CONTINUOUS 3 PERIODIC 3 COMMENTS

Χ

By Geotech.

By Geotech.

Ref. Note 6

F'c 2500 psi

A325 89 A490

Ref. Note 4

AND

POR-RETIREMEN' 802 OCEAN AVE. PO

8/28/2015

**REVISED DATE** /1\ 9/22/2015

SHEET

**GENERAL STRUCTURAL NOTES** 

60 psf

100 psf

125 psf

20 psf

40 psf

Pf= 40 psf

Ce= 1.0

I= 1.0

Ct= 1.1

**ROOF TRUSSES:** 

ITEM

CONCRETE

E. Maximum stress load of trusses not to exceed 0.90 CSI for any trusses

Building Code 2010 Edition in accordance with section 1704, shall be

performed by a qualified person who can demonstrate competence for

inspections shall be performed in addition to the inspections required

the particular type of construction being inspected. The special

by the Florida Building Code, The plans and Specifications, The

A. All construction shall be inspected in conformance with the

B. All items noted as required special inspection per the Florida

STRUCTURAL INSPECTION AND TESTING:

Grading, Excavation & Backfill

Final Foundation Inspection

Reinforcing Placement

Reinforcing Welding

Anchor Bolts & Inserts

Concrete Placement

STRUCTURAL STEEL

**Embedded Plates** 

Fillet Welds > 5/16"

SHOP WELDING

FIELD WELDING

Fillet Welds > 5/16"

Prefab. Construction

Plywood Nailing

Holdown Installation

WOOD

High Strength Bolting

**Epoxy Anchor Placement** 

Preparation of Test Specimens

**Expansion Anchor Placement** 

Welding of Anchors & Studs

Welding Stairs/Railing System

Single Pass Fillet Welds ≤ 5/16"

Partial/Complete Penetration

Single Pass Fillet Welds ≤ 5/16"

Partial/Complete Penetration

2009 Edition International Building Code.

Architect of Record, and the Building Officials.

All material and workmanship shall conform to the requirements of the 2009 International Building Code.

**DESIGN LOADS:** 

A. Floor live load Deck live load Common live load Storage live load B. Roof live load C. Roof snow load

Flat-roof snow load Snow exposure factor Snow load importance factor Thermal factor

D. Basic wind speed Importance factor Occupancy category Wind exposure

E. Seismic design category Importance factor Occupancy category Spectral response accelerations Site class Spectral response coefficients Seismic-force-resisting system(s) Design base shear Seismic response coefficient(s)

Response modification factor(s)

Analysis procedure used

Sds= 0.253 Sd1= 0.087 V= 0.039 W Cs = 0.039R= 6.5 Equivalent lateral force procedure

Ss= 0.316 S1= 0.077 ASCE 7-05 Method 12.8:A.13 & G.1

100 mph (3) second gust

A. Contractor shall be responsible for all construction methods, techniques, sequencing and safety required to complete construction. B. Contractor shall verify all dimensions and details prior to proceeding

or Engineer of record. C. Contractor shall verify all required openings on Architectural, Mechanical

# FOUNDATION:

- A. Footing have been designed for a maximum allowable soil bearing

A. Concrete to develop a unit compressive stress of 2,500 psi minimum at

REINFORCING STEEL FOR CONCRETE: A. All reinforcing steel shall be rail steel deformed bars conforming to

grade 60 bars. B. Details of reinforcing steel shall conform to ASTM Manual of Standard practice, Code of Standard Practice for detailing reinforcing materials,

REINFORCING PROTECTION:

C. Slabs = 3/4 inches.

shall be pressured treated.

weights in accordance with ASTM B 695, Class 55 minimum. Connectors that absence of manufacturer's recommendations, a minimum of ASTM A 653,

preservative-treated wood in an interior, dry environment shall be

1-1/2"

FOR 3 PLY 2x6 COLUMNS 2 ROWS

OF 30d COMMON WIRE NAILS

FOR 2 PLY 2x4 COLUMNS 1 ROW

STAGGERED BETWEEN FACES

NAILING (note 1)

2-8d

2-8d

3-8d

16d-16"oc

16d-24" oc

16d-6" oc

3-8d toenail

8d @ 6" oc

2-16d

4-8d

3-16d

3-16d

3-8d

2-8d

2-8d

3-8d

16d-24" oc

4-8d toenail or 2-16d endnail 2-20d nails @ 3x sole plates

16d @ 16" oc along ea. edge

20d @ 32"oc T&B and stagger 2-20d @ ends and at splices

2-16d at each bearing

8d corrosion resist

6d .113"°

8d .131" °

10d .148" °

16d .162" °

20d .192" °

OF 10d COMMON WIRE NAILS

STAGGERED BETWEEN FACES

- with constuction. All discrepancies shall be approved by the Architect
- and Electrical plans.

- pressure of 4,000 psf on native material or properly compacted structural
- B. All footings shall be constructed as shown on the plans and in accordance with S. W. Cole Engineering, Inc. report for project 14-1188s dated 01/16/2015.
- C. No excavation shall be made below any footing closer than one to one slope to the bottom of same.
- D. All bottom of exterior footings to be a minimum of 54" below finish grade. E. Back fill all pipe trench excavations below footings with lean concrete to the bottom of the footings.

28 days per I.B.C. section 1905 with 5 sacks of cement/ cubic yd. minimum.

ASTM A615, grade 60 bars except where welding is required use A706,

by CTSI and WCRSI (latest edition).

C. All concrete slab reinforcing steel shall be supported at the required D. Provide 2'-0" X 2'-0" corner bars for all horizontal wall steel at all

corners and intersections. E. Splices shall be lapped 36 bar diameters or 2'-0" minimum unless detailed otherwise.

A. Concrete deposited against earth = 3 inches B. Concrete formed surfaces exposed to ground or weather: #5 rebar and smaller = 1 1/2 inches. #6 rebar and larger = 2 inches.

A. All anchor bolts to be ASTM A-307 minimum. B. All wedge anchors to be Simpson Wedge-all or approved equivalent.

STRUCTURAL WOOD:

A. All sawn lumber, excluding stud wall framing, to be #1/#2 grade SPF, except 4x lumber to be No. 2 Douglas Fir Larch, 6x lumber or larger to be No. 1 Douglas Fir Larch, per I.B.C. section 2303.1.1.. All stud wall framing to be stud grade SPF or better.

B. The contractor shall furnish and install all bolts and plates as required to complete the job. C. All bolt heads and nuts bearing on wood to be provided with standard

cut washers, except where otherwise shown. D. All wood members in contact with concrete, masonry or exposed to weather

dipped zinc coated galvanized steel, silicon bronze or copper. Fasteners other than nails, timber rivets, wood screws and lag screws shall be permitted to be of mechanically deposited zinc-coated steel with coating are used in exterior applications and in contact with preservativetreated wood shall have coating types and weights in accordance with the treated wood or connector manufacturer's recommendations. In the type G185 zinc-coated galvanized steel, or equivalent, shall be used. Exception: Plan carbon steel fasteners in SBX/DOT and zinc borate

F. All nailing not shown shall be called for in I.B.C. table 2304.9.1, fastening

ROOF COMPONENT AND CLADDING (LOADING DIAGRAM)

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FRAMING MEMBER

NAILS @ PANEL FIELD

-NAILS @ DIAPHRAGM

 $\bigcirc$ 

FOR 4 PLY 2x6 COLUMNS 1/2" DIA.

**BOLTS POSITIONED AS SHOWN** 

1-1/2"

FOR 2 PLY 2x6 COLUMNS 2 ROWS

OF 10d COMMON WIRE NAILS

STAGGERED BETWEEN FACES

**BUILT UP COLUMN** 

NO. CONNECTION

Joist to sill or girder, toenail

Bridging to joist, toenail each end

1x6" subfir or less to each joist, face nail

2" subfir to joist or girder, blind & face nail

Sole plate to joist/blocking, face nail

Doubled top plates, typical face nail

Blocking between joists/rafters to top plate

Top plates, laps and intersections, face nail

Ceiling joists, laps over partitions, face nail

1" brace to each stud and plate, face nail

1x8" shthg or less to each bearing, face nail

Wider than 1x8" shthg to each bearing, face nail

Wood structural panels and particleboard (note 2)

Combination subfloor-underlayment to framing

NAILING SCHEDULE

Subfloor, roof and wall sheathing to framing

Ceiling joist to parallel rafters, face nail

Double top plates, at lap splices

Continuous header, two pieces

Ceiling joists to plate, toenail

Rafter to plate, toenail

Built up corner studs

1/2" and less

1-1/8" - 1-1/4"

3/4" and less

1-1/8" - 1-1/4"

1/2" and less

1. Refer to nail diameters for nail size.

2. Nail spaced 4" oc edges, 10"oc at intermediate

diaphragms and shear walls, refer to schedules.

supports, except 4" oc at all supports where spans

are 48" or more. For nailing of plyw'd and particleboard

Panel siding to framing

7/8" - 1"

2" planks

Built up girder and beams

Continuous header to stud, toenail

Top plate to stud, end nail

Double studs, face nail

Rim joist to top plate

Stud to sole plate

Wider than 1x6" subfloor to ea. joist, face nail

(NDS 15.3.2 REQUIREMENTS)

NAILING SCHEDULE

FOR 4 PLY 2x4 COLUMNS 1/2" DIA.

BOLTS AT CENTER LINE SPACE AS

ROOF - 12" o.c.

ROOF - 6" o.c.

- NAILS @ PANEL

ROOF - 6" o.c.

\*\*SEE SHEET VG4 FOR

9-1/2"

12"

14-1/2"

12"

6-1/2"

9-1/2"

- STRAP AT DBL. TOP PLATE

PER DETAIL

BATT. INSULATION

2 ROWS 16d @ 24" O.C.

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ASSEMBLY "C"

THROUGH BOLTED

12" O.C.

1010

760

680

CONNECTION

2 ROWS 1/2" 2 ROWS 1/2"

BOLTS AT AT BOLTS AT AT

24" O.C.

380 340

4 PCS. 1-3/4"

SHEATHING GRADE

ROOF FRAM. - 8d @ ROOF

SLOPES OVER 2:12 AND

10d @ ROOF SLOPES EQ.

TO OR LESS THAN 2:12

DIAPHRAGM BOUNDARY

SCALE: 1/2" = 1'-0"

**GAUGE** 

PENETRATION

12"

10"

10"

(2) ROWS 16d @ 24" O.C.-

COMMON\_

NAIL

**SPACING** 

DIAPHRAGM NAILING

\*\* VALID FOR LATERAL LOADS ONLY \*\*

3-1/2"

6-1/2"

8-1/2"

10"

2-1/2"

5-1/2"

6-1/2"

3-1/2"

4-1/2"

5-1/2"

6-1/2"

1. PENETRATION IS THE DEPTH OF EMEDMENT OF THE STAPLE INTO THE MAIN MEMBER

REQUIRED TO ATTAIN ITS FULL CAPACITY (SHEAR VALUE) FOR LATERAL LOADING.

2. TABLE IS ONLY INTENDED FOR USE IN DIAPHRAGM NAILING OF ROOF, FLOOR, AND

STAPLES AND NAILS

(TABLE OF EQUIVALENT)

**TYPICAL CORNER** 

ASSEMBLY "B"

NAILED CONNECTION

2 ROWS 16d

AT 12" O.C.

COMMON WIRE

3 PCS. 1-3/4"

SCALE: 1" = 1'-0"

ASSEMBLY "A"

ASSEMBLY

NUMBER

2 PCS. 1-3/4"

EQUIVAILENT O.C. SPACING OF STA[LES

10"

3-1/2"

6-1/2"

10"

2-1/2"

3-1/2"

NAILING DIAMETERS

G. Roof sheathing shall be 15/32" CDX plywood with a span rating of 24/16.

applicable workmanship provisions of the code. Special inspection testing requirements apply equally to all bidder designed components. 2. Special inspection is not required for work performed by an approved E. Fasteners in contact with preservative-treated wood shall be of hotfabricator per 2010 F.B.C. section 1704.2.2. 3. Continuous special inspection means that the special inspector is on site at all times observing the work requiring the special inspection (2010 F.B.C. section 1704). Periodic special inspection means that the special inspector is on the site at the time intervals necessary to confirm that all work requiring special inspection is in compliance. 4. All welds shall be visually inspected. 5. All complete penetration welds shall be tested ultrasonically or by using approved method. 6. Periodic special inspection is only required for welding of ASTM A706 reinforcing steel not greater then No. 5 used for embedments, provided the materials, qualifications of welding procedures and welders are

verified prior to the start of work: periodic inspections are made of work in process: and a visual inspection of all welds is made prior to completion or prior to shipment of shop welding. 7. Inspection for prefabricated construction shall be the same as if the

material used in the construction took place on site. Continuous inspection will not be required during prefabrication if the approved agency certifies the construction and furnishes evidence of compliance.

1. The items marked with a 'X' shall be inspected in accordance with 2010

F.B.C. Section 1704 by a certified special inspector from an established

test agency. For material sampling and testing requirements, refer to the

material sampling and testing section, the project specifications and the specific general notes sections. The testing agency shall send copies

engineer, contractor and building official. Any materials which fail to meet

of all structural testing and inspection reports directly to the Architect,

the project specifications shall immediately be brought to the attention

proper design authority and to the building official. The special inspector

of the architect. All discrepancies shall be brought to the immediate

attention of the contractor for correction, then, if uncorrected, to the

shall submit a final signed report stating whether the work requiring

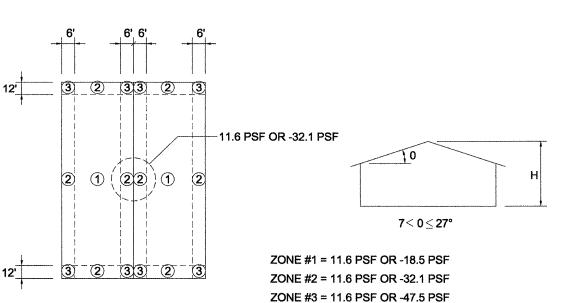
special inspection was to the best of the inspector's knowledge, in

conformance with the approved plans and specifications and the

Snug tight. 9. Turn of the nut method.

C. Contractor to retain an approved special inspector to observe and approve all high strength bolt installations.

ZONE #4 = 20.2 PSF OR -21.8 PSF ZONE #5 = 20.2 PSF OR -27.0 PSF



**GABLE ROOF** SCALE: N.T.S.

**MULTIPLE MEMBER BEAMS** (SIDE LOADED CONNECTION)

COMMON WIRE

530

AT 12" O.C.

(MINIMUM REQUIREMENTS)

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Ref. Note 4 Ref. Note 5 Ref. Note 4 Ref. Note 4 Ref. Note 5 Ref. Note 7 Χ

