

GENERAL NOTES

A. DESIGNED IN ACCORDANCE WITH THE ME STATE BUILDING CODE, CONSISTING OF THE IBC 2003 BUILDING CODE, WITH STATE AMENDMENTS.

B. SOIL BEARING CAPACITY USED IN DESIGN OF NEW FOUNDATIONS: 2,000 PSF, BASED ON A GEOTECHNICAL ENGINEERING REPORT BY S.W. COLE ENGINEERING, INC. ALL FOOTINGS TO BEAR A MINIMUM OF 4'-6" BELOW FINISHED GRADE ON PREPARED NATIVE SOILS OR STRUCTURAL FILL (12" MIN. UNDERLIFT IN ROCK) HAVING THE SAME BEARING CAPACITY AS ASSUMED IN THE FOUNDATION DESIGN. IF UNSUITABLE SOILS ARE ENCOUNTERED, OR IF ROCK IS ENCOUNTERED PRIOR TO THE PROPOSED BOTTOM OF FOOTING, NOTIFY THE ENGINEER IMMEDIATELY.

C. STRUCTURE DESIGNED FOR THE FOLLOWING LOADS:

ROOF DEAD LOADS:	FLOOR LIVE LOADS:	= 125 PSF
MEMBRANE, INSULATION & DECK = 5 PSF	LIGHT STORAGE	= 10 PSF
COLLATERAL ALLOWANCE = 5 PSF	WIND LOADS:	BASIC WIND SPEED (V) = 100 MPH
BAR JOISTS = 5 PSF	EXPOSURE = "B"	IMPORTANCE FACTOR = 1.0
SUBTOTAL TO BAR JOISTS = 15 PSF	SEISMIC DESIGN DATA:	SEISMIC USE GROUP = 1
BEAMS AND GIRDERS = 15 PSF	SEISMIC DESIGN CATEGORY = "C"	SITE SOIL CLASS = "D"
TOTAL ROOF = 20 PSF	SEISMIC DESIGN CATEGORY = "C"	S _s = 0.373
		S ₁ = 0.100

ROOF LIVE LOADS:

GROUND SNOW LOAD = 70 PSF	FLOOR ROOF SNOW LOAD = 50 PSF (MODIFIED FOR DRIFTING AT ROOF PARAPETS).
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FLOOR DEAD LOADS:

SLAB & DECK = 45 PSF	COLLATERAL ALLOWANCE = 5 PSF PARTITIONS
BAR JOISTS = 5 PSF	TOTAL FLOOR = 55 PSF

BASIC STRUCTURAL SYSTEM AND SEISMIC RESISTING SYSTEM:
STRUCTURAL STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE.

ANALYZED USING THE EQUIVALENT LATERAL FORCE PROCEDURE.

D. IF ANY FIELD CONDITIONS PRECLUDE COMPLIANCE WITH THESE DRAWINGS AND/OR CONDITIONS SPECIFIED, THE CONTRACTOR SHALL NOTIFY THE ENGINEER IMMEDIATELY AND SHALL NOT PROCEED WITH THE AFFECTED WORK. THE CONTRACTOR SHALL VERIFY ALL BUILDING DIMENSIONS, DETAILS, AND CONDITIONS PRIOR TO START OF CONSTRUCTION THAT MAY BE IMPACTED BY VARIATIONS FROM THE CONDITIONS SHOWN HEREIN.

E. ALL COMPACTED FILL BEING PLACED MUST BE TESTED BY AN APPROVED TESTING LABORATORY DURING THE FILL OPERATION. SLABS AND FOOTINGS SHALL NOT BE PLACED UNTIL FILL TEST REPORTS HAVE BEEN CHECKED AND APPROVED BY THE GEOTECHNICAL ENGINEER. PRIOR TO PLACING CONCRETE, EXISTING SOIL CONDITIONS SHALL BE INSPECTED AND VERIFIED BY THE GEOTECHNICAL ENGINEER. ALL UNSUITABLE SOIL SHALL BE REMEDIATED AS DIRECTED BY THE SOILS ENGINEER. BUILDING PAD PREPARATION SHALL BE IN ACCORDANCE WITH THE SITE DRAWINGS AND SPECIFICATIONS.

SITE NOTES

A. VERIFY ALL UNDERGROUND UTILITY LOCATIONS PRIOR TO STARTING EXCAVATION WORK.

B. WORK AREAS SHALL BE MARKED, FENCED AND OTHERWISE SECURED SO AS TO PROVIDE PROPER PROTECTION FOR THE PUBLIC AND AS REQUIRED BY THE BUILDING INSPECTOR.

MATERIAL NOTES

CONCRETE SPECIFICATIONS:

A. ALL CONCRETE WORK SHALL CONFORM WITH A.C.I. "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE", ACI 318 - LATEST EDITION, AND "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES", ACI 315 - LATEST EDITION.

B. ALL CONCRETE FORM WORK SHALL CONFORM WITH A.C.I. "RECOMMENDED PRACTICES FOR CONCRETE FORM WORK" - ACI 304.

C. ALL CONCRETE SHALL HAVE MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS OF 3000 PSI. ALL STRUCTURAL CONCRETE SHALL CONFORM WITH A.C.I. "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" - ACI 301.

D. CONTRACTOR SHALL SUBMIT MIX DESIGNS FOR APPROVAL. MIX DESIGN SHALL INDICATE 7 AND 28 DAYS STRENGTHS, CEMENT CONTENT, AIR CONTENT, WATER-CEMENT RATIO, SLUMP, AMOUNT OF FINE AND COARSE AGGREGATES AND ADMIXTURES. ALL EXTERIOR CONCRETE AND CONCRETE EXPOSED TO WEATHER SHALL BE AIR-ENRICHED (5% TO 7%).

E. MAXIMUM WATER-CEMENT RATIO = 0.53
MAXIMUM SLUMP LIMIT = 4"
MAXIMUM AGGREGATE SIZE:
FOOTINGS & FOUNDATIONS = 3/4" TO 1 1/2"
SLAB-ON-GRADE = 3/4" TO 1"
CONCRETE FILL = 1/2"

F. CEMENT SHALL BE PORTLAND CEMENT, TYPE I OR II, CONFORMING TO ASTM-C-150.
CONCRETE AGGREGATES SHALL CONFORM TO ASTM C-33.
AIR-ENRICHING ADMIXTURES SHALL CONFORM TO ASTM C-260.
NON-SHRINK GROUT SHALL CONFORM TO ASTM C-109.
EXPANSION JOINTS SHALL BE 1/2" THICK ASPHALT IMPREGNATED FIBERBOARD JOINT MATERIAL CONFORMING TO ASTM D-1751.
CURING COMPOUND SHALL BE CLEAR, CONFORMING TO ASTM C-309.

G. COLD WEATHER CONCRETE WORK, WHEN APPLICABLE, SHALL CONFORM TO ACI 306.
F. HOT WEATHER CONCRETE WORK, WHEN APPLICABLE, SHALL CONFORM TO ACI 305.
G. ALL FLOOR SLABS SHALL BE CONSTRUCTED TO HAVE A MINIMUM FLATNESS OF F1=50 AND A MINIMUM LEVELNESS OF F2=25.

H. ALL BAR REINFORCING FOR CONCRETE TO CONFORM TO ASTM A615, GRADE 60. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185.

I. CONCRETE ACCESSORIES TO BE ADEQUATE TO MAINTAIN REINFORCING ACCURATELY IN PLACE AND BE NON-CORROSIVE, NON-STAINING TYPE.

J. LAP ALL BAR REINFORCING PER ACI 318. STAGGER SPICES IN HORIZONTAL WALLS AND SLABS.

K. REINFORCEMENT COVER, UNLESS NOTED OTHERWISE:
1) FOOTINGS AND GRADE BEAMS - BOTT. 3"
2) WALLS/PIERS - OUTSIDE 2", INSIDE 1"
3) SLABS - 1 1/2" FROM TOP

L. WELDED WIRE FABRIC SHALL HAVE MINIMUM END AND SIDE LAPS OF 1'-0".

M. ALL AROUND OPENINGS IN CONCRETE SLABS AND WALLS, ADD (4)-#5 DIAGONAL BARS EXTENDING 2'-0" BEYOND EACH SIDE OF OPENING.

N. ALL CONCRETE WORK, REINFORCING PLACEMENT, FORM WORK AND SHORING SHALL BE INSPECTED BY AN INDEPENDENT TESTING AGENCY RETAINED BY THE OWNER FOR THE FOLLOWING ITEMS:
INSPECT BOTTOM OF FOOTING SOIL CONDITIONS.
INSPECT FORM WORK AND PLACEMENT OF REINFORCEMENT.
SAMPLING FRESH CONCRETE PER ASTM C-172, MODIFIED FOR SLUMP BY ASTM C-94.
SLUMP PER ASTM C-143, ONE TEST AT POINT OF DISCHARGE FOR EACH DAY'S POUR FOR EACH TYPE OF CONCRETE.
AIR CONTENT PER ASTM C-173 (VOLUMETRIC) OR ASTM C-231 (PRESSURE). ONE TEST FOR EACH DAY'S POUR FOR EACH TYPE OF CONCRETE.
CONCRETE TEMPERATURE - TEST EACH TIME A SET OF COMPRESSION TEST IS TAKEN.
COMPRESSION TEST SPECIMEN PER ASTM C-31 (ONE SET OF 4 STANDARD CYLINDERS)
COMPRESSION STRENGTH TEST PER ASTM C-39. (ONE SET FOR EACH DAY'S POUR).
PLUS ADDITIONAL SETS FOR EACH 50 CY OVER AND ABOVE THE FIRST 25 CY.
ONE SPECIMEN TESTED AT 7 DAYS, TWO SPECIMENS TESTED AT 28 DAYS, AND ONE SPECIMEN RETAINED IN RESERVE FOR LATER TESTING IF REQUIRED.

STRUCTURAL MASONRY SPECIFICATIONS:

A. MASONRY WALLS (AS DESIGNATED ON DWGS) ARE DESIGNED IN ACCORDANCE WITH ACI 530/ASCE 5/TMS 402.

B. CONSTRUCTION SHALL CONFORM TO ACI 530/ASCE 6/TMS 602 - LATEST EDITION. "SPECIFICATIONS FOR MASONRY STRUCTURES." ALL MASONRY CONSTRUCTED SHALL BE INSPECTED AND TESTED IN ACCORDANCE WITH THE SPECIAL PROVISIONS.

C. MASONRY UNITS SHALL CONFORM TO ASTM C90 WITH COMPRESSIVE STRENGTH OF 1900 PSI, LIGHTWEIGHT BLOCK. NET MASONRY STRENGTH USED FOR DESIGN = 1500 PSI.

D. MASONRY MORTAR SHALL BE TYPE "M" OR "S" CONFORMING TO ASTM C270.

E. GROUT SHALL CONFORM TO ASTM C94 OR C476, WITH A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS AND A SLUMP BETWEEN 8-10 INCHES.

F. ALL BAR REINFORCING FOR MASONRY SHALL CONFORM TO ASTM A615, GRADE 60.

G. ALL REBAR SHALL BE GROUTED FULL HEIGHT USING LOW-LIFT METHOD.

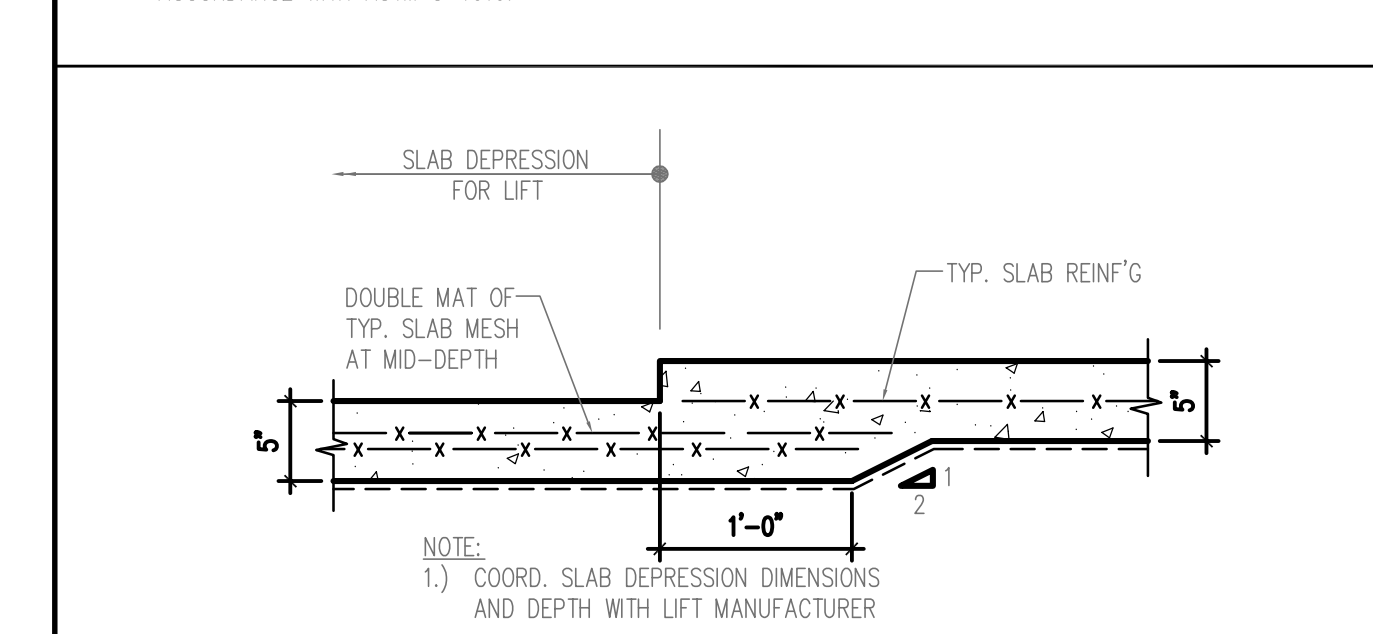
H. ALL BOND BEAMS SHALL BE GROUTED SOLID.

I. PROVIDE GALV. No. 9 WIRE STD. TRUSS-TYPE HORIZONTAL REINFORCEMENT @ 24" OC TYPICAL, ABOVE AND BELOW OPENINGS AND AT HORIZONTAL JOINTS WITH WALL TIES TO STEEL BEAMS, UNLESS OTHERWISE NOTED. (HORIZONTAL REINFORCEMENT TO BE BY "DUR-O-WALL", OR APPROVED EQUAL.)

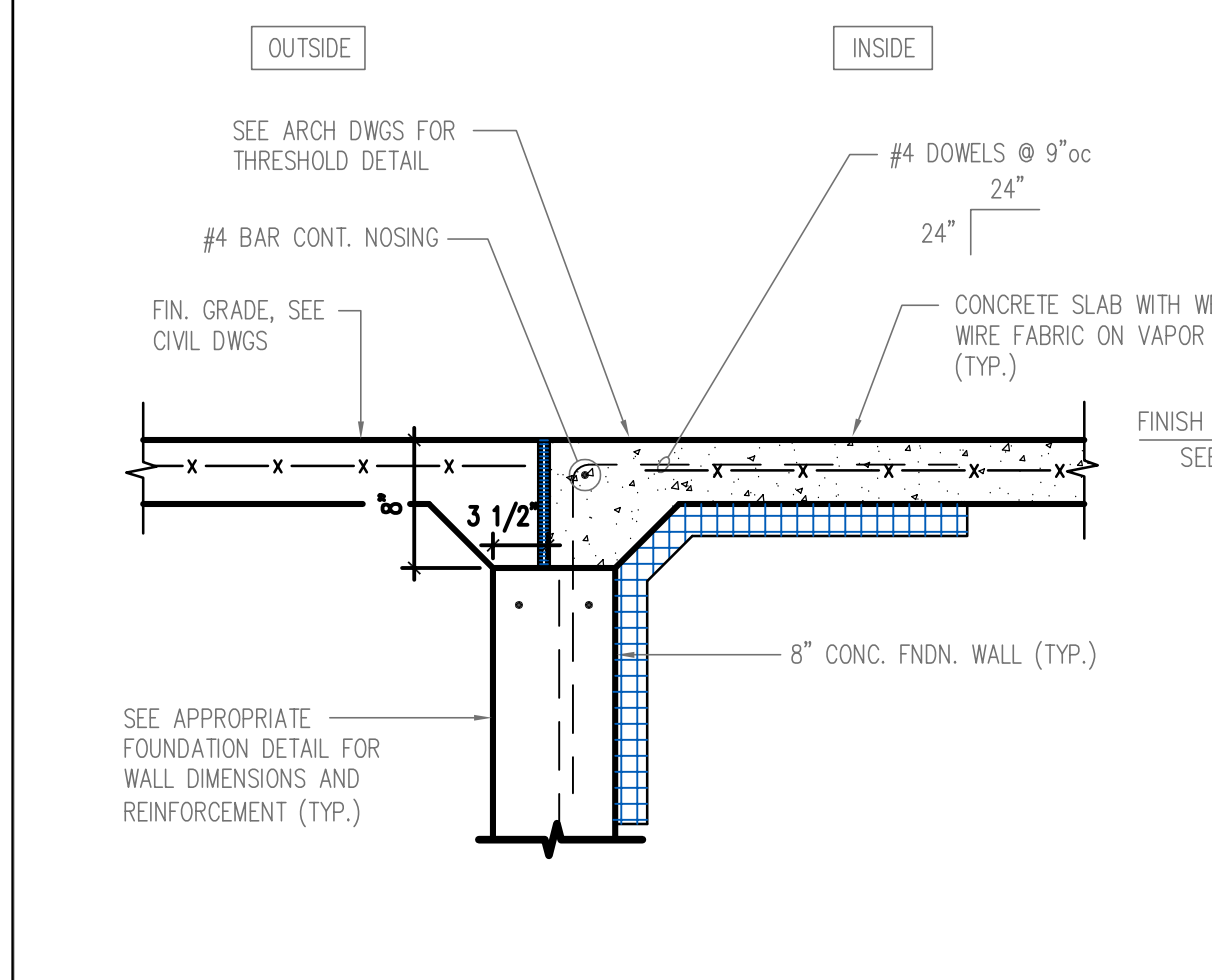
J. ALL BELOW GRADE MASONRY SHALL BE GROUTED SOLID.

K. CONTRACTOR SHALL SUBMIT MATERIAL CERTIFICATIONS FOR CONCRETE MASONRY UNITS, REINFORCEMENT AND MIX DESIGNS FOR APPROVAL.

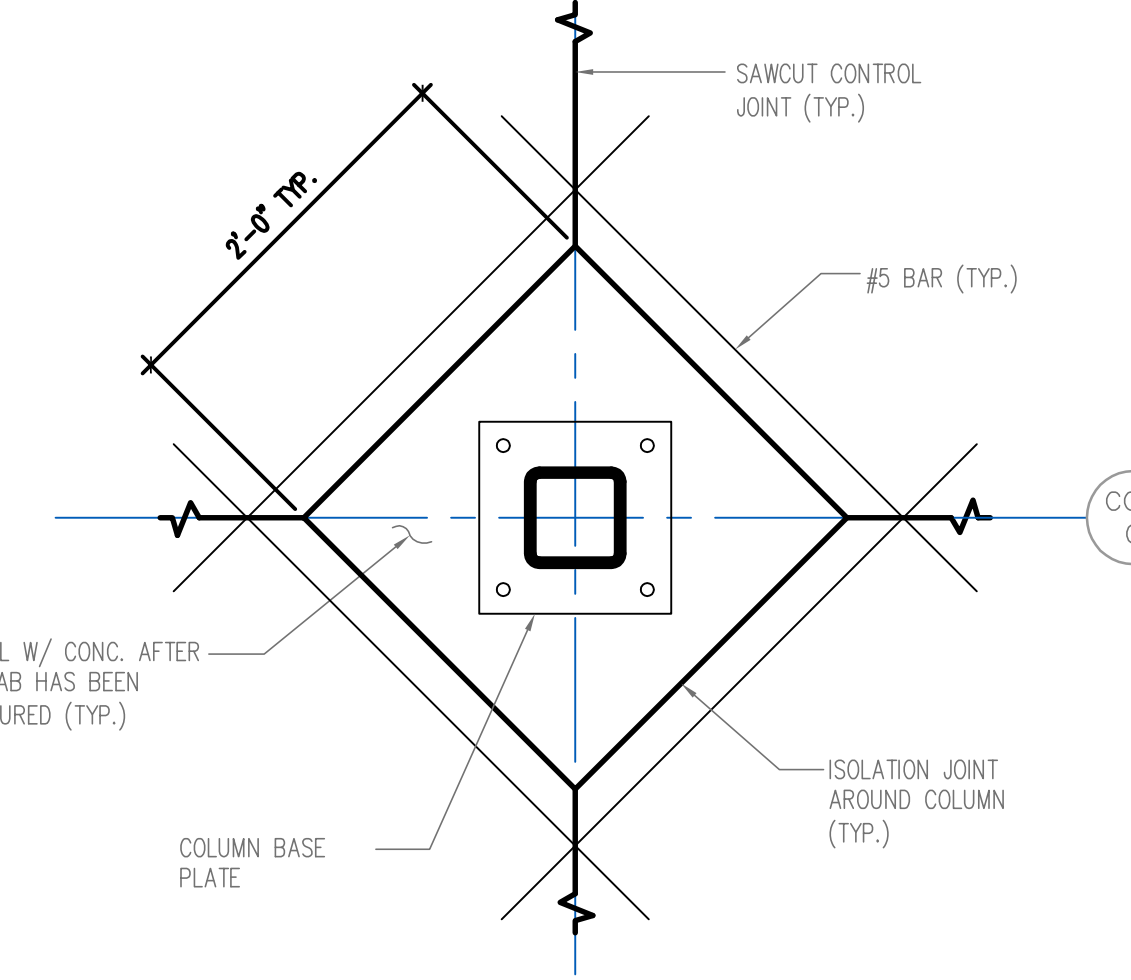
L. ALL STRUCTURAL MASONRY WORK, REINFORCING PLACEMENT AND SHORING SHALL BE INSPECTED BY AN INDEPENDENT TESTING AGENCY RETAINED BY THE OWNER FOR THE FOLLOWING ITEMS:
REVIEW MATERIAL CERTIFICATES FOR BLOCK, REINFORCEMENT, MORTAR AND GROUT.
INSPECT PLACEMENT OF REINFORCEMENT, INCLUDING FOUNDATION DOWELS. TEST AND EVALUATE MORTAR IN ACCORDANCE WITH ASTM C-760. TEST AND EVALUATE GROUT IN ACCORDANCE WITH ASTM C-1019.



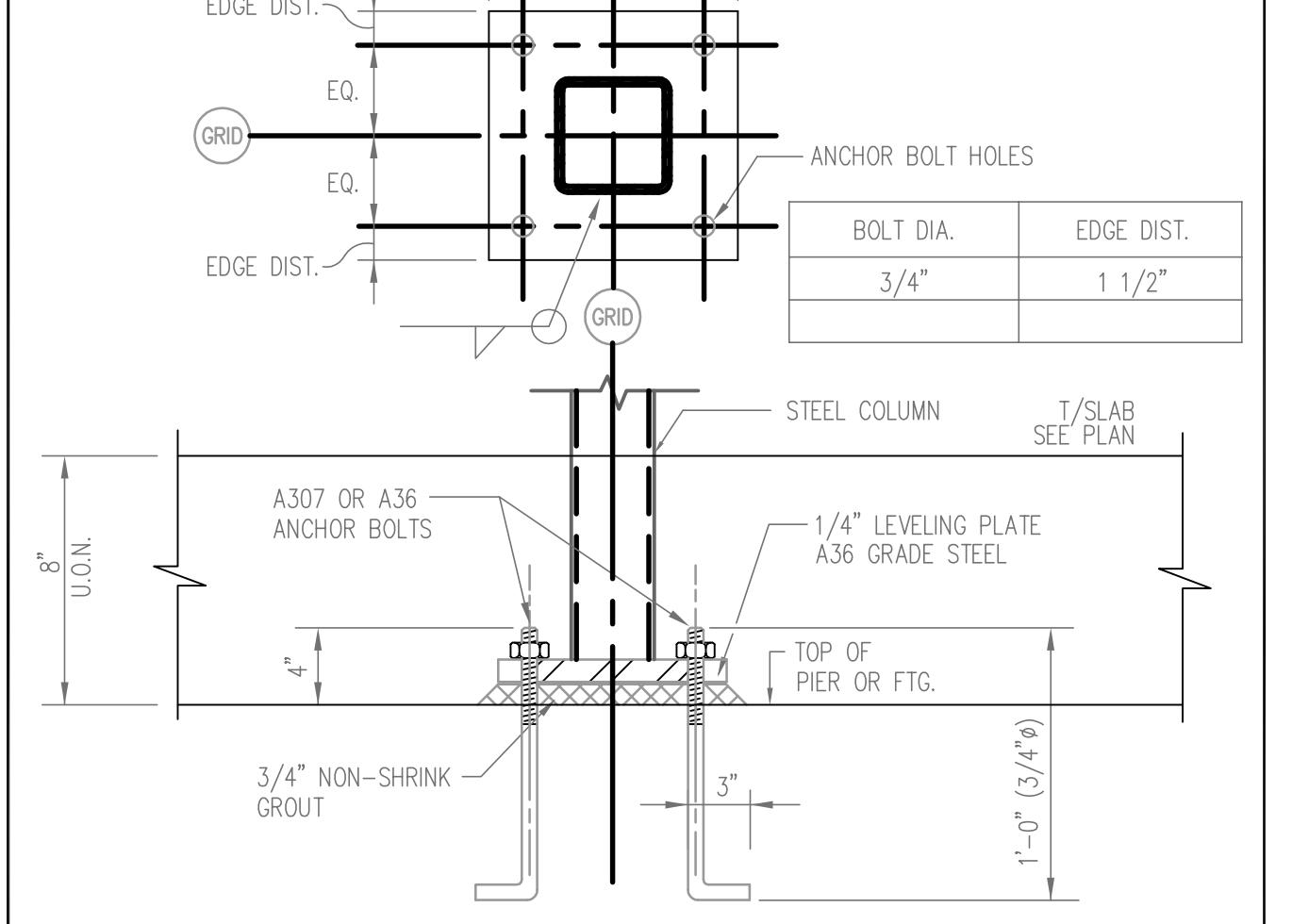
A14 SLAB DEPRESSION DETAIL
N.T.S.



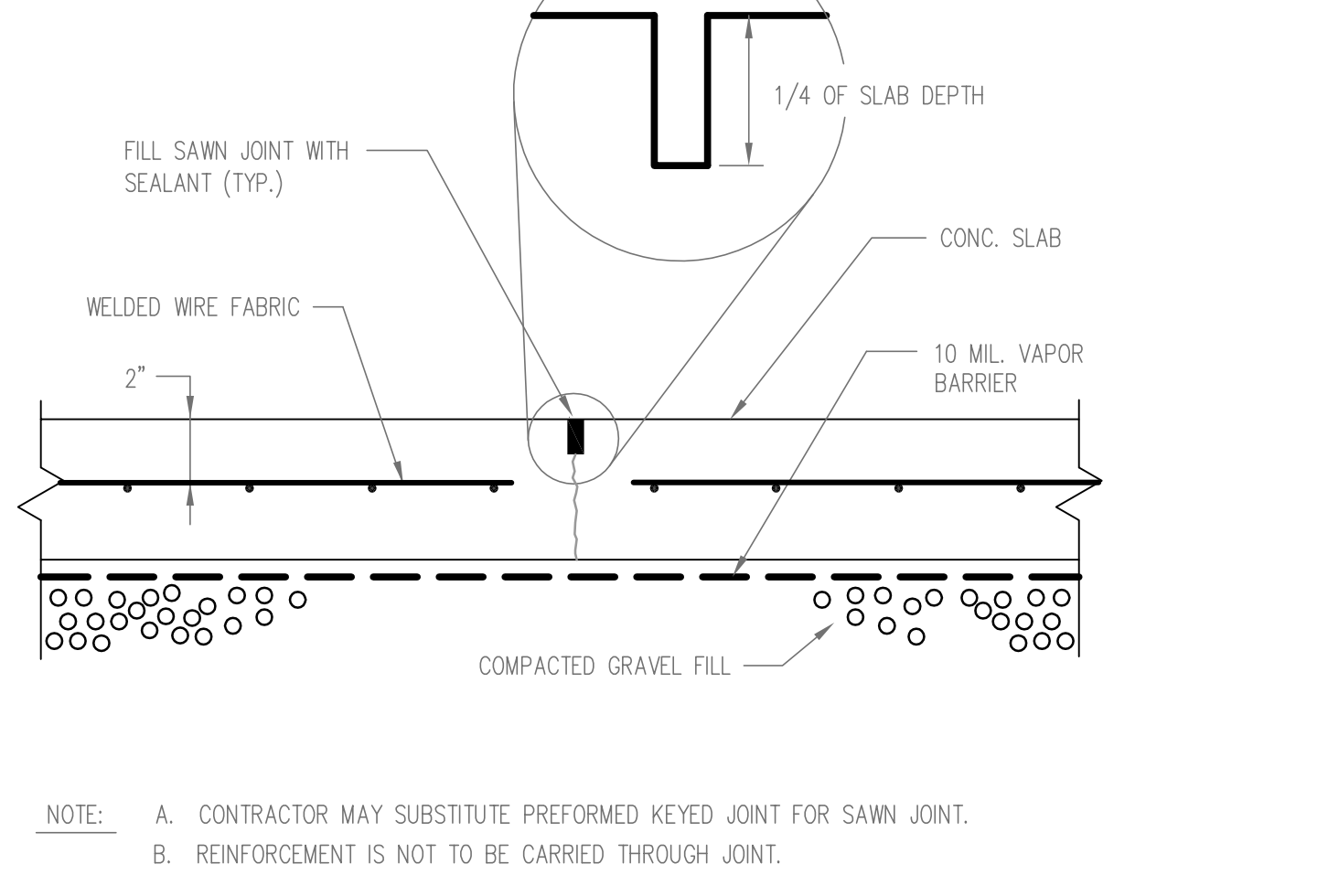
K11 SLAB AT DOOR DETAIL
N.T.S.



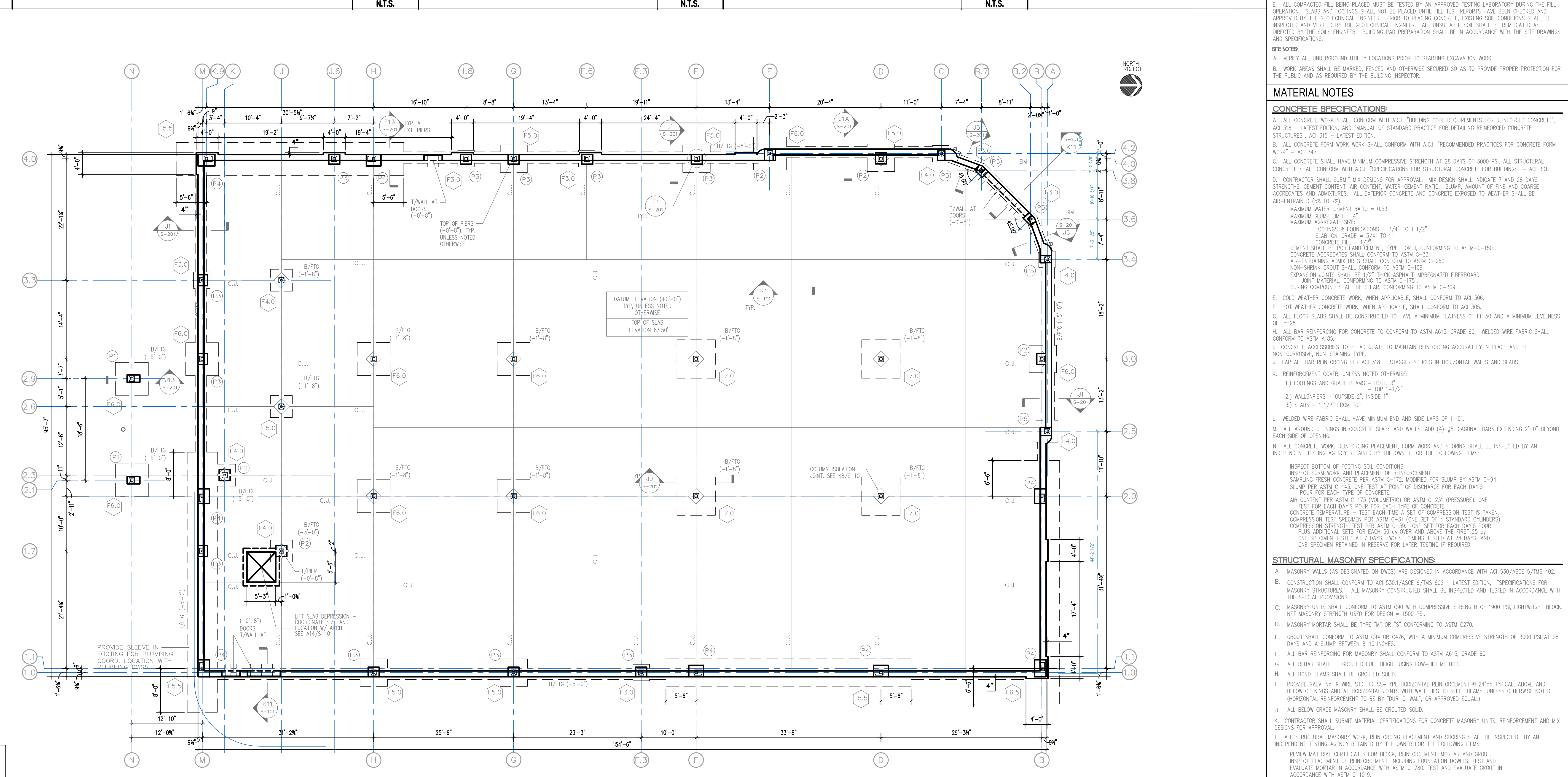
K8 COLUMN ISOLATION JOINT
N.T.S.



K5 TYPICAL BASE PLATE DETAIL
N.T.S.



K1 SLAB CONTROL JOINT
N.T.S.



FOUNDATION NOTES:

- SLAB-ON-GRADE SHALL CONSIST OF 5" CONCRETE SLAB REINFORCED WITH 6x6-W1.4W1.4 WELDED WIRE FABRIC ON 12" OF GRANULAR FILL.
- C.J. INDICATES CONTROL JOINT. SEE K1/S-101.
- FOR DOOR OPENING SIZES AND LOCATIONS SEE ARCH. DWGS. FOR SLAB AND FOUNDATION PENETRATIONS SEE MECH AND ELEC. DWGS.
- B/FTG. (-#'-#") INDICATES DISTANCE BELOW DATUM FINISHED FIRST FLOOR SLAB ELEVATION.
- (F#) INDICATES FOOTING TYPE. SEE FOOTING SCHEDULE FOR DETAILS.
- (P#) INDICATES PIER TYPE. SEE PIER SCHEDULE FOR DETAILS.
- THE SUBSTITUTION OF CAST-IN-PLACE CONCRETE FOUNDATION WALLS IS PERMITTED. THE CONTRACTOR SHALL PROVIDE TYPICAL DETAILS FOR APPROVAL.

FOOTING SCHEDULE

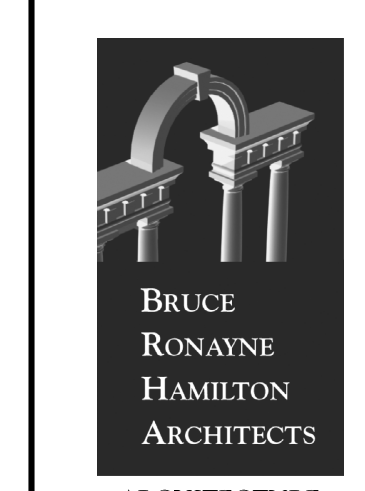
FTG MARK	SIZE (LxWxH)	REINFORCEMENT
F3.0	3'-0" x 3'-0" x 1'-0"	(4)-#5 BARS, EACH WAY
F4.0	4'-0" x 4'-0" x 1'-0"	(5)-#5 BARS, EACH WAY
F5.0	5'-0" x 5'-0" x 1'-0"	(6)-#5 BARS, EACH WAY
F6.0	6'-0" x 6'-0" x 1'-0"	(6)-#6 BARS, EACH WAY
F7.0	7'-0" x 7'-0" x 1'-0"	(7)-#6 BARS, EACH WAY
F5.5	5'-6" x SEE PLAN x 1'-0"	(11)-#7 BARS LONGITUDINAL, BOTT. #5 BARS AT 12" TRANSVERSE, BOTT.
F6.5	6'-6" x SEE PLAN x 1'-0"	(14)-#7 BARS LONGITUDINAL, BOTT. #5 BARS AT 12" TRANSVERSE, BOTT.

PIER SCHEDULE

PIER MARK	SIZE (LxW)	REINFORCEMENT
P1	2'-4" x 1'-4"	(12)-#5 VERT. DOWELS
P2	2'-0" x 2'-0"	(8)-#6 VERT. DOWELS
P3	2'-0" x 1'-10"	(8)-#6 VERT. DOWELS
P4	3'-0" x 2'-0"	(14)-#6 VERT. DOWELS
P5	2'-0" x 1'-6"	(6)-#6 VERT. DOWELS

PROVIDE #3 TIES @ 10" OC (TRIPLED AT TOP AND BOTTOM OF PIER)

A1 FOUNDATION PLAN
1/8" = 1'-0"



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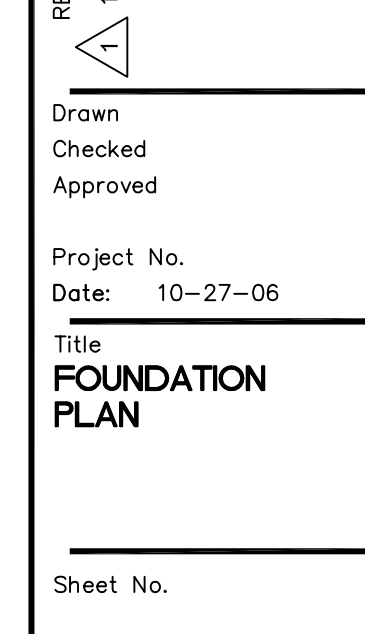
ISSUED FOR BUILDING PERMIT/CG-BIDDING

NO SCALE DRAWING
TM PROJECT NO.: 46648-00

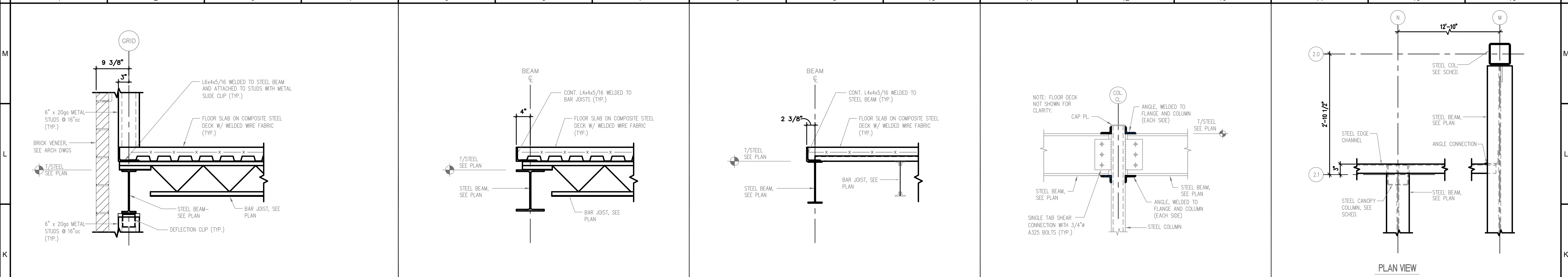
MATERIAL NOTES

CONCRETE SPECIFICATIONS:

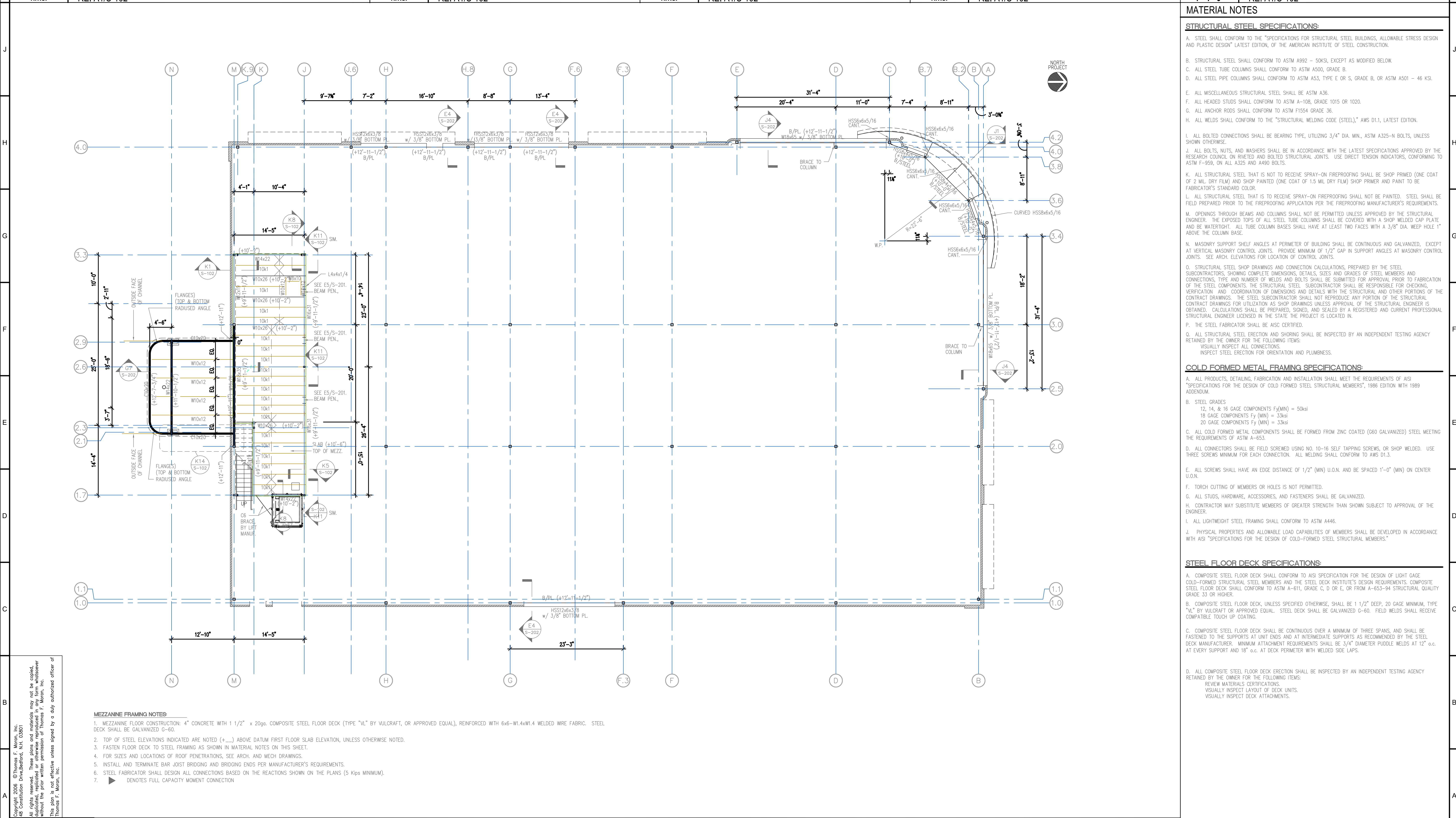
STRUCTURAL MASONRY SPECIFICATIONS:



A14 SLAB DEPRESSION DETAIL
N.T.S.



K1 N.T.S.	MEZZANINE DETAIL RE: A1/S-102	K5 N.T.S.	MEZZANINE DETAIL RE: A1/S-102	K8 N.T.S.	MEZZANINE DETAIL RE: A1/S-102	K11 N.T.S.	BEAM CONNECTION DETAIL RE: A1/S-102	K14 1"=1'-0"	CANOPY FRAMING PLAN RE: A1/S-102
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A1 1/8" = 1'-0"	MEZZANINE LEVEL FRAMING PLAN
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MATERIAL NOTES

STRUCTURAL STEEL SPECIFICATIONS:

- STEEL SHALL CONFORM TO THE "SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS, ALLOWABLE STRESS DESIGN AND PLASTIC DESIGN" LATEST EDITION, OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION.
- STRUCTURAL STEEL SHALL CONFORM TO ASTM A992 - 50KSI, EXCEPT AS MODIFIED BELOW.
- ALL STEEL TUBE COLUMNS SHALL CONFORM TO ASTM A500, GRADE B.
- ALL STEEL PIPE COLUMNS SHALL CONFORM TO ASTM A53, TYPE E OR S, GRADE B, OR ASTM A501 - 46 KSI.
- ALL MISCELLANEOUS STRUCTURAL STEEL SHALL BE ASTM A36.
- ALL HEADED STUDS SHALL CONFORM TO ASTM A-100, GRADE 1015 OR 1020.
- ALL ANCHOR RODS SHALL CONFORM TO ASTM F1554 GRADE 36.
- ALL WELDS SHALL CONFORM TO THE "STRUCTURAL WELDING CODE (STEEL)," AWS D1.1, LATEST EDITION.
- ALL BOLTED CONNECTIONS SHALL BE BEARING TYPE, UTILIZING 3/4" DIA. MIN., ASTM A325-N BOLTS, UNLESS SHOWN OTHERWISE.
- ALL BOLTS, NUTS, AND WASHERS SHALL BE IN ACCORDANCE WITH THE LATEST SPECIFICATIONS APPROVED BY THE RESEARCH COUNCIL ON RIVETED AND BOLTED STRUCTURAL JOINTS. USE DIRECT TENSION INDICATORS, CONFORMING TO ASTM F-959, ON ALL A325 AND A490 BOLTS.
- ALL STRUCTURAL STEEL THAT IS NOT TO RECEIVE SPRAY-ON FIREPROOFING SHALL BE SHOP PRIMED (ONE COAT OF 2 MIL DRY FILM) AND SHOP PAINTED (ONE COAT OF 1.5 MIL DRY FILM) SHOP PRIMER AND PAINT TO BE FABRICATOR'S STANDARD COLOR.
- ALL STRUCTURAL STEEL THAT IS TO RECEIVE SPRAY-ON FIREPROOFING SHALL NOT BE PAINTED. STEEL SHALL BE FIELD PREPARED PRIOR TO THE FIREPROOFING APPLICATION PER THE FIREPROOFING MANUFACTURER'S REQUIREMENTS.
- OPENINGS THROUGH BEAMS AND COLUMNS SHALL NOT BE PERMITTED UNLESS APPROVED BY THE STRUCTURAL ENGINEER. THE EXPOSED TOPS OF ALL STEEL TUBE COLUMNS SHALL BE COVERED WITH A SHOP WELDED CAP PLATE, AND BE "WATERTIGHT." ALL TUBE COLUMN BASES SHALL HAVE AT LEAST TWO FACES WITH A 3/8" DIA. "KEEP HOLE" 1" ABOVE THE COLUMN BASE.
- MASONRY SUPPORT SHELF ANGLES AT PERIMETER OF BUILDING SHALL BE CONTINUOUS AND GALVANIZED, EXCEPT AT VERTICAL MASONRY CONTROL JOINTS. PROVIDE MINIMUM OF 1/2" GAP IN SUPPORT ANGLES AT MASONRY CONTROL JOINTS. SEE ARCH. ELEVATIONS FOR LOCATION OF CONTROL JOINTS.
- STRUCTURAL STEEL SHOP DRAWINGS AND CONNECTION CALCULATIONS, PREPARED BY THE STEEL SUBCONTRACTORS, SHOWING COMPLETE DIMENSIONS, DETAILS, SIZES AND GRADES OF STEEL MEMBERS AND CONNECTIONS, TYPE AND NUMBER OF WELDS AND BOLTS SHALL BE SUBMITTED FOR APPROVAL PRIOR TO FABRICATION OF THE STEEL COMPONENTS. THE STRUCTURAL STEEL SUBCONTRACTOR SHALL BE RESPONSIBLE FOR CHECKING, VERIFICATION AND COORDINATION OF DIMENSIONS AND DETAILS WITH THE STRUCTURAL AND OTHER PORTIONS OF THE CONTRACT DRAWINGS. THE STEEL SUBCONTRACTOR SHALL NOT REPRODUCE ANY PORTION OF THE STRUCTURAL CONTRACT DRAWINGS FOR UTILIZATION AS SHOP DRAWINGS UNLESS APPROVAL OF THE STRUCTURAL ENGINEER IS OBTAINED. CALCULATIONS SHALL BE PREPARED, SIGNED, AND SEALED BY A REGISTERED AND CURRENT PROFESSIONAL STRUCTURAL ENGINEER LICENSED IN THE STATE THE PROJECT IS LOCATED IN.
- THE STEEL FABRICATOR SHALL BE AISC CERTIFIED.
- ALL STRUCTURAL STEEL ERECTION AND SHORING SHALL BE INSPECTED BY AN INDEPENDENT TESTING AGENCY RETAINED BY THE OWNER FOR THE FOLLOWING ITEMS:
VISUALLY INSPECT ALL CONNECTIONS.
INSPECT STEEL ERECTION FOR ORIENTATION AND PLUMBNESS.

COLD FORMED METAL FRAMING SPECIFICATIONS:

- ALL PRODUCTS, DETAILING, FABRICATION AND INSTALLATION SHALL MEET THE REQUIREMENTS OF AISI "SPECIFICATIONS FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS", 1986 EDITION WITH 1989 ADDENDUM.
- STEEL GRADES
12, 14, & 16 GAGE COMPONENTS $F_y(MIN) = 50ksi$
18 GAGE COMPONENTS $F_y(MIN) = 33ksi$
20 GAGE COMPONENTS $F_y(MIN) = 33ksi$
- ALL COLD FORMED METAL COMPONENTS SHALL BE FORMED FROM ZINC COATED (G60 GALVANIZED) STEEL MEETING THE REQUIREMENTS OF ASTM A-653.
- ALL CONNECTORS SHALL BE FIELD SCREWED USING NO. 10-16 SELF TAPPING SCREWS, OR SHOP WELDED. USE THREE SCREWS MINIMUM FOR EACH CONNECTION. ALL WELDING SHALL CONFORM TO AWS D1.3.
- ALL SCREWS SHALL HAVE AN EDGE DISTANCE OF 1/2" (MIN) U.O.N. AND BE SPACED 1'-0" (MIN) ON CENTER U.O.N.
- TORCH CUTTING OF MEMBERS OR HOLES IS NOT PERMITTED.
- ALL STUDS, HARDWARE, ACCESSORIES, AND FASTENERS SHALL BE GALVANIZED.
- CONTRACTOR MAY SUBSTITUTE MEMBERS OF GREATER STRENGTH THAN SHOWN SUBJECT TO APPROVAL OF THE ENGINEER.
- ALL LIGHTWEIGHT STEEL FRAMING SHALL CONFORM TO ASTM A446.
- PHYSICAL PROPERTIES AND ALLOWABLE LOAD CAPABILITIES OF MEMBERS SHALL BE DEVELOPED IN ACCORDANCE WITH AISI "SPECIFICATIONS FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS."

STEEL FLOOR DECK SPECIFICATIONS:

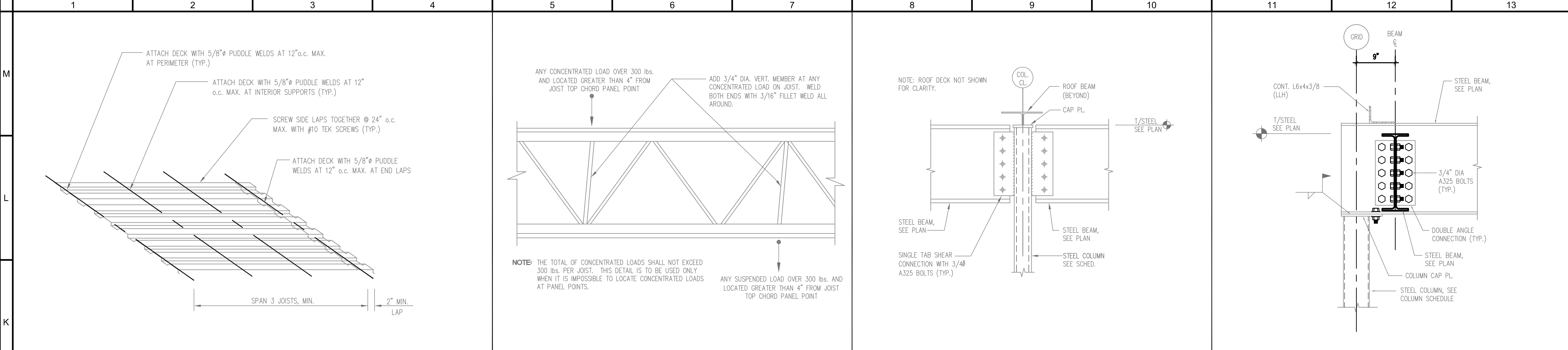
- COMPOSITE STEEL FLOOR DECK SHALL CONFORM TO AISI SPECIFICATION FOR THE DESIGN OF LIGHT GAGE COLD-FORMED STRUCTURAL STEEL MEMBERS AND THE STEEL DECK INSTITUTE'S DESIGN REQUIREMENTS. COMPOSITE STEEL FLOOR DECK SHALL CONFORM TO ASTM A-611, GRADE C, D OR E, OR FROM A-653-94 STRUCTURAL QUALITY GRADE 33 OR HIGHER.
- COMPOSITE STEEL FLOOR DECK, UNLESS SPECIFIED OTHERWISE, SHALL BE 1 1/2" DEEP, 20 GAGE MINIMUM, TYPE "VL" BY VULCRAFT OR APPROVED EQUAL. STEEL DECK SHALL BE GALVANIZED G-60. FIELD WELDS SHALL RECEIVE COMPATIBLE TOUCH UP COATING.
- COMPOSITE STEEL FLOOR DECK SHALL BE CONTINUOUS OVER A MINIMUM OF THREE SPANS, AND SHALL BE FASTENED TO THE SUPPORTS AT UNIT ENDS AND AT INTERMEDIATE SUPPORTS AS RECOMMENDED BY THE STEEL DECK MANUFACTURER. MINIMUM ATTACHMENT REQUIREMENTS SHALL BE 3/4" DIAMETER PUDDLE WELDS AT 12" O.C. AT EVERY SUPPORT AND 18" O.C. AT DECK PERIMETER WITH WELDED SIDE LAPS.
- ALL COMPOSITE STEEL FLOOR DECK ERECTION SHALL BE INSPECTED BY AN INDEPENDENT TESTING AGENCY RETAINED BY THE OWNER FOR THE FOLLOWING ITEMS:
REVIEW MATERIALS CERTIFICATIONS.
VISUALLY INSPECT LAYOUT OF DECK UNITS.
VISUALLY INSPECT DECK ATTACHMENTS.

MEZZANINE FRAMING NOTES

- MEZZANINE FLOOR CONSTRUCTION: 4" CONCRETE WITH 1 1/2" x 20ga. COMPOSITE STEEL FLOOR DECK (TYPE "VL" BY VULCRAFT, OR APPROVED EQUAL), REINFORCED WITH 6x6-WI-4XW1.4 WELDED WIRE FABRIC. STEEL DECK SHALL BE GALVANIZED G-60.
- TOP OF STEEL ELEVATIONS INDICATED ARE NOTED (+...) ABOVE DATUM FIRST FLOOR SLAB ELEVATION, UNLESS OTHERWISE NOTED.
- FASTEN FLOOR DECK TO STEEL FRAMING AS SHOWN IN MATERIAL NOTES ON THIS SHEET.
- FOR SIZES AND LOCATIONS OF ROOF PENETRATIONS, SEE ARCH. AND MECH DRAWINGS.
- INSTALL AND TERMINATE BAR JOIST BRIDGING AND BRIDGING ENDS PER MANUFACTURER'S REQUIREMENTS.
- STEEL FABRICATOR SHALL DESIGN ALL CONNECTIONS BASED ON THE REACTIONS SHOWN ON THE PLANS (5 Kips MINIMUM).
- ▶ DENOTES FULL CAPACITY MOMENT CONNECTION

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MATERIAL NOTES

STEEL ROOF DECK SPECIFICATIONS:

A. STEEL ROOF DECK SHALL CONFORM TO AISI SPECIFICATION FOR THE DESIGN OF LIGHT GAGE, COLD-FORMED STRUCTURAL STEEL MEMBERS AND THE STEEL DECK INSTITUTE'S DESIGN REQUIREMENTS. STEEL ROOF DECK SHALL CONFORM TO ASTM A-611, GRADE C, D OR E, OR FROM A-653-94 STRUCTURAL QUALITY GRADE 33 OR HIGHER.

B. STEEL ROOF DECK, UNLESS SPECIFIED OTHERWISE, SHALL BE 1 1/2" DEEP, 20 GAGE MINIMUM, TYPE "B" BY VULCRAFT, OR APPROVED EQUAL. STEEL DECK SHALL BE PAINTED. FIELD WELDS SHALL RECEIVE TOUCH UP PAINT PRIOR TO ROOFING INSTALLATION.

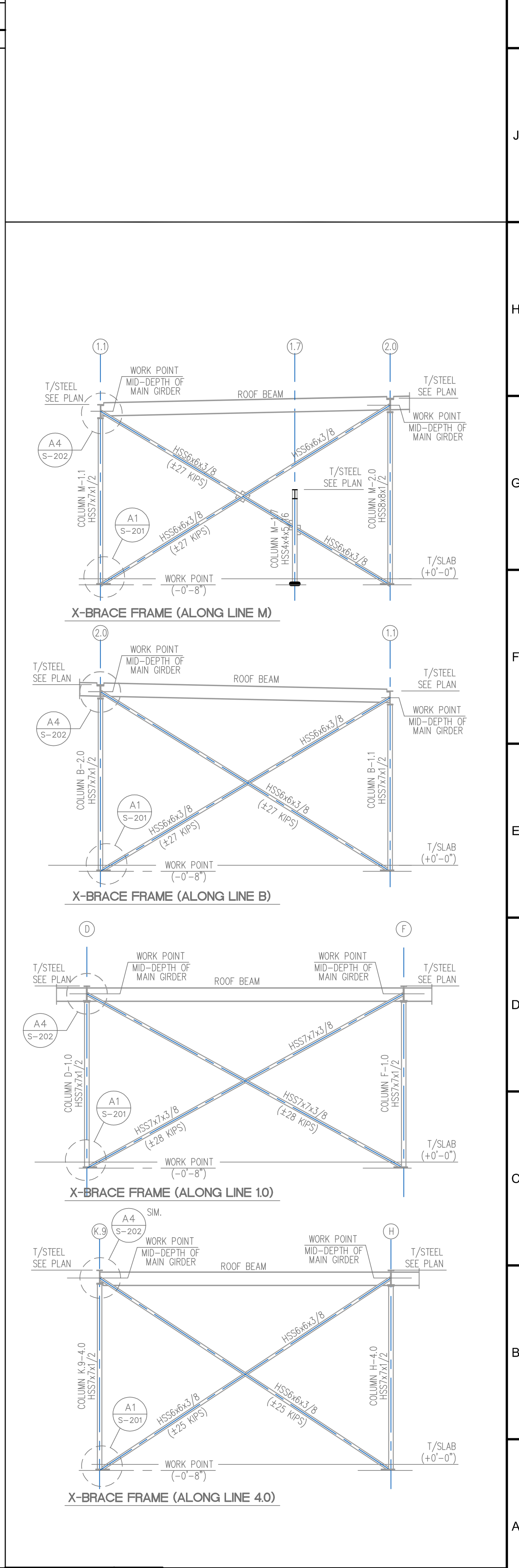
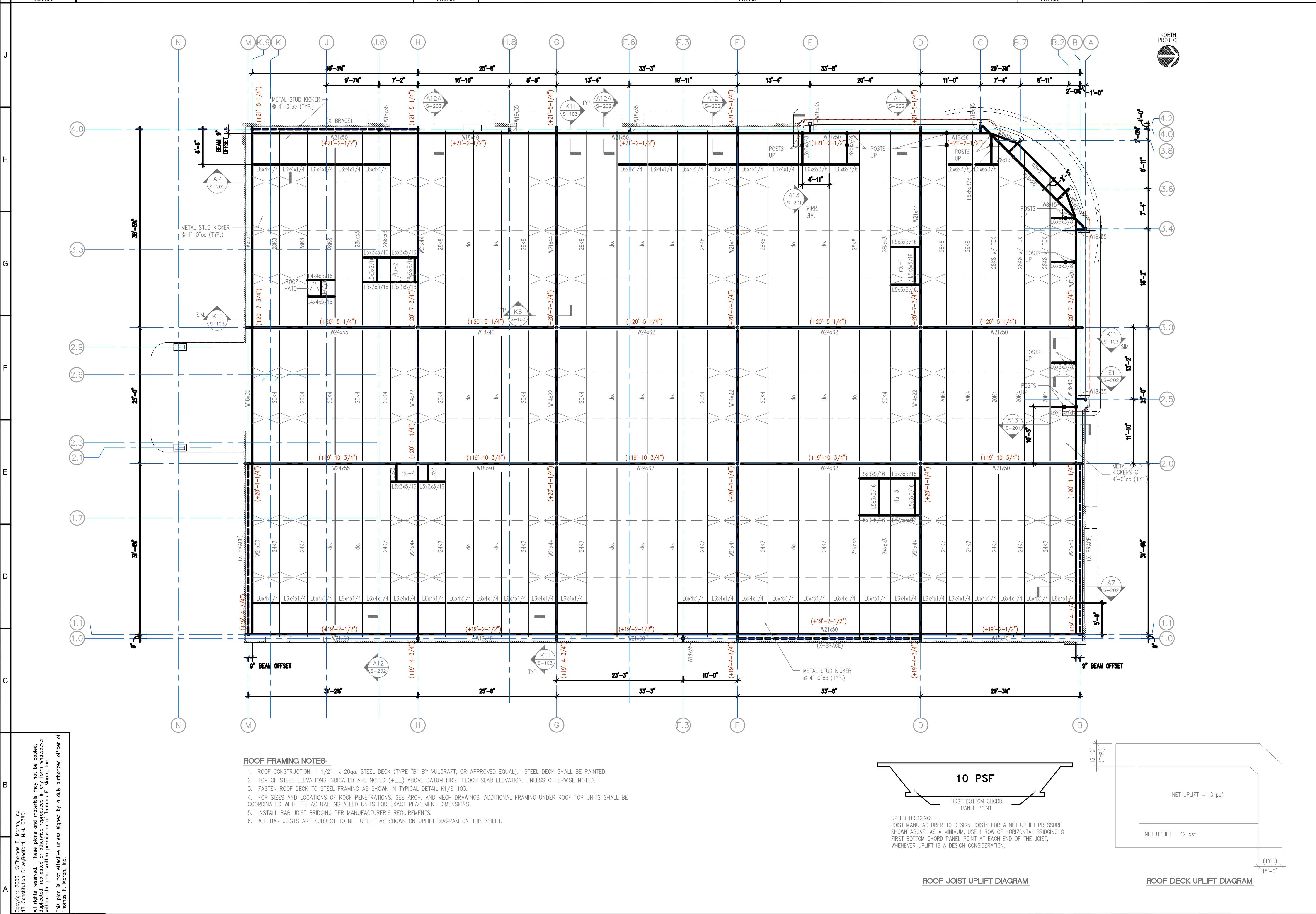
C. STEEL ROOF DECK SHALL BE CONTINUOUS OVER A MINIMUM OF THREE SPANS, AND SHALL BE FASTENED TO THE SUPPORTS AT UNIT ENDS AND AT INTERMEDIATE SUPPORTS AS RECOMMENDED BY THE STEEL DECK MANUFACTURER. MINIMUM ATTACHMENT REQUIREMENTS SHALL BE 5/8" DIAMETER PUDDLE WELDS AT 12" o.c. AT EVERY SUPPORT AND AT DECK PERIMETER AND WITH #10 TEK SCREWS AT 24" o.c. AT ALL SIDE LAPS. SEE TYPICAL DECK ATTACHMENT DETAIL FOR FURTHER CLARIFICATION.

D. ALL STEEL ROOF DECK ERECTION SHALL BE INSPECTED BY AN INDEPENDENT TESTING AGENCY RETAINED BY THE OWNER FOR THE FOLLOWING ITEMS:
 REVIEW MATERIALS CERTIFICATIONS.
 VISUALLY INSPECT LAYOUT OF DECK UNITS.
 VISUALLY INSPECT DECK ATTACHMENTS.

SHOP DRAWINGS

A. THE GENERAL CONTRACTOR (OR CONSTRUCTION MANAGER) SHALL SUBMIT SHOP DRAWINGS FOR ALL STRUCTURAL ELEMENTS SHOWN ON THE CONTRACT DOCUMENTS FOR APPROVAL. THE STRUCTURAL ENGINEER WILL NOT BE RESPONSIBLE FOR THE STRUCTURAL CERTIFICATION AND DESIGN OF THE PROJECT IF THE GENERAL CONTRACTOR FAILS TO OBTAIN APPROVAL OF THE SHOP DRAWINGS. THE GENERAL CONTRACTOR SHALL INFORM THE STRUCTURAL ENGINEER IN WRITING CONCERNING DEVIATIONS AND/OR OMISSIONS FROM THE CONTRACT DOCUMENTS AT THE TIME OF SHOP DRAWING SUBMISSION.

K1 ROOF DECK ATTACHMENT DETAIL N.T.S. K5 BAR JOIST REINFORCING DETAIL N.T.S. K8 TYPICAL STEEL CONNECTION N.T.S. K11 PERIMETER STEEL DETAIL N.T.S.



A14 BRACE ELEVATIONS N.T.S.

A1 ROOF FRAMING PLAN 1/8" = 1'-0"

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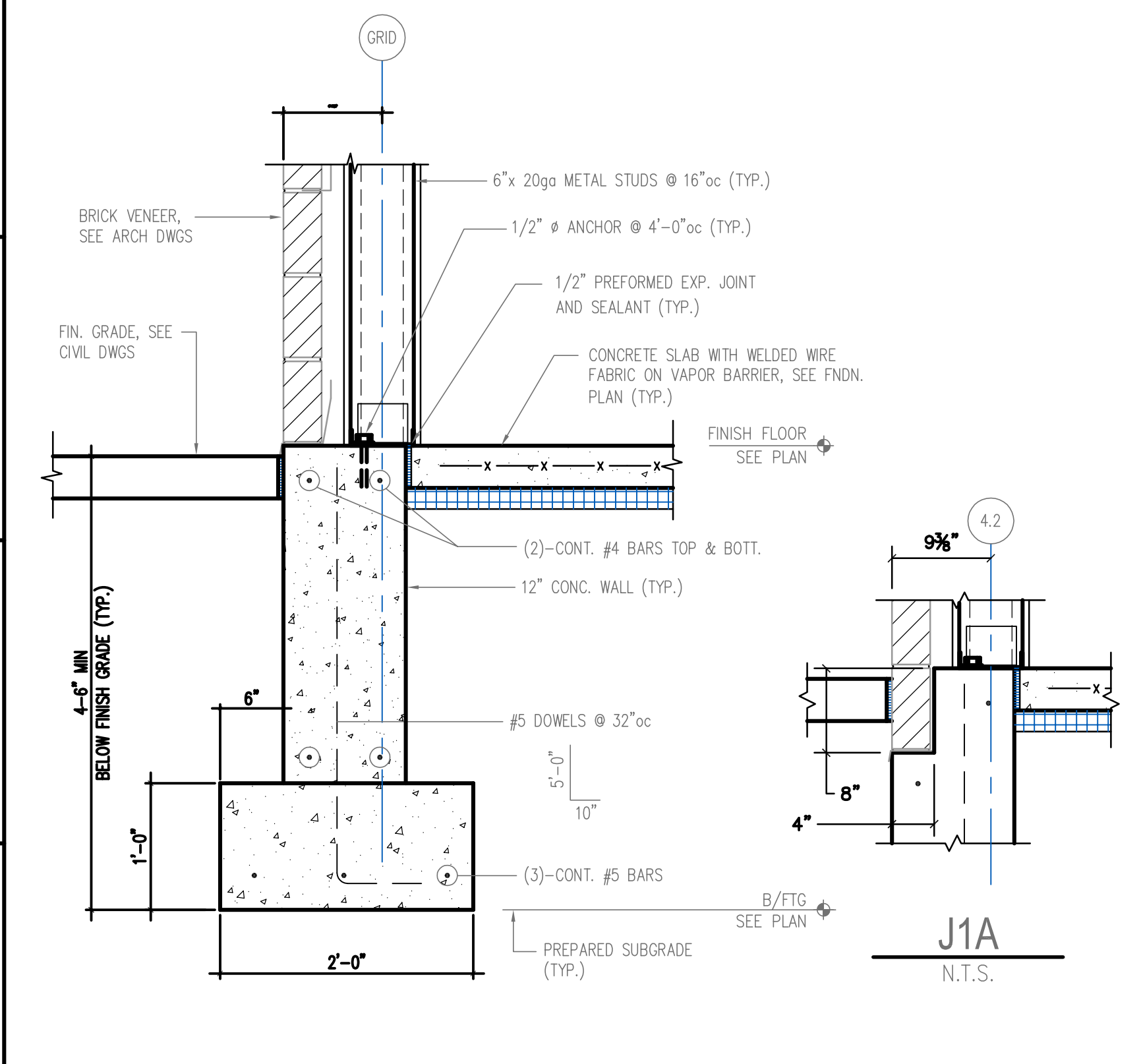
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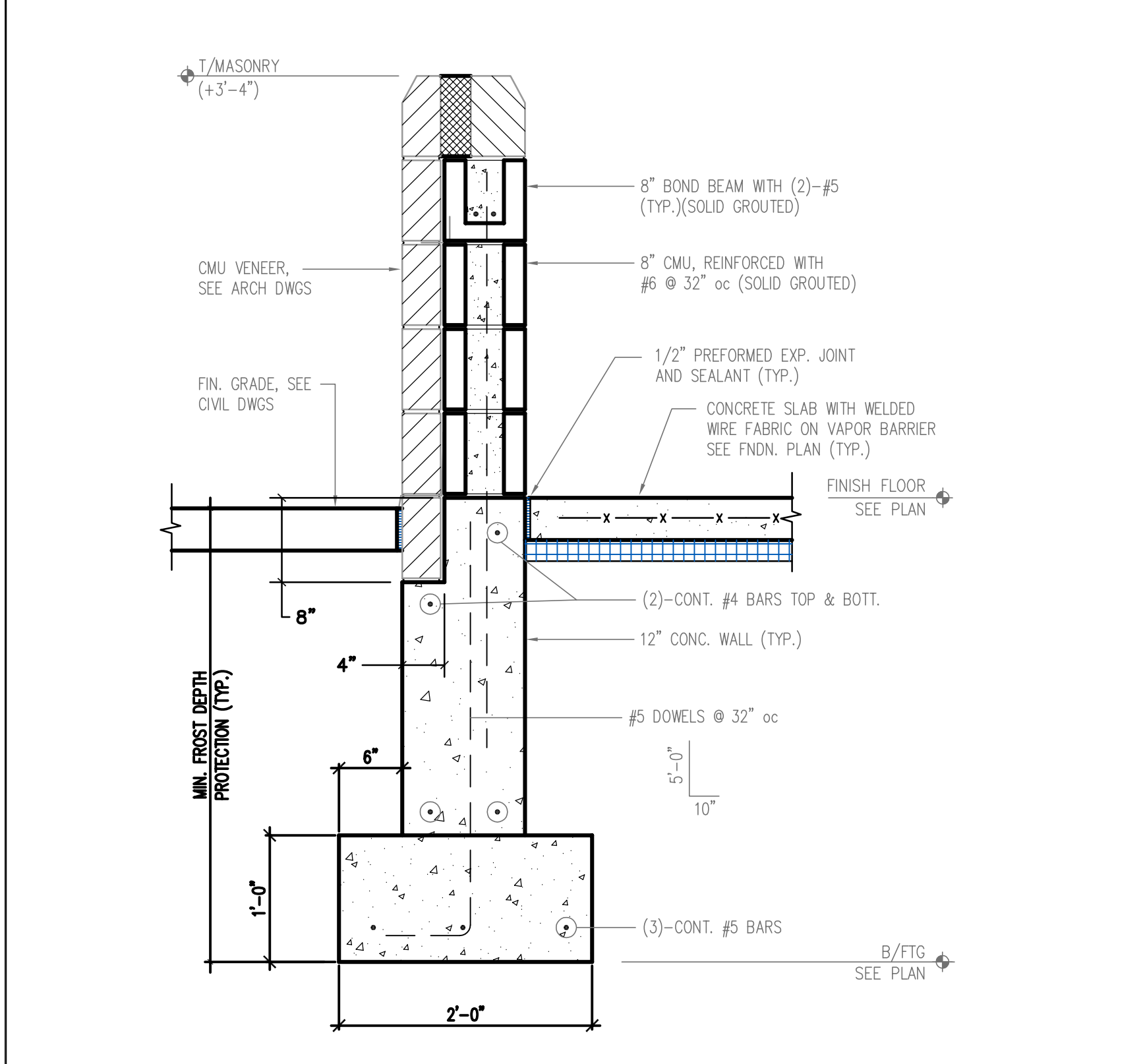
ISSUED FOR BUILDING PERMIT/CG BIDDING
 REVISIONS
 10-27-06
 Drawn
 Checked
 Approved
 Project No.
 Date: 10-27-06
 Title
ROOF FRAMING PLAN
 Sheet No.
S-103

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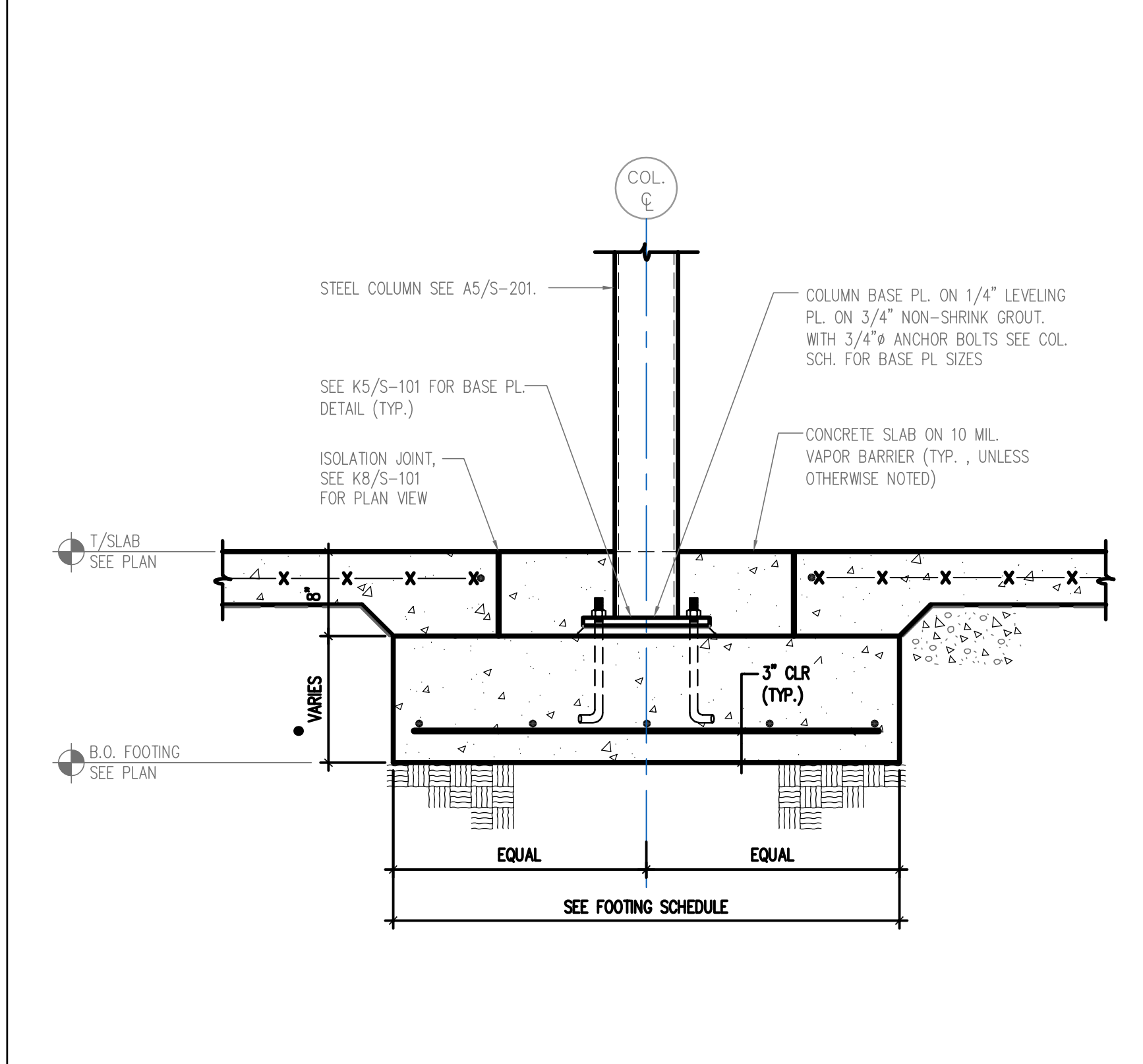
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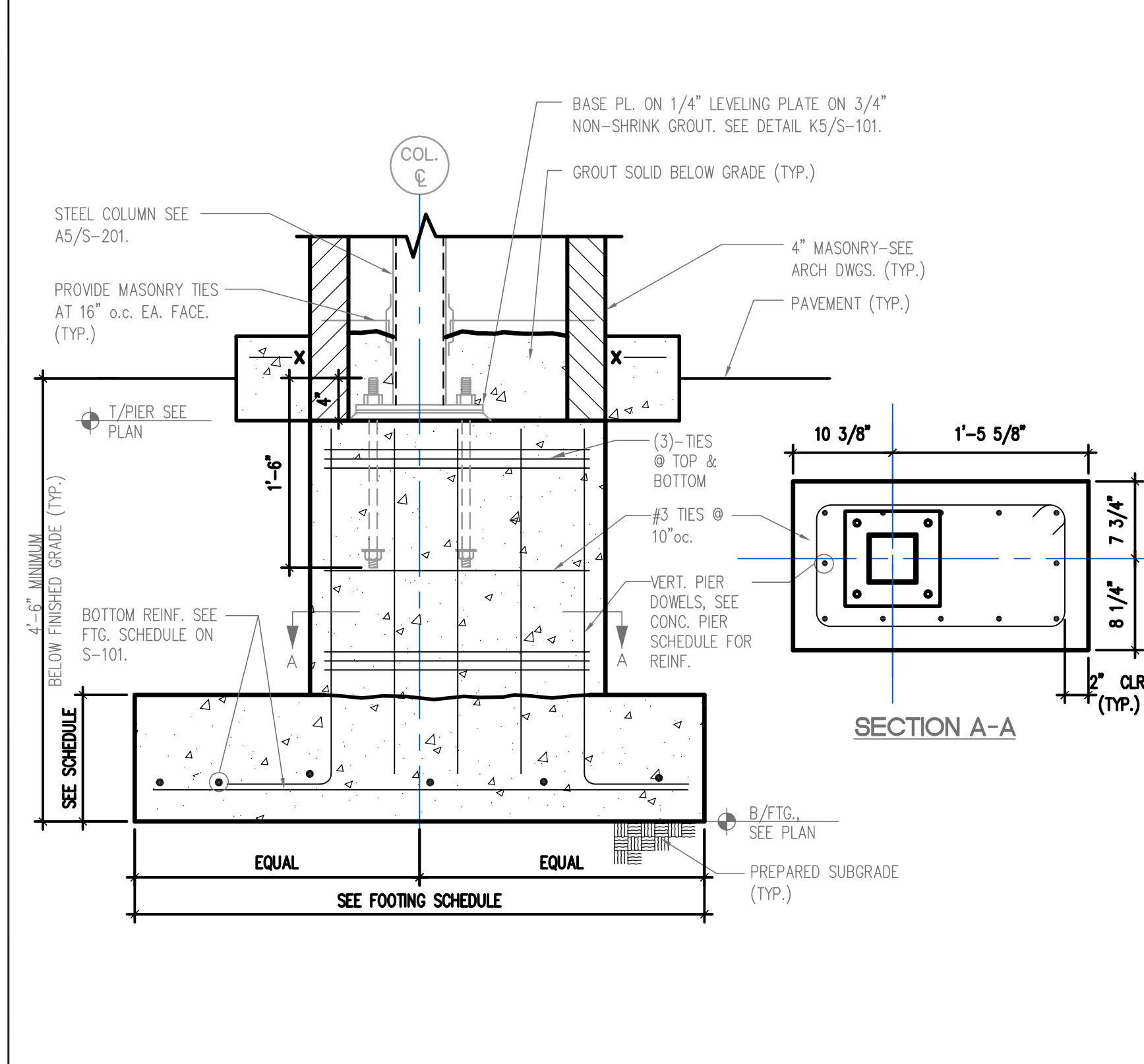
J1 FOUNDATION WALL DETAIL
N.T.S.



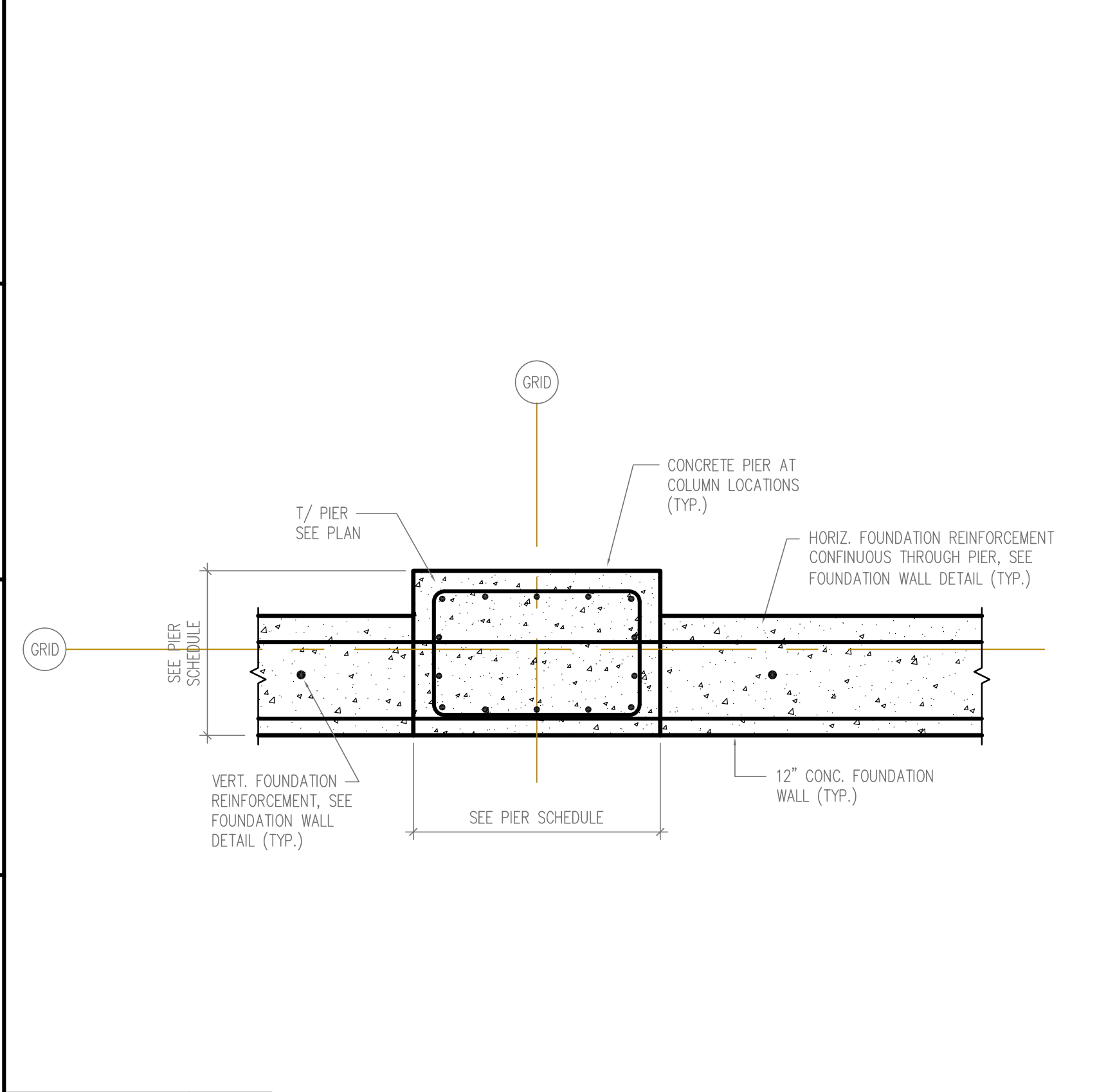
J5 FOUNDATION WALL DETAIL
N.T.S.



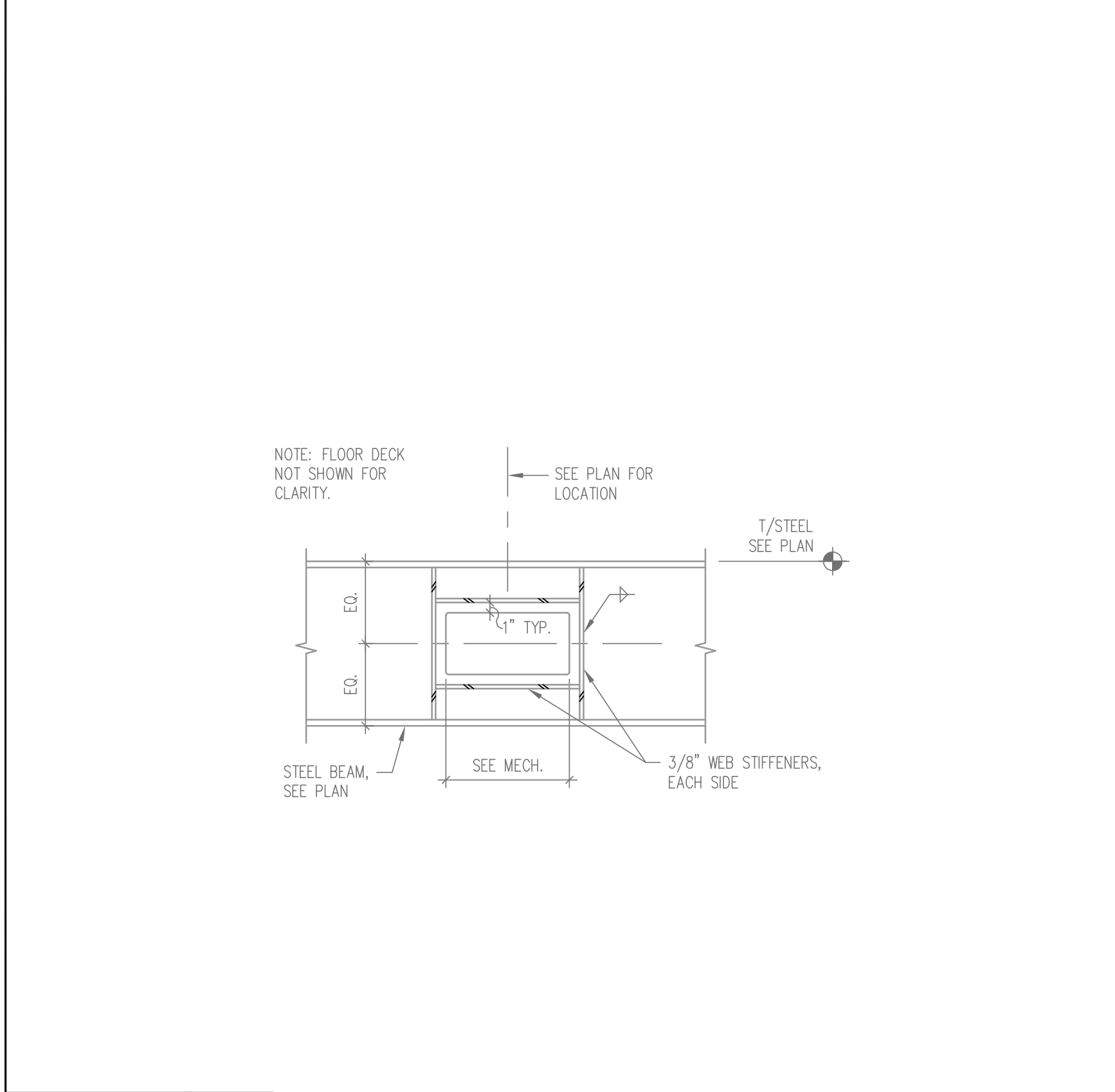
J9 INTERIOR COLUMN FOOTING DETAIL
N.T.S.



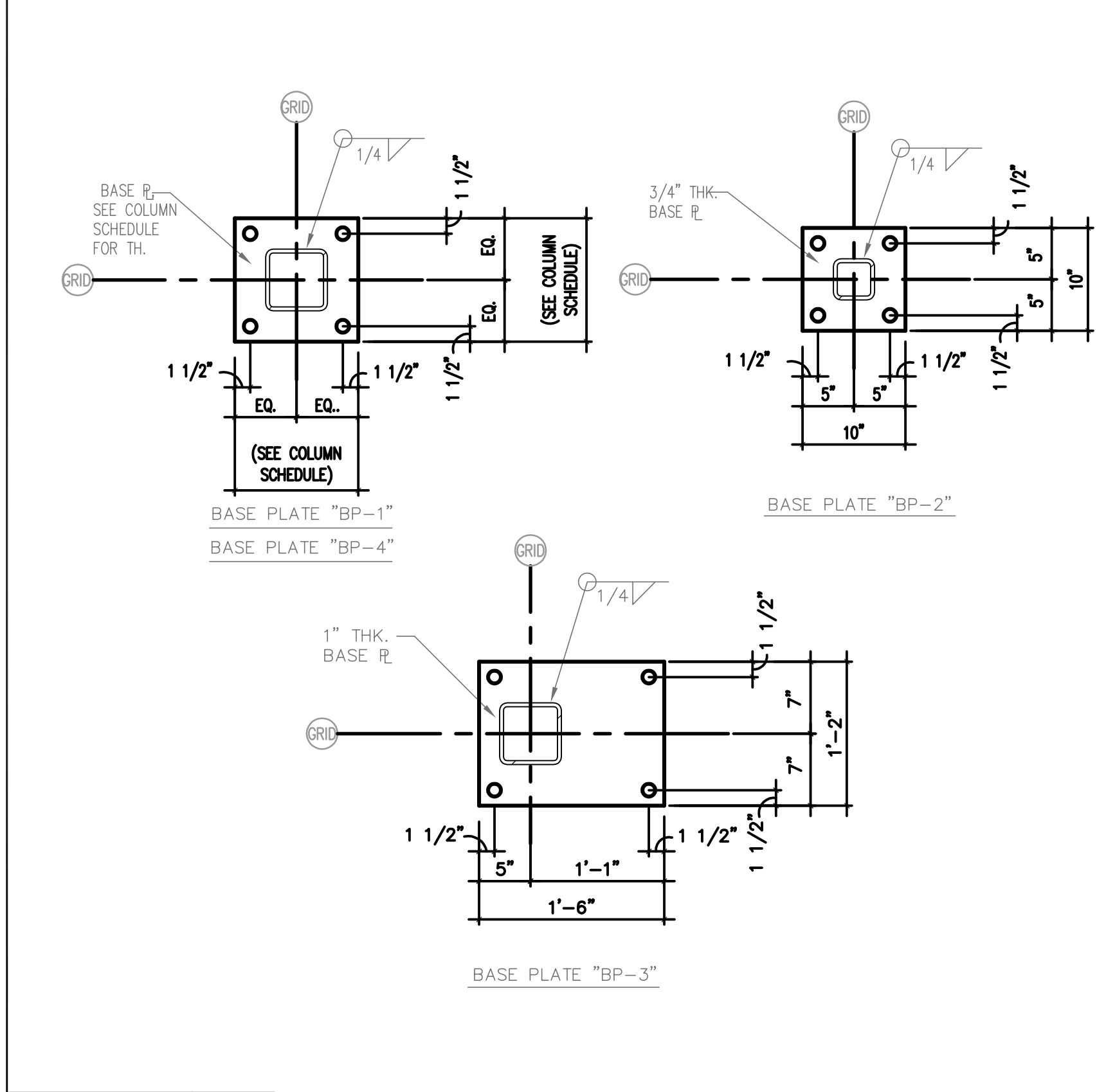
J13 EXTERIOR CANOPY FOOTING DETAIL
N.T.S.



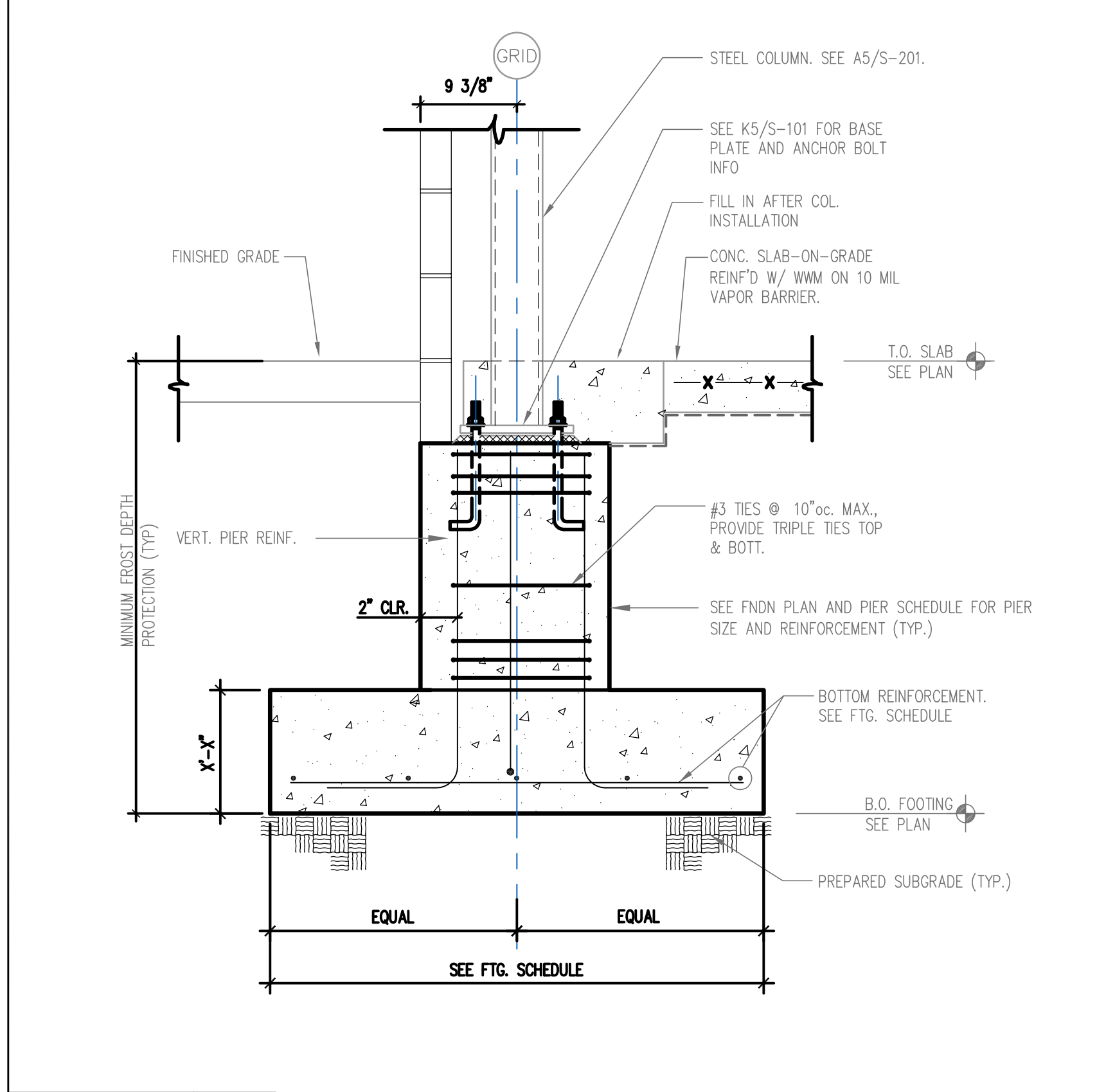
E1 FOUNDATION PIER - PLAN VIEW
N.T.S.



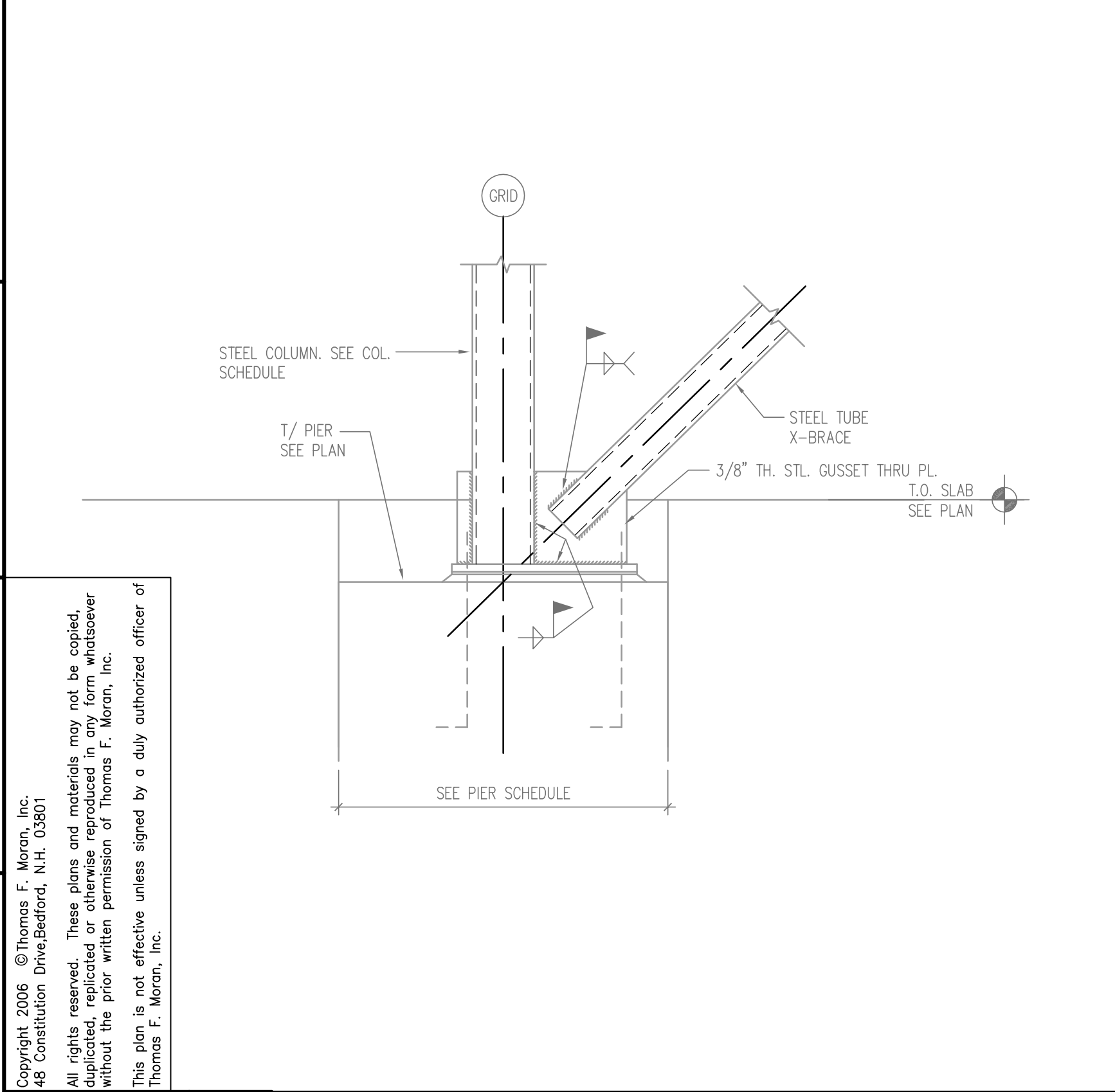
E5 BEAM PENETRATION DETAIL
N.T.S.



E9 BASE PLATE DETAILS
N.T.S.



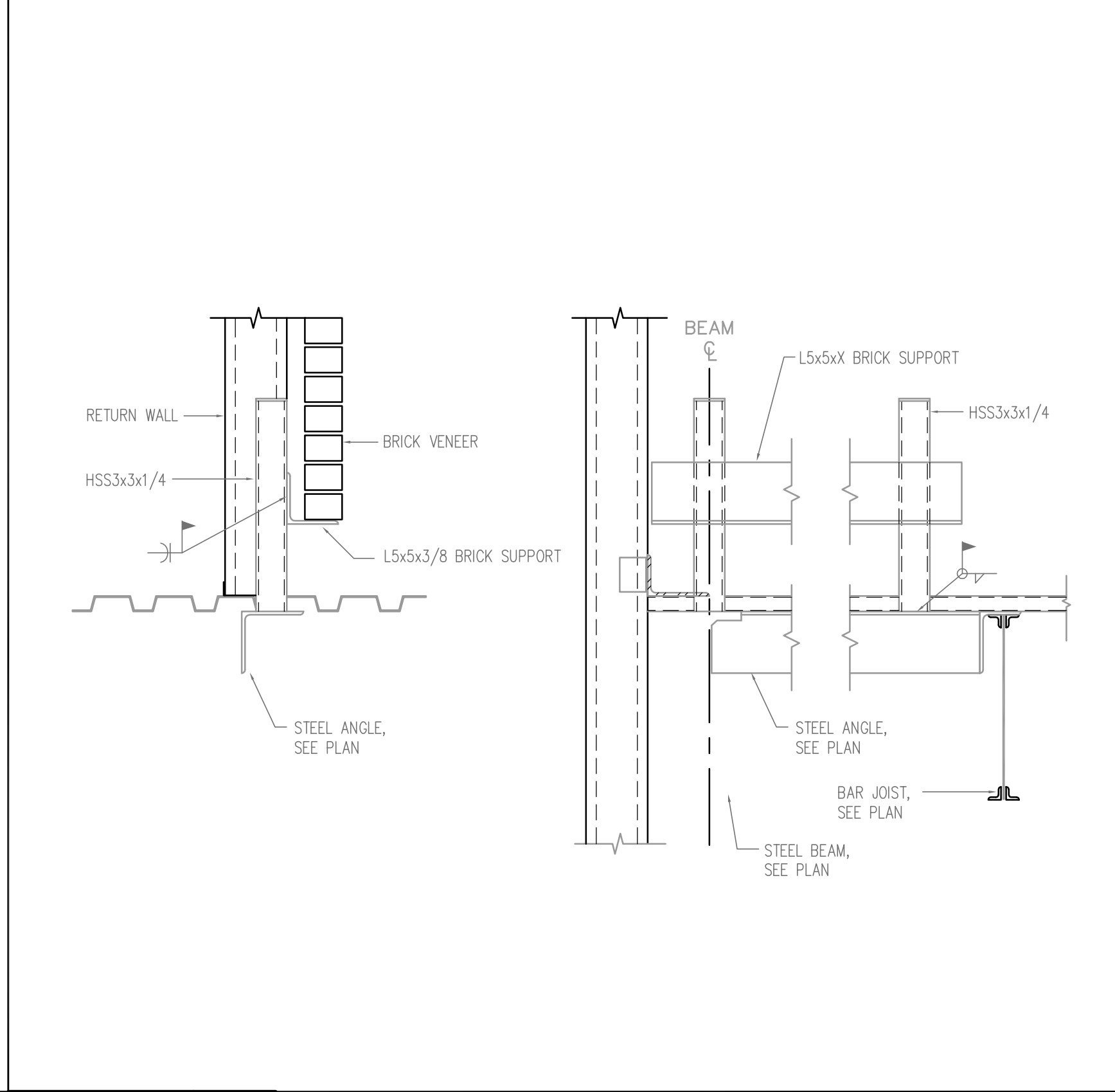
E13 EXTERIOR COLUMN DETAIL
N.T.S.



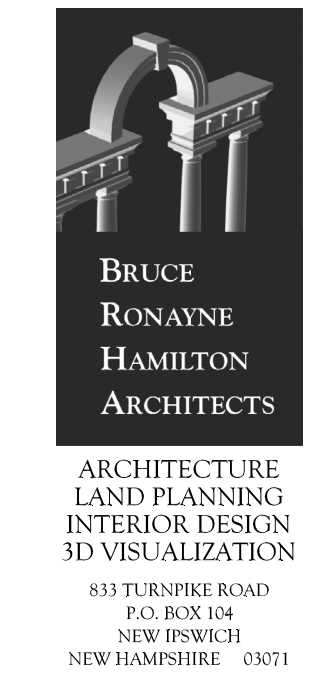
A1 BRACE COLUMN DETAIL
N.T.S.

COLUMN LOCATION	A2.5, A3.4, B3.0, C4.2, D4.0, E4.2, F.3-1.0, F.6-4.0, J.6-4.0, H.8-4.0, H1.0	B1.1, B2.0, D1.0, F1.0, H4.0, K.9-4.0, M1.1	D2.0, D3.0, F2.0, F3.0, G2.0, G3.0, H2.0, H3.0	M2.0	B.2-3.6, B.7-3.8	F4.0	G1.0, G4.0, M3.0	J1.7, J2.6, J3.3, M1.7, M3.3	K2.3	N2.1, N2.9
CAP PLATE	10"x6"x1/2"	10"x6"x1/2"	6"x6"x1/4"	10"x6"x1/2"	10"x6"x1/2"	12"x6"x1/2"	12"x6"x1/2"	4"x4"x1/4"	4"x4"x1/4"	10"x6"x1/2"
ELEV. VARIES										
ROOF										
CANOPY										
ELEV. (+10'-2") MEZZANINE LEVEL	HSS6x6x1/4	HSS12x12x1/2	HSS8x6x1/8	HSS6x6x1/2	HSS4x4x1/8	HSS6x6x1/8	HSS6x6x1/8	HSS4x4x1/8	HSS4x4x1/8	HSS6x6x1/8
ELEV. (+0'-0") (DIMENSION TO B/BASE PL. TYP.)										
BASE PLATE	BP-1 12"x3/4"x12"	BP-3 18"x1"x14"	BP-4 14"x1"x14"	BP-3 18"x1"x14"	BP-2 10"x3/4"x10"	BP-1 12"x3/4"x12"	BP-1 12"x3/4"x12"	BP-2 10"x3/4"x10"	BP-2 10"x3/4"x10"	BP-1 12"x1"x12"
ANCHOR BOLTS (F1554)	(4) -3/4"	(4) -1"	(4) -3/4"	(4) -1"	(4) -3/4"	(4) -3/4"	(4) -3/4"	(4) -3/4"	(4) -3/4"	(4) -1"

A5 COLUMN SCHEDULE
N.T.S.



A13 BRICK SUPPORT DETAIL
N.T.S.



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 NEW HAMPSHIRE 03071

50 NOT SCALE DRAWING
 TM PROJECT NO. 46048.00

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Project No.
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 Title
STRUCTURAL DETAILS

Sheet No.

S-201

