1999-0190
1021 Ocean Ave.
Building Addition/Retail
William Gribizis

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CITY OF PORTLAND, MAINE DEVELOPMENT REVIEW APPLICATION PLANNING DEPARTMENT PROCESSING FORM

19990150	
LD: Mumber	 _

William Gribizis			10/22/99
Applicant		_	Application Date
285 Clifton Street, Portland, ME 04103		_	Ocean Ave 1021
Applicant's Malling Address			Project Name/Description
SAA Consultant/Agent		1021 Ocean Ave, Portland Ma	aine 04103
773-6034		Address of Proposed Site	
Applicant or Agent Daytime Telephone, F	2X	415-B-004 Assessor's Reference: Chart-B	look I as
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Proposed Building square Feet or # of Un		age of Site	IM Zoning
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Site Plan (major/minor)	Subdivision # of lots	LI PAD Review	14-403 Streets Review
☐ Flood Hazard	☐ Shoreland	☐ HistoricPreservation	DEP Local Certification
☐ Zoning Conditional Use (ZBA/PB)	Zoning Variance		C Other
Fees Paid: Site Plan \$400.	00 Subdivision	Engineer Review	Date: 10/22/99
Planning Approval Status	7 a	Reviewer	ing personal and an english and personal and are resident of the personal and an english and an english and an
☐ Approved	Approved w/Conditions See Attached	Denied	
Approval Date	Approval Expiration	Extension to	☐ Additional Sheets
OK to Issue Building Permit	···		Attached
	signature	date	NHINNIZANGAZIZIZIZIZIZIZIZIZIZIZIZIZIZIZIZIZIZIZI
Performance Guarantee	☐ Required*	C] Not Required	Value to the state of the state
* No building permit may be issued until a	performance guarantee has be	en submitted as indicated below	
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Inspection Fee Paid			·
	date	amount	
Building Permit Issued		<u></u>	
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Performance Guarantee Reduced		·	
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Temporary Certificate of Occupancy		Conditions (See Attached)	
	date		
Final Inspection			
Certificate Of Occupancy	date	signature	
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Defect Guarantee Submitted	submitted date	amount	Amfunktion -l-t-
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CITY OF PORTLAND, MAINE DEVELOPMENT REVIEW APPLICATION PLANNING DEPARTMENT PROCESSING FORM

19970117	
LD: Number	 -

Applicant 285 Clifton St, Portland, ME 04103			12/19/97 Application Date Ocean Ave 1021
Applicant's Mailing Address SAA Consultant/Agent 773-6034 Applicant or Agent Daytime Telephone, Fa	x	1021 Ocean Ave Address of Proposed Site 415-B-004 Assessor's Reference: Chart-B	Project Name/Description
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Site Plan (major/minor)	Subdivision # of lots	PAD Review	14-403 Streets Review
Flood Hazard	Shoreland	☐ HistoricPreservation	DEP Local Certification
Zoning Conditional Use (ZBA/PB)	Zoning Variance		Other
Fees Paid: Site Plan \$300.00	Subdivisio	Engineer Review	Date 12/19/97
Planning Approval Status:		Reviewer Kandi Talbot	
	Approved w/Conditions See Attached	Denied	
Approval Date 1/2/98	Approval Expiration 1/2/9	9 Extension to	Additional Sheets
OK to Issue Building Permi	Kandi Talbot signature	1/2/98 date	Attached
Performance Guarantee	Required*	Not Required ■	1914 (1914 (1914 1914 1914 1914 1914 191
* No building permit may be issued until a	performance guarantee has beer	n submitted as indicated below	
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Inspection Fee Paid	date	amount	<u> </u>
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Temporary Certificate of Occupancy	datedate	remaining balance Conditions (See Attached)	signature
Final inspection			
Certificate Of Occupancy	date	signature	
Performance Guarantee Released	date		
Defect Guarantee Submitted	date	signature	
Defect Guarantee Released	submitted date	amount	expiration date
	date	signature	

CITY OF PORTLAND, MAINE DEVELOPMENT REVIEW APPLICATION PLANNING DEPARTMENT PROCESSING FORM

19970117	
I. D. Number	

Gribizis, William Applicant 285 Clifton St, Portland, ME 04103 Applicant's Mailing Address SAA Consultant/Agent 773-6034 Applicant or Agent Daytime Telephone, Fax Proposed Development (check all that appl		1021 Ocean Ave Address of Proposed Site 415-B-004 Assessor's Reference: Chart-B	
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Flood Hazard	Shoreland	☐ HistoricPreservation	DEP Local Certification
Zoning Conditional Use (ZBA/P8)	Zoning Variance		Other
Fees Paid: Site Plan \$300.00	Subdivision	Engineer Revie	Date: 12/19/97
DRC Approval Status:	andaman (66 gans) (100 gg) (10	Reviewer Jim Wendel	
☐ Approved ⊠	Approved w/Conditions see attache	☐ Denied	
Approval Date 1/2/98	Approval Expiration1/2/9	99 Extension to	Additional Sheets
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Performance Guarantee	Required*	Not Required	
* No building permit may be issued until a p	erformance guarantee has bee	n submitted as indicated below	
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Final Inspection			
Certificate Of Occupancy	date date	signature	
Performance Guarantee Released	data		
Defect Guarantee Submitted	date	signature	
Defect Guarantee Released	submitted date	amount	expiration date
	date	signature	

CITY OF PORTLAND, MAINE DEVELOPMENT REVIEW APPLICATION PLANNING DEPARTMENT PROCESSING FORM ADDENDUM

19970117	
I. D. Number	

Gribizis, William		12/19/97
Applicant		Application Date
285 Clifton St. Portland, ME 04103		Ocean Ave 1021
Applicant's Mailing Address		Project Name/Description
SAA	1021 Ocean Ave	
Consultant/Agent	Address of Proposed Site	
773-6034	415-B-004	
Applicant or Agent Daytime Telephone, Fax	Assessor's Reference: Ch	nart-Block-Lot
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	ions of Approval	
- that the applicant utilize crush stone for the proposed drivewa	ay.	
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Planning Cond	litions of Approval	
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	nditions of Approval	
1.The pavement setback is required to be 10' instead of the 7'	shown	

Fire Conditions of Approval

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- (11) THERE ARE TREES ALINE THE SIDES

 OF BOTH PROPERTY LINES. THERE ARE

 PLANTS WEAR THE PRINCESOD BUILDING THAT WILL

 PRESENDED. THE PROPERTY IS VERY WILL

 NAWD SCHPED.



March 25, 1993

William G Gribizis

285 Clifton Street Portland ME 04103 4614

KMC LOAN NO.: 845286-6

The above referenced loan was paid in full on 03-11-93. Enclosed are the indicated documents.

Mortgage Note:

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Mortgage Deed:

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Release Document:

Recording Instructions:

Other:

These documents are for your records unless otherwise specified.

We would like to take this opportunity to thank you for your business.

Knutson Mortgage Corporation 1-800-648-4800

SMM

PD004 013

LOAN NUMBER: 845286-6

ADJUSTABLE RATE NOTE 1 Year Treasury Index—Rate Caps

457528052-

THIS NOTE CONTAINS PROVISIONS ALLOWING FOR CHANGES IN MY INTEREST RATE AND MY MONTHLY PAYMENT. THIS NOTE LIMITS THE AMOUNT MY INTEREST RATE CAN CHANGE AT ANY ONE TIME AND THE MAXIMUM RATE I MUST PAY.

August 23, 19 89	Fortland,	Maine
· :	[City]	[State]
1021 Ocean Avenue	, Portland, Maine 04103	
	Property Address	
CONTRACTOR AND AN AN AN AND AN AND AN AND AN AN AND AND		
1. BORROWER'S PROMISE TO PAY		0.00
in return for a loan that I have received	, I promise to pay U.S. S. 102,40	0.00 (this amount is called "princi
cipal"), plus interest, to the order of the Lende Home Owners Savings Bank	< F.S.B.	
I understand that the Lender may transfer this receive payments under this Note is called the	s Note. The Lender or anyone who tak	ces this Note by transfer and who is entitled to
	pal until the full amount of principal he I will pay will change in accordance	nas been paid. I will pay interest at a yearly rate
The interest rate required by this Section 2 an scribed in Section 7(B) of this Note.	d Section 4 of this Note is the rate I v	will pay both before and after any default de
3. PAYMENTS (A) Time and Place of Payments I will pay principal and interest by mak	ting payments every month.	
	first day of each month beginning on _	October 1, 19 89
I may owe under this Note. My monthly payme 2014 , I still owe amounts under this No	ents will be applied to interest before p	rincipal. If, on September 1,
I will make my monthly payments at		
or at a different place if req	uired by the Note Holder.	
(B) Amount of My Initial Monthly Pay	yments	
Each of my initial monthly payments w	vill be in the amount of U.S. \$ 926	. 90
(C) Monthly Payment Changes	effect changes in the unpaid principal (of my loan and in the interest rate that I mus
4. INTEREST RATE AND MONTHLY PAT (A) Change Dates	YMENT CHANGES	
The interest rate I will pay may change on	the first day of September	
day every 12th month thereafter. Each date on (B) The Index	which my interest rate could change	is called a "Change Date."
	constant maturity of 1 year, as made	k. The "Index" is the weekly average yield on available by the Federal Reserve Board. The is called the "Current Index."
	te Holder will choose a new index which	h is based upon comparable information. The
(C) Calculation of Changes		m
Before each Change Date, the Note Holde Ouarter percentage points (adding Three and One ex. The Note Holder will then round the result
of this addition to the nearest one-eighth of one rounded amount will be my new interest rate to	e percentage point (0.125%). Subject t	to the limits stated in Section 4(D) below, this
		would be sufficient to repay the unpaid princi-
pal that I am expected to owe at the Change Dat The result of this calculation will be the new a		v interest rate in substantially equal payments.
(D) Limits on Interest Rate Changes		, , , , , , , , , , , , , , , , , , , ,
	will never be increased or decreased at (2.00 %) from the rate of	e greater than <u>11.95</u> % or less than I on any single Change Date by more than interest I have been paying for the preceding
twelve months. My interest rate will never be a	greater than <u>15.95</u> %.	
(E) Effective Date of Changes My new interest rate will become effective	e on each Change Date. I will pay the ar	mount of my new monthly payment beginning

(F) Notice of Changes

erange gray in

The Note Holder will deliver or mail to me a notice of any changes in my interest rate and the amount of my monthly payment before the effective date of any change. The notice will include information required by law to be given me and also the title and

on the first monthly payment date after the Change Date until the amount of my monthly payment changes again.

GRIBIZIS ADDITION

1021 Ocean Avenue Portland, Maine PSE Project No. 119-99

STRUCTURAL DRAWINGS, SPECIFICATIONS, AND SKETCHES

Drawings S1 and S2 Sketches SK-1 thru SK-22

Prepared for:

Bill Gribizis 285 Clifton Street Portland, ME 04103 Tel: (207) 773-6034

Prepared by:

David A. Price, P.E.
Price Structural Engineers
75 Farms Edge Road
North Yarmouth, ME 04097

Tel: (207) 846-0099 Fax: (207) 846-1633

August 25, 1999



40° x 40° ADDITION FOR BILL GRIBIZIS Project Location: 1021 Ocean Avenue; Portland, Maine August 23, 1999

GENERAL STRUCTURAL NOTES

DIVISION 1 - GENERAL REQUIREMENTS

Section 01010 - Summary of Work

- 1. Work of this Contract is comprised of the structural framing and foundation for a new 40' x 40' addition at the above referenced project location. Elevated floor is designed for a 50 psf office live load.
- 2. Work and materials shall conform to the 1996 BOCA National Building Code, State of Maine Building Codes, and other applicable codes and standards and shall meet the requirements of local authorities having jurisdiction.
- 3. Coordinate work schedule, daily hours of construction, location of material storage, access to utilities, and final cleanup requirements with owner prior to construction.
- 4. Structural drawings and specifications do not include provisions for sitework, watertightness of building, NFPA fire code requirements, Americans with Disabilities Act (ADA) requirements, egress requirements, or other architectural features.
- 5. The following list of drawings and sketches form a part of this specification:
 - S1 Foundation and 2nd Floor Framing Plan
 - S2 Roof Plan
 - Structural Sketches SK-1 through SK-22 (attached to this specification)
- 6. The structural design is based on the full interaction of all its connected parts. No provisions have been made for any temporary conditions that may arise during construction prior to the completion of the structure. The Contractor shall be responsible for adequate design and construction of all forms, shoring and temporary bracing during the progress of the project.
- 7. Alternate connection details may be used if such details are submitted to the Structural Engineer for review and acceptance is granted. However, the Structural Engineer shall be the sole judge of acceptability and the Contractor's Bid shall anticipate the use of those specific details shown on the Drawings. The Contractor shall be responsible for the design of any alternate details which he proposes.

- 8. The Contractor shall be completely responsible for the safety of adjacent structures, property, and the public. The Contractor shall comply with all federal, state and local requirements.
- 9. Do not scale from Drawings.
- 10. All materials shall be new except those labeled "EXG" (existing).
- 11. Work not indicated on a part of the Drawings but reasonably implied to be similar to that shown at corresponding places shall be included.
- 12. Any modification or alteration of these Construction Documents or changes in construction from the intent of these documents by the Contractor without written approval of the Engineer shall remove all professional and liable responsibility on the part of the Engineer.
- 13. The Contractor is required to examine the Drawing and Specifications carefully, visit the site and fully inform themselves as to all existing conditions and limitations, prior to submitting their Bid. Failure to visit the site and familiarize themselves with the existing conditions, interferences and other limitations will in no way relieve the successful Bidder from furnishing any materials or performing any work in accordance with Drawings and specifications (at no additional cost to the Owner).
- 14. Contractor shall obtain all necessary permits prior to proceeding with construction. Coordinate temporary dust enclosure requirements and security requirements with Owner. Submit detailed construction schedule to Owner prior to construction.
- 15. Remove and legally dispose of demolished materials.
- 16. Contractor shall take all necessary precautions to ensure that existing building components are not damaged during construction. All damaged areas shall be completely restored to the full satisfaction of the Owner at no additional cost to the Owner.
- 17. Stored materials shall be kept under cover and dry. Protect from weather and contact with damp or wet surfaces. Stack materials in such a manner that prevents warping or crushing.
- 18. Pre-manufactured materials shall be installed in accordance with manufacturer's requirements and recommendations.
- 19. Except where slope is specified, new materials shall be installed plumb, level, and square.

20. Substitutions for specified pre-manufactured materials may be made but only after specific written approval has been provided by the owner's engineer prior to installation.

DIVISION 2 - FOUNDATIONS

Section 02200 - Foundations and Backfill

- 1. Foundation excavations shall extend to undisturbed soil capable of providing sound, stable bearing below footings. Engineer assumes no responsibility for subsurface soil conditions. Owner is advised to obtain the services of a qualified geotechnical engineer.
- 2. No footings shall be placed in water or on frozen ground.
- 3. Foundations shall be carried down a minimum of 5 ft -0 in below grade, and shall be configured per Structural Drawings to resist uplifting during freeze thaw cycles.
- 4. Contractor will inspect and certify excavation bottoms, footing bearings, and structural backfill for conformance with Specifications and applicable codes.
- 5. Structural fill adjacent to foundations shall be a coarse granular material consisting of hard, durable, angular material conforming to the following gradation (MDOT 703.06 Type B Aggregate):

<u>Sieve Size</u>	% Passing by Weight
4 in.	100
½ in.	35-75
¼ in.	25-60
No. 40	0-25
No. 200	0-5

6. Structural backfill shall be placed to 95% of maximum density compaction as determined by ASTM D 1557. Lifts shall be no greater than 6 in. each and compacted with a vibratory compactor at each lift.

DIVISION 3 - CONCRETE

<u>Section 03300 - Cast-in-Place Concrete</u>

- 1. All concrete work shall conform to American Concrete Institute (ACI) "Specifications for Structural Concrete for Buildings" (ACI 301) and "Building Code Requirements for Reinforced Concrete" (ACI 318).
- 2. All concrete slabs shall have a minimum compressive strength of 4000 psi at 28 days, with water/cement ratio not exceeding 0.45. Concrete shall be made with 34" stone aggregate; shall have 5-7% air entrainment; shall be made with Type I

- or Type II cement; and shall have a 3"-4" slump. Concrete for slabs shall contain polypropylene fibers engineered for use in concrete complying with ASTM-C116, Type III, ½" to 1 ½" long, 1.5 pounds per cubic yd.
- 3. Other concrete shall be as specified above in Note 2, except that the minimum compressive strength may be reduced to 3000 psi at 28 days, and polypropylene fibers may be omitted.
- 4. Shop Drawings shall be prepared and submitted for all steel reinforcing within concrete.
- 5. No foundations shall be placed in water or on frozen ground.
- 6. All embedments in concrete, including anchor bolts, shall be firmly secured by tie wire to prevent movement during concrete placement.
- 7. All concrete materials, reinforcement and forms shall be free from frost or debris.
- 8. Concrete shall be maintained above 50 degrees F, and in moist condition for at least the first seven days after placement.
- 9. Consolidate all conrete with a vibrator or other means recommended by ACI 301.
- 10. All concrete reinforcing bars shall conform to ASTM A615, Grade 60.
- 11. Reinforcing bars may not be welded except where designated in writing by the Structural Engineer.
- 12. Vapor barrier below slabs on grade shall be "Vaporshield" by J-Pro.
- 13. Apply curing compound to slab surface in accordance with manufacturer's guidelines.

DIVISION 5 - METALS

Section 05120 - Structural Steel

- All structural steel work shall conform to the recommendations and requirements contained in the "Manual of Steel Construction, Allowable Stress Design," AISC Ninth Edition (including AISC Code of Standard Practice for Steel Buildings and Bridges), and "Structural Steel Welding Code – Steel," (AWS D1.1, latest edition).
- 2. No change in size or position of the structural elements shall be made without prior written approval of the Structural Engineer.

- 3. Temporary erection bracing shall be provided to hold structural steel securely in position. Remove temporary bracing and connections only after permanent members are in place and final connections are in place.
- 4. Shop connections unless otherwise noted, shall be made by welding. Connect structural steel components together using high strength bolts, %-inch diameter A325N "Tension-Control" type bolts (fully tensioned shear/bearing).
- 5. All shop and field welds shall be made by certified welders, and shall conform to the American Welding Society Code, AWS D1.1, latest edition, using E70-18 electrodes. Carefully control welding technique to avoid distortion, including clamping prior to welding. Minimum weld size shall be 3/16" fillet.
- 6. Shop drawings for steel shall be submitted for review and approval. Connections shown on these Drawings are generally schematic. They are intended to define the spatial relationship of the framed members and show a feasible method of making the connections. Any connection that is not shown or is not completely detailed on the Structural Drawings shall be designed by a licensed Professional Engineer in the state of Maine retained by the fabricator. Beam-to-beam and beam-to-post connections shall be designed for an end reaction equal to one-half the uniform load capacity of the beam as given in Part 2 ("Beams and Girders") of the AISC Manual, 9th Edition. Minimum shear capacity of 12 kips shall be provided. Completely detailed means the following information is shown on the Shop Detail Drawings for review by the engineer:
 - a) All plate dimensions and grades.
 - b) All weld sizes, pitches, and returns.
 - c) All hole sizes and spacings.
 - d) Number and type of bolts: Where bolts are shown but no number is given, the connection has not been completely detailed.
 - e) Where partial information is given, it shall be the minimum requirement for the connection.
 - f) Minimum plate thickness shall be 1/4".
- 7. Structural steel components shall be shop primed with fabricator's standard primer, except that structural steel exposed to weather shall be primed with Tnemec 90-97 primer (steel shall have SP-6 blast finish). Provide field touch-up as necessary.

8. Structural steel rolled shapes, plates, bars and tubes shall conform to the following:

ASTM A-572, Grade 50: All wide flange sections ("W" shapes), Fy = 50 ksi ASTM A-36: Other rolled shapes, plates and bars, Fy = 36 ksi

ASTM A-500, Grade B: Steel Tubes ("TS" shapes), Fy = 46 ksi

ASTM A-53, Grade B: Steel pipe, Fy = 35 ksi

ASTM A-36: Threaded rods

ASTM A-307: Anchor bolts in concrete (unless otherwise noted)

Note: Bolts and rods exposed to weather shall be galvanized.

9. Non-shrink grout shall be 5000 psi (minimum) compression strength.

10. Coordinate final painting of steel components with owner's requirements.

<u>DIVISION 6 – WOOD</u>

Section 06000 - Carpentry (General)

- Lumber shall bear the grade and trademark of the association under whose rules it is produced and a mark of mill identification. Lumber shall be sound, seasoned, kiln-dried to a moisture content not exceeding 19% and surfaced four sides.
 - a) Pressure Treated (PT) lumber shall be Southern Yellow Pine, Number 2 grade.
 - b) Except as noted above or designated otherwise, remaining lumber shall be No. 2 grade Spruce, Pine, Fir, planed four sides.
- 2. Lumber and wood in exterior applications, at sills, at porches and in contact with concrete and masonry shall be pressure treated using CCA preservative with a minimum net retention of 0.40 pcf.
 - a) All fasteners (including nails, lag screws, and bolts) for pressure treated lumber shall be hot-dip galvanized.
 - b) Cut ends of pressure treated (PT) lumber and timber posts and sills shall be dipped in a preservative treatment for a minimum of fifteen minutes.
- 3. Fabricate horizontal and inclined members, units of less than 1:1 slope, with natural convex bow (crown) up to provide camber.
- 4. Carpentry work shall comply with AFPA's "National Design Specification for Wood Construction," 1991 Edition. Wood components shall be securely attached with sound connections and without splitting. As a minimum, wood fasteners

- shall conform to BOCA 1996, Table 2305.2, "Fastening Schedule" unless otherwise noted.
- 5. Reference to "Simpson" on Drawings indicates metal connectors manufactured by Simpson Strong-Tie.
- At locations where portions of wood floor or roof deck are added or replaced, the finish floor elevation of the new wood deck shall match the adjacent existing wood floor elevation.
- 7. Plywood for floors and roof shall be installed with both suitable adhesive and 10d nails at 6" o.c. at supported edges and 12" o.c. elsewhere.
- 8. Floor framing around chase openings for mechanical ducts and stairs shall consist of the following (unless otherwise noted):
 - a) Double floor length members and joists each side of opening with member depth same as adjacent floor framing.
 - b) Members connected with Simpson double joist hangers.
- 9. Plywood for floors shall be ¾" thick, APA rated sheathing with 48/24 span rating, tongue and groove. Use full size sheets as much as possible.
- 10. Plywood for roofs shall be "4" thick, APA rated sheathing with 48/24 span rating. Install "H" clips where recommended by APA. Use full size sheets as much as possible.
- 11. Plywood end joints for floors and roof shall be staggered. Plywood surface grain shall be transverse to joist span.
- 12. Exterior wall sheathing shall be 5/8" thick, APA rated sheathing suitable for exterior use. Use full size sheets as much as possible. Wall sheathing shall be fastened with galvanized 10d common nails having 6" spacing at supported edges and 12" spacing elsewhere.
- 13. Cornerboards, fascias, soffits, and other exterior finish trim shall be 5/4" Western Red Cedar, No. 1 Grade (unless authorized otherwise by the owner).
- 14. Air infiltration barrier between exterior plywood sheathing and wood siding shall be Tyvek Housewrap, manufactured by DuPont Company or approved equal.
- 15. All sheathing shall conform to APA "Plywood Specification Grade Guide" and Product Standard PS-1. Plywood construction shall conform to APA Design/Construction Guide Residential and Commercial, Form E30B for required applications.

- 16. Provide additional temporary bracing (not shown on drawings) for trusses as recommended by Truss Plate Institute (TPI).
- 17. Trusses shall be anchored to top of walls with Simpson H2.5 Hurricane Ties at each end.
- 18. Stair width shall not exceed 4'-0" wide and shall be supported by (3) 2x12 stringers, equally spaced. Center stringer shall be reinforced with a continuous 2x4 nailed to both sides, and outside stringers shall have continuous 2x4 nailed to one side. Spacing between supports for stringers shall not exceed 6'-4" on center. Upper ends of stringers shall be supported by either suitable steel joist hangers or continuous wood ledger.

Section 06192 - Metal-Plate Connected Wood Trusses

PART 1 - GENERAL

1.1 RELATED DOCUMENTS

A. Drawings and general provisions of the Contract, including General and Supplementary Conditions and Division 1 Specification Sections, apply to this Section.

1.2 SUMMARY

- A. This Section includes the following:
 - 1. Triangular-pitched roof trusses.
 - Scissor roof trusses.
 - Truss accessories.

1.3 **DEFINITIONS**

A. Metal-plate-connected wood trusses include planar structural units consisting of metal-plateconnected members fabricated from dimension lumber and cut and assembled before delivery to Project site.

1.4 PERFORMANCE REQUIREMENTS

- A. Structural Performance: Engineer, fabricate, and erect metal-plate-connected wood trusses to withstand design loads within limits and under conditions required.
 - Design Loads: As indicated.
 - Design trusses to withstand design loads without deflections greater than the following:
 - a. Roof Trusses: Vertical deflection of 1/240 of span due to total load.
- B. Engineering Responsibility: Engage a fabricator who uses a qualified professional engineer to prepare calculations, Shop Drawings, and other structural data for metal-plate-connected wood trusses.

1.5 SUBMITTALS

- A. General: Submit each item in this Article according to the Conditions of the Contract and Division 1 Specification Sections.
- Product Data for lumber, metal-plate connectors, metal framing connectors, bolts, and fasteners.

- C. Shop Drawings detailing location, pitch, span, camber, configuration, and spacing for each type of truss required; species, sizes, and stress grades of lumber to be used; splice details; type, size, material, finish, design values, and orientation and location of metal connector plates; and bearing details.
 - To the extent truss design considerations are indicated as fabricator's responsibility, include structural analysis data signed and sealed by the qualified professional engineer responsible for their preparation.
 - 2. Include truss Shop Drawings signed and sealed by the qualified professional engineer responsible for their preparation.
- D. Product certificates signed by officer of truss fabricating firm certifying that metal-plateconnected wood trusses supplied for Project comply with specified requirements and Shop Drawings.
- E. Qualification data for firms and persons specified in "Quality Assurance" Article to demonstrate their capabilities and experience. Include lists of completed projects with project names and addresses, names and addresses of architects and owners, and other information specified.
- F. Material test reports from a qualified independent testing agency indicating and interpreting test results relative to compliance of fire-retardant-treated wood products with requirements indicated.
- G. Warranty of chemical treatment manufacturer for each type of treatment.
- H. Material certificates for dimension lumber specified to comply with minimum allowable unit stresses. Indicate species and grade selected for each use and design values approved by the American Lumber Standards Committee (ALSC) Board of Review.
- I. For fire-retardant-treated wood products, include certification by treating plant that treated materials comply with specified standard and other requirements as well as data relative to bending strength, stiffness, and fastener-holding capacities of treated materials.
- J. Research or evaluation reports of the model code organization acceptable to authorities having jurisdiction that evidence the following products' compliance with building code in effect for Project.
 - 1. Fire-retardant-treated wood.
 - Metal-plate connectors.
 - 3. Metal framing connectors.

1.6 QUALITY ASSURANCE

- A. Installer Qualifications: Engage an experienced Installer who has completed wood truss installation similar in material, design, and extent to that indicated for this Project and with a record of successful in-service performance.
- B. Fabricator's Qualifications: Engage a firm that complies with the following requirements for quality control and is experienced in fabricating metal-plate-connected wood trusses similar to those indicated for this Project and with a record of successful in-service performance:

- Fabricator participates in a recognized quality-assurance program that involves inspection by SPIB; Timber Products Inspection, Inc.; Truss Plate Institute (TPI); or other independent inspecting and testing agency acceptable to Architect and authorities having jurisdiction.
- C. Comply with applicable requirements and recommendations of the following publications:
 - 1. ANSI/TP1 1, "National Design Standard for Metal-Plate-Connected Wood Truss Construction."
 - 2. TPI HIB "Commentary and Recommendations for Handling Installing & Bracing Metal Plate Connected Wood Trusses."
 - TPI DSB "Recommended Design Specification for Temporary Bracing of Metal Plate Connected Wood Trusses."
- D. Metal-Plate Connector Manufacturer's Qualifications: A manufacturer that is a member of TPI and that complies with TPI quality-control procedures for manufacture of connector plates published in ANSI/TPI 1.
- E. Single-Source Responsibility for Connector Plates: Provide metal connector plates from one source and by a single manufacturer.
- F. Wood Structural Design Standard: Comply with applicable requirements of AFPA's "National Design Specification for Wood Construction" and its "Supplement."
- G. Single-Source Engineering Responsibility: Provide trusses engineered by metal-plate connector manufacturer to support superimposed dead and live loads indicated, with design approved and certified by a qualified professional engineer.
- H. Professional Engineer Qualifications: A professional engineer who is legally authorized to practice in the jurisdiction where Project is located and who is experienced in providing engineering services of the kind indicated that have resulted in installing metal-plate-connected wood trusses similar to those indicated for this Project and with a record of successful in-service performance.

1.7 DELIVERY, STORAGE, AND HANDLING

- A. Handle and store trusses with care and comply with manufacturer's written instructions and TPI recommendations to avoid damage and lateral bending.
- B. Inspect trusses showing discoloration, corrosion, or other evidence of deterioration. Discard and replace trusses that are damaged or defective.

1.8 SEQUENCING AND SCHEDULING

A. Time delivery and erection of trusses to avoid extended on-site storage and to avoid delaying progress of other trades whose work must follow erection of trusses.

PART 2 - PRODUCTS

2.1 MANUFACTURERS

- A. Available Manufacturers: Subject to compliance with requirements, manufacturers offering products that may be incorporated into the Work include, but are not limited to, the following:
- B. Manufacturers: Subject to compliance with requirements, provide products by one of the following:
 - 1. Fire-Retardant-Treated Materials, Interior Type A:
 - a. Baxter: J. H. Baxter Co.
 - b. Chemical Specialties, Inc.
 - c. Continental Wood Preservers, Inc.
 - d. Hickson Corporation.
 - e. Hoover Treated Wood Products, Inc.
 - 2. Fire-Retardant-Treated Materials, Exterior Type:
 - a. American Wood Treaters, Inc.
 - b. Hoover Treated Wood Products, Inc.
 - Mctal Connector Plates:
 - a. Alpine Engineered Products, Inc.
 - b. Computrus, Inc.
 - c. Mitek Industries, Inc.
 - d. Robbins Manufacturing Company.
 - c. Tee-Lok Corporation.
 - f. Truswal Systems Corporation.
 - 4. Metal Framing Anchors:
 - a. Cleveland Steel Specialty Co.
 - b. Harlen Metal Products, Inc.
 - c. Silver Metal Products, Inc.
 - d. Simpson Strong-Tie Company, Inc.
 - e. Southeastern Metals Manufacturing Co., Inc.
 - f. United Steel Products Co.

2.2 DIMENSION LUMBER

- A. Lumber Standards: Comply with DOC PS 20, "American Softwood Lumber Standard," and with applicable grading rules of inspection agencies certified by ALSC's Board of Review.
- B. Inspection Agencies: Inspection agencies, and the abbreviations used to reference them, include the following:
 - 1. NELMA Northeastern Lumber Manufacturers Association.
 - 2. NLGA National Lumber Grades Authority (Canadian).
 - 3. SPIB Southern Pine Inspection Bureau.

- WCLIB West Coast Lumber Inspection Bureau.
- WWPA Western Wood Products Association.
- C. Grade Stamps: Provide lumber with each piece factory marked with grade stamp of inspection agency evidencing compliance with grading rule requirements and identifying grading agency, grade, species, moisture content at time of surfacing, and mill.
- D. Provide dressed lumber, S4S, manufactured to actual sizes required by DOC PS 20 for moisture content specified, to comply with requirements indicated below:
 - Provide dry lumber with 19 percent maximum moisture content at time of dressing.
- E. Grade and Species: Provide dimension lumber of any species for truss chord and web members, graded visually or mechanically, and capable of supporting required loads without exceeding allowable design values according to AFPA's "National Design Specification for Wood Construction" and its "Supplement."

2.3 FIRE-RETARDANT-TREATED MATERIALS (where required by code)

- A. General: Where fire-retardant-treated wood is indicated, comply with applicable requirements of AWPA C20 (lumber). Identify fire-retardant-treated wood with appropriate classification marking of UL, U.S. Testing, Timber Products Inspection, or another testing and inspecting agency acceptable to authorities having jurisdiction.
 - Research or Evaluation Reports: Provide fire-retardant-treated wood acceptable to authorities having jurisdiction and for which a current model code research or evaluation report exists that evidences compliance of fire-retardant-treated wood for application indicated.
- B. Interior Type A: For interior locations, use chemical formulation that produces treated lumber with the following properties under conditions present after installation:
 - 1. Bending strength, stiffness, and fastener-holding capacities are not reduced below values published by manufacturer of chemical formulation under elevated temperature and humidity conditions simulating installed conditions when tested by a qualified independent testing agency.
 - 2. No form of degradation occurs due to acid hydrolysis or other causes related to treatment.
 - Contact with treated wood does not promote corrosion of metal fasteners.
- C. Exterior Type: Use for exterior locations and where indicated.
- D. Inspect each piece of treated lumber after drying and discard damaged or defective pieces.

2.4 METAL CONNECTOR PLATES

- A. General: Fabricate connector plates from metal complying with requirements indicated below.
- B. Hot-Dip Galvanized Steel Sheet: Structural-quality steel sheet, zinc coated by hot-dip process complying with ASTM A 653, G60 (ASTM A 653M, Z180) coating designation; Grade 33 and not less than 0.0359 inch (0.91 mm) thick.

- C. Electrolytic Zinc-Coated Steel Sheet: ASTM A 591 (ASTM A 591M), structural-(physical) quality steel sheet, zinc coated by electrodeposition; 33,000-psi (230-MPa) minimum yield strength, coating class C, and not less than 0.0474 inch (1.20 mm) thick.
- D. Aluminum-Zinc Alloy-Coated Steel Sheet: Structural-(physical) quality steel sheet, aluminum-zinc alloy-coated by hot-dip process complying with ASTM A 792, AZ50 (ASTM A 792M, AZ150) coating designation; Grade 33 and not less than 0.0359 inch (0.91 mm) thick.
- E. Stainless-Steel Sheet: ASTM A 666, Type 304 or 316, chromium nickel steel sheet; 33,000-psi (230-MPa) minimum yield strength and not less than 0.035 inch (0.89 mm) thick.

2.5 FASTENERS

- A. General: Provide fasteners of size and type indicated that comply with requirements specified below for material and manufacture.
 - Where truss members are exposed to weather or to high relative humidities, provide fasteners with a hot-dip zinc coating per ASTM A 153 or of stainless steel, Type 304 or 316.
- B. Nails, Wire, Brads, and Staples: FS FF-N-105.
- C. Power-Driven Fasteners: CABO NER-272.
- D. Wood Screws: ASME B18.6.1.
- E. Lag Bolts and Screws: ASME B18,2,1 (ASME B18,2,3,8M).
- F. Bolts: Steel bolts complying with ASTM A 307, Grade A (ASTM F 568, Property Class 4.6); with ASTM A 563 (ASTM A 563M) hex nuts and, where indicated, flat washers.

2.6 METAL FRAMING ANCHORS

- A. General: Provide metal framing anchors of structural capacity, type, size, metal, and finish indicated that comply with requirements specified, including the following:
 - Research or Evaluation Reports: Provide products for which model code research or
 evaluation reports exist that are acceptable to authorities having jurisdiction and that
 evidence compliance of metal framing anchors for application indicated with building
 code in effect for this Project.
 - 2. Allowable Design Loads: Provide products with allowable design loads, as published by manufacturer, that meet or exceed those indicated. Manufacturer's published values shall be determined from empirical data or by rational engineering analysis, and demonstrated by comprehensive testing performed by a qualified independent testing agency.
- B. Galvanized Steel Sheet: Hot-dip, zinc-coated steel sheet complying with ASTM A 653, G60 (ASTM A 653M, Z180) coating designation; structural, commercial, or lock-forming quality, as standard with manufacturer for type of anchor indicated.

C. Stainless-Steel Sheet: ASTM A 666, Type 304 or 316, chromium nickel steel sheet; 33,000-psi (230-MPa) minimum yield strength.

2.7 MISCELLANEOUS MATERIALS

- A. Galvanizing Repair Paint: SSPC-Paint 20 or DOD-P-21035, with dry film containing a minimum of 94 percent zinc dust by weight.
- B. Protective Coatings: Provide one of the following coating systems:
 - 1. SSPC-Paint 22, epoxy-polyamide primer.
 - SSPC-Paint 16, coal-tar epoxy-polyamide black or dark red paint.
 - SSPC-Paint 27 and SSPC-Paint 12, basic zinc chromate-vinyl butyral wash primer and cold-applied asphalt mastic.

2.8 FABRICATION

- A. Cut truss members to accurate lengths, angles, and sizes to produce close-fitting joints.
- B. Fabricate metal connector plates to size, configuration, thickness, and anchorage details required to withstand design loadings for types of joint designs indicated.
- C. Assemble truss members in design configuration indicated using jigs or other means to ensure uniformity and accuracy of assembly with joints closely fitted to comply with tolerances of ANSI/TPI 1. Position members to produce design camber indicated.
 - 1. Fabricate wood trusses within manufacturing tolerances of ANSI/TPI 1.
- D. Connect truss members by metal connector plates located and securely embedded simultaneously into both sides of wood members by air or hydraulic press.

PART 3 - EXECUTION

3.1 INSTALLATION

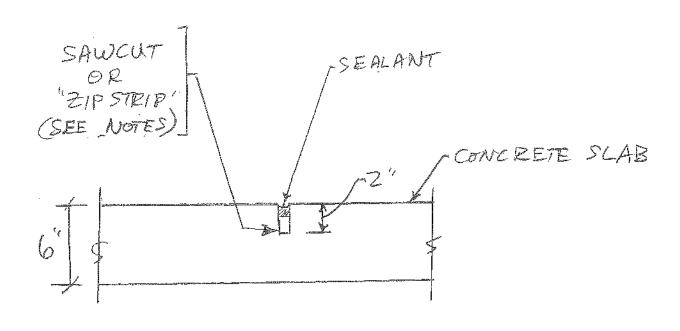
- A. Do not install wood trusses until supporting construction is in place and is braced and secured.
- B. Before installing, splice trusses delivered to Project site in more than one piece.
- C. Hoist trusses in place by lifting equipment suited to sizes and types of trusses required, exercising care not to damage truss members or joints by out-of-plane bending or other causes.
- D. Install and brace trusses according to recommendations of TPI and as indicated.
- E. Install trusses plumb, square, and true to line and securely fasten to supporting construction.
- F. Space, adjust, and align trusses in location before permanently fastening and as follows:

- 1. Truss Spacing: As indicated.
- G. Anchor trusses securely at all bearing points using metal framing anchors. Install fasteners through each fastener hole in metal framing anchor according to manufacturer's fastening schedules and written instructions.
- H. Securely connect each truss ply required for forming built-up girder trusses.
 - I. Anchor trusses to girder trusses as indicated.
- I. Install and fasten permanent bracing during truss erection and before construction loads are applied. Anchor ends of permanent bracing where terminating at walls or beams.
 - 1. Install and fasten strongback bracing vertically against vertical web of parallel-chord floor trusses at centers indicated.
- J. Install wood trusses within installation tolerances of ANSI/TPI 1.
- K. Do not cut or remove truss members.
- L. Return wood trusses that are damaged or do not meet requirements to fabricator and replace with trusses that do meet requirements.
 - 1. Do not alter trusses in the field.

3.2 REPAIRS AND PROTECTION

- A. Repair damaged galvanized coatings on exposed surfaces with galvanized repair paint according to ASTM A 780 and manufacturer's written instructions.
- B. Protective Coating: Clean and prepare exposed surfaces of embedded-metal connector plates. Brush apply primer, when part of coating system, and one coat of protective coating.
 - 1. Apply materials to provide minimum dry film thickness recommended by manufacturer of coating system.

END OF SECTION 06192

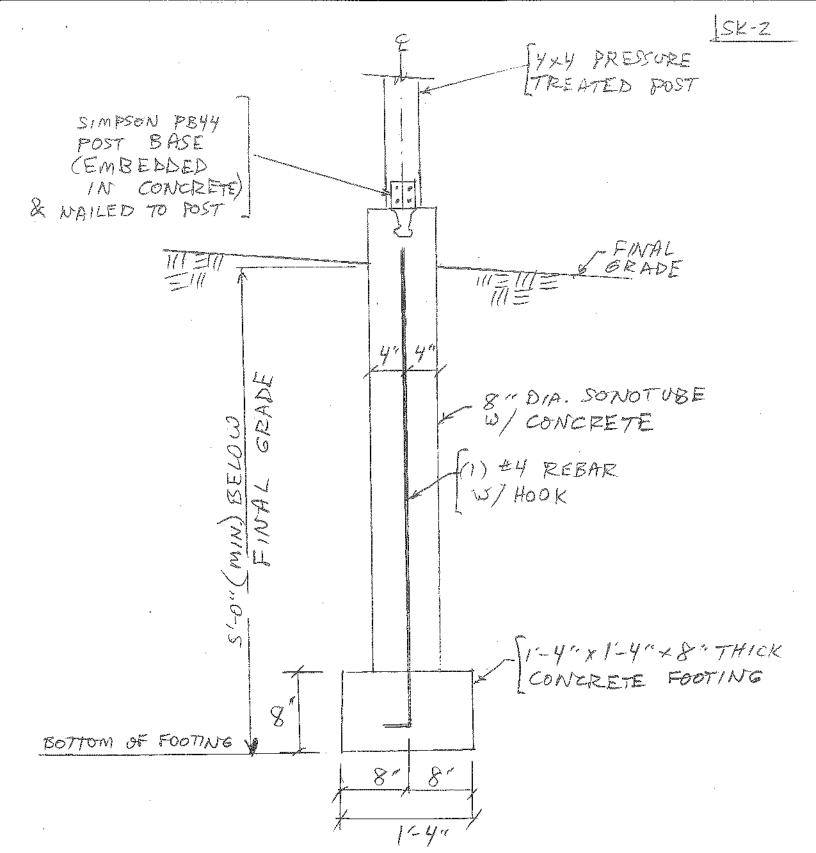


CONTROL JOINT DETAIL BY Not to Scale

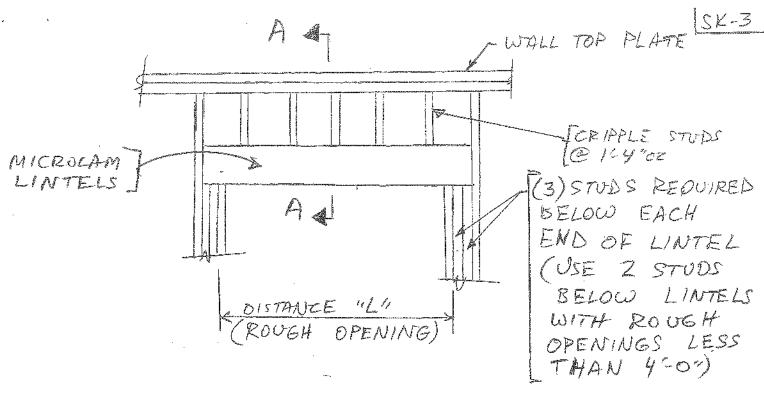
Notes:

1. Slab shall be sawed as soon as possible without dislodging aggregate of fresh concrete, but under no circumstances longer than 6 hours efter concrete slab has been placed.

2. Control joints shall be straight and shall be spaced not note than 10'-0" on center



PIER DETAIL (3)



WALL ELEVATION @ ROUGH OPENING Not to Scale

LINTEL SCHEDULE	
Distance "L" Rough Opening	Microlam Lintel (See Notes)
10-0	(3) 13/4×117/8
6'-6"	(3) 13/4× 9/4
3'-0"	$(2) 1^{3/4} \times 5^{1/2}$

MICROLAMS (See Notes) SEE LINTEL SCHEDULE 刚刚

LINTEL SCHEDULE

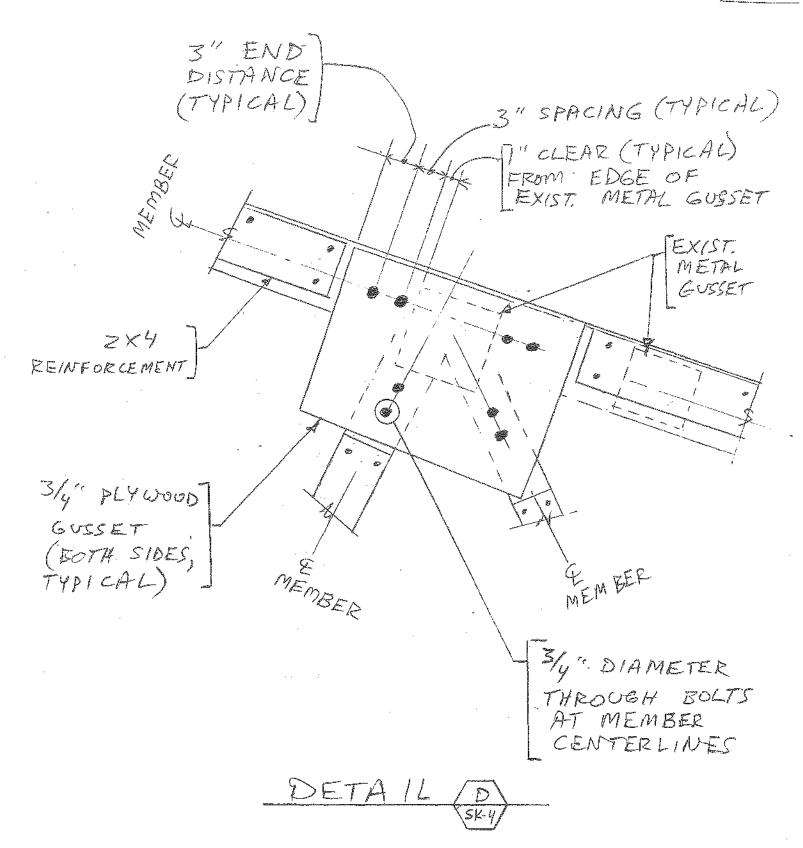
Not to Scale

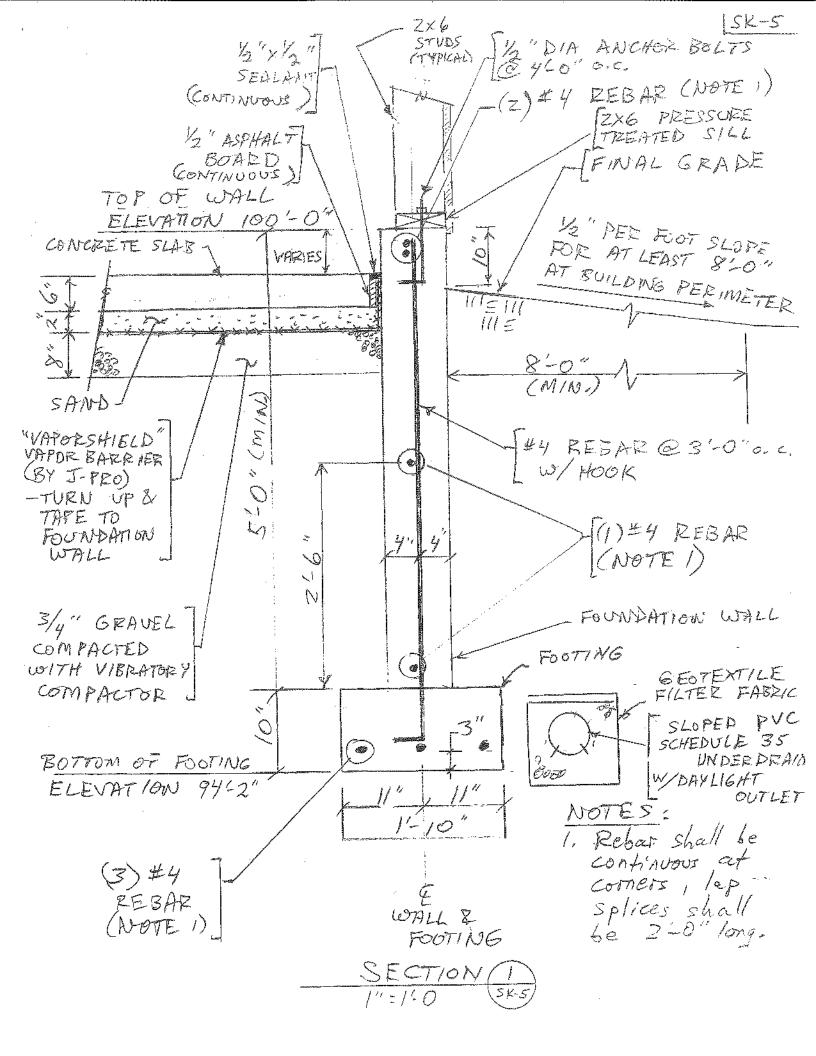
SECTION A-A

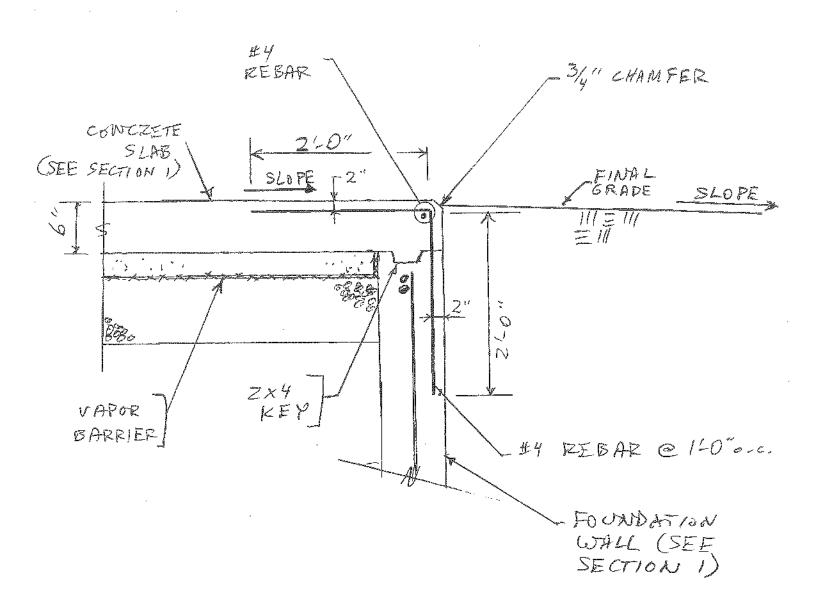
Notes:

1. Microlans as fabricated by Trus Joist Mannillan or Z. At 3-0" Lintel, center mande

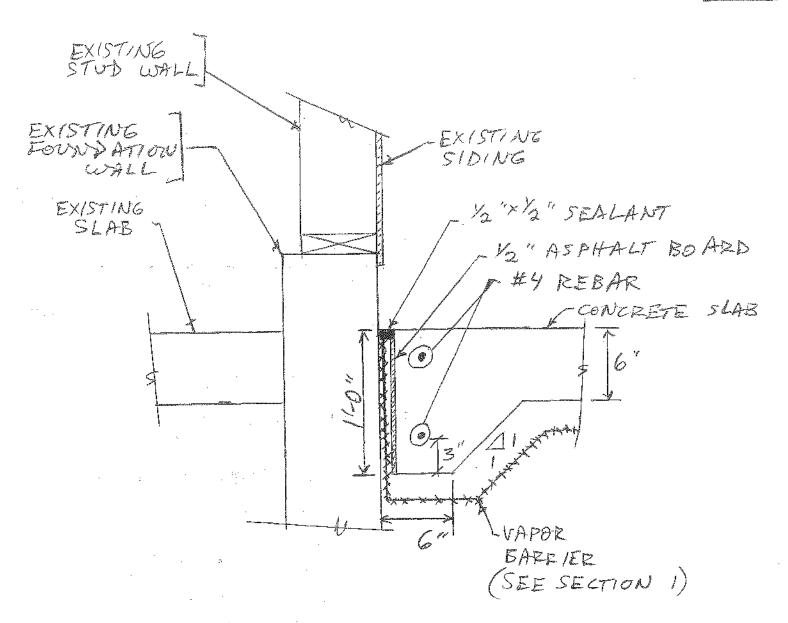
shall be Zx6, #2SPF

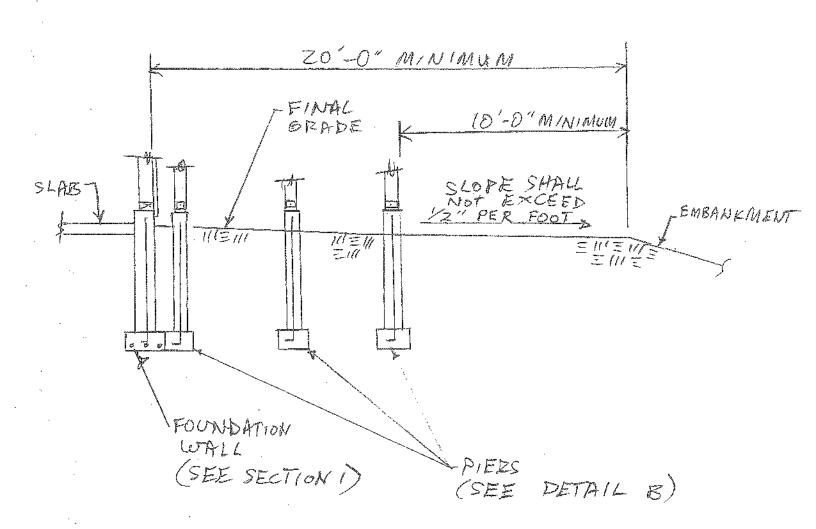




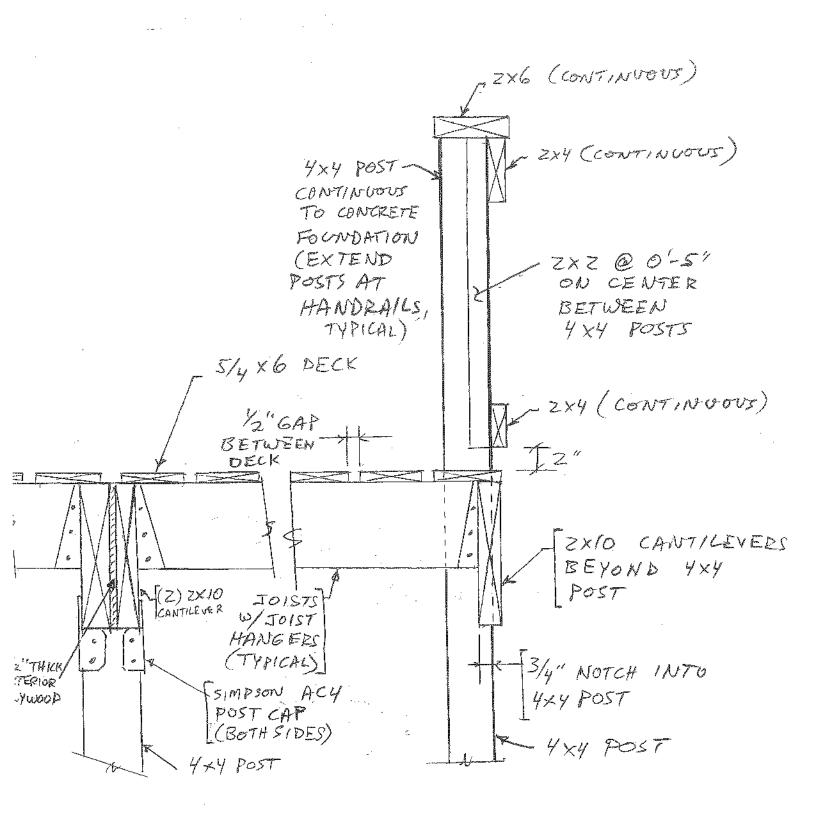


SECTION 2 Not to Scale (sk.6)

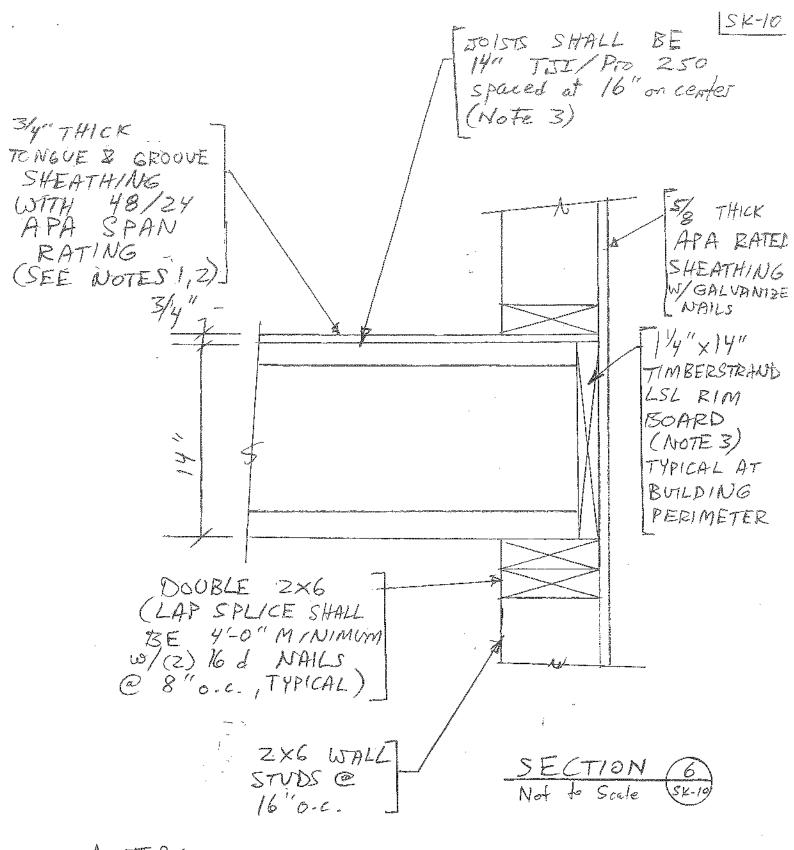




SECTION 4 Not to Scale SK-8



SECTION 5 Not to Scale (K.9)

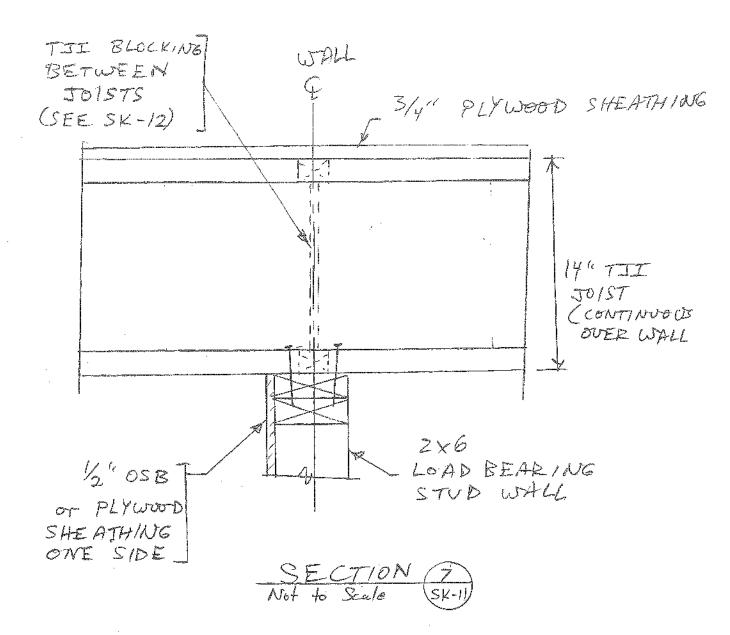


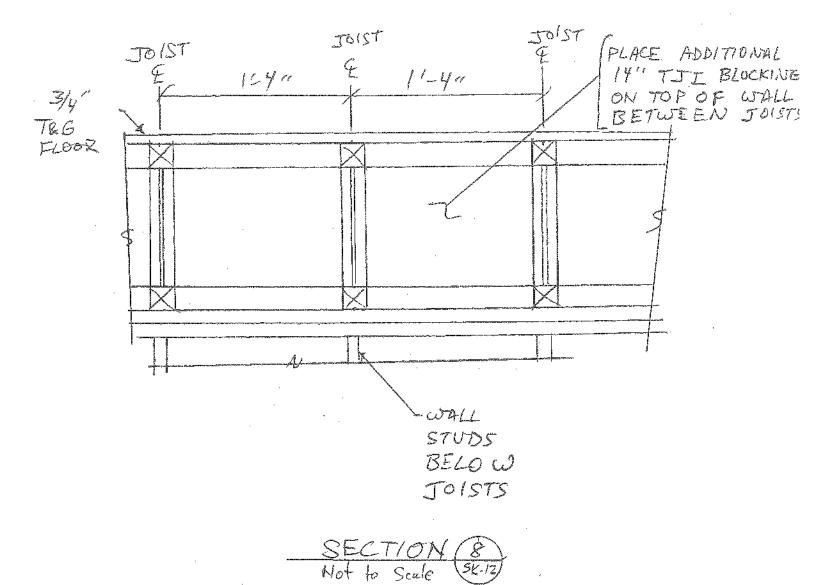
NOTES:

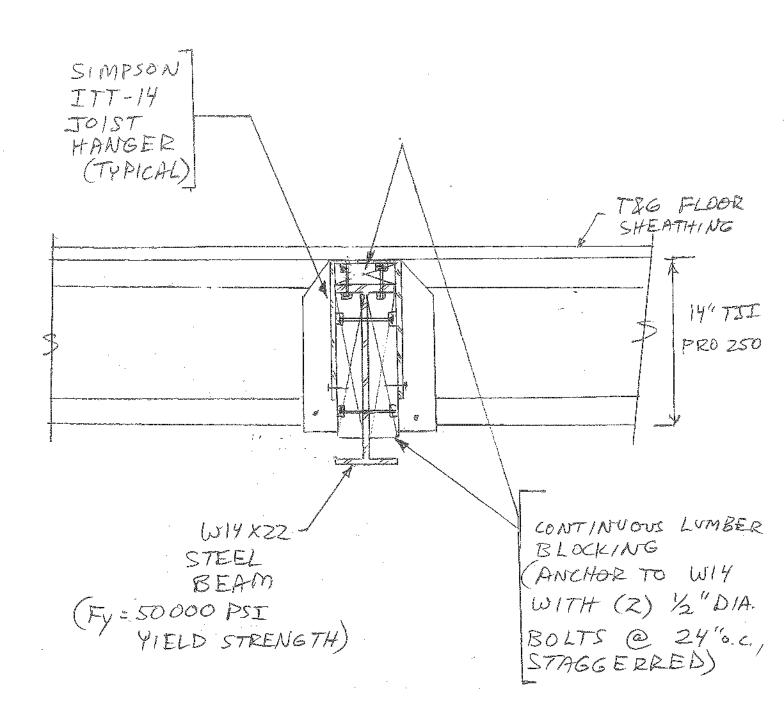
1. Floor sheathing shell be placed with long dimension perpendicular to supports.

2. Glue sheathing to joints and pail with 10 d nails. Nail spacing shall be 6" on center at supported edges and 150 on couter elsewhere.

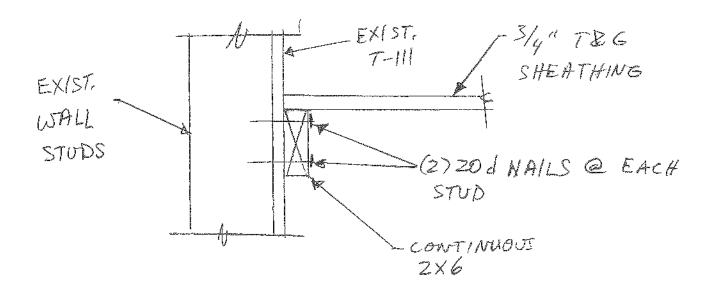
3. TII & LSL manufactured by Trus Joint Macmillan or said.





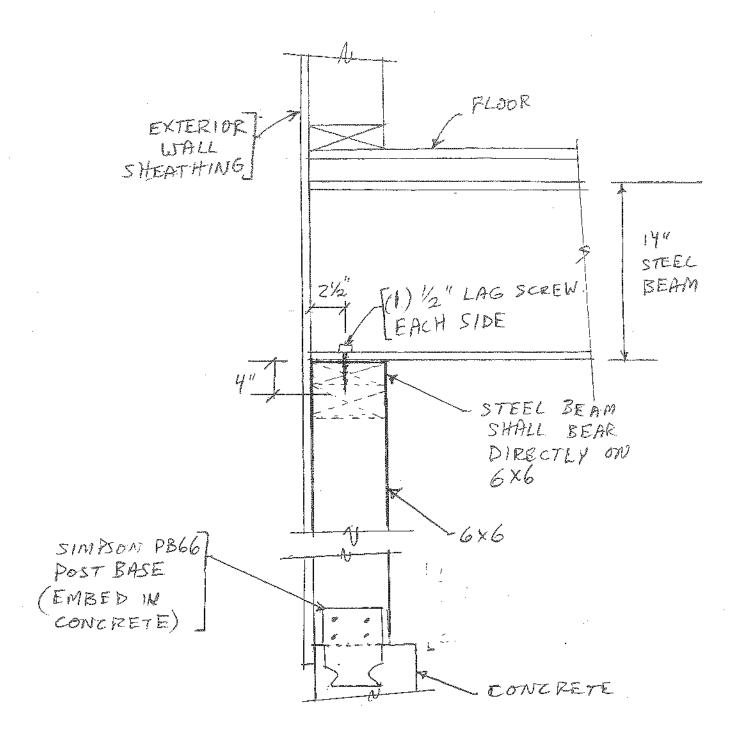


SECTION 9 Not to Scale (SK-13)

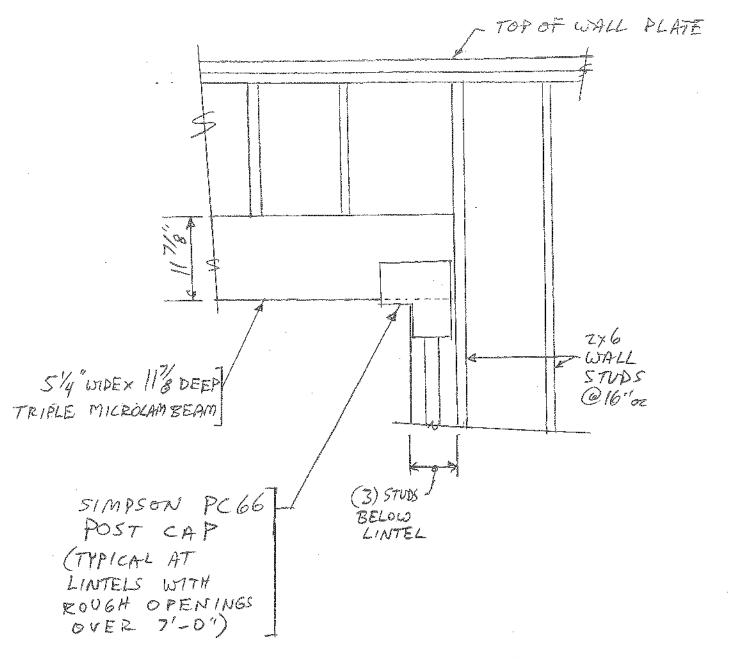


SECTION 10 Not to Scale SK-14

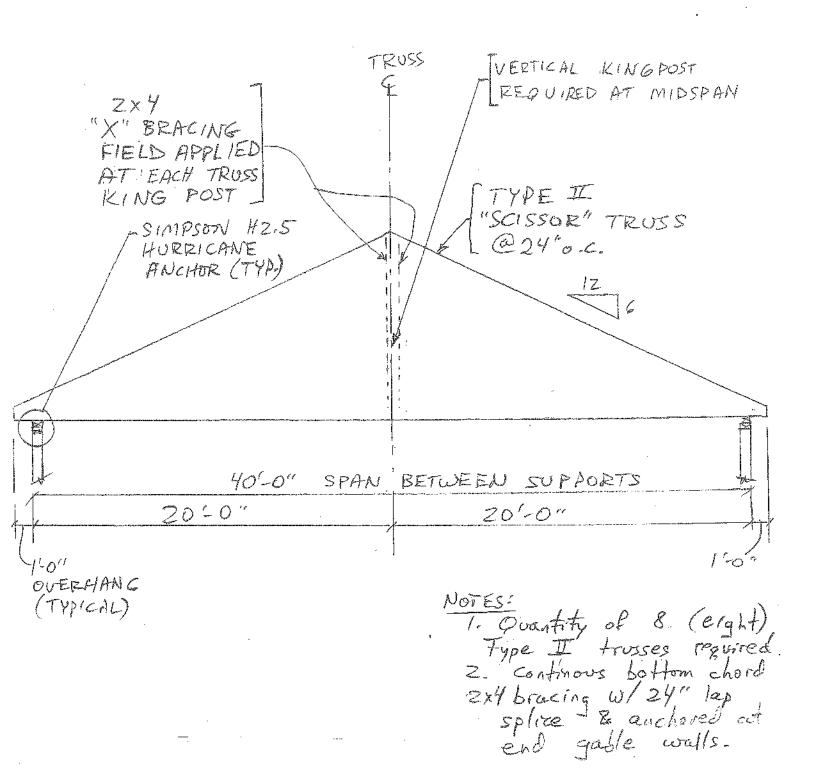
NOTE: other details not shown for clarity WOOD NAILER 14" STEEL BEAM SOTE SIDES - PE 5/8×6"×0'-6" 3/16 STEEL PIPE COLUMN (4) 3/" DIA EXPANSION BOLTS BASE A 8/x0'8" CONVERETE SLAB 3/16 3/4" NONSHRINK GROVT REBAR CONTERSTE FOOTING 3"CLEAR 14'x4' FOOTING (4) #4.



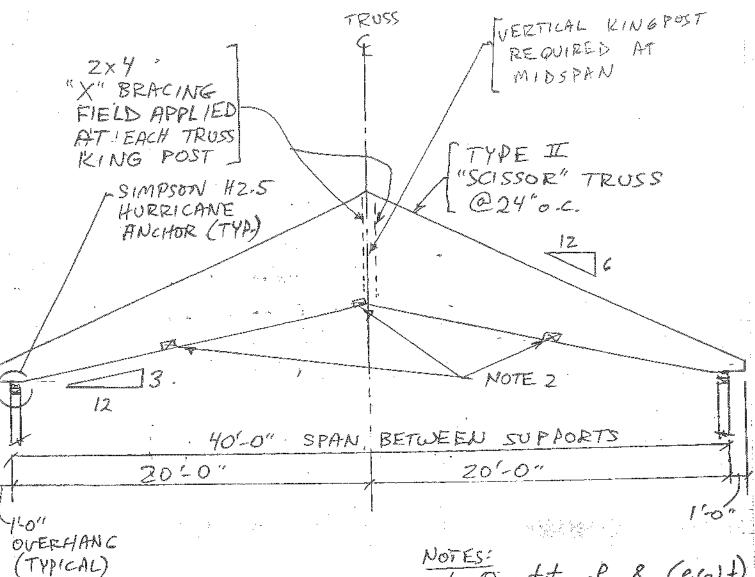
SECTION (12) Not to Scale (5K-16)



SECTION (13) Not to Scale (SK-17)



TYPE II TRUSS PROFILE 14



Noves:

1. Quantity of 8. (eight)

Type II trusses required

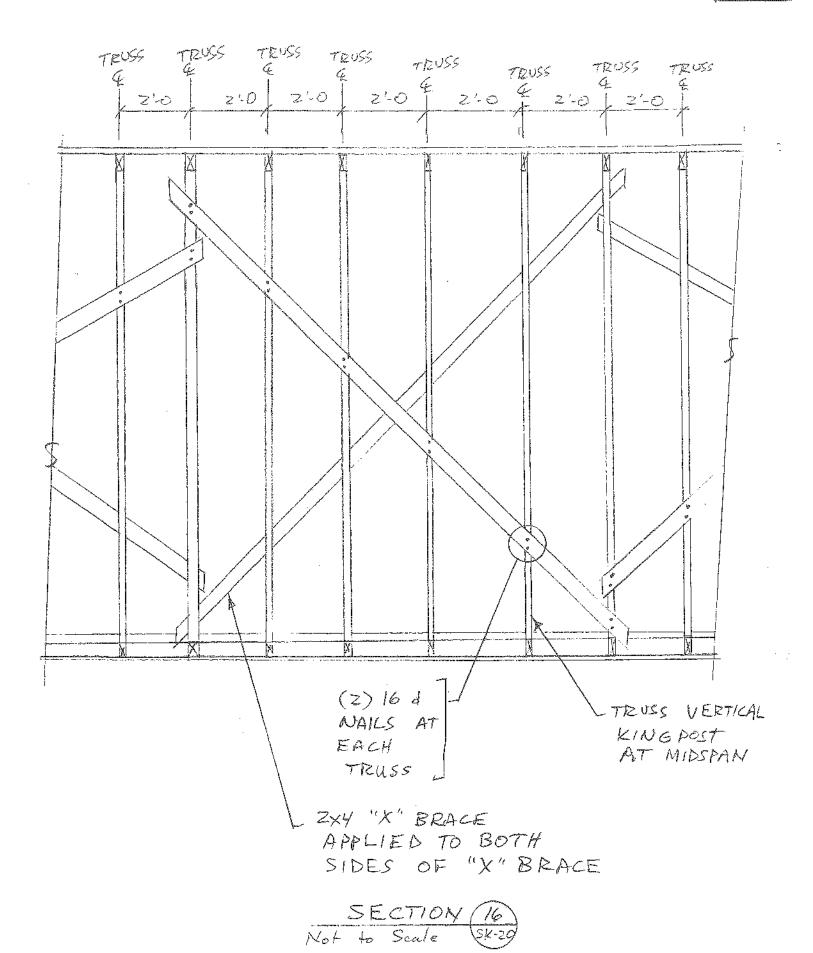
2. Continous bottom choird

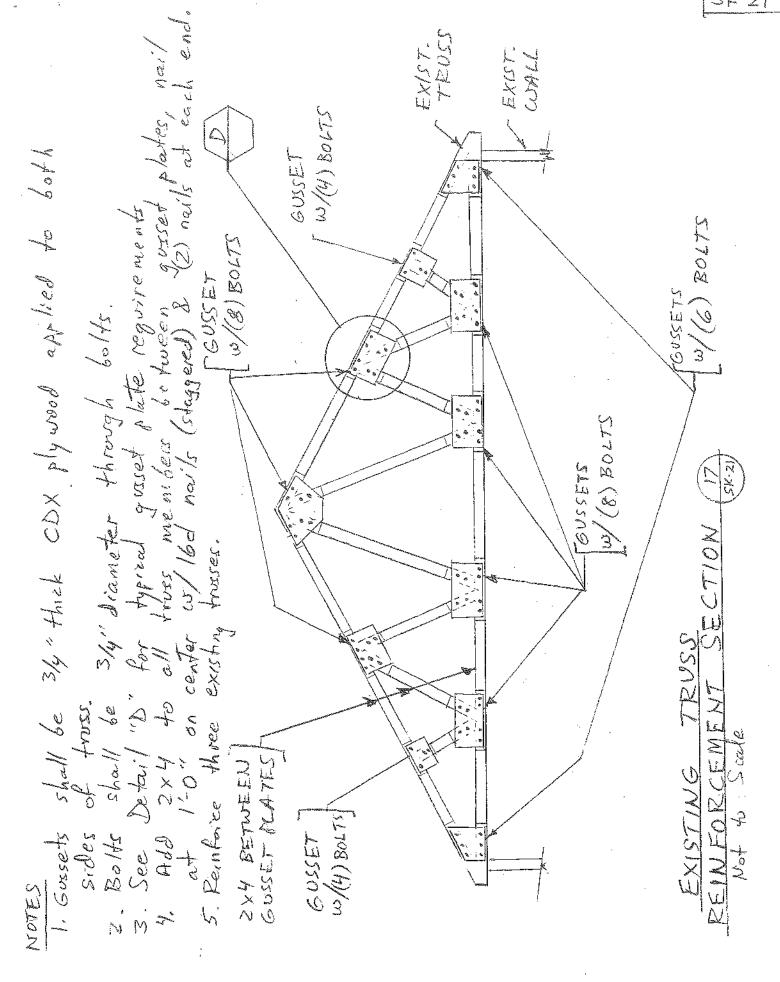
2x4 bracing w/24" lap

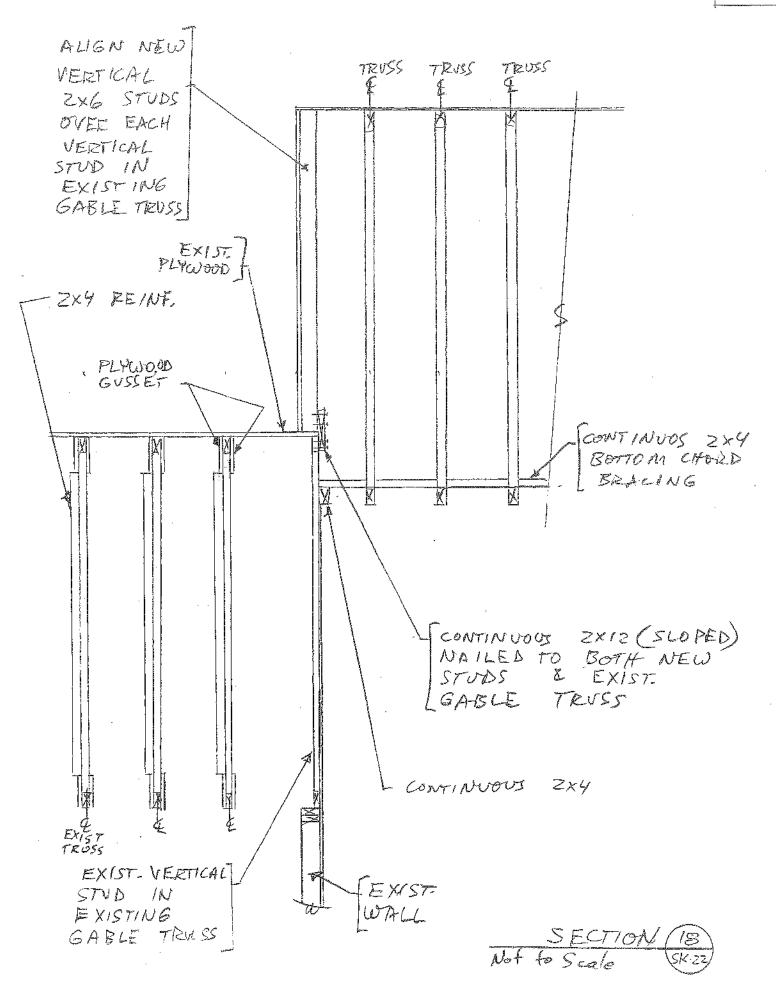
splice & auchored at

end gable walls.

TYPE IF TRUSS PROFILE (15)









DETECA-PORTMAN ASSOCIATES UNC. CONSTRUCTING ENGINEERS

778 MAIN STREET SUITE 8 SOUTH PORTLAND, MAINE 04:06 TEL. 207 775 1121 FAN 207 879 0896 # ROADWAY DIGICIN

🗱 ENVIRONMENTAL ENGINEERING 👚

灣 FRABBIC STUDIES AND MANAGEMENT

M MODIFICATIONS

M AIRPORT ENGINEERING

■ SITE PLANNING

👼 CONSTRUCTION ADMINISTRATION

MEMORANDUM

TO:

Kandi Talbot, Planner

FROM:

Steve Bushey, Acting Development Review Coordinator

RE:

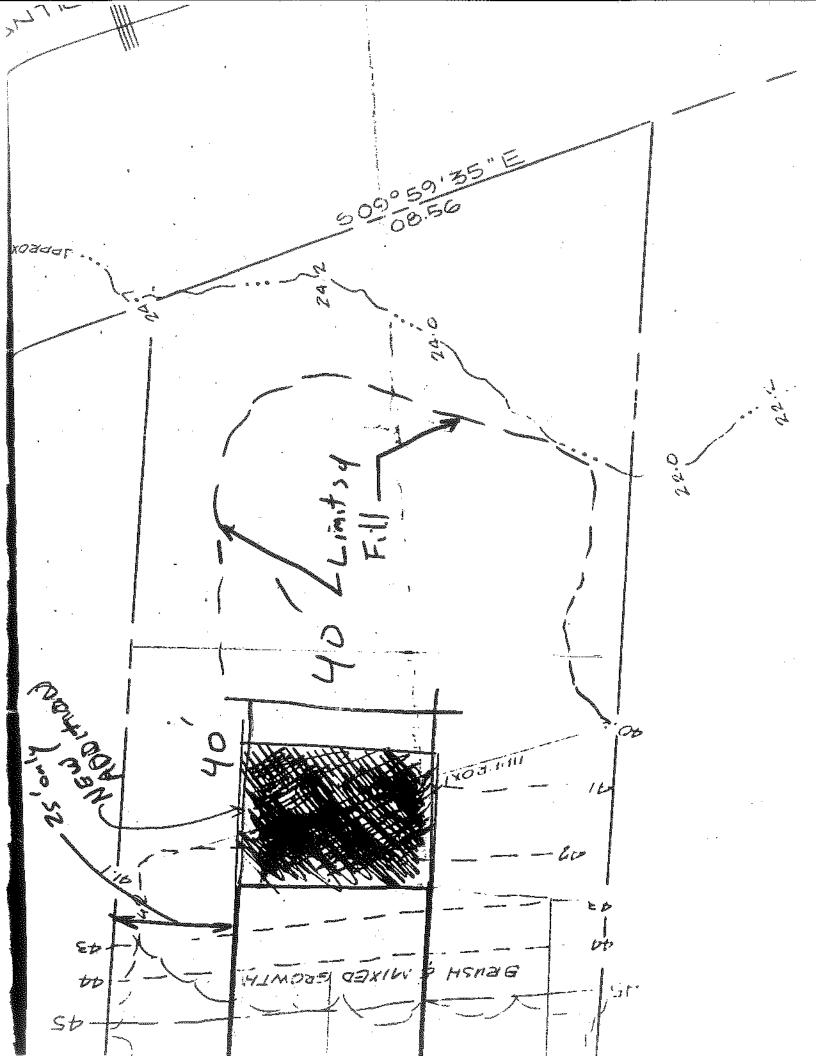
William Gribizis - 1021 Ocean Avenue

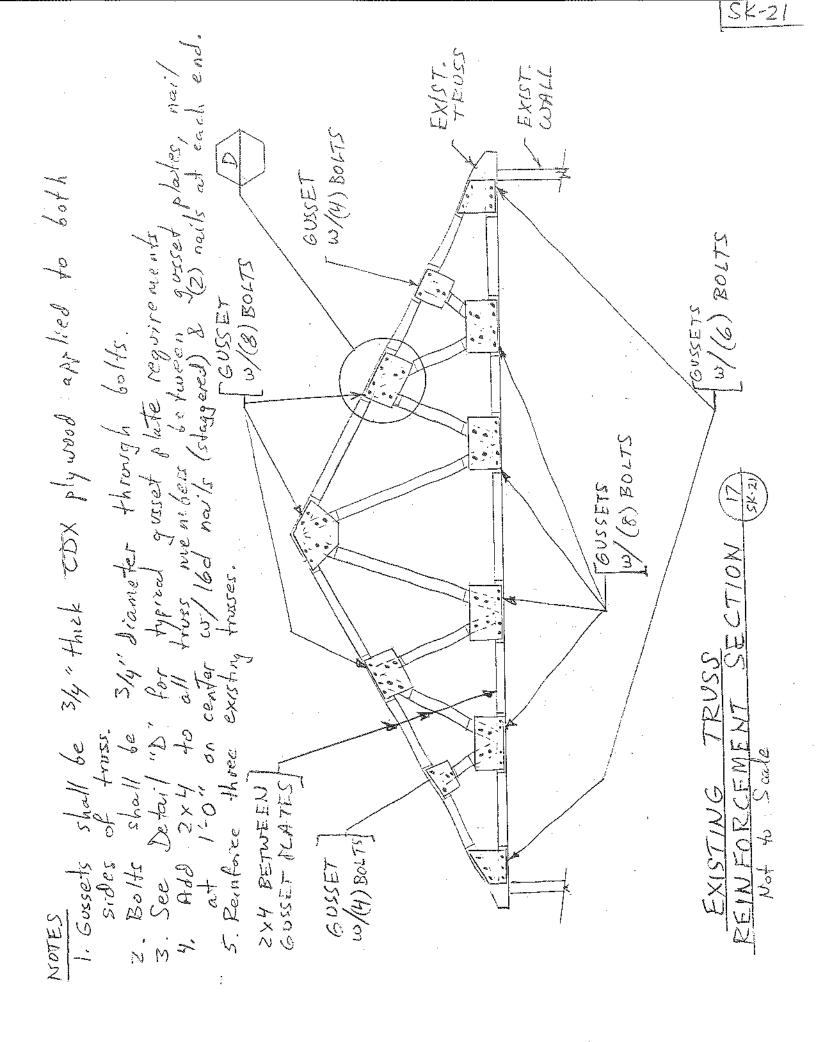
DATE:

November 9, 1999

I have reviewed the application materials dated 10/22/99 and plans dated 9/2/99 and provide the following comments:

- 1. The applicant should review permitting issues related to the MeDEP Natural Resources Protection Act (NRPA) for work within 100 feet of the stream. Evidence of a NRPA permit approval or sign-off by the MeDEP should be provided. I have observed that the site has already been extensively filled from the end of the existing building out to the stream. I met the owner onsite today and he felt that MeDEP had previously signed off on the fill, although he was unaware of the actual MeDEP standards prohibiting fill placement within 25 feet of a stream. As it exists, fill has been placed up to the stream channel. It is my opinion that the applicant should provide evidence of MeDEP sign-off.
- 2. The applicant should provide evidence of adequate sanitary disposal area on the site since two new bathrooms are proposed according to sheet A-2 of the drawings. The septic field location is not shown on the plans. The Plumbing Inspector must review and approve the plans. The owner implied that the septic field was along the existing building; however, there does <u>not</u> appear to be sufficient room available to meet the Maine Subsurface Wastewater Disposal System setbacks.
- 3. Other zoning requirements including parking, landscaping, etc. should be addressed by the applicant.
- 4. The site plan does not show a 40' x 40' expansion. It should be revised to accurately depict the building expansion limits as well as the limits of previously placed fill.
- 5. The site plan should be revised to include the limits of proposed disturbance, proposed driveway/paving, grading, spot grades around the building and drainage measures.
- 6. The site plan should be revised to include erosion control measures and an erosion control narrative outlining winter construction methods. I informed the owner that the fill area next to the existing building must be stabilized. The fill height down to the stream is approximately 25 feet and is at a 1:1 slope or steeper. I directed him to immediately install silt fence and to place riprap along the fill slope.





.6/99

DEPARTMENT OF ENVIRONMENTAL PROTECTION (DEP) PERMIT BY RULE NOTIFICATION FORM (For use with DEP Regulation, Chapter 305)

PLEASE TYPE OR PRINT IN E	LACK INK ONLY (3 C	opies, please bean	DOWN)		200
Name of Applicant:	MAM G	RIBIZIS	Name of Owner:	5AMR.	ABO
Mailing Address: 28	35 <u>CLIFT</u>	ON ST	Town/City:	PORTAR	ND,
State: MA-IWC	Zip Code:	04/03	Daytime Telephone ((Include area code)	16: 207-	773 6034
Name of Wetland, Water E	Jody or Stream:			WWW-978AbhadokuidashidasW2+A57-A+	
Detailed Directions to Site	92	102100	CAN AVE	IN PORT	LAND.
					and the second s
	KAND M	ap#: /3 ~-4/	Lot #: 4/5		JUMBERLA
Description of Project:	Yu Hunsa	APPITION LEAST 3	,	6-ARABE	
STREAM.	1 Be At	Lenst 3	6 10 40' FL Part of a larger pr		20 M ARY NOX
(CHECK ONE) This proje	ect: does 🔲 d	loes not invo	ive work below mean i	ow water.	iiniii iii
I am filing notice of my in Regulation, Chapter 305. standards.	itent to carry out wi I have a copy of I	ork which meets the PBR Sections che	ne requirements for Percked below. I have rea	rmit By Rule (PBR) ad and will comply v	under DEP vith all of the
🗝 Sec. (2) Soil Disturbanc	9	Sec. (8) Shoreline		Sec. (14) Piers, Wh	
Sec. (3) Intake Pipes	č Chrysta ros	Sec. (9) Utility Cro	= -	Sec. (15) Public Box	
Sec. (4) Replacement of Sec. (5) REPEALED	Subcoles	Sec. (10) Stream (ansportation Facilities	Sec. (16) Coestal S	=
Sec. (6) Movement of R	ocks or Vegetation		ion of Natural Areas	Sec. (18) Maintenar	
Sec. (7) Outfall Pipes		Sec. (13) F&W Cr Quality Improveme	eation/Enhance/Water nt		
I authorize staff of the De access the project site fo not valid until approved	r the purpose of de	termining complia	nce with the rules. I al	so understand that	this permit is
I have attached all of the NECESSARY ATTACHME	following required NTS:	submittals. NOTIF	CATION FORMS CAN	NOT BE ACCEPTED	WITHOUT THE
	topo map or Mair	ne Atlas & Gazetto	ole to: "Treasurer, St eer map with the proj is (unless not require	ect site clearly ma	
Signature of Applicant:	William	July	Date	11/ /9	7
Keep the bottom copy as Environmental Protection Office as evidence of the notice. Permits are valid action.	n at the appropriate DEP's receipt of r	te regional office notification. No fur	listed below. The DE ther authorization by E	EP will send a copy DEP will be issued a	to the Town Ifter receipt of
AUGUSTA DEP STATE HOUSE STAT AUGUSTA, ME 0433 (207)287-2111	TION 17 312 CA 33-0017 PORTL	AND DEP NGO ROAD AND, ME 04103 22-6300	BANGOR DEP 106 HOGAN ROAD BANGOR, ME 0440 (207)941-4570	PRESQUE ISL 1235 CENTRA 1 PRESQUE ISL (207)764-0477	L DRIVE E, ME 04769
OFFICE USE ONLY	Ck#096/	/	Staff MSC	Staff	
PBR#24054	FP 57).00	Date 7 9	Acc. 1//17/99	Def. Date	After Photos

16/16/29

- In the DEP HAS SIGNED OFF. THEY will mail you oursence.
- I THIS HOUSE IS OVER 90 YEARS OLD

 THE SEPTE HAS BEEN REPLACED

 E DO NOT KNOW WHERE THE KERCAF

 FIGURIS. WE HAVE NOT HAD ANY

 PROBLEMS WITH THE PROMIBING.
- 3. PARKING WILL BE IN THE DRIVEWAY.
 THE DRIVE WAY 15 AT LEAST 12'
 WIDE 100' LONG-
- 4. IT IS NOW to Scale.
- 5. THERE WILL BE NO DISTURBANCE. NO GRADING WILL BE DONE
- 6. We will Put SILT FEWERE PROUND Rev. meten. AS I STATED TO The ENGINCER. WE will SOD THE SLOPE:

CITY OF PORTLAND, MAINE DEVELOPMENT REVIEW APPLICATION PLANNING DEPARTMENT PROCESSING FORM

19990150 I. D. Number 1900,03

William Gribizis			10/22/99			
Applicant	-	Application Date				
285 Clifton Street, Portland, ME 04103		Ocean Ave 1021				
Applicant's Mailing Address		r.	Project Nar	ne/Description		
SAA		1021 Ocean Ave, Portland Main	e 04103			
Consultant/Agent		Address of Proposed Site				
773-6034		415-B-004				
Applicant or Agent Daytime Telephone, Fax		Assessor's Reference: Chart-Bloc	k-Lot			
Proposed Development (check all that apply)		ing Addition		tesidential		
☐ Office ☐ Retail ☐ Manufactur		☐ Parking Lot ☐ Other (
1600 sf	1 +		1M			
Proposed Building square Feet or # of Units	Acreage of Site		Zo	ning		
Check Review Required:						
Site Plan (major/minor)	Subdivision # of lots	☐ PAD Review		14-403 Streets Review		
10 Section Con April 2010 (Section Con Control	Shoreland	☐ HistoricPreservation		DEP Local Certification		
DE 1 - 445	6	, ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	_			
☐ Zoning Conditional ☐ Use (ZBA/PB)	Zoning Variance		Ц	Other		
Fees Paid: Site Plan \$400.00	Subdivision	Engineer Review	Date	10/22/99		
DRC Approval Status:	R	eviewer				
	Approved w/Conditions see attached	☐ Denied				
Approval Date	Approval Expiration	Extension to	□	riodidorial officeto		
☐ Condition Compliance				Attached		
signature date						
Performance Guarantee	Required*	☐ Not Required				
* No building permit may be issued until a pe	E1	30				
Performance Guarantee Accepted	Fromance guarantee has been submit	ted as indicated below				
☐ Inspection Fee Paid	date	amount		expiration date		
date		amount				
Building Permit date		- 5				
Performance Guarantee Reduced						
· ·	date	remaining balance		signature		
☐ Temporary Certificate Of Occupancy		☐ Conditions (See Attached)				
date						
☐ Final Inspection	2 77					
date signature Certificate Of Occupancy						
date						
☐ Performance Guarantee Released						
☐ Defect Guarantee Submitted	date	signature				
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WILLIAM GRIBIZIS

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7.	SEG PLAN
8.	NONE
9,	THERE WILL BE NO FINANCIALS
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1./	115.110



March 25, 1993

William G Gribizis

285 Clifton Street Portland ME 04103 4614

KMC LOAN NO.: 845286-6

The above referenced loan was paid in full on 03-11-93. Enclosed are the indicated documents.

Mortgage Note:

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Mortgage Deed:

S.º

Release Document:

Recording Instructions:

Other:

These documents are for your records unless otherwise specified.

We would like to take this opportunity to thank you for your business.

Knutson Mortgage Corporation 1-800-648-4800

SMM

PD004 013

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ADJUSTABLE RATE NOTE 1 Year Tressury Index—Rate Caps

454528052

THIS NOTE CONTAINS PROVISIONS ALLOWING FOR CHANGES IN MY INTEREST RATE AND MY MONTHLY PAYMENT. THIS NOTE LIMITS THE ANOUNT MY INTEREST RATE CAN CHANGE AT ANY ONE TIME AND THE MAXIMUM RATE I MUST PAY.

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(F) Proce of Changes

The Note Holder will deliver or mail to me a notice of any changes in my interest rate and the amount of my monthly payment.

The Note Holder will deliver or mail to me a notice of any changes in my interest rate and the amount of my monthly payment.

(F) Notice of Changes

7. BORROWER'S FAILURE TO PAY AS REQUIRED

(A) Late Charges for Overdue Payments

if the Note Holder has not received the full amount of any monthly payment by the end of _ calen 5,00 dar days after the date it is due, I will pay a late charge to the Note Holder. The amount of the charge will be my overdue payment of principal and interest. I will pay this late charge promptly but only once on each late payment.

(B) Default

If I do not pay the full amount of each monthly payment on the date it is due, I will be in default.

(C) Notice of Default

If I am in default, the Note Holder may send me a written notice telling me that if I do not pay the overdue amount by a certain date, the Note Holder may require me to pay immediately the full amount of principal which has not been paid and all the interest that I owe on that amount. That date must be at least 30 days after the date on which the notice is delivered or mailed to me.

(D) No Walver By Note Holder

Even if, at a time when I am in default, the Note Holder does not require me to pay immediately in full as described above. the Note Holder will still have the right to do so if I am in default at a later time.

(E) Payment of Note Holder's Costs and Expenses

If the Note Holder has required me to pay immediately in full as described above, the Note Holder will have the right to be paid back by me for all of its costs and expenses in enforcing this Note to the extent not prohibited by applicable law. Those expenses include, for example, reasonable attorneys' fees.

8. GIVING OF NOTICES

Unless applicable law requires a different method, any notice that must be given to me under this Note will be given by delivering it or by mailing it by first class mail to me at the Property Address above or at a different address if I give the Note Holder a notice of my different address.

Any notice that must be given to the Note Holder under this Note will be given by mailing it by first class mail to the Note Holder at the address stated in Section 3 (A) above or at a different address if I am given a notice of that different address.

9. OBLIGATIONS OF PERSONS UNDER THIS NOTE

If more than one person signs this Note, each person is fully and personally obligated to keep all of the promises made in this Note, including the promise to pay the full amount owed. Any person who is a guarantor, surety or endorser of this Note is also obligated to do these things. Any person who takes over these obligations, including the obligations of a guarantor, surety or endorser of this Note, is also obligated to keep all of the promises made in this Note. The Note Holder may enforce its rights under this Note against each person individually or against all of us together. This means that any one of us may be required to pay all of the amounts owed under this Note.

10. WAIVERS

I and any other person who has obligations under this Note waive the rights of presentment and notice of dishonor. "Presentment" means the right to require the Note Holder to demand payment of amounts due. "Notice of dishonor" means the right to require the Note Holder to give notice to other persons that amounts due have not been paid.

11. UNIFORM SECURED NOTE

This Note is a uniform instrument with limited variations in some jurisdictions. In addition to the protections given to the Note Holder under this Note, a Mortgage, Deed of Trust or Security Deed (the "Security Instrument"), dated the same date as this Note, protects the Note Holder from possible losses which might result if I do not keep the promises which I make in this Note. That Security Instrument describes how and under what conditions I may be required to make immediate payment in full of all amounts I owe under this Note. Some of those conditions are described as follows:

Transfer of the Property or a Beneficial interest in Borrower. If all or any part of the Property or any interest in it is sold or transferred for if a beneficial interest in Borrower is sold or transferred and Borrower is not a natural person) without Lender's prior written consent, Lender may, at its option, require immediate payment in full of all sums secured by this Security Instrument. However, this option shall not be exercised by Lender if exercise is prohibited by federal law as of the date of this Security Instrument. Lender also shall not exercise this option if: (a) Borrower causes to be submitted to Lender information required by Lender to evaluate the intended transferce as if a new loan were being made to the transferce; and (b) Lender reasonably determines that Lender's security will not be impaired by the loan assumption and that the risk of a breach of any covenant or agreement in this Security Instrument is acceptable to Lender.

To the extent permitted by applicable law, Lender may charge a reasonable fee as a condition to Lender's consent to the loan assumption. Lender may also require the transferee to sign an assumption agreement that is acceptable to Lender and that obligates the transferee to keep all the promises and agreements made in the Note and in this Security Instrument, Borrower will continue to be obligated under the Note and this Security Instrument unless Lender releases Borrower in writing.

If Lender exercises the option to require immediate payment in full, Lender shall give Borrower notice of acceleration. The notice shall provide a period of not less had 30 days from the date the notice is delivered or mailed within which Borrower must pay all sums secured by the Security Instrument. If Borrower fails to pay

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GRIBIZIS ADDITION

1021 Ocean Avenue Portland, Maine PSE Project No. 119-99

STRUCTURAL DRAWINGS, SPECIFICATIONS, AND SKETCHES

Drawings S1 and S2 Sketches SK-1 thru SK-22

Prepared for:

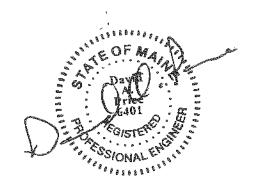
Bill Gribizis 285 Clifton Street Portland, ME 04103 Tel: (207) 773-6034

Prepared by:

David A. Price, P.E. Price Structural Engineers 75 Farms Edge Road North Yarmouth, ME 04097

Tel: (207) 846-0099 Fax: (207) 846-1633

August 25, 1999



40' x 40' ADDITION FOR BILL GRIBIZIS Project Location: 1021 Ocean Avenue; Portland, Maine August 23, 1999

GENERAL STRUCTURAL NOTES

DIVISION 1 - GENERAL REQUIREMENTS

Section 01010 - Summary of Work

- 1. Work of this Contract is comprised of the structural framing and foundation for a new 40' x 40' addition at the above referenced project location. Elevated floor is designed for a 50 psf office live load.
- 2. Work and materials shall conform to the 1996 BOCA National Building Code, State of Maine Building Codes, and other applicable codes and standards and shall meet the requirements of local authorities having jurisdiction.
- 3. Coordinate work schedule, daily hours of construction, location of material storage, access to utilities, and final cleanup requirements with owner prior to construction.
- 4. Structural drawings and specifications do not include provisions for sitework, watertightness of building, NFPA fire code requirements, Americans with Disabilities Act (ADA) requirements, egress requirements, or other architectural features.
- 5. The following list of drawings and sketches form a part of this specification:
 - S1 Foundation and 2nd Floor Framing Plan
 - S2 Roof Plan
 - Structural Sketches SK-1 through SK-22 (attached to this specification)
- 6. The structural design is based on the full interaction of all its connected parts. No provisions have been made for any temporary conditions that may arise during construction prior to the completion of the structure. The Contractor shall be responsible for adequate design and construction of all forms, shoring and temporary bracing during the progress of the project.
- 7. Alternate connection details may be used if such details are submitted to the Structural Engineer for review and acceptance is granted. However, the Structural Engineer shall be the sole judge of acceptability and the Contractor's Bid shall anticipate the use of those specific details shown on the Drawings. The Contractor shall be responsible for the design of any alternate details which he proposes.

- 8. The Contractor shall be completely responsible for the safety of adjacent structures, property, and the public. The Contractor shall comply with all federal, state and local requirements.
- 9. Do not scale from Drawings.
- 10. All materials shall be new except those labeled "EXG" (existing).
- 11. Work not indicated on a part of the Drawings but reasonably implied to be similar to that shown at corresponding places shall be included.
- 12. Any modification or alteration of these Construction Documents or changes in construction from the intent of these documents by the Contractor without written approval of the Engineer shall remove all professional and liable responsibility on the part of the Engineer.
- 13. The Contractor is required to examine the Drawing and Specifications carefully, visit the site and fully inform themselves as to all existing conditions and limitations, prior to submitting their Bid. Failure to visit the site and familiarize themselves with the existing conditions, interferences and other limitations will in no way relieve the successful Bidder from furnishing any materials or performing any work in accordance with Drawings and specifications (at no additional cost to the Owner).
- 14. Contractor shall obtain all necessary permits prior to proceeding with construction. Coordinate temporary dust enclosure requirements and security requirements with Owner. Submit detailed construction schedule to Owner prior to construction.
- 15. Remove and legally dispose of demolished materials.
- 16. Contractor shall take all necessary precautions to ensure that existing building components are not damaged during construction. All damaged areas shall be completely restored to the full satisfaction of the Owner at no additional cost to the Owner.
- 17. Stored materials shall be kept under cover and dry. Protect from weather and contact with damp or wet surfaces. Stack materials in such a manner that prevents warping or crushing.
- 18. Pre-manufactured materials shall be installed in accordance with manufacturer's requirements and recommendations.
- 19. Except where slope is specified, new materials shall be installed plumb, level, and square.

20. Substitutions for specified pre-manufactured materials may be made but only after specific written approval has been provided by the owner's engineer prior to installation.

DIVISION 2 - FOUNDATIONS

Section 02200 - Foundations and Backfill

- 1. Foundation excavations shall extend to undisturbed soil capable of providing sound, stable bearing below footings. Engineer assumes no responsibility for subsurface soil conditions. Owner is advised to obtain the services of a qualified geotechnical engineer.
- 2. No footings shall be placed in water or on frozen ground.
- 3. Foundations shall be carried down a minimum of 5 ft -0 in. below grade, and shall be configured per Structural Drawings to resist uplifting during freeze thaw cycles.
- 4. Contractor will inspect and certify excavation bottoms, footing bearings, and structural backfill for conformance with Specifications and applicable codes.
- 5. Structural fill adjacent to foundations shall be a coarse granular material consisting of hard, durable, angular material conforming to the following gradation (MDOT 703.06 Type B Aggregate):

<u>Sieve Size</u>	<u>% Passing by Weight</u>
4 in.	100
½ in.	35-75
¼ in.	25-60
No. 40	0-25
No. 200	0-5

6. Structural backfill shall be placed to 95% of maximum density compaction as determined by ASTM D 1557. Lifts shall be no greater than 6 in. each and compacted with a vibratory compactor at each lift.

DIVISION 3 - CONCRETE

Section 03300 - Cast-in-Place Concrete

- 1. All concrete work shall conform to American Concrete Institute (ACI) "Specifications for Structural Concrete for Buildings" (ACI 301) and "Building Code Requirements for Reinforced Concrete" (ACI 318).
- 2. All concrete slabs shall have a minimum compressive strength of 4000 psi at 28 days, with water/cement ratio not exceeding 0.45. Concrete shall be made with %" stone aggregate; shall have 5-7% air entrainment; shall be made with Type I

- or Type II cement; and shall have a 3"-4" slump. Concrete for slabs shall contain polypropylene fibers engineered for use in concrete complying with ASTM-C116, Type III, ½" to 1 ½" long, 1.5 pounds per cubic yd.
- 3. Other concrete shall be as specified above in Note 2, except that the minimum compressive strength may be reduced to 3000 psi at 28 days, and polypropylene fibers may be omitted.
- 4. Shop Drawings shall be prepared and submitted for all steel reinforcing within concrete.
- 5. No foundations shall be placed in water or on frozen ground.
- All embedments in concrete, including anchor bolts, shall be firmly secured by tie wire to prevent movement during concrete placement.
- 7. All concrete materials, reinforcement and forms shall be free from frost or debris.
- 8. Concrete shall be maintained above 50 degrees F, and in moist condition for at least the first seven days after placement.
- 9. Consolidate all conrete with a vibrator or other means recommended by ACI 301.
- 10. All concrete reinforcing bars shall conform to ASTM A615, Grade 60.
- 11. Reinforcing bars may not be welded except where designated in writing by the Structural Engineer.
- 12. Vapor barrier below slabs on grade shall be "Vaporshield" by J-Pro.
- 13. Apply curing compound to slab surface in accordance with manufacturer's guidelines.

DIVISION 5 - METALS

Section 05120 - Structural Steel

- 1. All structural steel work shall conform to the recommendations and requirements contained in the "Manual of Steel Construction, Allowable Stress Design," AISC Ninth Edition (including AISC Code of Standard Practice for Steel Buildings and Bridges), and "Structural Steel Welding Code Steel," (AWS D1.1, latest edition).
- 2. No change in size or position of the structural elements shall be made without prior written approval of the Structural Engineer.

- 3. Temporary erection bracing shall be provided to hold structural steel securely in position. Remove temporary bracing and connections only after permanent members are in place and final connections are in place.
- 4. Shop connections unless otherwise noted, shall be made by welding. Connect structural steel components together using high strength bolts, %-inch diameter A325N "Tension-Control" type bolts (fully tensioned shear/bearing).
- 5. All shop and field welds shall be made by certified welders, and shall conform to the American Welding Society Code, AWS D1.1, latest edition, using E70-18 electrodes. Carefully control welding technique to avoid distortion, including clamping prior to welding. Minimum weld size shall be 3/16" fillet.
- 6. Shop drawings for steel shall be submitted for review and approval. Connections shown on these Drawings are generally schematic. They are intended to define the spatial relationship of the framed members and show a feasible method of making the connections. Any connection that is not shown or is not completely detailed on the Structural Drawings shall be designed by a licensed Professional Engineer in the state of Maine retained by the fabricator. Beam-to-beam and beam-to-post connections shall be designed for an end reaction equal to one-half the uniform load capacity of the beam as given in Part 2 ("Beams and Girders") of the AISC Manual, 9th Edition. Minimum shear capacity of 12 kips shall be provided. Completely detailed means the following information is shown on the Shop Detail Drawings for review by the engineer:
 - a) All plate dimensions and grades.
 - b) All weld sizes, pitches, and returns.
 - c) All hole sizes and spacings.
 - d) Number and type of bolts: Where bolts are shown but no number is given, the connection has not been completely detailed.
 - e) Where partial information is given, it shall be the minimum requirement for the connection.
 - f) Minimum plate thickness shall be 4".
- 7. Structural steel components shall be shop primed with fabricator's standard primer, except that structural steel exposed to weather shall be primed with Tnemec 90-97 primer (steel shall have SP-6 blast finish). Provide field touch-up as necessary.

8. Structural steel rolled shapes, plates, bars and tubes shall conform to the following:

ASTM A-572, Grade 50: All wide flange sections ("W" shapes), Fy = 50 ksi ASTM A-36: Other rolled shapes, plates and bars, Fy = 36 ksi

ASTM A-500, Grade B: Steel Tubes ("TS" shapes), Fy = 46 ksi

ASTM A-53, Grade B: Steel pipe, Fy = 35 ksi

ASTM A-36: Threaded rods

ASTM A-307: Anchor bolts in concrete (unless otherwise noted)

Note: Bolts and rods exposed to weather shall be galvanized.

9. Non-shrink grout shall be 5000 psi (minimum) compression strength.

10. Coordinate final painting of steel components with owner's requirements.

DIVISION 6 - WOOD

Section 06000 - Carpentry (General)

- Lumber shall bear the grade and trademark of the association under whose rules
 it is produced and a mark of mill identification. Lumber shall be sound,
 seasoned, kiln-dried to a moisture content not exceeding 19% and surfaced four
 sides.
 - a) Pressure Treated (PT) lumber shall be Southern Yellow Pine, Number 2 grade.
 - b) Except as noted above or designated otherwise, remaining lumber shall be No. 2 grade Spruce, Pine, Fir, planed four sides.
- 2. Lumber and wood in exterior applications, at sills, at porches and in contact with concrete and masonry shall be pressure treated using CCA preservative with a minimum net retention of 0.40 pcf.
 - a) All fasteners (including nails, lag screws, and bolts) for pressure treated lumber shall be hot-dip galvanized.
 - b) Cut ends of pressure treated (PT) lumber and timber posts and sills shall be dipped in a preservative treatment for a minimum of fifteen minutes.
- 3. Fabricate horizontal and inclined members, units of less than 1:1 slope, with natural convex bow (crown) up to provide camber.
- 4. Carpentry work shall comply with AFPA's "National Design Specification for Wood Construction," 1991 Edition. Wood components shall be securely attached with sound connections and without splitting. As a minimum, wood fasteners

- shall conform to BOCA 1996, Table 2305.2, "Fastening Schedule" unless otherwise noted.
- 5. Reference to "Simpson" on Drawings indicates metal connectors manufactured by Simpson Strong-Tie.
- 6. At locations where portions of wood floor or roof deck are added or replaced, the finish floor elevation of the new wood deck shall match the adjacent existing wood floor elevation.
- 7. Plywood for floors and roof shall be installed with both suitable adhesive and 10d nails at 6" o.c. at supported edges and 12" o.c. elsewhere.
- 8. Floor framing around chase openings for mechanical ducts and stairs shall consist of the following (unless otherwise noted):
 - a) Double floor length members and joists each side of opening with member depth same as adjacent floor framing.
 - b) Members connected with Simpson double joist hangers.
- 9. Plywood for floors shall be ¾" thick, APA rated sheathing with 48/24 span rating, tongue and groove. Use full size sheets as much as possible.
- 10. Plywood for roofs shall be ¾" thick, APA rated sheathing with 48/24 span rating. Install "H" clips where recommended by APA. Use full size sheets as much as possible.
- 11. Plywood end joints for floors and roof shall be staggered. Plywood surface grain shall be transverse to joist span.
- 12. Exterior wall sheathing shall be 5/8" thick, APA rated sheathing suitable for exterior use. Use full size sheets as much as possible. Wall sheathing shall be fastened with galvanized 10d common nails having 6" spacing at supported edges and 12" spacing elsewhere.
- 13. Cornerboards, fascias, soffits, and other exterior finish trim shall be 5/4" Western Red Cedar, No. 1 Grade (unless authorized otherwise by the owner).
- 14. Air infiltration barrier between exterior plywood sheathing and wood siding shall be Tyvek Housewrap, manufactured by DuPont Company or approved equal.
- 15. All sheathing shall conform to APA "Plywood Specification Grade Guide" and Product Standard PS-1. Plywood construction shall conform to APA Design/Construction Guide Residential and Commercial, Form E30B for required applications.

- 16. Provide additional temporary bracing (not shown on drawings) for trusses as recommended by Truss Plate Institute (TPI).
- 17. Trusses shall be anchored to top of walls with Simpson H2.5 Hurricane Ties at each end.
- 18. Stair width shall not exceed 4'-0" wide and shall be supported by (3) 2x12 stringers, equally spaced. Center stringer shall be reinforced with a continuous 2x4 nailed to both sides, and outside stringers shall have continuous 2x4 nailed to one side. Spacing between supports for stringers shall not exceed 6'-4" on center. Upper ends of stringers shall be supported by either suitable steel joist hangers or continuous wood ledger.

Section 06192 - Metal-Plate Connected Wood Trusses

PART I - GENERAL

1.1 RELATED DOCUMENTS

A. Drawings and general provisions of the Contract, including General and Supplementary Conditions and Division 1 Specification Sections, apply to this Section.

1.2 SUMMARY

- A. This Section includes the following:
 - Triangular-pitched roof trusses.
 - 2. Scissor roof trusses.
 - Truss accessories.

1.3 DEFINITIONS

A. Metal-plate-connected wood trusses include planar structural units consisting of metal-plate-connected members fabricated from dimension lumber and cut and assembled before delivery to Project site.

1.4 PERFORMANCE REQUIREMENTS

- A. Structural Performance: Engineer, fabricate, and erect metal-plate-connected wood trusses to withstand design loads within limits and under conditions required.
 - 1. Design Loads: As indicated.
 - 2. Design trusses to withstand design loads without deflections greater than the following:
 - a. Roof Trusses: Vertical deflection of 1/240 of span due to total load.
- B. Engineering Responsibility: Engage a fabricator who uses a qualified professional engineer to prepare calculations, Shop Drawings, and other structural data for metal-plate-connected wood trusses.

1.5 SUBMITTALS

- A. General: Submit each item in this Article according to the Conditions of the Contract and Division I Specification Sections.
- B. Product Data for lumber, metal-plate connectors, metal framing connectors, bolts, and fasteners.

- C. Shop Drawings detailing location, pitch, span, camber, configuration, and spacing for each type of truss required; species, sizes, and stress grades of lumber to be used; splice details; type, size, material, finish, design values, and orientation and location of metal connector plates; and bearing details.
 - To the extent truss design considerations are indicated as fabricator's responsibility, include structural analysis data signed and sealed by the qualified professional engineer responsible for their preparation.
 - 2. Include truss Shop Drawings signed and sealed by the qualified professional engineer responsible for their preparation.
- D. Product certificates signed by officer of truss fabricating firm certifying that metal-plateconnected wood trusses supplied for Project comply with specified requirements and Shop Drawings.
- E. Qualification data for firms and persons specified in "Quality Assurance" Article to demonstrate their capabilities and experience. Include lists of completed projects with project names and addresses, names and addresses of architects and owners, and other information specified.
- F. Material test reports from a qualified independent testing agency indicating and interpreting test results relative to compliance of fire-retardant-treated wood products with requirements indicated.
- G. Warranty of chemical treatment manufacturer for each type of treatment.
- H. Material certificates for dimension lumber specified to comply with minimum allowable unit stresses. Indicate species and grade selected for each use and design values approved by the American Lumber Standards Committee (ALSC) Board of Review.
- I. For fire-retardant-treated wood products, include certification by treating plant that treated materials comply with specified standard and other requirements as well as data relative to bending strength, stiffness, and fastener-holding capacities of treated materials.
- J. Research or evaluation reports of the model code organization acceptable to authorities having jurisdiction that evidence the following products' compliance with building code in effect for Project.
 - Fire-retardant-treated wood.
 - Metal-plate connectors.
 - 3. Metal framing connectors.

1.6 QUALITY ASSURANCE

- A. Installer Qualifications: Engage an experienced Installer who has completed wood truss installation similar in material, design, and extent to that indicated for this Project and with a record of successful in-service performance.
- B. Fabricator's Qualifications: Engage a firm that complies with the following requirements for quality control and is experienced in fabricating metal-plate-connected wood trusses similar to those indicated for this Project and with a record of successful in-service performance:

- 1. Fabricator participates in a recognized quality-assurance program that involves inspection by SPIB; Timber Products Inspection, Inc.; Truss Plate Institute (TPI); or other independent inspecting and testing agency acceptable to Architect and authorities having jurisdiction.
- C. Comply with applicable requirements and recommendations of the following publications:
 - 1. ANSI/TP1 1, "National Design Standard for Metal-Plate-Connected Wood Truss Construction."
 - 2. TPI HIB "Commentary and Recommendations for Handling Installing & Bracing Metal Plate Connected Wood Trusses."
 - 3. TPI DSB "Recommended Design Specification for Temporary Bracing of Metal Plate Connected Wood Trusses."
- D. Metal-Plate Connector Manufacturer's Qualifications: A manufacturer that is a member of TPI and that complies with TPI quality-control procedures for manufacture of connector plates published in ANSI/TPI 1.
- E. Single-Source Responsibility for Connector Plates: Provide metal connector plates from one source and by a single manufacturer.
- F. Wood Structural Design Standard: Comply with applicable requirements of AFPA's "National Design Specification for Wood Construction" and its "Supplement."
- G. Single-Source Engineering Responsibility: Provide trusses engineered by metal-plate connector manufacturer to support superimposed dead and live loads indicated, with design approved and certified by a qualified professional engineer.
- H. Professional Engineer Qualifications: A professional engineer who is legally authorized to practice in the jurisdiction where Project is located and who is experienced in providing engineering services of the kind indicated that have resulted in installing metal-plate-connected wood trusses similar to those indicated for this Project and with a record of successful in-service performance.

1.7 DELIVERY, STORAGE, AND HANDLING

- A. Handle and store trusses with care and comply with manufacturer's written instructions and TPI recommendations to avoid damage and lateral bending.
- B. Inspect trusses showing discoloration, corrosion, or other evidence of deterioration. Discard and replace trusses that are damaged or defective.

1.8 SEQUENCING AND SCHEDULING

A. Time delivery and erection of trusses to avoid extended on-site storage and to avoid delaying progress of other trades whose work must follow erection of trusses.

PART 2 - PRODUCTS

2.1 MANUFACTURERS

- A. Available Manufacturers: Subject to compliance with requirements, manufacturers offering products that may be incorporated into the Work include, but are not limited to, the following:
- B. Manufacturers: Subject to compliance with requirements, provide products by one of the following:
 - 1. Fire-Retardant-Treated Materials, Interior Type A:
 - a. Baxter: J. H. Baxter Co.
 - b. Chemical Specialties, Inc.
 - c. Continental Wood Preservers, Inc.
 - d. Hickson Corporation.
 - e. Hoover Treated Wood Products, Inc.
 - 2. Fire-Retardant-Treated Materials, Exterior Type:
 - a. American Wood Treaters, Inc.
 - b. Hoover Treated Wood Products, Inc.
 - Mctal Connector Plates:
 - a. Alpine Engineered Products, Inc.
 - b. Computrus, Inc.
 - c. Mitek Industries, Inc.
 - d. Robbins Manufacturing Company.
 - e. Tee-Lok Corporation.
 - f. Truswal Systems Corporation.
 - 4. Metal Framing Anchors:
 - a. Cleveland Steel Specialty Co.
 - b. Harlen Metal Products, Inc.
 - c. Silver Metal Products, Inc.
 - d. Simpson Strong-Tie Company, Inc.
 - e. Southeastern Metals Manufacturing Co., Inc.
 - f. United Steel Products Co.

2.2 DIMENSION LUMBER

- A. Lumber Standards: Comply with DOC PS 20, "American Softwood Lumber Standard," and with applicable grading rules of inspection agencies certified by ALSC's Board of Review.
- B. Inspection Agencies: Inspection agencies, and the abbreviations used to reference them, include the following:
 - 1. NELMA Northeastern Lumber Manufacturers Association.
 - 2. NLGA National Lumber Grades Authority (Canadian).
 - 3. SPIB Southern Pine Inspection Bureau.

- 4. WCLIB West Coast Lumber Inspection Bureau.
- WWPA Western Wood Products Association.
- C. Grade Stamps: Provide lumber with each piece factory marked with grade stamp of inspection agency evidencing compliance with grading rule requirements and identifying grading agency, grade, species, moisture content at time of surfacing, and mill.
- D. Provide dressed lumber, S4S, manufactured to actual sizes required by DOC PS 20 for moisture content specified, to comply with requirements indicated below:
 - 1. Provide dry lumber with 19 percent maximum moisture content at time of dressing.
- E. Grade and Species: Provide dimension lumber of any species for truss chord and web members, graded visually or mechanically, and capable of supporting required loads without exceeding allowable design values according to AFPA's "National Design Specification for Wood Construction" and its "Supplement."

2.3 FIRE-RETARDANT-TREATED MATERIALS (where required by code)

- A. General: Where fire-retardant-treated wood is indicated, comply with applicable requirements of AWPA C20 (lumber). Identify fire-retardant-treated wood with appropriate classification marking of UL, U.S. Testing, Timber Products Inspection, or another testing and inspecting agency acceptable to authorities having jurisdiction.
 - Research or Evaluation Reports: Provide fire-retardant-treated wood acceptable to authorities having jurisdiction and for which a current model code research or evaluation report exists that evidences compliance of fire-retardant-treated wood for application indicated.
- B. Interior Type A: For interior locations, use chemical formulation that produces treated lumber with the following properties under conditions present after installation:
 - 1. Bending strength, stiffness, and fastener-holding capacities are not reduced below values published by manufacturer of chemical formulation under elevated temperature and humidity conditions simulating installed conditions when tested by a qualified independent testing agency.
 - 2. No form of degradation occurs due to acid hydrolysis or other causes related to treatment.
 - 3. Contact with treated wood does not promote corrosion of metal fasteners.
- C. Exterior Type: Use for exterior locations and where indicated.
- D. Inspect each piece of treated lumber after drying and discard damaged or defective pieces.

2.4 METAL CONNECTOR PLATES

- A. General: Fabricate connector plates from metal complying with requirements indicated below.
- B. Hot-Dip Galvanized Steel Sheet: Structural-quality steel sheet, zinc coated by hot-dip process complying with ASTM A 653, G60 (ASTM A 653M, Z180) coating designation; Grade 33 and not less than 0.0359 inch (0.91 mm) thick.

- C. Electrolytic Zinc-Coated Steel Sheet: ASTM A 591 (ASTM A 591M), structural-(physical) quality steel sheet, zinc coated by electrodeposition; 33,000-psi (230-MPa) minimum yield strength, coating class C, and not less than 0.0474 inch (1.20 mm) thick
- D. Aluminum-Zinc Alloy-Coated Steel Sheet: Structural-(physical) quality steel sheet, aluminum-zinc alloy-coated by hot-dip process complying with ASTM A 792, AZ50 (ASTM A 792M, AZ150) coating designation; Grade 33 and not less than 0.0359 inch (0.91 mm) thick.
- E. Stainless-Steel Sheet: ASTM A 666, Type 304 or 316, chromium nickel steel sheet; 33,000-psi (230-MPa) minimum yield strength and not less than 0.035 inch (0.89 mm) thick.

2.5 FASTENERS

- A. General: Provide fasteners of size and type indicated that comply with requirements specified below for material and manufacture.
 - Where truss members are exposed to weather or to high relative humidities, provide fasteners with a hot-dip zinc coating per ASTM A 153 or of stainless steel, Type 304 or 316.
- B. Nails, Wire, Brads, and Staples: FS FF-N-105.
- C. Power-Driven Fasteners: CABO NER-272.
- D. Wood Screws: ASME B18.6.1.
- E. Lag Bolts and Screws: ASME B18.2.1 (ASME B18.2.3.8M).
- F. Bolts: Steel bolts complying with ASTM A 307, Grade A (ASTM F 568, Property Class 4.6); with ASTM A 563 (ASTM A 563M) hex nuts and, where indicated, flat washers.

2.6 METAL FRAMING ANCHORS

- A. General: Provide metal framing anchors of structural capacity, type, size, metal, and finish indicated that comply with requirements specified, including the following:
 - Research or Evaluation Reports: Provide products for which model code research or
 evaluation reports exist that are acceptable to authorities having jurisdiction and that
 evidence compliance of metal framing anchors for application indicated with building
 code in effect for this Project.
 - 2. Allowable Design Loads: Provide products with allowable design loads, as published by manufacturer, that meet or exceed those indicated. Manufacturer's published values shall be determined from empirical data or by rational engineering analysis, and demonstrated by comprehensive testing performed by a qualified independent testing agency.
- B. Galvanized Steel Sheet: Hot-dip, zinc-coated steel sheet complying with ASTM A 653, G60 (ASTM A 653M, Z180) coating designation; structural, commercial, or lock-forming quality, as standard with manufacturer for type of anchor indicated.

C. Stainless-Steel Sheet: ASTM A 666, Type 304 or 316, chromium nickel steel sheet; 33,000-psi (230-MPa) minimum yield strength.

2.7 MISCELLANEOUS MATERIALS

- A. Galvanizing Repair Paint: SSPC-Paint 20 or DOD-P-21035, with dry film containing a minimum of 94 percent zinc dust by weight.
- B. Protective Coatings: Provide one of the following coating systems:
 - 1. SSPC-Paint 22, epoxy-polyamide primer.
 - SSPC-Paint 16, coal-tar epoxy-polyamide black or dark red paint.
 - SSPC-Paint 27 and SSPC-Paint 12, basic zinc chromate-vinyl butyral wash primer and cold-applied asphalt mastic.

2.8 FABRICATION

- A. Cut truss members to accurate lengths, angles, and sizes to produce close-fitting joints.
- B. Fabricate metal connector plates to size, configuration, thickness, and anchorage details required to withstand design loadings for types of joint designs indicated.
- C. Assemble truss members in design configuration indicated using jigs or other means to ensure uniformity and accuracy of assembly with joints closely fitted to comply with tolerances of ANSI/TPI 1. Position members to produce design camber indicated.
 - 1. Fabricate wood trusses within manufacturing tolerances of ANSI/TPI 1.
- D. Connect truss members by metal connector plates located and securely embedded simultaneously into both sides of wood members by air or hydraulic press.

PART 3 - EXECUTION

3.1 INSTALLATION

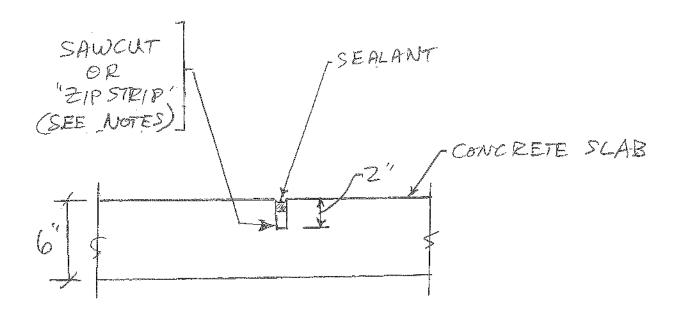
- A. Do not install wood trusses until supporting construction is in place and is braced and secured.
- B. Before installing, splice trusses delivered to Project site in more than one piece.
- C. Hoist trusses in place by lifting equipment suited to sizes and types of trusses required, exercising care not to damage truss members or joints by out-of-plane bending or other causes.
- D. Install and brace trusses according to recommendations of TPI and as indicated.
- E. Install trusses plumb, square, and true to line and securely fasten to supporting construction.
- F. Space, adjust, and align trusses in location before permanently fastening and as follows:

- 1. Truss Spacing: As indicated.
- G. Anchor trusses securely at all bearing points using metal framing anchors. Install fasteners through each fastener hole in metal framing anchor according to manufacturer's fastening schedules and written instructions.
- H. Securely connect each truss ply required for forming built-up girder trusses.
 - Auchor trusses to girder trusses as indicated.
- I. Install and fasten permanent bracing during truss crection and before construction loads are applied. Anchor ends of permanent bracing where terminating at walls or beams.
 - Install and fasten strongback bracing vertically against vertical web of parallel-chord floor trusses at centers indicated.
- J. Install wood trusses within installation tolerances of ANSI/TPI 1.
- K. Do not cut or remove truss members.
- L. Return wood trusses that are damaged or do not meet requirements to fabricator and replace with trusses that do meet requirements.
 - 1. Do not alter trusses in the field.

3.2 REPAIRS AND PROTECTION

- A. Repair damaged galvanized coatings on exposed surfaces with galvanized repair paint according to ASTM A 780 and manufacturer's written instructions.
- B. Protective Coating: Clean and prepare exposed surfaces of embedded-metal connector plates. Brush apply primer, when part of coating system, and one coat of protective coating.
 - 1. Apply materials to provide minimum dry film thickness recommended by manufacturer of coating system.

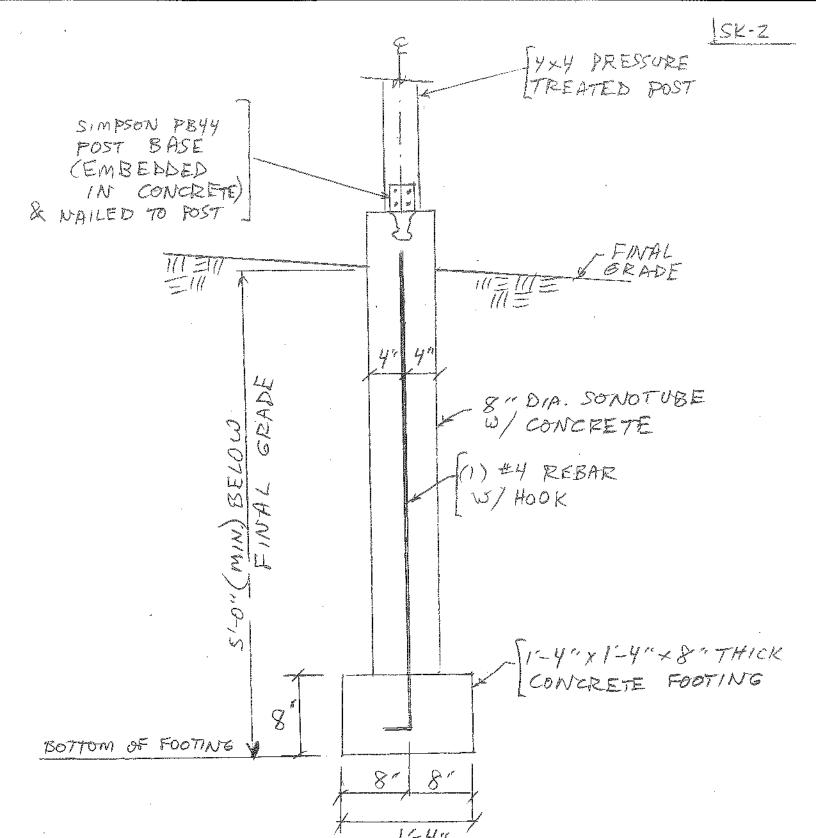
END OF SECTION 06192



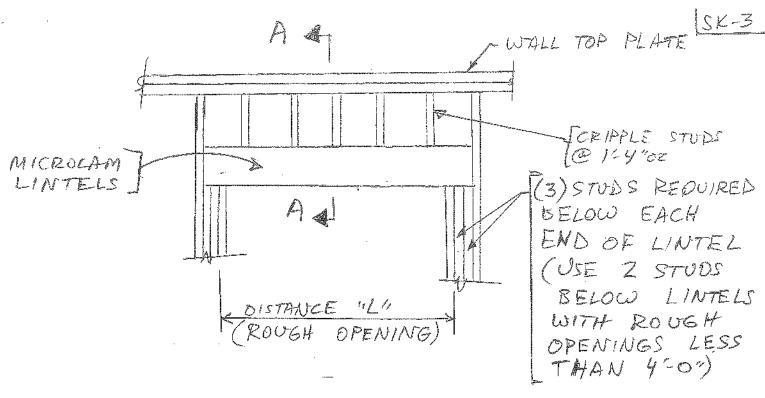
Notes:

1. Slab shall be sawed as soon as possible without dislodging aggregate of fresh concrete, but under no circumstances longer than 6 hours efter concrete slab has been placed.

2. Control joints shall be straight and shall be spaced not hour than 10'- or on center



PIER DETAIL (B)



WALL ELEVATION @ ROUGH OPENING
Not to Scale

LINTEL SC	HEDULE
Distance "L"	Microlan
Rough Opening	Lintel (See Notes,
10-0	(3) 13/4×117/8
6'-6"	(3) 13/4× 9/4
3'-0"	(2) 13/4 × 51/2

SEE LINTEL SCHEDULE

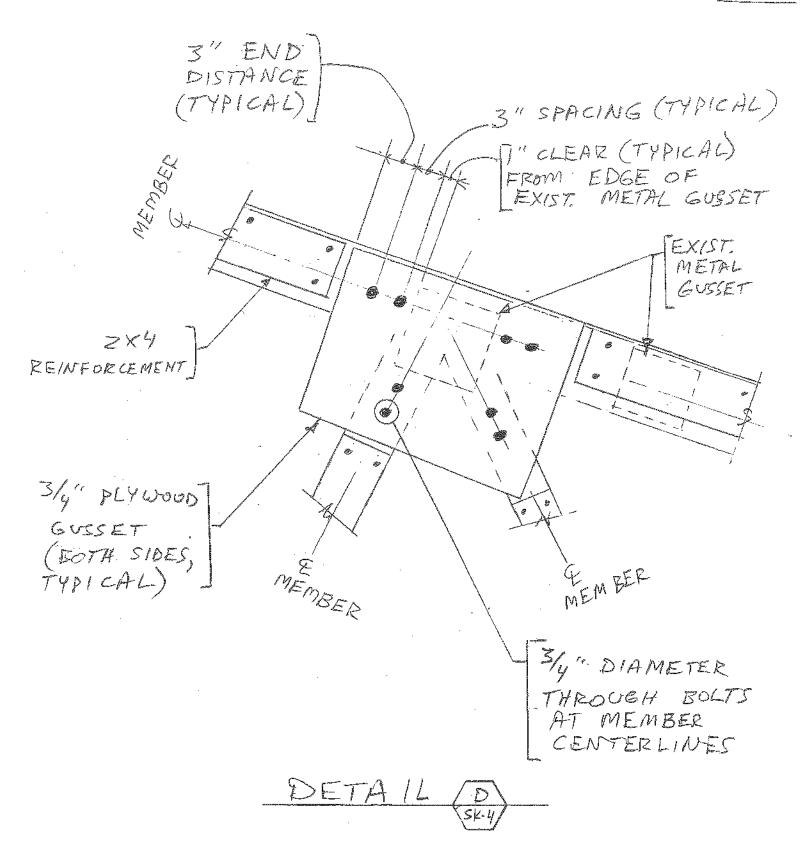
LINTEL SCHEDULE (C)
Not to Scale (SK-3)

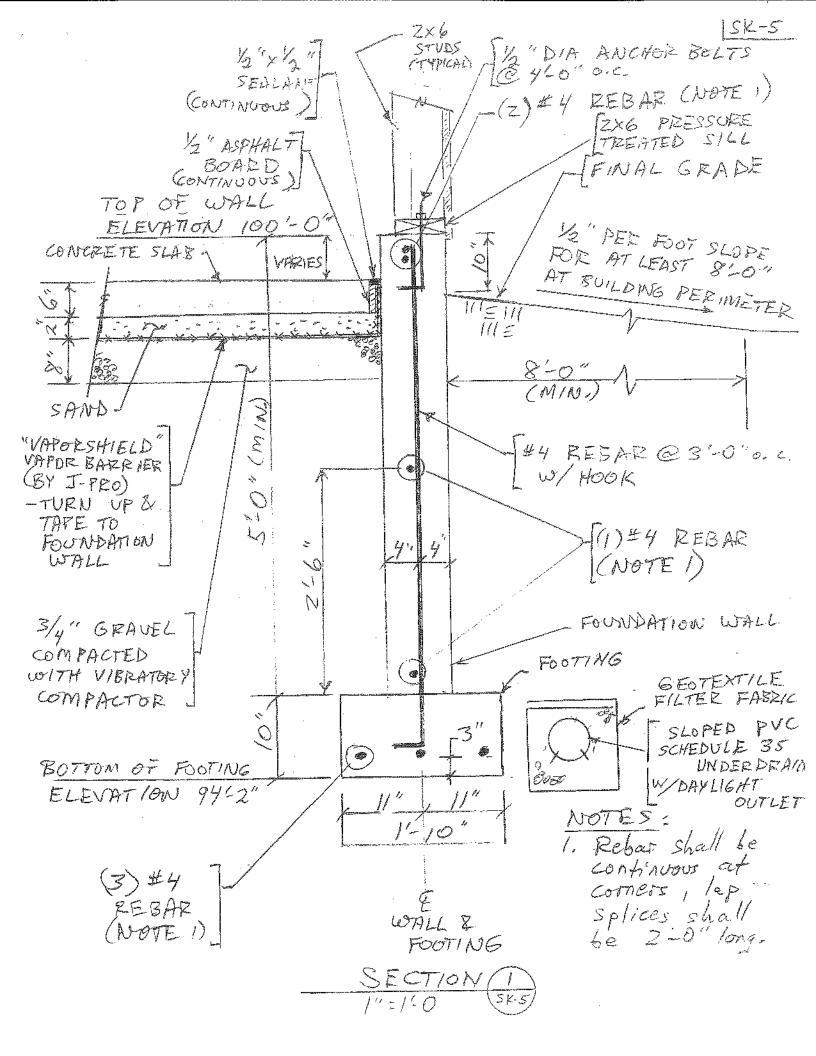
SECTION A-A Not to Scale

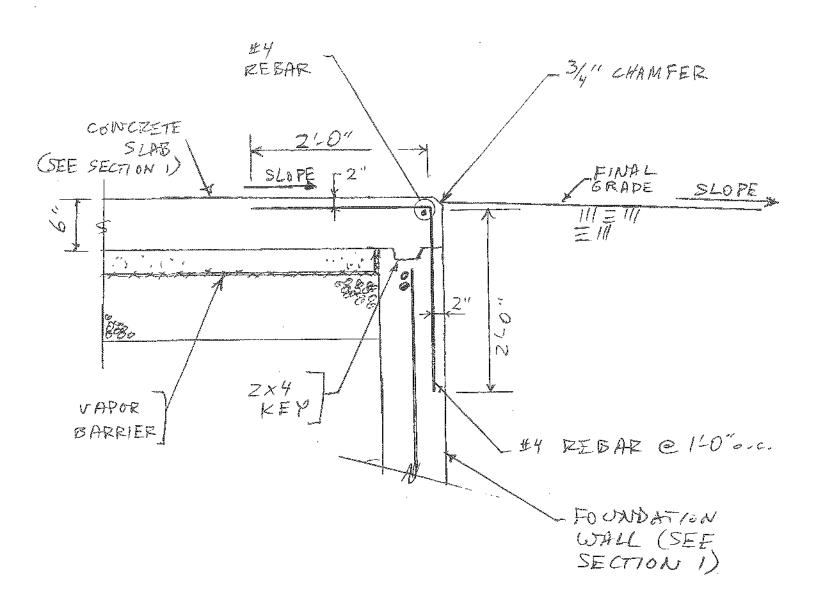
Notes:

1. Microlans as fabricated by Trus Joist Mannillan or approved equal 2. At 3-0" Lintel, center member

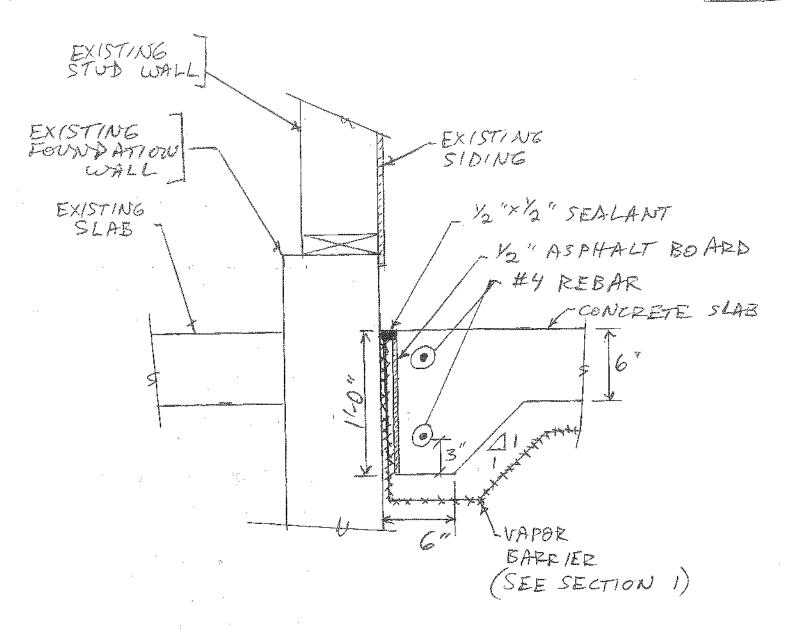
shall be ZX6, 42SPF

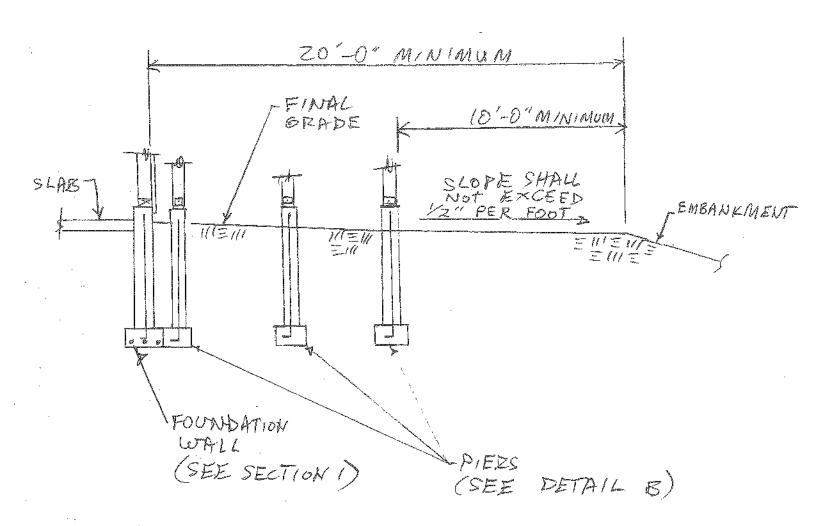


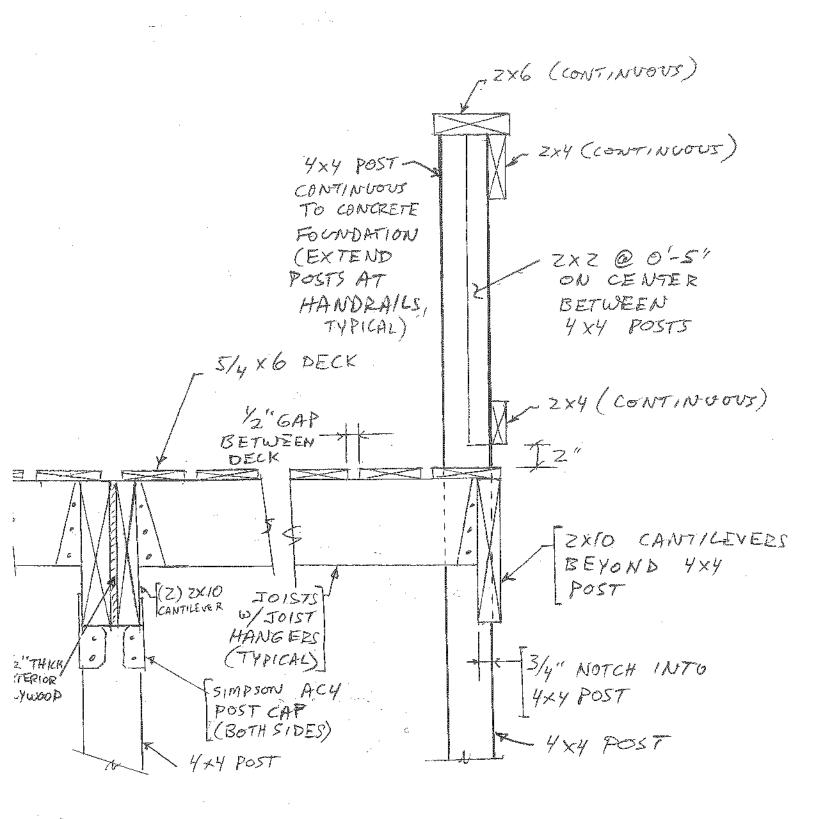




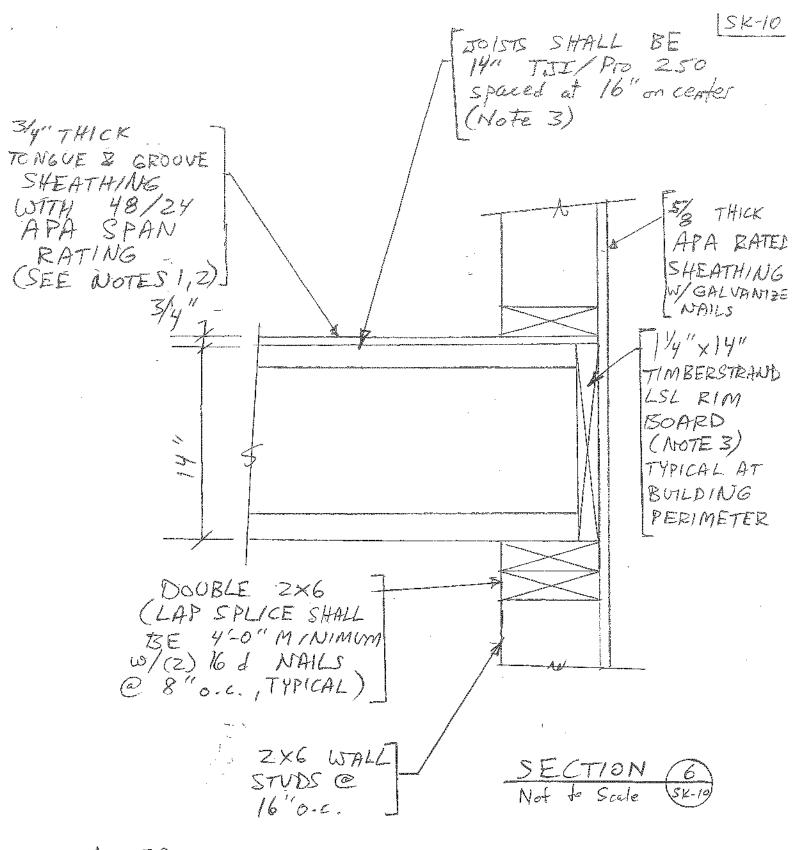
SECTION(2) Not to Scale SK-6)







SECTION 5 Not to Scale 5k.9

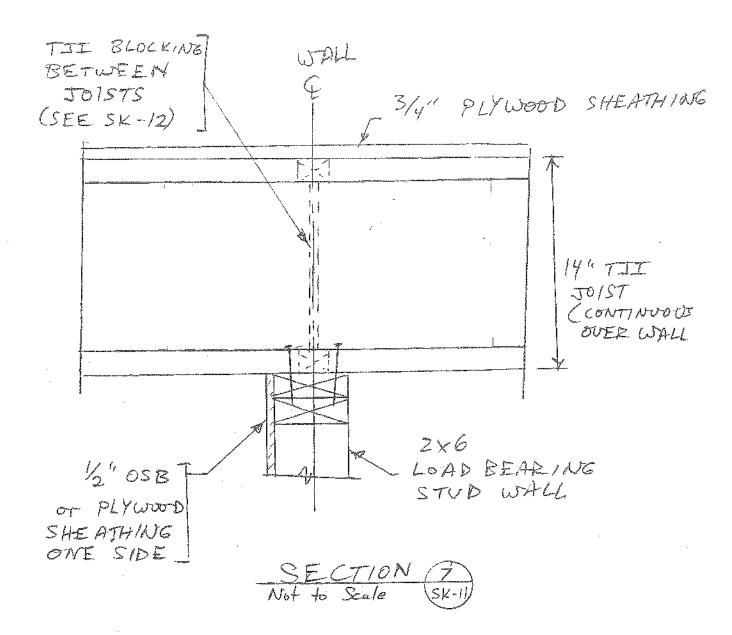


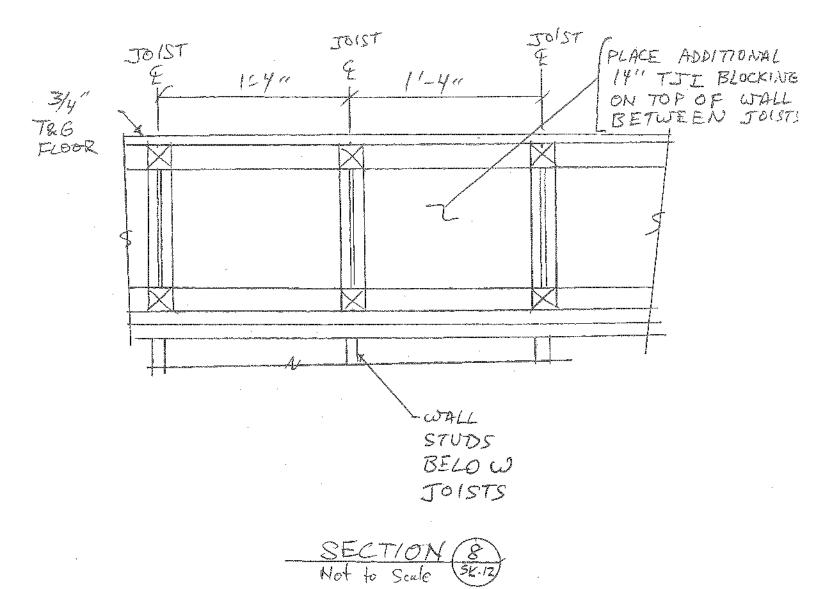
NOTES:

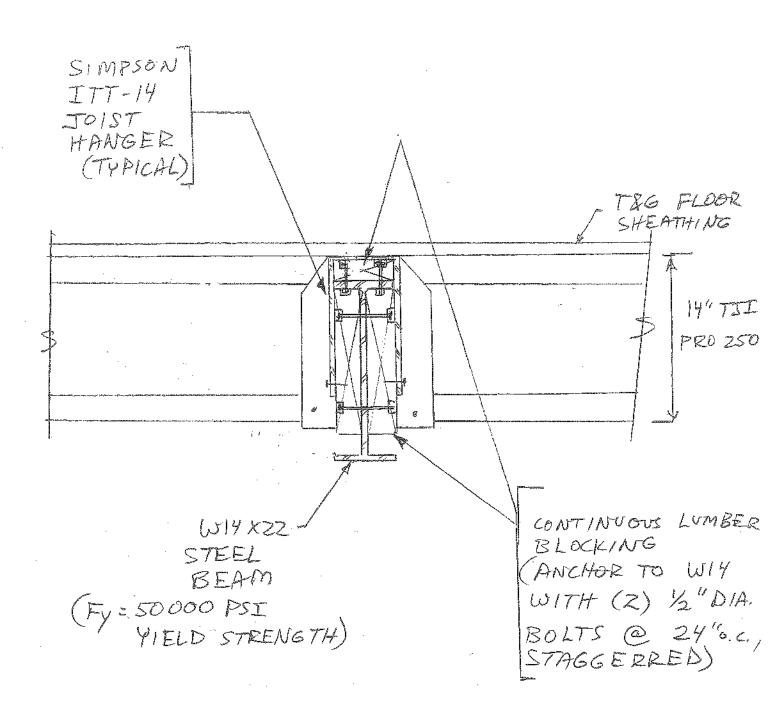
1. Floor sheathing shall be placed with long dinvension perpendicular to supports.

2. Give sheathing to joints and pail with 10d nacls. North spacing shall be 6" on center at supported edges and 1-0 on center at supported edges and 1-0 on southers.

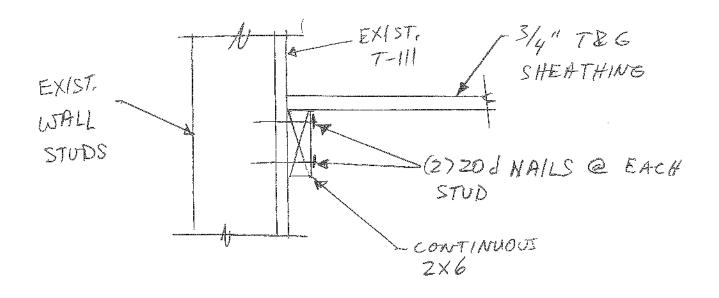
3. TJI & LSL manufactured by Trus Joint Macrillan or said.





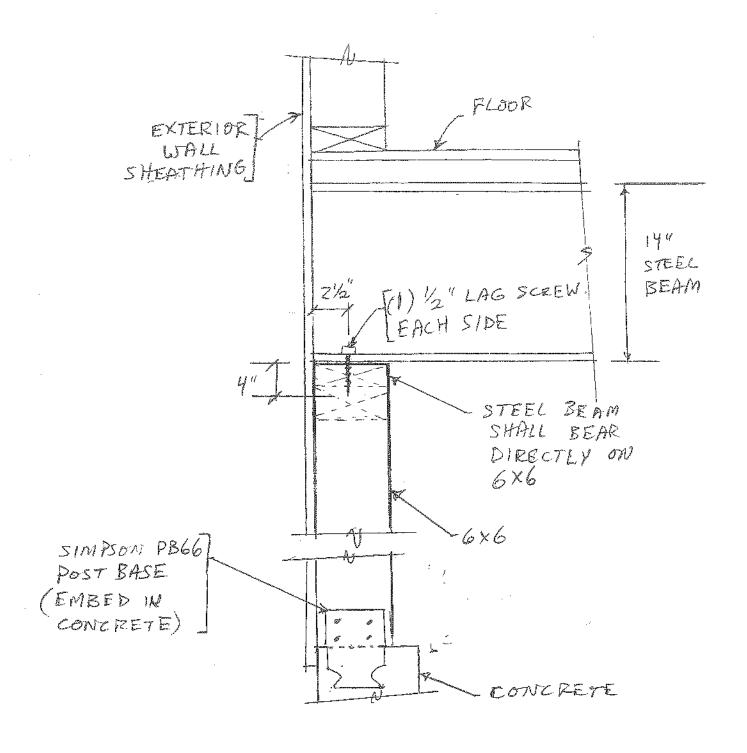


SECTION 9 Not to Scale (SK-13)

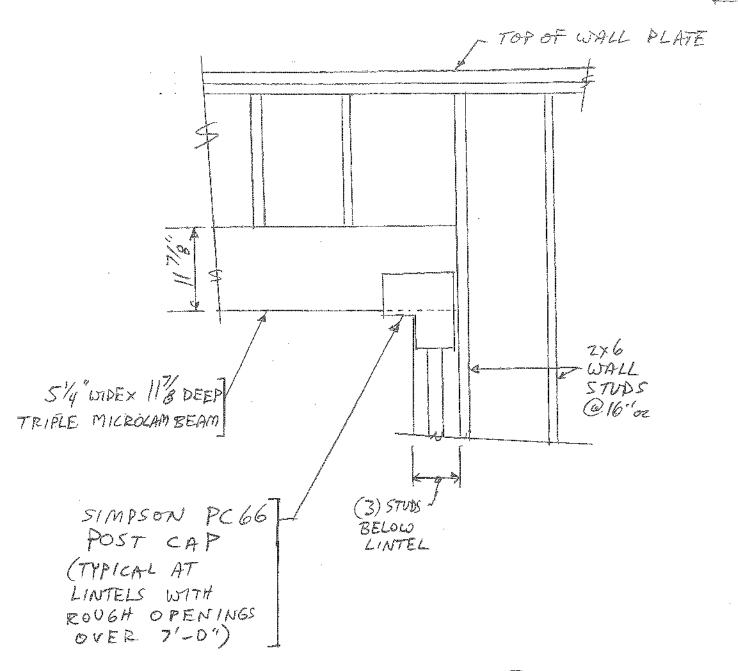


SECTION 10 Not to Scale SK-14

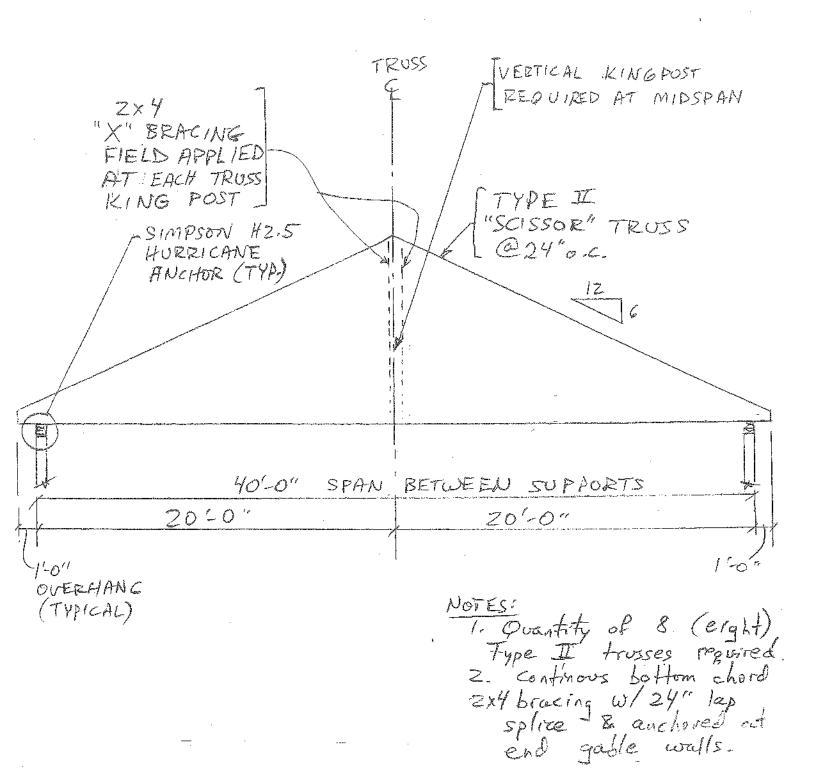
NOTE: other details not shown for clarity 4 "地" WOOD NAILER 14" STEEL BEAM SOTH SIDES PE 5/8×6"×0'-6" 3/16 STEEL PIPE COLUMN (4) 8 DIA EXPANSION BOLTS BASE 12 5/2 X 8"X 0'8" CONCRETE SLAB 3/16 NONSHRINK GROUT REBAR CONTEREST FOOTING 3"CLEAR 14'x4' FOOTING (4)#4.



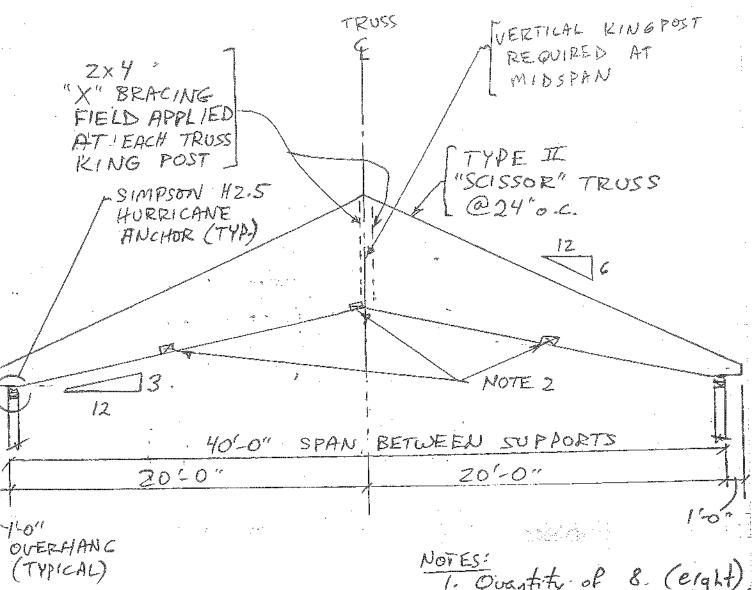
SECTION (12) Not to Scale SK-16



SECTION 13 Not to Scale (SK-17)



TYPE IF TRUSS PROFILE 14 SK-18



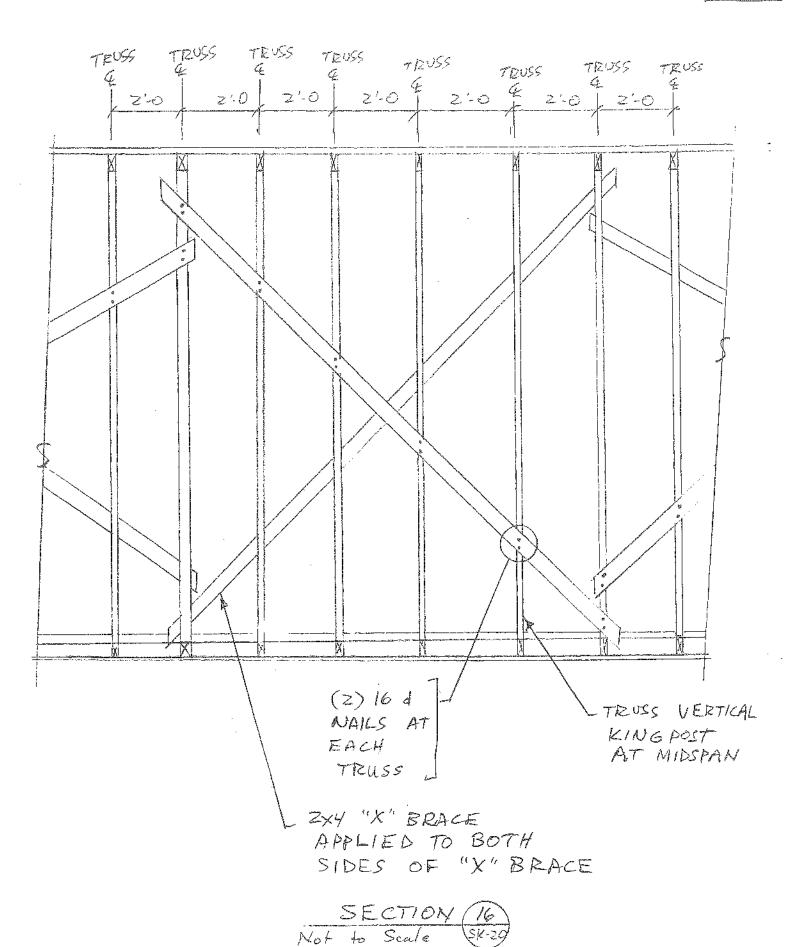
Type I trusses required.

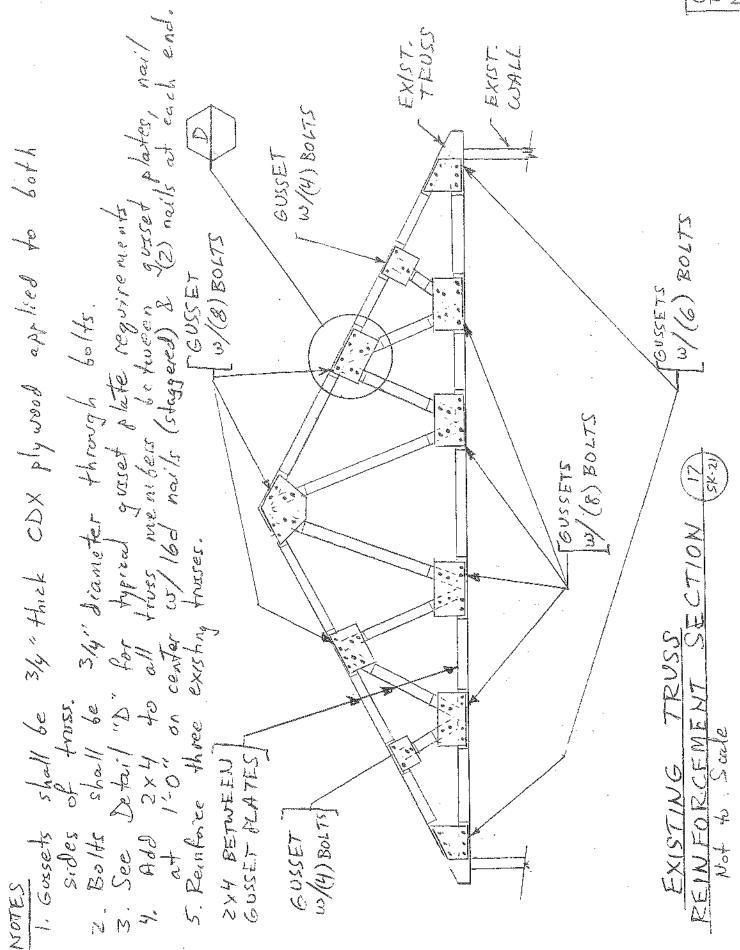
Z. Continous bottom chord

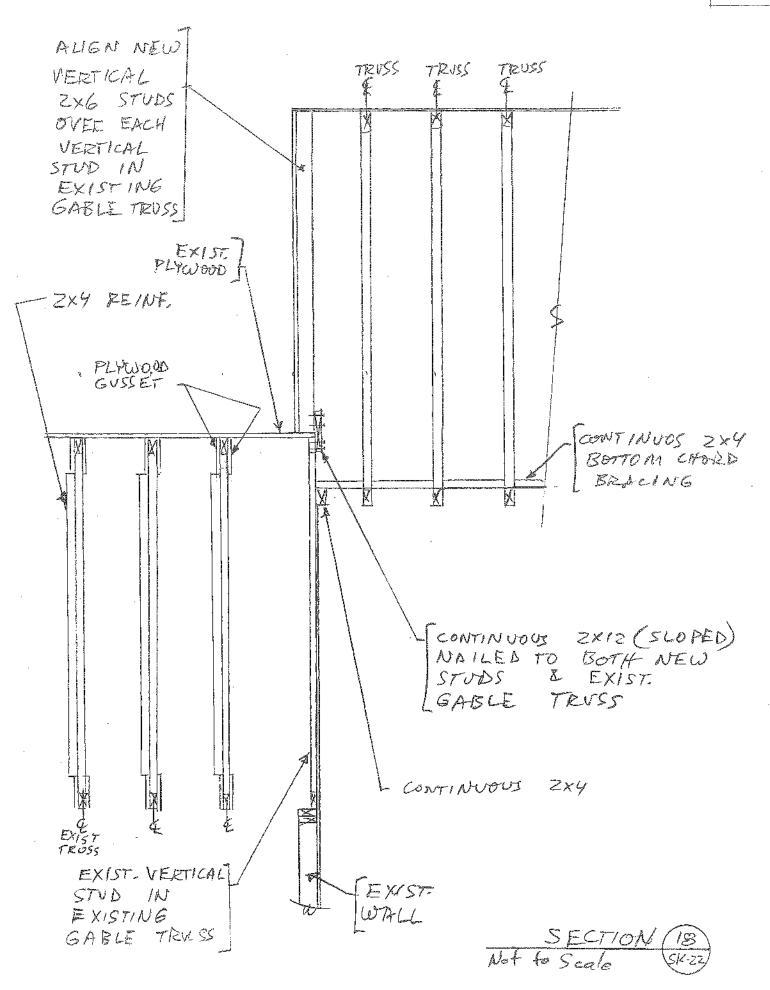
2x4 bracing w/24" lap

spline & anchored at
end gable walls.

TYPE IT TRUSS PROFILE (15)







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WILLIAM GRIBIZIS

1.	1ST FLOOR WILL BE USED AS STORAGE AND FLOOR WILL HAVE TWO OFFICES
2.	B. TOTAL BUILDING AREA 1600 S/F
3.	Nowe
4.	Nowe
5.	City water, Septic
_6-	THE 11 YRS I HAVE OWNED THE PROPORTY THERE HAS BEEN NO PROBLEM WITH SURFACE WATER RUN OFF.
7.	See PLAN
8.	NONE
9,	THERE WILL BE NO FINANCING
10.	
U	ASA NO

7. BORROWER'S FAILURE TO PAY AS REQUIRED

(A) Late Charges for Overdue Payments calen-If the Note Holder has not received the full amount of any monthly payment by the end of ____

dar days after the date it is due, I will pay a late charge to the Note Holder. The amount of the charge will be _ my overdue payment of principal and interest. I will pay this late charge promptly but only once on each late payment.

If I do not pay the full amount of each monthly payment on the date it is due, I will be in default.

If I am in default, the Note Holder may send me a written notice telling me that if I do not pay the overdue amount by a certain date, the Note Holder may require me to pay immediately the full amount of principal which has not been paid and all the interest that I owe on that amount. That date must be at least 30 days after the date on which the notice is delivered or mailed to me.

Even if, at a time when I am in default, the Note Holder does not require me to pay immediately in full as described above. the Note Holder will still have the right to do so if I am in default at a later time.

If the Note Holder has required me to pay immediately in full as described above, the Note Holder will have the right to be (E) Payment of Note Holder's Costs and Expenses paid back by me for all of its costs and expenses in enforcing this Note to the extent not prohibited by applicable law. Those expenses include, for example, reasonable attorneys' fees.

Unless applicable law requires a different method, any notice that must be given to me under this Note will be given by deliver-8. GIVING OF NOTICES ing it or by mailing it by first class mail to me at the Property Address above or at a different address if I give the Note Holder a notice of my different address.

Any notice that must be given to the Note Holder under this Note will be given by mailing it by first class mail to the Note Holder at the address stated in Section 3 (A) above or at a different address if I am given a notice of that different address.

9. OBLIGATIONS OF PERSONS UNDER THIS NOTE If more than one person signs this Note, each person is fully and personally obligated to keep all of the promises made in this Note, including the promise to pay the full amount owed. Any person who is a guarantor, surety or endorser of this Note is also obligated to do these things. Any person who takes over these obligations, including the obligations of a guarantor, surety or endorser of this Note, is also obligated to keep all of the promises made in this Note. The Note Holder may enforce its fights under this Note against each person individually or against all of us together. This means that any one of us may be required to pay all of the amounts owed under this Note.

I and any other person who has obligations under this Note waive the rights of presemment and notice of dishonor. "Presentment" means the right to require the Note Holder to demand payment of amounts due. "Notice of dishonor" means the right to require the Note Holder to give notice to other persons that amounts due have not been paid.

11. UNIFORM SECURED NOTE

This Note is a uniform instrument with limited variations in some jurisdictions. In addition to the protections given to the Note Holder under this Note, a Mortgage, Deed of Trust or Security Deed (the "Security Instrument"), dated the same date as this Note, protects the Note Holder from possible losses which might result if I do not keep the promises which I make in this Note. That Security Instrument describes how and under what conditions I may be required to make immediate payment in full of all amounts I owe under this Note. Some of those conditions are described as follows:

Transfer of the Property or a Beneficial interest in Borrower. If all or any part of the Property or any interest in it is sold or transferred (or if a beneficial interest in Borrower is sold or transferred and Borrower is not a natural person) without Lender's prior written consent, Lender may, at its option, require immediate payment in full of all sums secured by this Security Instrument. However, this option shall not be exercised by Lender if exercise is prohibited by federal law as of the date of this Security Instrument. Lender also shall not exercise this option if: (a) Borrower causes to be submitted to Lender information required by Lender to evaluate the intended transforce as if a new loan were being made to the transferee; and (b) Lender reasonably determines that Lender's security will not be impaired by the loan assumption and that the risk of a breach of any covenant or agreement in this Security Instrument is acceptable to Lender.

To the extent permitted by applicable law, Lender may charge a reasonable fee as a condition to Lender's consent to the loan assumption. Lender may also require the transferee to sign an assumption agreement that is acceptable to Lender and that obligates the transferee to keep all the promises and agreements made in the Note and in this Security Instrument. Borrower will continue to be obligated under the Note and this Security Instrument unless Lender releases Borrower in writing.

If Lender exercises the option to require immediate payment in full, Lender shall give Borrower notice of acceleration. The notice shall provide a period of not less had 30 days from the date the notice is delivered or mailed within which Borrower must pay all sums secured by the Security Instrument. If Borrower fails to pay these sums prior to the expiration of this period, Lender way invoke any remedies permitted by this Security

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Bill Gribizis

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CITY OF PORTLAND

January 2, 1998

William Gribizis 285 Clifton Street Portland, ME 04103

Re:

1021 Ocean Avenue

Dear Mr. Gribizis:

On January 2, 1998 the Portland Planning Authority granted minor site plan approval for a 4,000 sq. ft. addition to the existing garage at 1021 Ocean Avenue with the following condition:

that the applicant utilize crushed stone material for the proposed driveway.

The approval is based on the submitted site plan. If you need to make any modifications to the approved site plan, you must submit a revised site plan for staff review and approval.

Please note the following provisions and requirements for all site plan approvals:

- 1. The site plan approval will be deemed to have expired unless work in the development has commenced within one (1) year of the approval or within a time period agreed upon in writing by the City and the applicant. A one year extension may be granted by this department if requested by the applicant in writing prior to the expiration date of the site plan.
- A performance guarantee in a form acceptable to the City of Portland and an inspection fee equal to 1.7% of the performance guarantee will have to be posted before beginning any site construction or issuance of a building permit.
- 3. A defect guarantee, consisting of 10% of the performance guarantee, must be posted before the performance guarantee will be released.
- 4. Prior to construction, a preconstruction meeting shall be held at the project site with the contractor, development review coordinator, Public Work's representative and owner to review the construction schedule and critical aspects of the site work. At that time, the site/building contractor shall provide three (3) copies of a detailed construction schedule to the attending City representatives. It shall be the contractor's responsibility to arrange a mutually agreeable time for the preconstruction meeting.

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- 5. If work will occur within the public right-of-way such as utilities, curb, sidewalk and driveway construction, a street opening permit(s) is required for your site. Please contact Carol Merritt at 874-8300, ext. 8828. (Only excavators licensed by the City of Portland are eligible.)
- The Development Review Coordinator (874-8300 ext. 8722) must be notified five (5) working days prior to date required for final site inspection. Please make allowances for completion of site plan requirements determined to be incomplete or defective during the inspection. This essential as all site plan requirements must be completed and approved by the Development Review Coordinator prior to issuance of a Certificate of Occupancy. <u>Please</u> schedule any property closing with these requirements in mind.

If there are any questions, please contact the Planning Staff.

Sincerely,

Joseph E. Gray, Jr.

. Director of Planning and Urban Development

oc: Alexander Jaegerman, Chief Planner

-Kandice Talbot, Planner

P. Samuel Hoffses, Building Inspector

Marge Schmickal, Zoning Administrator

Tony Lombardo, Project Engineer

Development Review Coordinater

William Bray, Deputy Director/City Traffic Engineer

Jeff Tarling, City Arborist

Matalie Burns, Associate Corporation Counsel

Lt. Gaylen McDougall, Fire Prevention

Mary Gresik, Building Permit Secretary

Kathleen Brown, Director of Economic Development

Susan Doughty, Assessor's Office

Approval Letter File