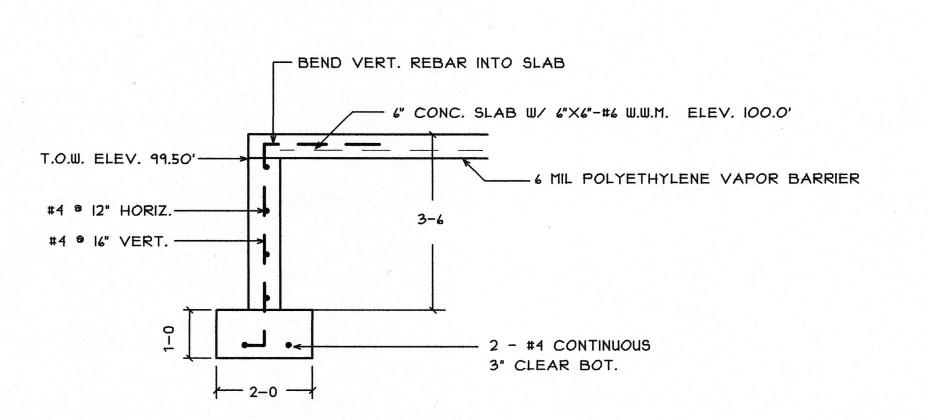
SHEET TITLE

FOUNDATION PLAN

SHEET NUMBER

5-1

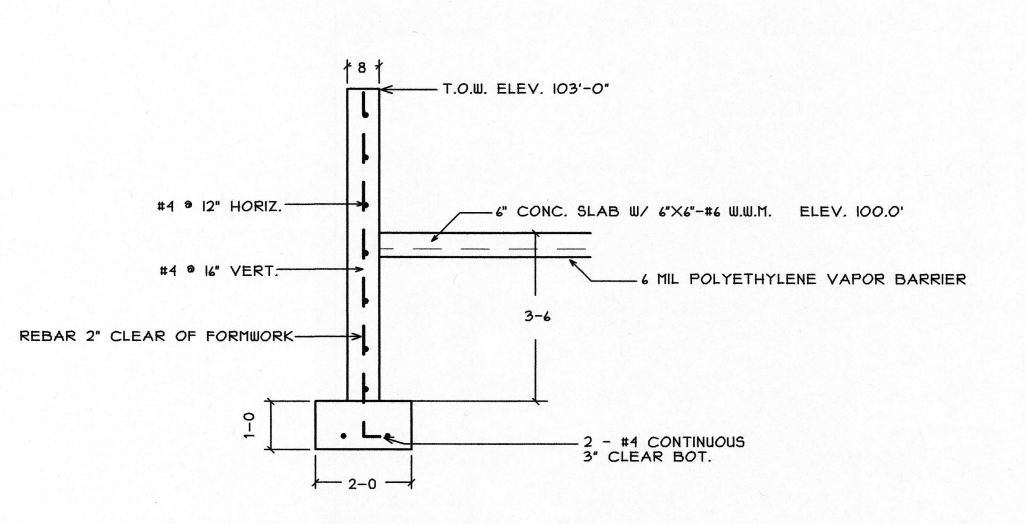
SHEET I OF 3



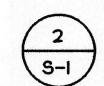
SECTION THRU DOOR OPENINGS

SCALE: 1/2"=1'-0"

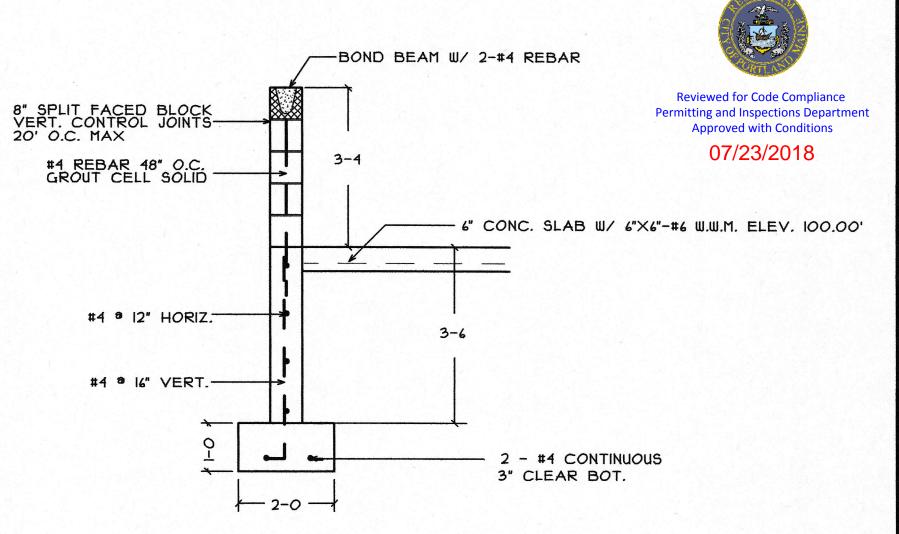




WALL SECTION

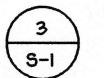


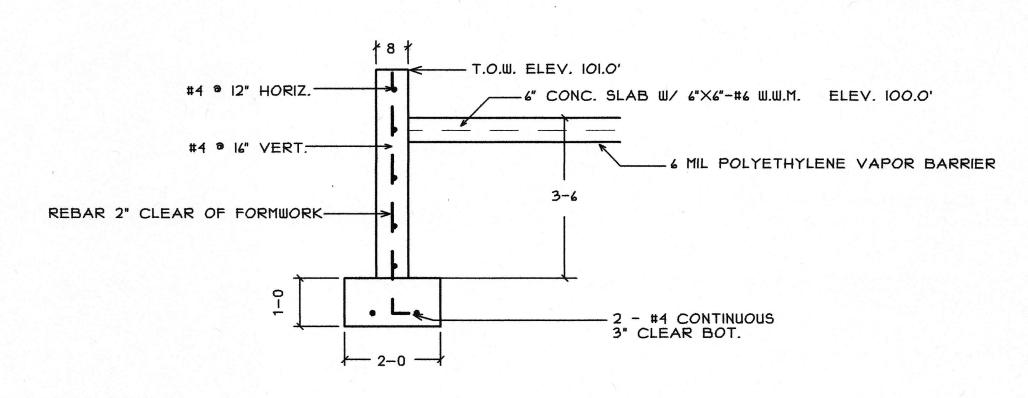
SCALE: 1/2"=1'-0"



WALL SECTION

SCALE: 1/2"=1'-0"

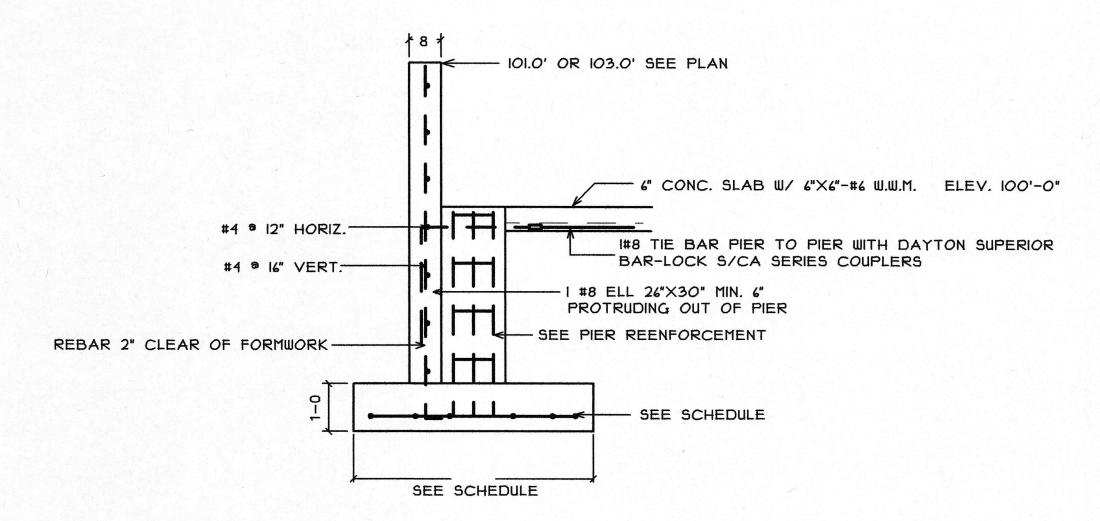




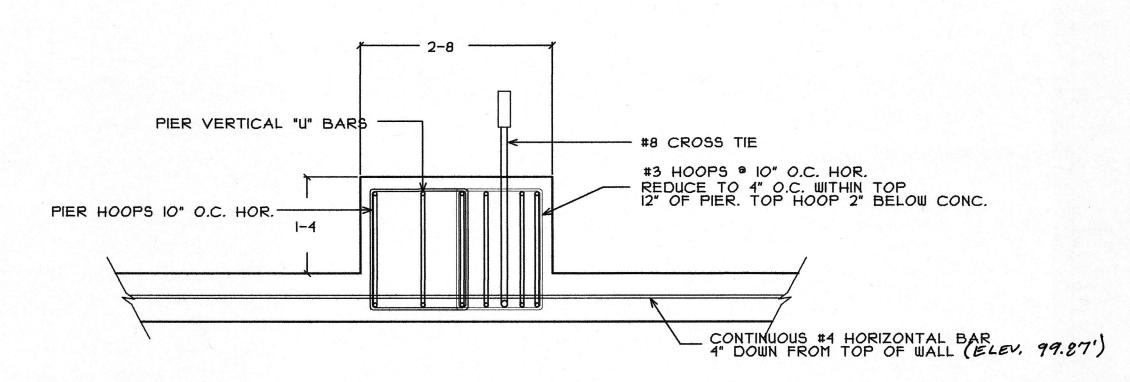
WALL SECTION

SCALE: 1/2"=1'-0"

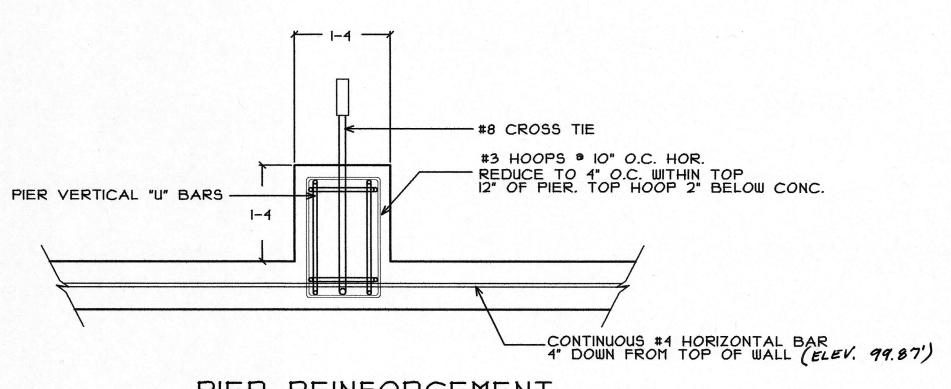




WALL SECTION @ PIERS 2A - 6A, 2D - 6D



PIER REINFORCEMENT 4A, 5A SCALE: 3/4" = 1'-0"

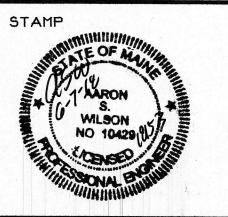


PIER REINFORCEMENT 2A, 3A, 6A 2D THRU 6D SCALE: 3/4" = 1'-0"

BISKUP CONSTRUCTION, INC. 16 DANIELLE DRIVE WINDHAM, MAINE 04062 TEL. (201) 892-9800 FAX. (201) 892-9895

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CONSULATANT ASSOCIATED DESIGN
PARTNERS INC. 80 Leighton Road Office: (207) 878-1751 Falmourth, Maine 04105 Fax: (207) 878-1788 E-Mail: adp@adpengineerin



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R	EVISIONS
DATE	DESCRIPTION

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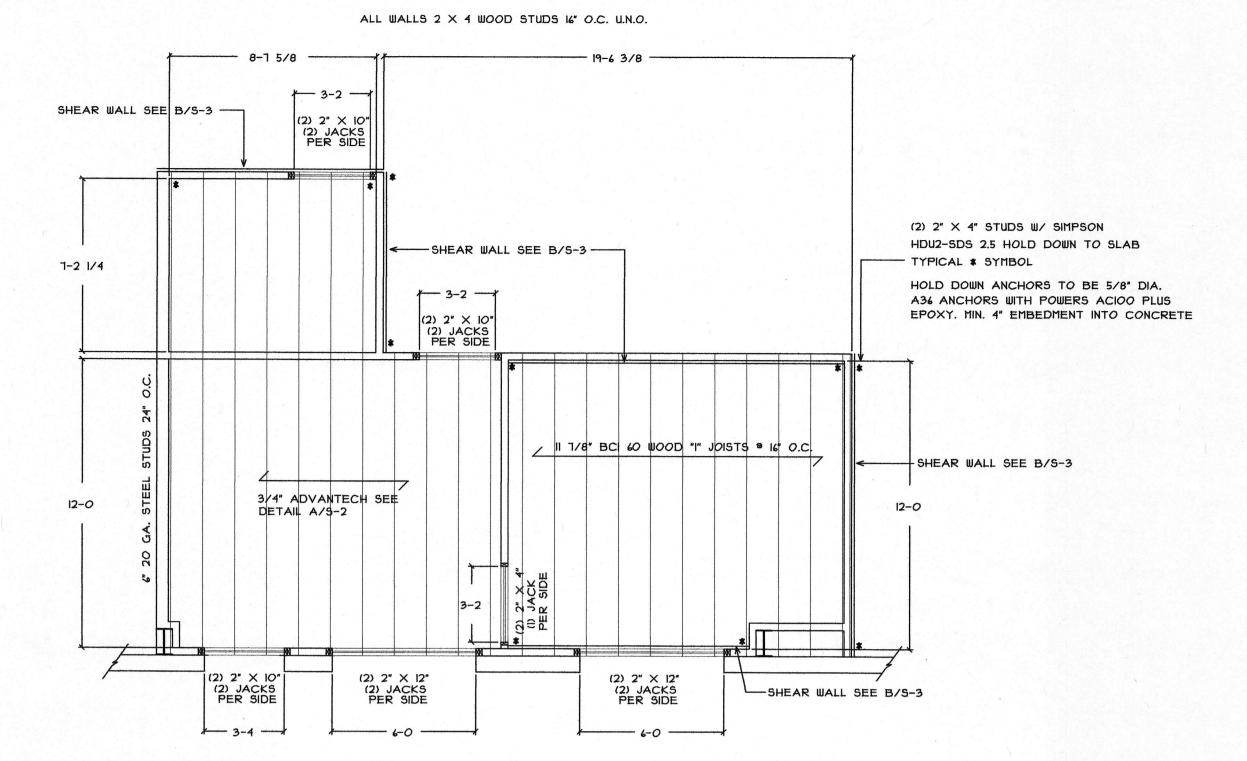
SHEET TITLE

FOUNDATION DETAILS

SHEET NUMBER

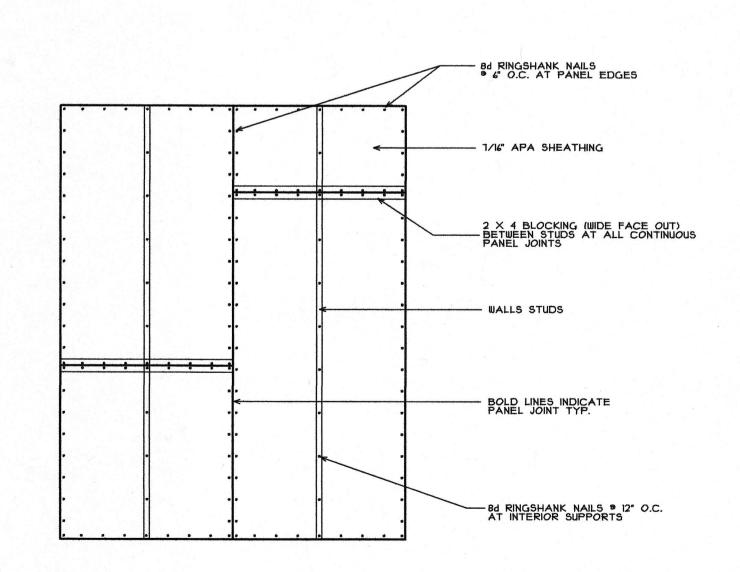
S - 2

SHEET 2 OF 3



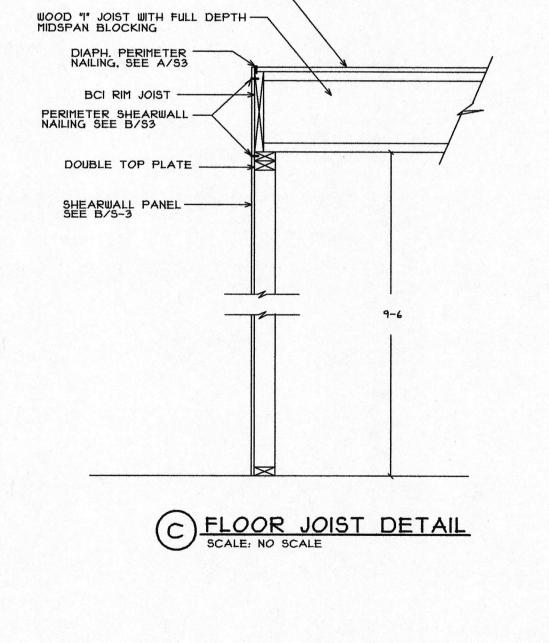
FLOOR DESIGN LOAD: 125 PSF LIGHT WAREHOUSE STORAGE SCALE: 1/4'' = 1'-0''

TYPICAL (3) TENANT SPACES

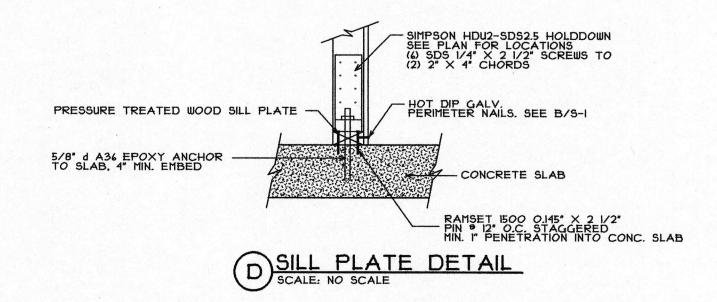


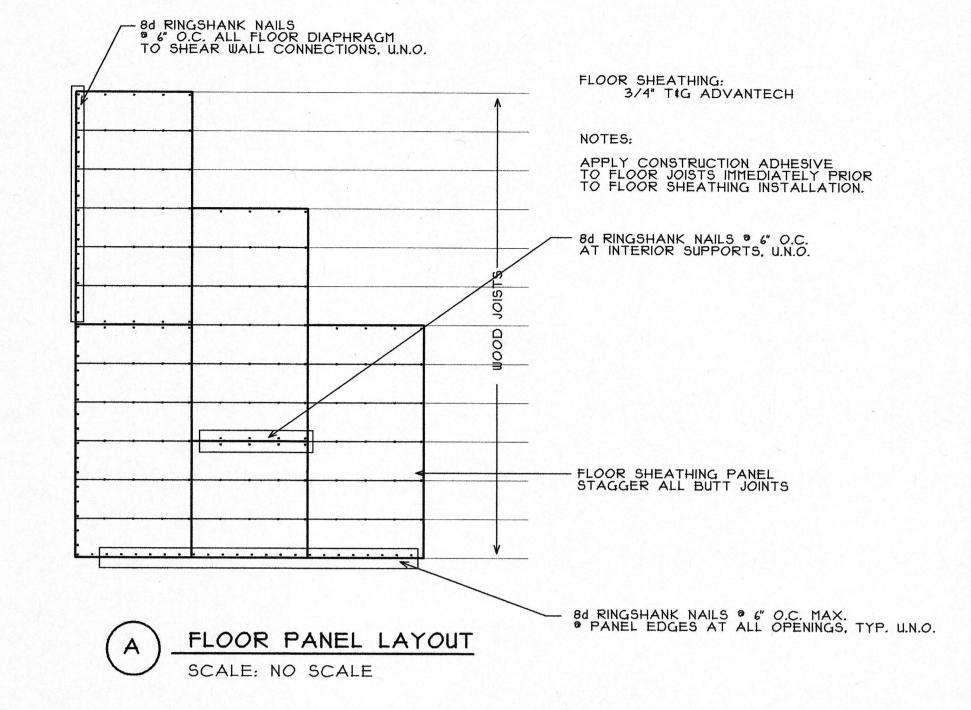
* OPTIONAL: PANELS MAY BE INSTALLED LONG SIDE HORIZONTAL WITH CONTINUOUS HORIZONTAL BLOCKING AT SEAMS SHEARWALL PANEL LAYOUT

SCALE: 1/2" = 1'-0"



FLOOR SHEATHING SEE A/S-3---





GENERAL STRUCTURAL NOTES

I. ALL WORK SHALL CONFORM TO THE REQUIREMENTS OF ALL APPLICABLE STATE AND LOCAL CODES, INCLUDING BUT NOT -IBC BUILDING CODE 2015 ED

-ANSI-ASCE 7-05 -ACI 318-05 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" -ACI 301 "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR -AISC STEEL CONSTRUCTION MANUAL 9TH ED ASD -AISI COLD FORMED STEEL DESIGN MANUAL, 2001

2. DESIGN LOADS 2.1. GRAVITY ROOF DESIGN LOADS:

-ANSI-AF#PA NDS-2005

MEZZ LIVE LOAD 125 PSF MEZZ DEAD LOAD 15 PSF

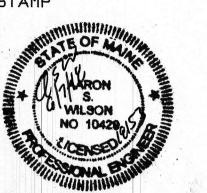
THIS P.E. REVIEW COVERS STRUCTURAL FLOOR AND WALL FRAMING ELEMENTS FOR GRAVITY AND LATERAL LOADS DETAILED ON SHEET THIS SHEET ONLY. THIS P.E. REVIEW IS BASED ON THE MINIMUM DESIGN LOADS AND CONSTRUCTION STANDARDS SET FORTH THE IBC 2015 BUILDING CODE. DESIGN AND DETAIL FOR ALL OTHER FRAMING ELEMENTS, WIND/SEISMIC DESIGN, FRAMING CONNECTIONS, VENEER DESIGN, COMPONENTS AND CLADDING, FINISHES, FLOOR PLAN LAYOUT. THERMAL/MOISTURE PROTECTION (INCLUDING ROOFING AND INSULATION) LIFE SAFETY CODE REQUIREMENTS, AND OTHER ELEMENTS NOT SPECIFICALLY REPRESENTED HEREIN HAVE NOT BEEN REVIEWED AND ARE BEYOND THE PURVIEW OF THIS P.E. SEAL.

BISKUP CONSTRUCTION, INC. 16 DANIELLE DRIVE WINDHAM, MAINE 04062 TEL. (201) 892-9800 FAX. (207) 892-9895

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BUILDING TRE PRC 320 314

REVISIONS						
DATE	DESCRIPTION					
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DATE: 6/6/2018 SCALE: AS SHOWN DESIGNER: AW CHECKED BY: JB © COPYRIGHT

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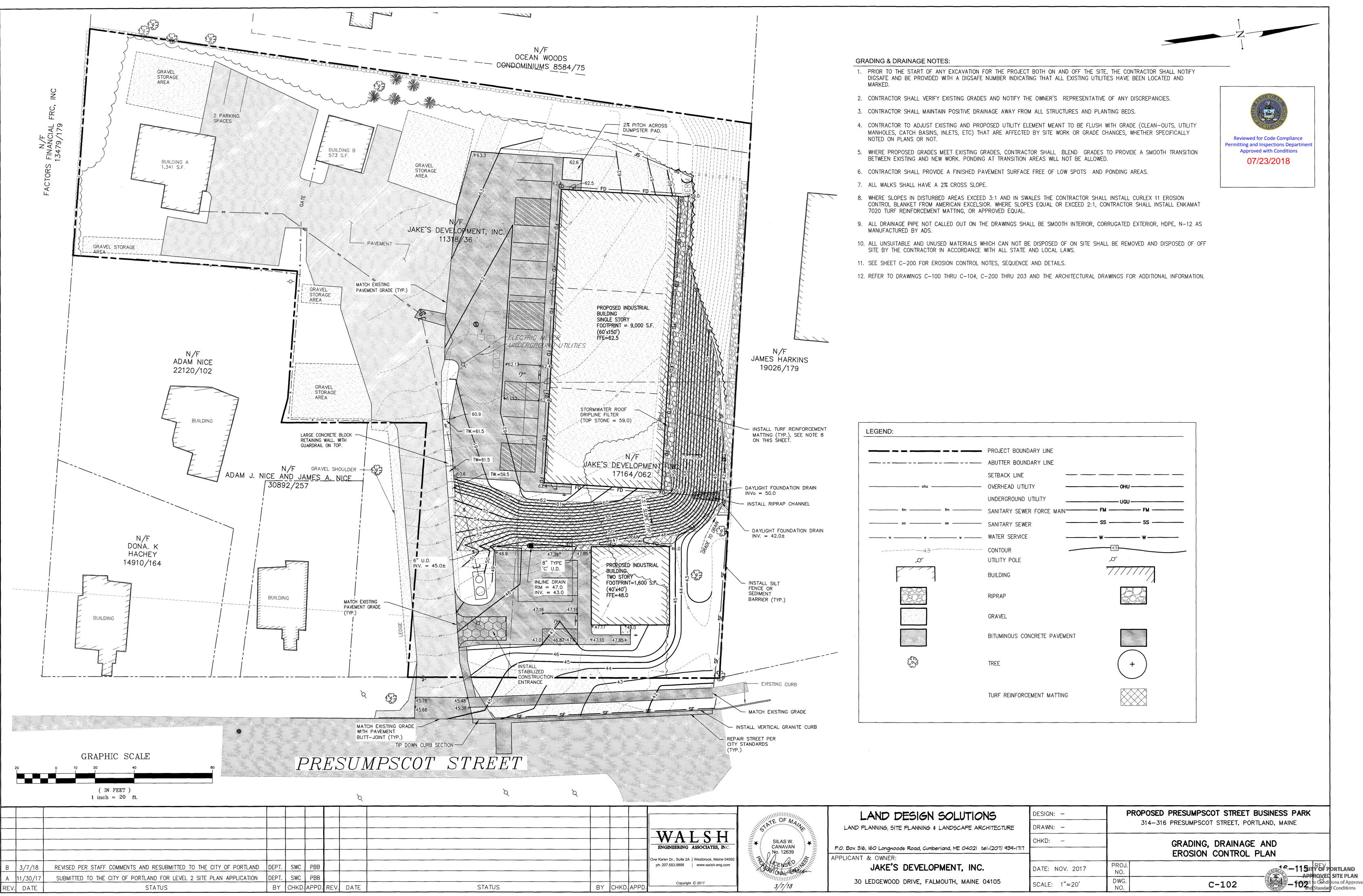
SHEET TITLE

FRAMING DETAILS

SHEET NUMBER

S - 3

SHEET 3 OF 3



IBC 2015: CODE DATA	
OCCUPANT LOAD - TABLE 1004.1.2	91 IND. 7,683/100=17 B 1,317/100=14
USE GROUP CLASSIFICATION - SECTION 306.2, 304.1	F-I FACTORY INDUSTRIAL, B BUSINESS
TYPE OF CONSTRUCTION - TABLE 601	V B
ACTUAL BUILDING AREA	9,000 S.F.
BUILDING AREA LIMITATION - TABLE 506.2	8,500 S.F. (NO SPRINKLER)
STREET FRONTAGE INCREASE - 504.3.3	850 S.F. (SEE BELOW)
ALLOWABLE BUILDING AREA	9,350 S.F.
BUILDING HEIGHT LIMITATION - TABLE 504.4	1 STORY
BUILDING HEIGHT	24'-0" STORY
FIRE SUPPRESSION: 903.2.4	NOT REQUIRED
FIRE RESISTANT CONSTRUCTION:	
STRUCTURAL FRAME	NA
EXTERIOR BEARING WALLS	NA
INTERIOR BEARING WALLS	NA
EXTERIOR NONBEARING WALLS	NA
FIRE WALLS & PARTY WALLS TABLE 508.4	NO SEPARATION REQUIRED
FLOOR CONSTRUCTION	NA .
ROOF CONSTRUCTION	NA .
STAIR ENCLOSURES	NA
EXIT ACCESS TRAVEL DISTANCE TABLE 1017.2	200' (NO SPRINKLER SYSTEM)
COMMON PATH OF TRAVEL SEE NFPA 101-2009 TABLE AT.6	50' (NO SPRINKLER SYSTEM)
MINIMUM NUMBER OF EXITS - TABLE 1006.3.1	2
ACCESSORY USE	NA
FIRE EXTINGUISHERS - TABLE 906.3(1)	2-A, I AT EACH EXIT
GENERAL NOTES	
	·

STREET FRONTAGE INCREASE 504.3.3

AREA INCREASE = [150/420-0.25] 30/30 AREA INCREASE = 0.10 OR 850 S.F.

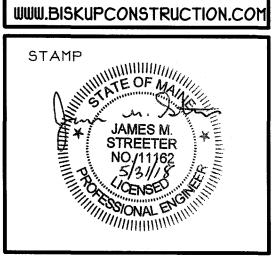
CCUPANT LOAD - TABLE 1.3.1.2	91 IND. 7,683/100=17 B 1,317/100=14
SE GROUP CLASSIFICATION 40.1.4.1.1, 38.1.4	GENERAL INDUSTRIAL, BUSINESS
YPE OF CONSTRUCTION - TABLE 8.1	V (000)
CTUAL BUILDING AREA	9,000 S.F.
UILDING HEIGHT	24'-0" STORY
RE SUPPRESSION: 40.3.5, 38.3.5	NOT REQUIRED
RE RESISTANT CONSTRUCTION:	
STRUCTURAL FRAME	NA
EXTERIOR BEARING WALLS	NA
INTERIOR BEARING WALLS	NA
EXTERIOR NONBEARING WALLS	NA
FIRE WALLS & PARTY WALLS 6.1.14.3.2 MIXED USE	NO SEPARATION REQUIRED
FLOOR CONSTRUCTION	NA
ROOF CONSTRUCTION	NA
STAIR ENCLOSURES	NA
XIT ACCESS TRAVEL DISTANCE - TABLE 40.2.6	200' (NO SPRINKLER SYSTEM)
OMMON PATH OF TRAVEL - TABLE 40.2.5	50' (NO SPRINKLER SYSTEM)
NIMUM NUMBER OF EXITS 40.2.4.I.I	
CCESSORY USE	NA
IRE EXTINGUISHERS	2-A, I AT EACH EXIT
ENERAL NOTES	

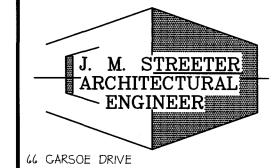
OCCUPANT LOAD - TABLE 14.8.1.2	91 IND. 7,683/100=T7 B 1,317/100=14
USE GROUP CLASSIFICATION - SECTION 6.1.12 6.1.11	GENERAL INDUSTRIAL, BUSINESS
TYPE OF CONSTRUCTION - TABLE A.12.2.1	V (000)
ACTUAL BUILDING AREA	9,000 S.F.
BUILDING HEIGHT	24'-0" STORY
FIRE SUPPRESSION:	NONE REQUIRED
FIRE WALLS & PARTY WALLS	NA
COMMON PATH OF TRAVEL 14.10.1.1.4	50' (NO SPRINKLER SYSTEM)
MINIMUM NUMBER OF EXITS 14.9.1.1	2
FIRE EXTINGUISHERS - TABLE 13.6.8.2.1.1	2-A, I AT EACH EXIT
GENERAL NOTES	



BISKUP CONSTRUCTION, INC.
16 DANIELLE DRIVE
WINDHAM, MAINE 04062

TEL. (201) 892-9800 FAX. (201) 892-9895





66 CARSOE DRIVE PORTLAND, MAINE 04103 (207) 797-3093

> PROPOSED BUILDING 320 P STREET, LLC 14 PRESUMPSCOT STREET PORTLAND, MAINE

<u>σ</u>	
1	REVISIONS
DATE	DESCRIPTION

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SCALE: 1/8" = 1'-0"

DESIGNER: JS

CHECKED BY: JB

BISKUP CONSTRUCTION, INC.

SHEET TITLE

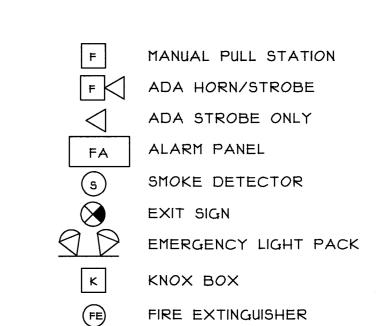
LIFE SAFETY PLAN

SHEET NUMBER

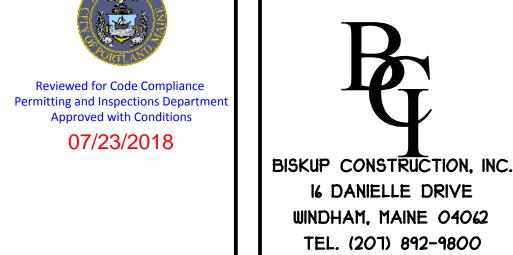
A-2

SHEET 2 OF 5

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60-0	17-4 3/8 135 CD 135 CD 134 1/22
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1	44-1 1/2







FAX. (201) 892-9895

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REVISIONS DATE DESCRIPTION

DATE: 5/4/2018 SCALE: 1/8" = 1'-0"DESIGNER: JS CHECKED BY: JB

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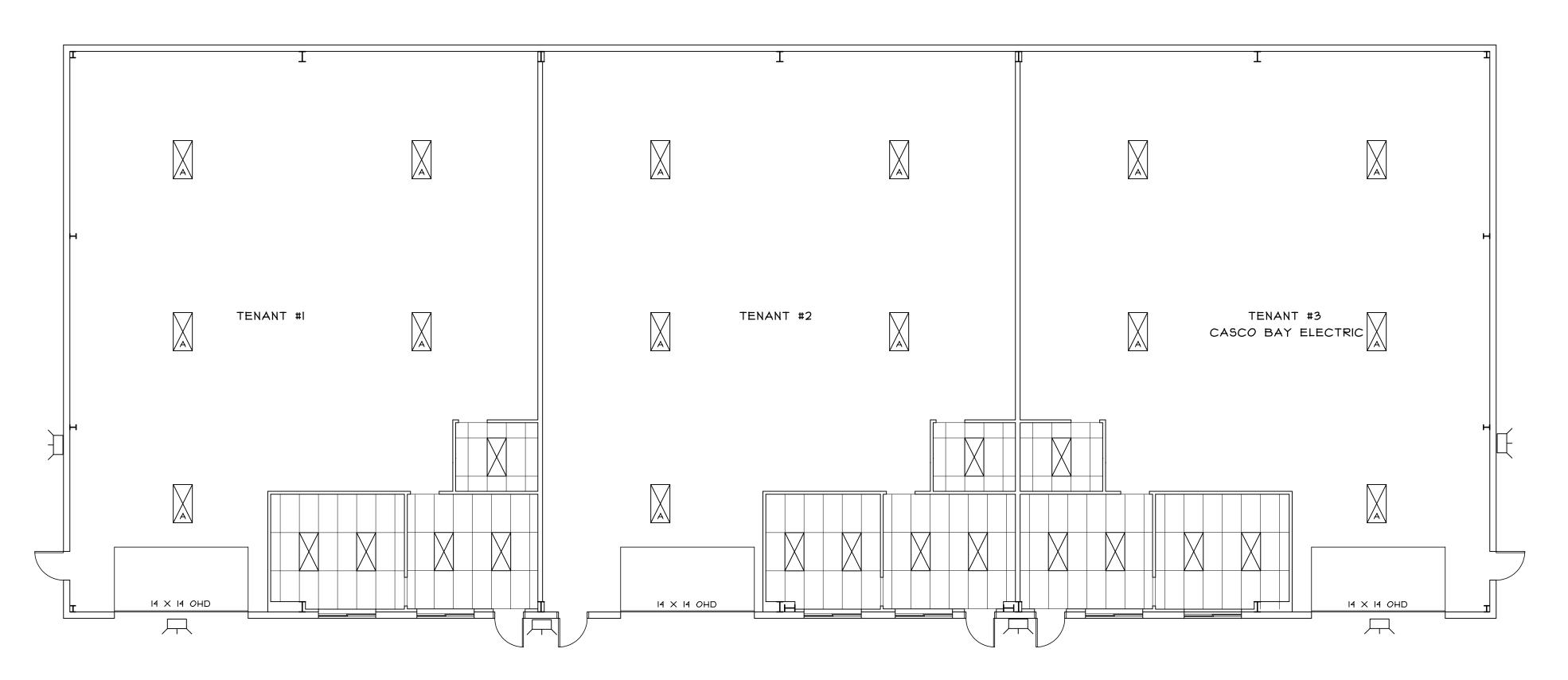
SHEET TITLE

LIGHTING PLAN

SHEET NUMBER

E-2

SHEET 2 OF 2



ALL CEILINGS 8'-3" A.F.F.

RAB LED FULL CUTOFF WPLED52N

ALEO LED SLB2-165/850/F XE G3

ALEO LED LT-CD-24LE-34/840 XE G3





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PROPOSED BUILD 320 P STREET, I PRESUMPSCOTT 9 PORTLAND, MAIN

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<u>g</u>	
F	REVISIONS
DATE	DESCRIPTION
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DATE: 5/4/2018 SCALE: 1/8" = 1'-0" DESIGNER: JS CHECKED BY: JB

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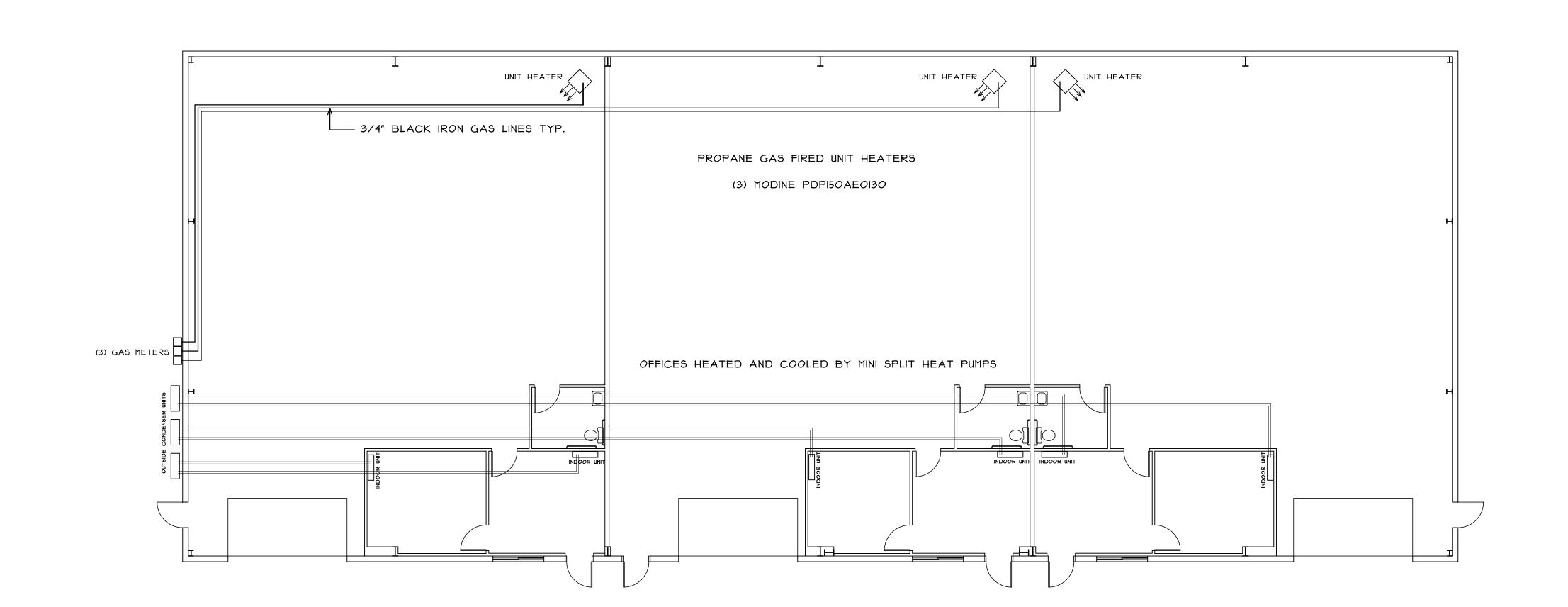
SHEET TITLE

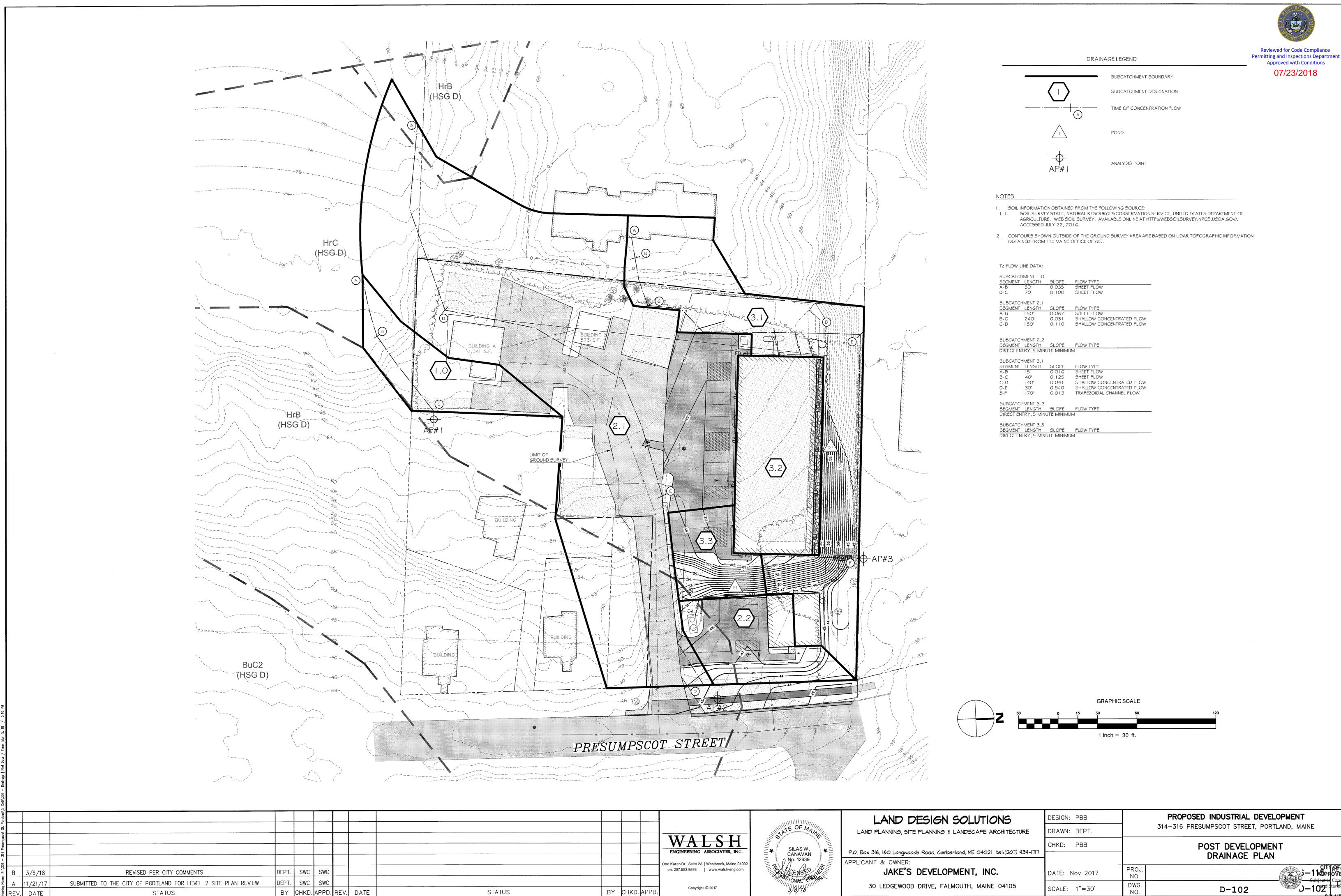
MECHANICAL PLAN

SHEET NUMBER

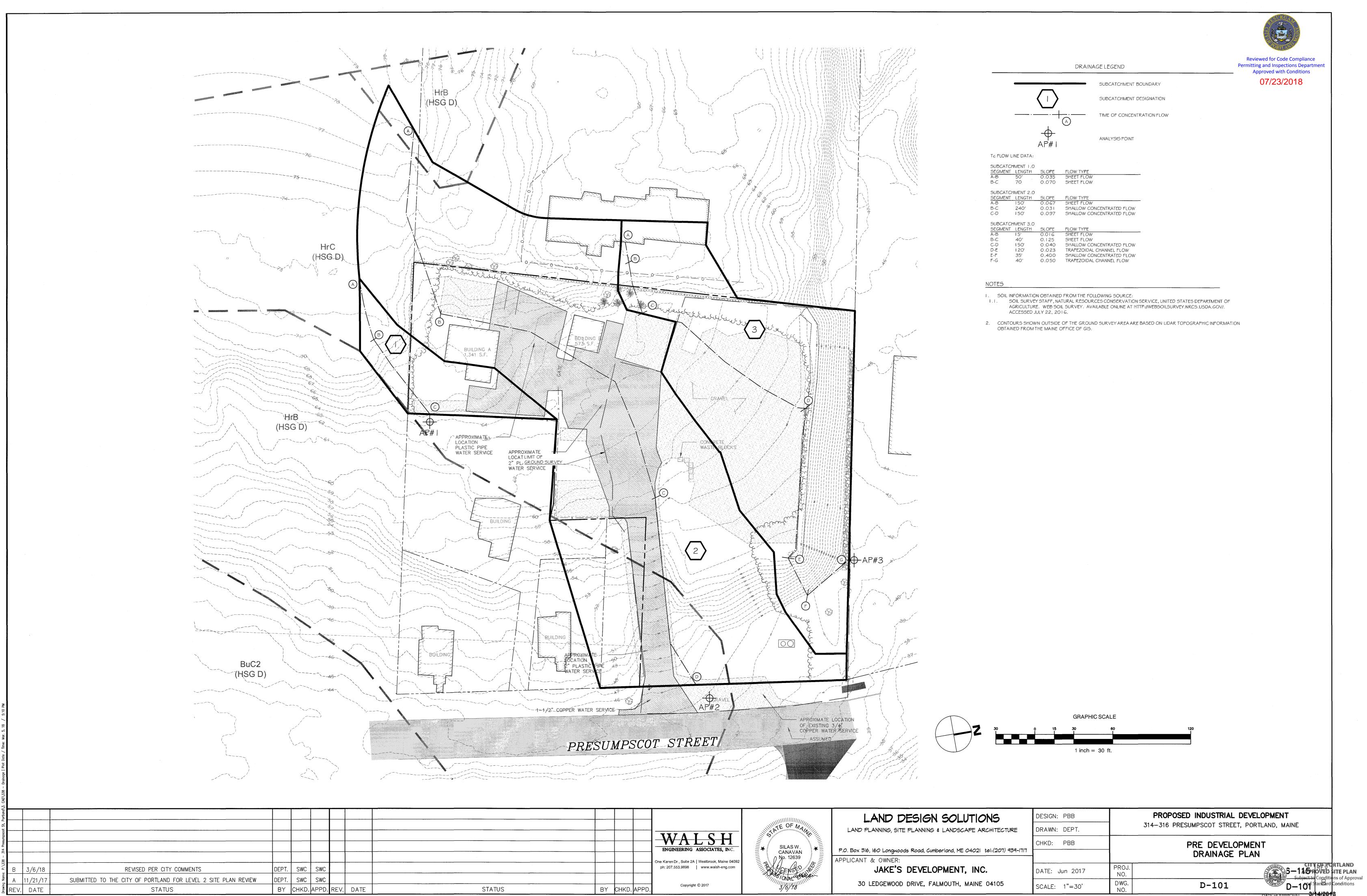
M-1

SHEET | OF |





DATE of APPROVAL 3/14/2018
PLANNER Christian Roadman



D—TOT

DATE OF APPROVAL 3/14/2018

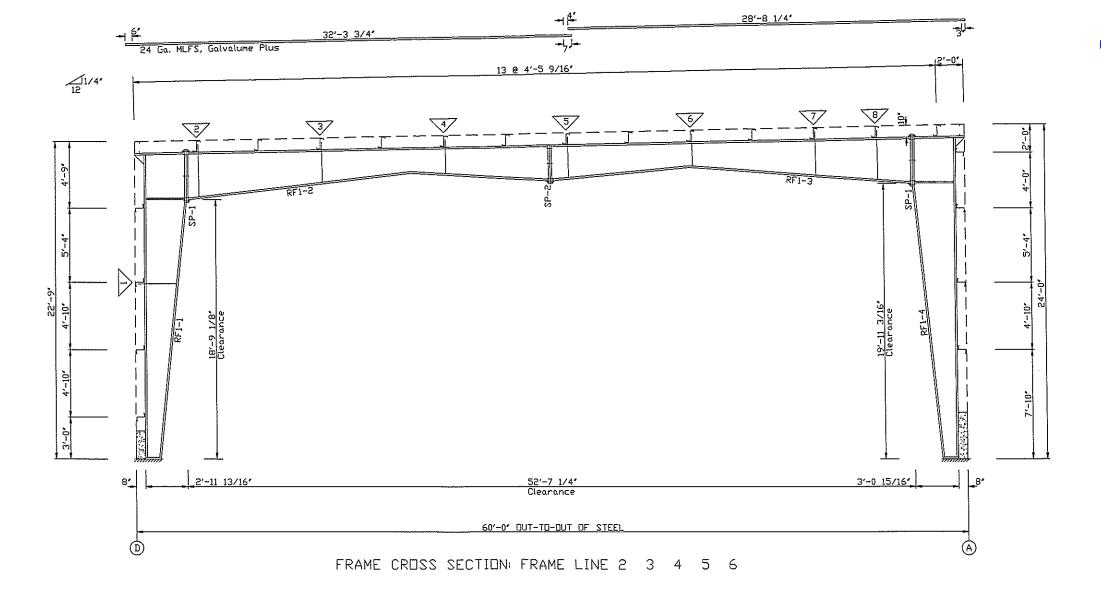
PLANNER Christian Roadman

MEMBER	TABLE				
	14.114	Web Depth	Web Plate	Outside Flange	Inside Flange
Mark	Weight	Start/End 1	hick Lenoth	W x Thk x Length	W x <u>Thk</u> x Length
RF1-1	904	10.5/29.1 0.	188 168.Ö	6 × 5/16" × 262.3	6 × 1/2" × 222.1
		29.1/35.0 0.	313 95.0	6 × 1/2′ × 43.3	
RF1-2	976	38.0/19.0 0.	250 195.6	6 x 5/16" x 314.8	6 × 5/16" × 196.5
		19.0/28.0 10.	188 120.0	1	6 x 5/16" x 120.3
RF1-3	1002	28.0/19.0 lo.	188 120.0	6 x 5/16" x 314.2	6 × 5/16' × 120.3
		19.0/37.0 0.	250 194.2		6 × 3/8' × 194.2
RF1-4	1179		313 96.9	8 x 1/2" x 44.3	8 x 5/8" x 236.2
			188 180.0	8 x 5/16' x 277.0	



Reviewed for Code Compliance Permitting and Inspections Department Approved with Conditions

07/23/2018





DRAWING FRXS-1

					WHITE THE	* -	
REV.		DESCRIP	TION:		DATE:	DRAFT	ENG.
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2							
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◬	PREL	IMINARY DRAWING NOT FOR CONSTRUC	TION / FOR PERMIT ON	1LY	CURRENT	REVISI	0 :NC
PAC	AGE	STEEL SYSTEMS, INC.	Biskup Construc	tion Inc.			
PROJE	CT	320 P Street LLC	RIGID FRAME X-SECTION	אנ		//	<u> </u>
ID		1805-076	DESIGN: ZRM	DESIGN CHECH	< :	/ PAL	KAGE
PROJE	CT	314 Presumpscott St.	DRAFT: TMZ	DRAFT CHECK		/ >=	_//

DATE: 5/30/18

ADDRESS Portland, ME 04103





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PROPOSED BUILDING 320 P STREET, LLC 14 PRESUMPSCOTT STREET PORTLAND, MAINE

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F	REVISIONS
DATE	DESCRIPTION

DATE: 5/4/2018

SCALE: 1/8" = 1'-0"

DESIGNER: JS

CHECKED BY: JB

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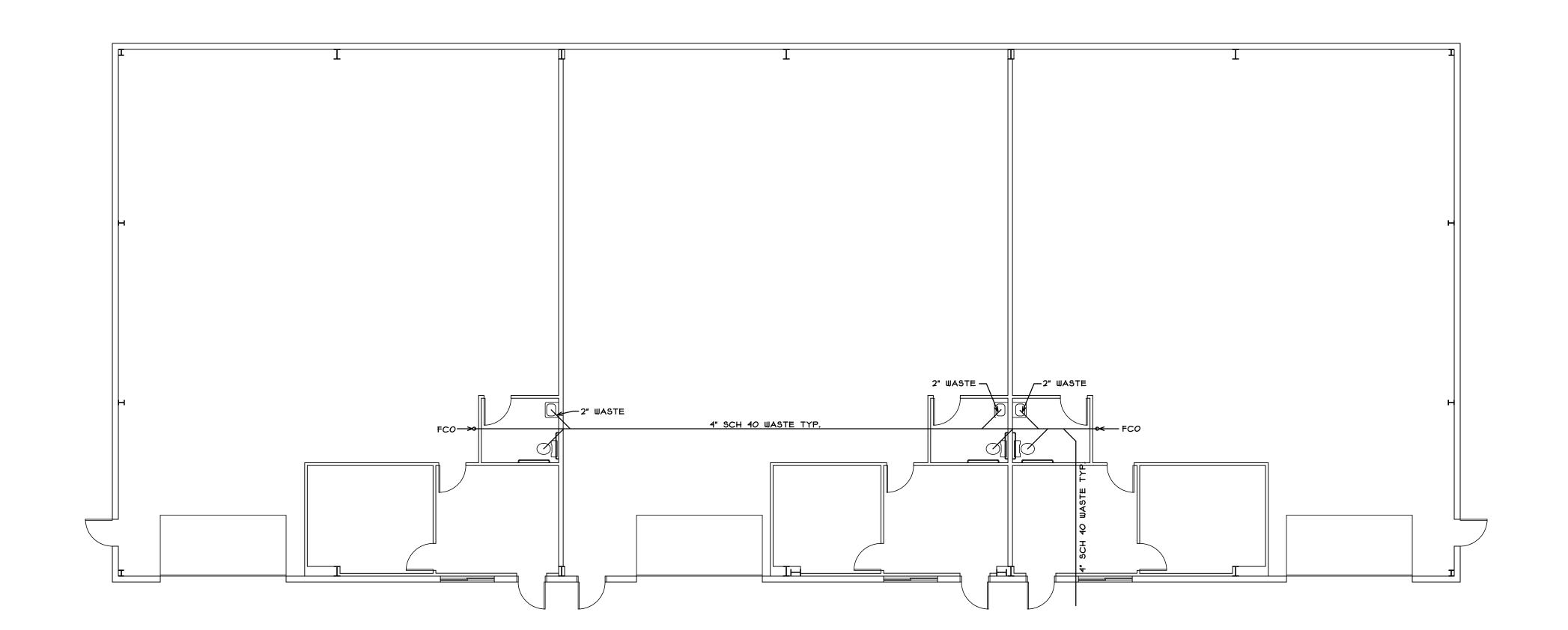
SHEET TITLE

SANITARY WASTE PLAN

SHEET NUMBER

P -

SHEET I OF 2



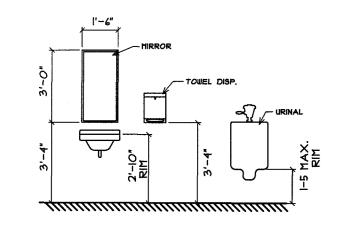
DOOR SCHEDULE

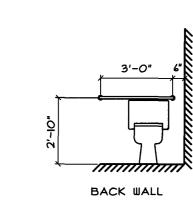
TAG	DOOR SLAB	SIZE	THICKNESS	SWING	FIRE LABEL	FRAME	WALL THICKNESS	REMARKS	
						1194		04 × 00 №0 61400	
1	HM	3010	1 3/4	LHR		HM	4 1/8	24 X 30 INS. GLASS	HRDWR SET #I
2	HM	3010	1 3/4	RHR		НМ	4 1/8	24 X 30 INS. GLASS	HRDWR SET #I
3	НМ	3010	1 3/4	LHR		НМ	4 1/8	24 X 30 INS. GLASS	HRDWR SET #I
4	HM	3010	1 3/4	RHR		HM	4 1/8	24 X 30 INS. GLASS	HRDWR SET #I
5	НМ	3010	1 3/4	LHR		HM	4 1/8	24 X 30 INS. GLASS	HRDWR SET #I
6	HM	3010	1 3/4	RHR		HM	4 1/8	24 X 30 INS. GLASS	HRDWR SET #I
7	HM	3010	1 3/4	LH		HM	4 1/8		HRDWR SET #2
8	HM	3010	1 3/4	RH		HM	4 1/8		HRDWR SET #2
9	НМ	3010	1 3/4	RH		HM	4 1/8		HRDWR SET #2
10	HM	3010	1 3/4	LH		HM	4 3/4		HRDWR SET #3
II .	нм	3010	1 3/4	RH		НМ	4 3/4		HRDWR SET #3
12	нм	3010	1 3/4	LH		HM	4 3/4		HRDWR SET #3
13	нм	3010	1 3/4	RH		HM	4 3/4		HRDWR SET #3
14	НМ	3010	1 3/4	LH		НМ	4 3/4		HRDWR SET #3
145	НМ	3010	1 3/4	RH		НМ	4 3/4		HRDWR SET #3
					HAR	OWARE	SCHEDUL	.E	
			SET #I	HINO	ES, CL	OSER, L	OCKSET	, WEATHERSTRIP, THRESHO	LD
			SET #2	HIN	GES, PR	RIVACY	SET		
			SET #3	HIN	GES. PA	SSAGE	SET		

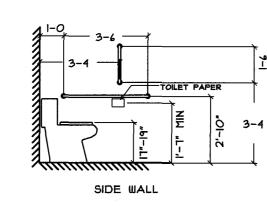
WINDOW SCHEDULE

TAG	SIZE	TYPE	MANUFACTURER	GLAZING	"u" VALUE	SHGC	REMARKS
A	72" × 48"	SLIDER	MARVIN INTEGRITY	LOW "E" WITH ARGON	0.31	0.33	

NOTE: PROVIDE SOLID WOOD BLOCKING IN WALL FOR MOUNTING OF ALL WALL MOUNTED ITEMS



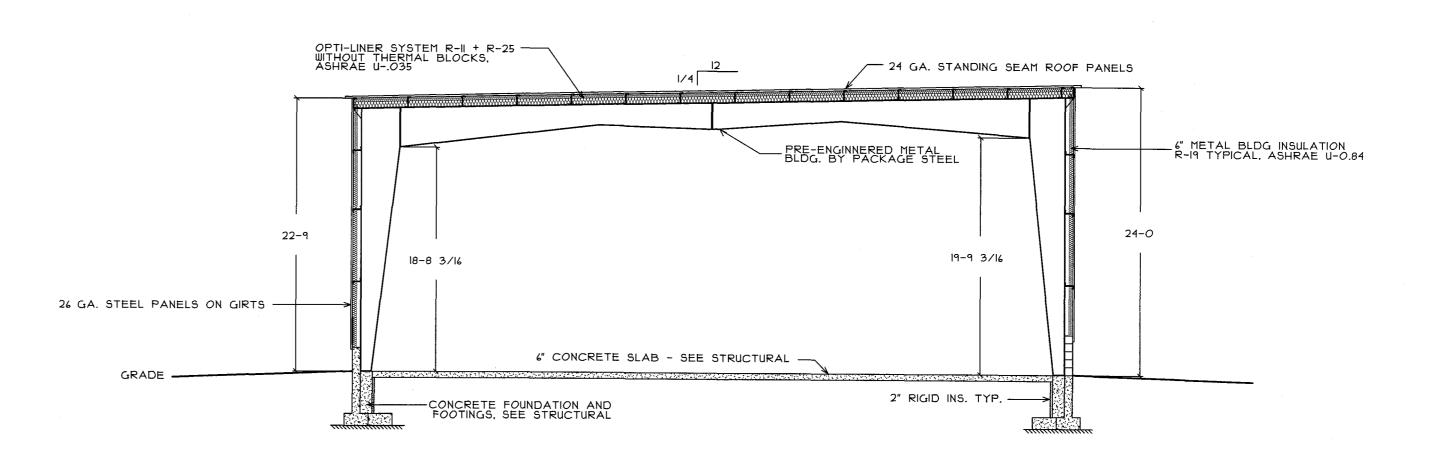




TOILET FIXTURE MOUNTING HEIGHTS

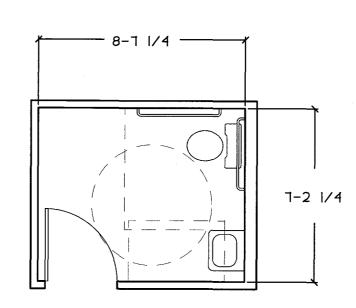
SCALE 1/4" = 1'-0"





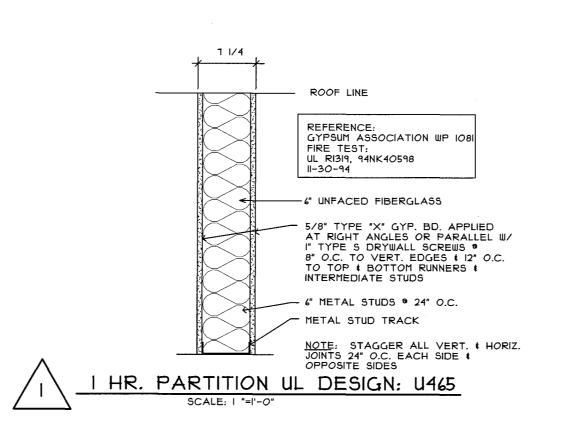
BUILDING SECTION

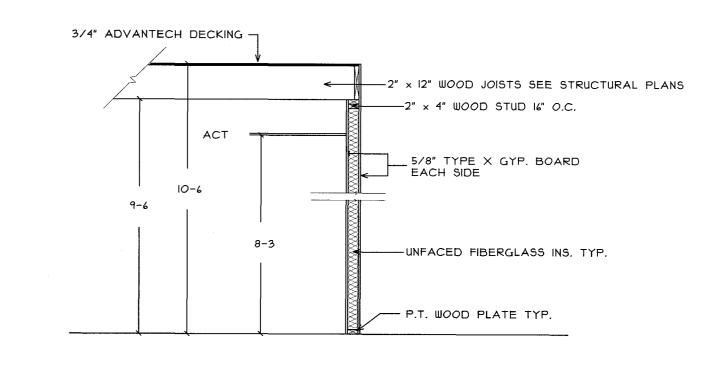
SCALE: 1/8" = 1'-0"



ENLARGED PLAN BATHROOM

SCALE 1/4" = 1'-0"





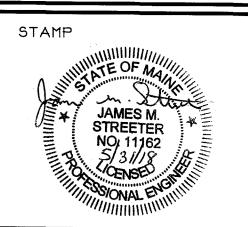
NON RATED WALL & DECK TYP. U.N.O.

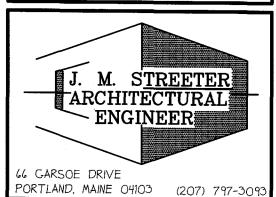
SCALE: 3/8" = 1'-0"

BISKUP CONSTRUCTION, INC.
16 DANIELLE DRIVE
WINDHAM, MAINE 04062

WWW.BISKUPCONSTRUCTION.COM

TEL. (201) 892-9800 FAX. (201) 892-9895





PROPOSED BUILDING 320 P STREET, LLC 314 PRESUMPSCOT STREET PORTLAND, MAINE

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REVISIONS						
DATE	DESCRIPTION					

DATE: 5/30/2018					
SCALE: 1/8" = 1'-0"					
DESIGNER: JS					
CHECKED BY: JB	-				

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SHEET TITLE

SCHEDULES & DETAILS

SHEET NUMBER

A-4

SHEET 4 OF 5

