

	03'10"E	W.17.1 + 6		M.04.94	M.09.50	M., 20, 10, 0	1°Ø4'32"E	1~20'43"E	1.01.30.IE	H.00,50.5	8°04'0 "W	2"30'46"E	BEARING		
	52.68'	130.01	204.73'	136.46	46.84	55.70	120 76'	130.08	118.65	109.501	54.28	26 58	LENGTU	DATA	
C 3 2 3 2 3 2 3 3 3 3 3 3 3 3 3 3 3 3 3				3 <mark>0</mark> 3	2	C6		2							
5101 5101	40.5	121.0	712	52.2	19.1	2.55	38	63.	256	287.	16:				

OCEAN RIDGE CONDOMINIUMS 852 OCEAN AVENUE PORTLAND, MAINE

UNITS 30, 31, & 32

ARCHITECT:	LIST OF DRAWIN
JOHN H. LEASURE ARCHITECT, INC.	1 - GRADING PI
SOUTH PORTLAND, MAINE 041 06	
PHONE: 767–4600 FAX: 767–4600	S1 - GENERAL I
	S2 - FOUNDATIO
<u>CIVIL ENGINEER:</u>	S3 - FOUNDATIO
SEBAGO TECHNICS	ST - SECOND F
ONE CHABOT STREET	S6 - THIRD FLO
WESTBROOK, MAINE 04098 PHONE: 856-0277	S7 - ROOF FRA
	S8 - FRAMING S
STRUCTURAL ENGINEER:	
L & L STRUCTURAL ENGINEERING SERVICES, INC.	A1 - FIRST FLO
6 Q STREET South Dortland Maine 041 06	A2 - SECOND F
PHONE: 767~4830	A3 – THIRD FLO
FAX: 799–5432	A4 - ROOF PLAI
	AT - EXTERIOR
	A8 - BUILDING S
	A9 - WALL SEC
	A10 - SECTIONS
	ATUA- SECTIONS
	ATT = WALL TIPE
	A13 - DOOR AND

MARCH 3, 2005

VINGS:

PLAN SHEET 2

NOTES ON PLAN ON SECTIONS AND DETAILS ON DETAILS FLOOR FRAMING PLAN OOR FRAMING PLAN MING PLAN SECTIONS AND DETAILS OR PLAN LOOR PLAN OR PLAN N ELEVATIONS ELEVATIONS **ELEVATIONS** SECTIONS TIONS AND DETAILS ES AND DETAILS CTIONS - DOOR AND WINDOW SCHEDULES



	- z
--	------------

GENERAL NOTES

- The notes on the drawings are not intended to replace specifications. See specifications for requirements in addition to general notes.
 Structural drawings shall be used in conjunction with job specifications and architectural, machanical, electrical, plumbing, and site drawings. Consult these drawings for locations and dimensions of openings, choses, inserts, regists, sleeves, depressions, and other details not shown on structural drawings.
 All dimensions on the details not shown on structural drawings discrepancies shall be brought to the attention of the engineer before proceeding with the affected part of the work.
 Do not scale plans.
 Sections and details shown on any structural drawings shall be considered typical for similar conditions.
 All proprietary products shall be installed in accordance with the manufacturers witten instructions.
 The structure is designed to be self supporting and stable after the Building is complete. It is the contractor's sole responsibility to determine erscino procedures and sequencing the support brocing, guye or tie downs. Such material shall remoner the process, guye or the down. Such material shall remoner the process, guye or the down. Such material shall remone the property of the contractor after completion of the project.
 All applicable federal, state, and municipal regulations shall be followed, including the federal department of labor occupationd safety and health act.

DESIGN LOADS

1. Building code: BOCA Basic Building Code (1999) 2 Design Live Loads: (Ground snow load = 60 PSF)

sigiri	LIVE LOGUS.	1044 -	. 00		/	
	Roof	 4	42	PSF	+	Drift

Living	areas	40 PSF	

- 3. Design wind loads ore based on exposure 8 using 85 mph basic
- wind speed. 4. Seismic design utilizes the following criteria:
- a Building framing system: Concentrically braced frames and shear walls.
- Analysis procedure: Equivalent Lateral Force Procedure. Selsmic hazard exposure group: "I" Seismic performance category "C" b. C.

- Seismic performance category "C" Soll profile type: "S1" Peak velocity-related acceleration (Av): "0.10"
- Peak acceleration (Aa): "0.10"
- h. Response modification factor (R): '5'
 i. Deflection amplification factor (Cd): '4 1/2"

FOUNDATION NOTES

- Foundations have been designed with a presumptive soil bearing capacity indicated in of 2000 PSF to be verified in the fidd.
 Interior spread footings and exterior strip factings shall be founded
- in notive soil or compacted situations on proceedings of the bolt reaction of the soil or compacted situation of the source of
- 3. Exterior strip and spread footings shall be founded on a minimum
- of 4'-0" below finished grade. Slabs on grade shall bear on a minimum of 12' of compacted
- Status of grade shall be on a minimum of 12 of compacted status of a minimum of 12 of compacted at the slab sub grade level, they shall be over excavated to the surface of the natural sail ond replaced with structural fill. Refer to drawings and specifications for vapor barrier requirements. Concrete slabs shall be moist cured.
- Structural fill shall be used at all locations below footings and slabs and adjacent to the foundation walls. Prior to placement of structural 5.
- fill remove oil topsoil and other unsuitable material Compacted structural fill shall consist of clean granular material free of organics. loom, trash, snow, be, frozen sall or any other objectionable material It shall be well graded within the following units:

SOREEN OR SIEM SIZE	PERCENT FINER BY WEIGHT
4 inch	100

4 1101	100
3 inch	90 ta 1 00
1/4 inch	25 to 90
NO. 40	Ota 30
NO. 200	0 to 5

- Structural fill beneath slabs shall be placed in layers not exceeding 12' 6. in locse measure and compacted by self propelled compaction equipment at approximate optimum moisture content to a dry density of at least 95% of the maximum in place dry density as determined by the modified proctor test (ATSM D-1557).
- 7 Under drains shall be placed as shown an the site drawings. Under drains
- Onder drains shall be placed as shown an the site drawings. Under drains shall be installed to positively drain to a suitable discharge paint away from the structure. Refer to the site drawings far additional Information.
 Exterior concrete dabs on grade, shall be underlain by at least 4 feet of structural fill meeting gradation and compaction requirements noted above. Reinforce slobs with 6x6 W1.4xW1.4 WNF.
- 9. Backfill both sides of foundation walls simultaneously.

CONCRETE NOTES

- All concrete work shall conform to ACI 318-Latest Edition.
- Concrete strength at 26 days shall be a. 4000 PSI for basement walls. b. 3000 PSI for footings, frost walls and piers. c. 4000 PSI for all dabs on grode. All concrete shall be air entrained 4%-6% with approved odmixtures. Concrete shall not be placed in water or on frozen ground. Provide PVC sleeves where pipes pass through concrete walls or
- slobs.
- Reinforcing bors shall conform to ASTM A615 Grade 60 deformed bars, and shall be detailed, fabricated and erected in accordonce with ACI 315-Lotest edition. 6.
- Welded wire fobric shall be provided in flat sheet
- Fiber reinforced concrete shall conform to ATSM C-1116. Complete shop drawings and schedules of all reinforcing steel shall be prepared by the contractor and submitted to the engineer for review prior to commencement of that portion of work. All accessories must be shown on the *shop* drawings. Submit (6) blue line prints and (1) reproducible (sepio) to the Architect.
- 10. Splices af reinforcing bars shall be in accordance with ACI 318. splices of WWF shall be 6 minimum. 11.
- Concrete finishes: See specifications and Architectural drawings. For additional information consult hardwood floor manufacturer for preferred concrete finish before placement.
- 12. Anchor bolts shall conform to ASTM A307 unless noted otherwise on plan.
- 13. Provide control/construction joints in foundation walls at a maximum spacing of 15 ft. from any corner or 30 ft. along length of wall. At contrd pints, discontinue every other horizontal bar. At construction joints all reinforcing shall be continuous through the joint.
- me general contractor shall be responsible for coordination of: door bond out locations. slob depression and other required band outs. Coordinate location of band outs with Architectural, Mechanical & Plumbing. Electrical and kitchen equipment vendors as necessary to properly install each specific item
- 15. Pravide control joints in dabs as follow:
 - a 15' x 15' (225 SF) with fibremesh reinfarcment
 - b. 20' x 20' (400 SF) with welded wire fabric reinfarcment

STRUCTURAL STEEL NOTES:

- 1. Structurd steel fabrication, erection, ond connection design shall conform to AISC 'Specification far the design, fabricotion. and erection of structural steel"-Latest edition.
- 2. Structural steel: a. Structural steel shall conform to ASTM A-36. b. Structural tubing shall conform to ASTM A-500 GR.B. c. Structural pipe shall conform to ASTM A-53, TYPE E or S.
- Design connections for the reactions shown on the drawings or the maximum end reaction that can be produced by a laterally supparted

- Field connections shall be bolted using 3/4* ASTM A325 high strength bolts except where field welding is indicated on the drawings.
 All welding shall conform to AWS 01.1-Latest edition. Welding electrodes shall be E70XX.

TIMBER TRUSS FRAMING:

- agreement. 6. 7.
- ASCE 7-99.

8.

TIMBER FRAMING:

- 3.

- 7.



1. Materials Stress graded lumber, metal plate connectors. Minimum grade No. 2 M.S.R. Southern Pine, kiln dried. 15% maximum M.C., or approved alternate. 2. Applicable specifications:

National Design Specification far stress graded lumber and

its fastening (NDS).
 b. Design specifications for light metal plate connected wood trusses (TPI-Latest edition)
 racing: The truss manufacturer shall specify all bracing required

3. Bracing: The truss manufacturer shall specify all bracing required both for temporary construction loading and for permonent lateral support of compression members. Submittals:

a Submit design colculations, shap drawings and exection procedures oil affixed with the sect of a professional structural engineer registered in the State of Maine. shop drawings shall show stress grade ond size of members size and location of plate connectors, size and location of 5. All fabricated trusses shall be inspected at the fabrication plant and approved trusses shall receive the TPI mork of approval in accordance with the truss plate institute in-plant inspection license

Connector plates shall be galvanized. Timber trusses shall be designed in accordance with BOCA and

Provide permanent bottom chord bracing in accordance with the truss plate institute (TPI-lotest edition). Trusses shall be designed for all potential load combinations of live

laads (snow) ond wind loads including unbalanced snow loads, drift loads and wind loads in accordance with BOCA 1999. 10. Maximum permissible floor live i wd deflection = L/480

See S8 for floor loadings

1. All timber framing shall be in accordonce with the AITC timber construction manual or the national design specifications (NDS)

 - latest edition.
 Individual timber froming members shall be visually graded, minimum grade #2 Spruce-Pine-Fir (SPF), kin dried to 19% maximum moisture content.

Pressure treated lumber shall be used where wood is in contact with ground, concrete or masonry. Timber shall be southern yellow pine treated with cca to 0.4 #/CF in accordance with AWPA C-18. Metal connectors shall be used at all timber to timber connections or as noted on the design drawings

5 Provide Simpson H2.5 hurricane anchors where timber framing and/or trusses bear on walls. 6. Nailing not specified sholl conform with BOCA 1999.

Exterior wall sheathing shall be 1/2" thick APA rated sheathing fastened with 10d nails @ 4 o.c. at panel edges and 6' o.c. intermediate, (typ unless otherwise noted)

8. Floor deckina shall be 3/4" thick APA rated "STURDI-FLOOR" plywood sheathing fastened with' construction othesive and 10d nails 0 6 ' o.c. ot panel edges and intermediate.

Roof sheathing shall be 5/8" thick APA rated sheathing fastened with 10d nails 0 6" o.c. at panel edges and intermediate.

10. All 2 x P.T. sill plates shall be installed on sill sealer.







FOUNDATION

		designed by: JHL	rev.	date	description			
	852 OCEAN AVENUE	drawn by: JML checked by: JHL				appro		ENGINEERING SERVICES, INC.
	PORTLAND, MAINE	scale: date: 2-23-05	-				•	SOUM PORTLAND. MAINE 04106
N	ENHYDATION1 PLAN2	plot date: - project #: 23035						PHONE (207) 767–4830 FAX (207) 799–5432 EMAiL: II.engineering@verizon.net





S2

AVENUE MAINE TAILS
5





NOTES 1. SEE GONEDAL MOTES ON S1. 2. V." INCLOTES VERSALAN BELIN MANAFACTURED B BORE CASCINES CORP. OR APPROVED EXUM. 3. PROVING CASCING SOME ALL SA MILLS MANUES AT BOTH ENDES OF HEADERS. (The U.M.O.)	50,-0,	2		- 34'-0"			Proder work
		designed by: JHL	rev. date	description op	pr'd		
	OCEAN RIDGE CONDOMINIUMS	drawn by: JML			HL-		ENGINEERING SERVICES INC
	PORTLAND MAINE	scole:				•	SIX O STREET
	SECOND FLOOR FRAMING PLAN	date: 2-23-05					SUUIT PURILAND, MAINE U4106
	UNITS 30, 31 & 32	piot date: - project # 23035					FAX (207) 799–5432 EMAIL: II.mg!+wering*iron.net





TCLL=40 PSF TCDL=10 PSF BCLL=0 PSF BCDC=10 PSF

1									
		designed by JH.	rev. date	description	17,000		[
							• •		_
		drawn by Ma				_	~		
							ר אר ר		
	1 832 UCEAN AVENUF	checked by JH.							
)	I I I I I I I I I I I I I I I I I I I							NG VERVICES INC	
	PORILAND. MAINE	scale:	_				· SIV O STREET		
							קא ע הואבר		
	POOF FRAMING PLAN	date: 2-23-05					SOUTH PORTLAN	ND, MAINE 04106	
	I COL LYAMING FLAN					-			
		plot agter -					PHONE: (207) 767	-4830	
		Droket & 23045					FAX: (207) 799-	-5432	
						-	EVAII: I andread	Ind American and	

PROJECT NORTH

"N" INDICATES VERSALAN BEAN IMMURACTURED BY BOSE CASCADES CORP. OR APPROVED EQUAL
 "INDICATES COLLIAN PROPERTIES SHALL BE "VERSA-LAN BEAN" 3000 Fb DF (E=2.0x10" PSI AND Fb=3000 PSI).

4. ROOF TRUSS LOADING SHALL BE AS FOLLOWS:

LEGEND

BEARING WALL

TRUSS TYPE 'A' @ 24" O.C.







	UNITS 30, 31 & 32
	SECOND FLOOR PLAN
	PORTLAND, MAINE
	852 OCEAN AVENUE
	OCEAN RIDGE CONDOMINIUMS
) :	
	AND WANDOWS UNLESS NODCATED OTHERWISE.

3) INTERIOR DIALENSIONS WE TO CENTERLARE OF WALLS/DOORS

<u>Notes:</u> 1) diffre Bulling Shull be springlered per NFM 139: 2) FOR WALL THPES, SEE DWG, A12.

JOHN IN LEASURE ARCHITECT, INC. 6 Q STREET SOUTH PORTLAND, MAINE 04106





PROJECT NORTH



NOTES; 1) ENTRE BUILDING SHALL BE SPRINKLERED PER NFPA 13R 2) FOR WALL TYPES, SEE DWG. AD. INTERIOR DIMENSIONS ARE TO CENTERLINE OF WALLS/DOORS AND WINDOWS UNLESS INDICATED OTHERWISE.



PROJECT NORTH

DATE 9-9-9





C

NOTES:

Entire Building Shall be sprinklered per NFPA 13R
 See A5 For Minimum Attic Ventilation Requirements.

OCEAN RIDGE CONDOMINIUMS 852 OCEAN AVENUE PORTLAND, MAINE

UNITS 30, 31 & 32



8 •

OCEAN RIDGE 852 OCEAN AVENUE PORTLAND, MAINE EAST ELEVATION UNITS 30, 31 & 32 32 CONDOMINIUMS

A5

ATTIC MINIMUM VENTILATION REQUIREMENTS (WITH WHYOR BURRER AT CELING)

JOHN H. LEASURE ARCHITECT, INC. 6 Q STREET SOUTH PORTLAND, MAINE 04106

STATUS DATE 3-3-0

<u>8</u>

TA TA TA

Ę

YHLT SHANGLES, TYP.











OCEAN RIDGE 852 OCEAN AVENUE PORTLAND, MAINE sections & details units 30, 31 & 32 CONDOMINIUMS A10

NSULATION

INSULATION

100⁰ SLAB EL=167⁰-11⁰ EL=168⁰-11¹ (UNIT 30)

3/4" ENT. PLYMOOD WHOR RELINCOR (SEE WERK SPEC) 11.F. FLOND (T/ FLYMOOD) EL-188-0" (INIT 30)

OS DOT WILL

WE BD, CELING

JOHN H. LEASURE ARCHITECT, INC. 6 Q STREET SOUTH PORTLAND, MAINE 04106

DATE STATUS 3-3-05











OCEAN RIDGE CONDOMINIUMS 852 OCEAN AVENUE UNITS 30, 31 & 32





8	TEMPERED SAFETY GLASS IN ALL STAIR WINDOWS, WITHIN 24" OF DOOI IS THAN 18" ABOVE FINISHED FLOOR NEXT TO A WALKING SURFACE.	E MOTES: PROVDE AND LES								
						¥, NR	IREA SHALL HAVE DTH, 24" IN HEIGH BOVE FINISHED FLO LEAR OPENING	n or sleeping / W WIN, 20" in W Ore Than 44" a JN 5.7 Sq. Ft. ci Each Bedroom)	HINDTE 1: EACH BEDROO EGRESS MINDO & SILL NOT M WITH A MINIM (MINIMUM ONE	
	R.O.	R.O.								
	<u> </u>	RÓ	E" HEAD HGT @ 7-8" AFF	NCOCK LUNDER WINDOW THP	3-0" X 3-8" HA	N/A	0H3644*	"HANCOCK" P	3 5 2	
20 1	HELE SUPPORT		E" *EGRESS WHOOW	NCOCK LUMBER WINDOW TYP	3-4" X 5'-0" "HA	N/A	10H4060* H4080-2*	"HANCOCX" P	5 5 ∞ >	_
			E" ANNING	NCOCK LUNBER WINDOW TYP	2'-2" X 2'-0" "HM	N/A	- 0H2860*	"HANCOCK" P	WIA E	-
-		WINDOWS		MARK	ROUGH OPENING RE	DIMENSION	AT NO. UNIT	MANUF C	NO. TYPE	
		Δ 				HEDU	SCI	DOW	N N	
	he rloor plans and rlevations	() - Indoor and door quantiles shall be calculated from in								
			C 19000	88 8	- B 8	10 80 90 10 10 10 10 10 10 10 10 10 10 10 10 10	13/8	2-6 x 6-8" PR 2-8 x 6-8"	8 8 8 8	
			C C W000	55	8 3	1908 10 10	13/8	Z-8×4-0	: <u>у</u> в	
	()	1	c w000	8	LOOKET	10	1.3/8	Z-5'x 5-8"	30 B	
1		-	C 1000	8	8	1000	1.3/8"	2-6"x 6-8"	27 B	
MAIES	NME2		C W000	888	3	10 000	1 3/8	2-51 5-5	a a 8. 8	
Ł			C C W000	5 5		5 10 5 10	3/8	2-6x 6-8	24 B	
	7-0	7-0	c 1000	8	78 5	1000	- 3/8°	2-5×6-6	23 ZZ	
			C W000	5 5	8 28	1908 1908	- 3/8°	PR Z-8x 6-8 1	B	1 .
		-1	C 1000	8	PRS	1900 100	3/8	2-5x 5-8 1	20 B	
			- C W000	8	- 91, 55, 73	19408	3/40	7-8"x 8-8" 1	15 B	
Τ			1 1 1 1	1 8	- P.S	I	3/8	2-8"x 6-6"	7 U	
	134 WOOD () 1 HR FRE ANDID METAL () NSWATED FLUSH METAL () NSWATED FLUSH METAL	SULD COME FLUSH NOOD () HOLLING COME FLU		/ 8	P.S.		3/8	2-6x 6-8 1	12 ■ B	
	WARS _ WARS _	WHE'		8	- NS, SH, LOOKSET,	10408 186A MIT.	3/4° 1 HR.	3-0'x 6'-8' 1	= i	
				8	- 35,00,13	10408 18CA MTL -	3/4° 1 HR	3-0"x 8'-8" 1	10 F	1
			C AUMA	×	- NS, 12, 00	PUL 18CA MIL -	3/8"	3-0'x 6'-8' 1	•	
7-10	<u>7-8</u>	<u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u></u>	C NULN	88 W000	NSL OH GWW	RLUS NOT	+	9-0'x 7-0'	8 n -	1
	100 SQ. NCH. WX.		c AUAA	88 W000	- NS, 12, TEMP, 13	0001 80101	3/8"	3-0'x 6-8' 1	2	1
	†] †	 	F.R. HEAD JAMB MAI. SILL HT.	S TYPE MAT.	ZE TYPE REMARKS	SET MAT. SI	HK. F.R.	SIZE T	VO. TYPE	
		() 3010 000 MIN 3050 TE () 3010	AME TYPES THRESHOLD				DOOR			1
		× × × × × × × × × × × × × × × × × × ×								1
	<u> </u>		5405 971 V 160607	XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX	HC HOLLOW COR					
			ACTION AND A CONTRACT		ES ELECTRIC STR F.J.P. FINGER JOINT FR FINE RATED					
			UNID UNID		D.K. DOOR NHOOK D.S. DOOR NHOOK DHO ELECTRO. HO	C r	r	ζ - -		
	ITPES		DINNE S. STEL DIN WETAL S.C. SOLD CORE HARDBOARD			 n	С Т П Г	ס ח	700	
	1()))))	HEDULE ABBREVIATIONS	DOOR SC	_					

