389 Congress Street, Location of Construction:	1 AS	Owner Name:		(201) 014-01		03-0621	JUN 2	0 2003	392 BO	025001
131 Hope Ave	10\ (V)	Goldeneye C	Corp				Over a con-		Phone:	
Business Name:		Contractor Nan			Con	6 Falmouth Rd	ALLA URE DU	BUI VIN		6667
		no contractor	/ self			ortland			Phone	
Lessec/Buyer's Name	· · · · · · · · · · · · · · · · · · ·	Phone:				nit Type:				
						ngle Family				Zone:
Past Use:		Proposed Use:			_==		Cost of Work		· · · · · · · · · · · · · · · · · · ·	12-2
foundation only - permit 030336	t number	build single f	amily 45	' x 75' -		\$674.00	\$93,000	102	O District:	1
		foundation pe	rmit 030	336 issued	FIR	E DEPT:	- 1-	NSPECTION INSPECTION	2	
		05/07/03				\		Use Group:		Type: == Z
						المحل	Denied		20	Type.
roposed Project Description					_			7	2011	1 <i>99</i>
ouild single family 45' x		on normalis 0200			7	N		4_		′ /
g	/J = loungain	on permit 0303	36 issue	d 05/07/03	Signa		\ s	ignature:	//_	
					PEDI	ESTRIAN ACTIV	TTIES DISTR	ICT (P.A.I).)	<u> </u>
				ů.	Actio	on: Approve	d Appro	ved w/Cond	litions 🖂	Denied
					Signa	iture:				
ermit Taken By: tmm	Date App						l name I	Date	:: 	
	06/05/					Zomig F	Approval			
This permit applicati	on does not p	reclude the	Speci	al Zone or Revie	ws	Zoning	Appeal	H	istoric Prese	Evation
Applicant(s) from me Federal Rules.			Shor	reland	4	[] Variance			ot in District	
Building permits do r septic or electrical we	ork.	į	☐ Wetl	and Approved	65k	Miscellane	ous	۵ □	oes Not Requ	tire Review
Building permits are within six (6) months	Of the date of	issuance	Floo	d Zone		Conditiona	l Use	☐ R	equires Revie	ew
False information ma permit and stop all we	y invalidate a ork	building	Subd	ivision		_ Interpretation	on	□ A ₁	pproved	-
			Site I	Plan		Approved		│ □ AI	proved w/Co	ondițions
PERMIT	SSUED		Мај 🗀	Minor MM]	Denied		De	nied /	1
JUN 20	2002		Date: 4	20/03		Date:		Date: /	10/201	12
	5(98)		l·	1				Date. L	701	<u> </u>
CITY OF POR	M AMO								V V	
-										
			CEL	TIPICA III						
reby certify that I am the ve been authorized by the diction. In addition, if a	owner of rec	ord of the name	_	RTIFICATIO		,				
ve been authorized by th	e owner to ma	ake this applica	ed prope	fly, or that the	propo	sed work is aut	horized by the	he owner	of record a	and that
we been authorized by the diction. In addition, if a have the authority to er permit.										
. There are authority to er	iter all areas c	Overed by each		105u	, 1 (ury mat me (wae official'	s authoriz	red repress	ntativo

ADDRESS

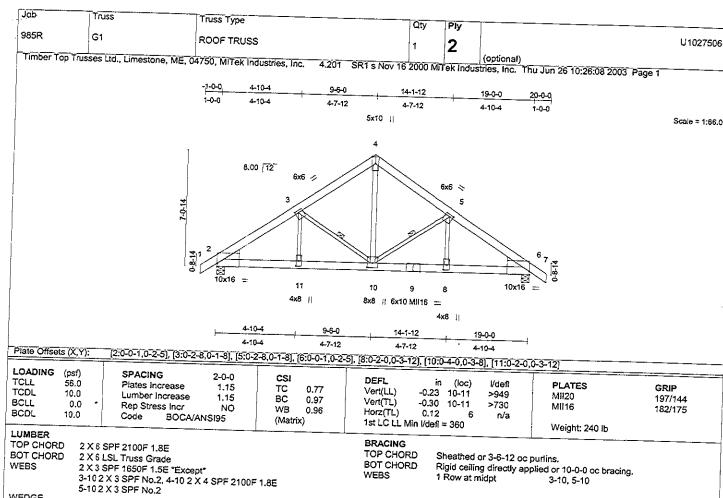
ONSIBLE PERSON IN CHARGE OF WORK, TITLE

DATE

DATE

PHONE

PHONE



Left: 2 X 8 SPF No.2, Right: 2 X 8 SPF No.2

REACTIONS (lb/size) 2=12948/0-6-7 (input: 0-5-8), 6=12948/0-6-7 (input: 0-5-8)

(lb/size) Z=1294010-0-7 (linpot.)
Max Horz 2=-201(load case 4)
Max Uplift 2=-2054(load case 6), 6=-2054(load case 6)
Max Grav 2=15018(load case 2), 6=15018(load case 3)

FORCES (lb) - First Load Case Only
TOP CHORD 1-2=54, 2-3=-16463, 3-4=-11430, 4-5=-11430, 5-6=-16463, 6-7=54
BOT CHORD WEBS 2-11=13144, 10-11=13516, 9-10=13516, 8-9=13516, 6-8=13144
3-11=5810, 3-10=-4972, 4-10=11674, 5-10=-4972, 5-8=5810

NOTES

WEDGE

- NOTES
 1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category III, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per EXPOSED FT and COMMENT AND STATE AND STATE

and 2054 ib uplift at joint 6.

8) This truss has been designed with ANSI/TPI 1-1995 criteria.

9) 2-ply truss to be connected together with 10d Common(.148"x3") Nails as follows: Top chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.

Bottom chords connected as follows: 2 X 6 - 3 rows at 0-5-0 oc.

Webs connected as follows: 2 X 3 - 1 row at 0-9-0 oc, 2 X 3 - 1 row at 0-9-0 oc.

LOAD CASE(S) Standard

1) Snow: Lumber Increase=1.15, Plate Increase=1.15

Continued on page 2

MiTek Canada, Inc.

100 Industrial Rd., P.O. Box 1329 Bradford, Ontario, L3Z 2B7



LOADING AND DIMENSIONS SPECIFIED BY FABRICATOR, SUBJECT TO VERIFICATION BY AUTHORITIES IN JURISDICTION.

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ROSS S

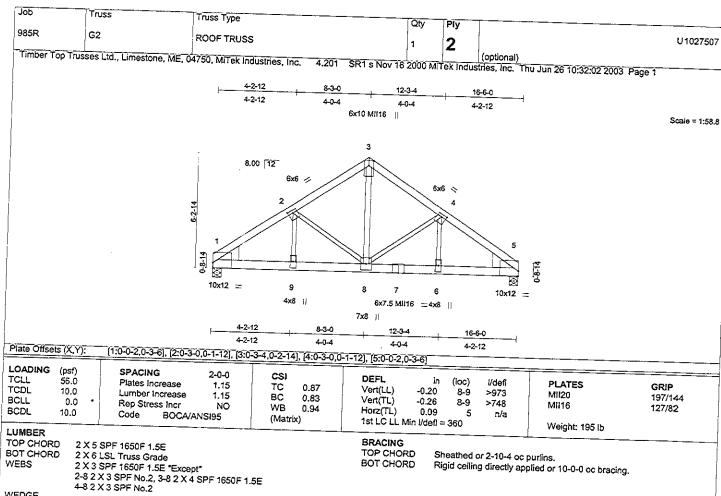
SOUSTERED A

Job	Truss	Truss Type		Qty	Pív	
985R	G1	ROOF TRUSS	í	1	2	U1027506
Timber Top Trus	ses Ltd., Limestone, ME, 0	4750, MiTek Industries, Inc. 4.20	1 SR1 s Nov 16	2000 MAT	ek ladus	(optional) stries, Inc. Thu Jun 26 10:26:08 2003 Page 2
					ev indus	stries, Inc. Thu Jun 26 10:26:08 2003 Page 2

LOAD CASE(S) Standard Uniform Loads (plf) Vert: 1-4=-132.0, 4-7=-132.0, 2-6=-1247.1







WEDGE

Left: 2 X 10 SPF No.2, Right: 2 X 10 SPF No.2

(lb/size) 1=11061/0-5-8, 5=11061/0-5-8

(lb/size) 1=1166(load case 4)

Max Horz 1=166(load case 4)

Max Uplift 1=1431(load case 6), 5=-1431(load case 6)

Max Grav 1=12818(load case 2), 5=12818(load case 3)

FORCES (Ib) - First Load Case Only
TOP CHORD 1-2=-13987, 2-3=-9728, 3-4=-9728, 4-5=-13987
BOT CHORD WEBS 1-9=11001, 8-9=11296, 7-8=11296, 6-7=11296, 5-6=11001
2-9=5076, 2-8=-4066, 3-8=9993, 4-8=-4066, 4-6=5076

NOTES

NOTES

1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category III, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANSi95 If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

2) Design load is based on 56.0 psf specified roof snow load.

3) Unbalanced snow loads have been considered for this design.

4) All plates are MII20 plates unless otherwise indicated.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.

6) WARNING: Required bearing size at joint(s) 1, 5 greater than input bearing size.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1431 lb uplift at joint 1

 Provide mechanical connection (by others) or truss to bearing piate capable or wire and 1431 lb uplift at joint 5.
 This truss has been designed with ANSI/TPI 1-1995 criteria.
 2-ply truss to be connected together with 10d Common(.148"x3") Nails as follows: Top chords connected as follows: 2 X 5 - 1 row at 0-9-0 oc.
 Bottom chords connected as follows: 2 X 6 - 3 rows at 0-5-0 oc.
 Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc. 2 X 3 - 1 row at 0-9-0 oc. 2 X Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc, 2 X 3 - 1 row at 0-9-0 oc, 2 X 3 - 1 row at 0-9-0 oc.

LOAD CASE(S) Standard
1) Snow: Lumber Increase=1.15, Plate Increase=1.15

Continued on page 2



MiTek Canada, Inc.



LOADING AND DIMENSIONS SPECIFIED BY FABRICATION, SUBJECT TO VERIFICATION BY AUTHORITIES IN JURISDICTION.

SONAL ENGINE

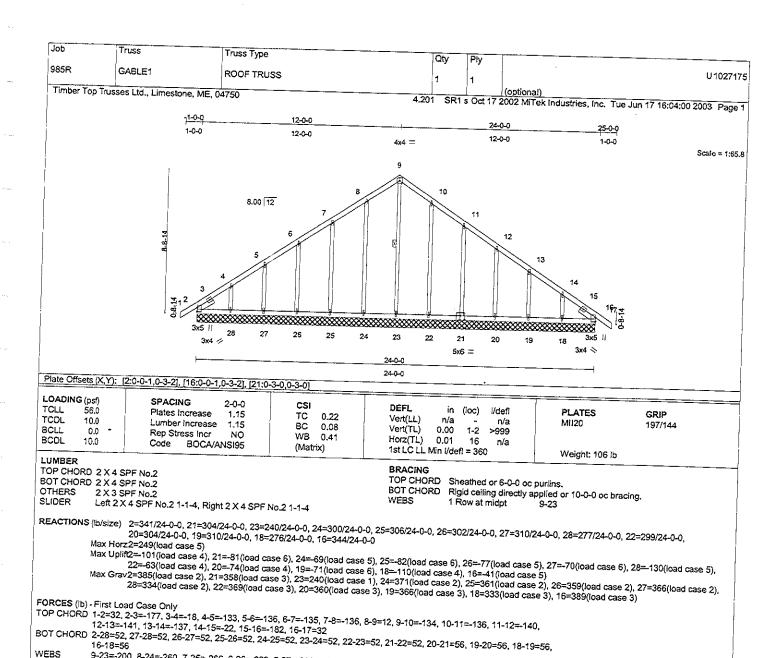
ROSS ROSS

Job	Truss	Truss Type			
985R	G2		Qty	Ply	
		ROOF TRUSS	1	2	U1027507
Timber Top Trus	ses Ltd., Limestone, ME, 0	4750, MITek Industries, Inc. 4.201 SR1 s Nov 16	2000 MiT	ek Indus	(optional) tries, Inc. Thu Jun 26 10:32:02 2003 Page 2

LOAD CASE(S) Standard Uniform Loads (plf) Vert: 1-3=-132.0, 3-5=-132.0, 1-5=-1247.1







NOTES

WEBS

1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category III, condition I enclosed building, If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

Cable End Detail*

3. All plates are 44 MINO and a significant or a significant of the face), see MiTek "Standard.

9-23=-200, 8-24=-260, 7-25=-266, 6-26=-263, 5-27=-269, 4-28=-239, 10-22=-260, 11-21=-264, 12-20=-262, 13-19=-270,

Gable End Detail*

3) All plates are 1x4 MII20 unless otherwise indicated.

4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

6) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.

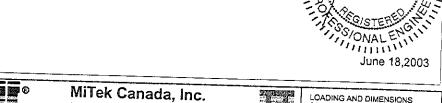
7) Provide members connection (by others) of truss to begin plate connection for the inject 2 at the unit of the inject 2.

the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 101 lb uplift at joint 2, 81 lb uplift at joint 269 lb uplift at joint 24, 82 lb uplift at joint 25, 77 lb uplift at joint 26, 70 lb uplift at joint 27, 130 lb uplift at joint 28, 63 lb uplift at joint 29, 71 lb uplift at joint 19, 110 lb uplift at joint 18 and 41 lb uplift at joint 16.

8) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(\$) Standard



MITEK CANADA, INC. GENERAL SPECIFICATIONS (U.S.A.) DATED APRIL 1, 1997 FORM AN INTEGRAL PART OF THIS DESIGN.

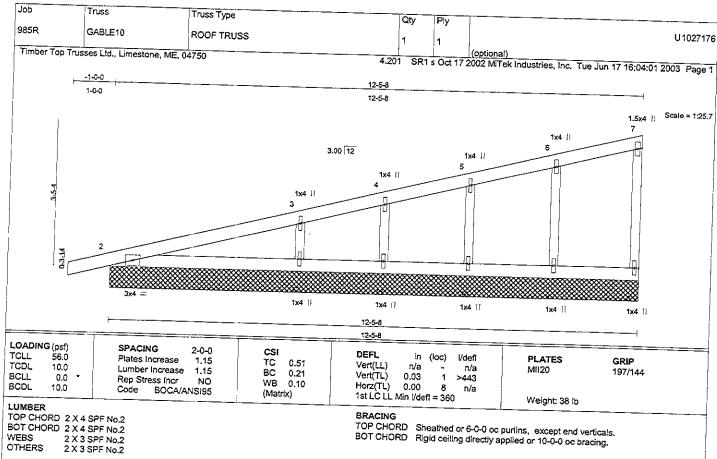


100 Industrial Rd., P.O. Box 1329 Bradford, Ontario, L3Z 2B7



LOADING AND DIMENSIONS SPECIFIED BY FABRICATOR, SUBJECT TO VERIFICATION BY AUTHORITIES IN JURISDICTION.

BOS BOS



REACTIONS (lb/size) 8=124/12-5-8, 2=428/12-5-8, 9=309/12-5-8, 10=343/12-5-8, 11=140/12-5-8, 12=666/12-5-8

Max Horz 2=222(load case 5)

Max Uplift8=-30(load case 5), 2=-108(load case 4), 9=-60(load case 4), 10=-65(load case 4), 11=-34(load case 4), 12=-119(load case 4)

FORCES (lb) - First Load Case Only TOP CHORD 1-2=29, 2-3=71, 3-4=-60, 4-5=33, 5-6=-38, 6-7=21, 7-8=-105 BOT CHORD 2-12=5, 11-12=5, 10-11=5, 9-10=5, 8-9=5 WEBS 6-9=-273, 5-10=-290, 4-11=-147, 3-12=-533

NOTES

1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load 5.0 psf bottom chord dead load, 100 ml from hurricane oceanline, on an occupancy category Itl, condition I enclosed building, If porches exist, they are not exposure C ASCE 7-93 per BOCA/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

Gable End Detail*

3) Gable requires castileting the second of th

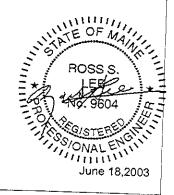
Gable End Detair

3) Gable requires continuous bottom chord bearing.

4) Gable studs spaced at 2-0-0 oc.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 30 ib uplift at joint 8, 108 lb uplift at joint 2, 60 lb uplift at joint 9, 65 lb uplift at joint 10, 34 lb uplift at joint 11 and 119 lb uplift at joint 12.
7) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard



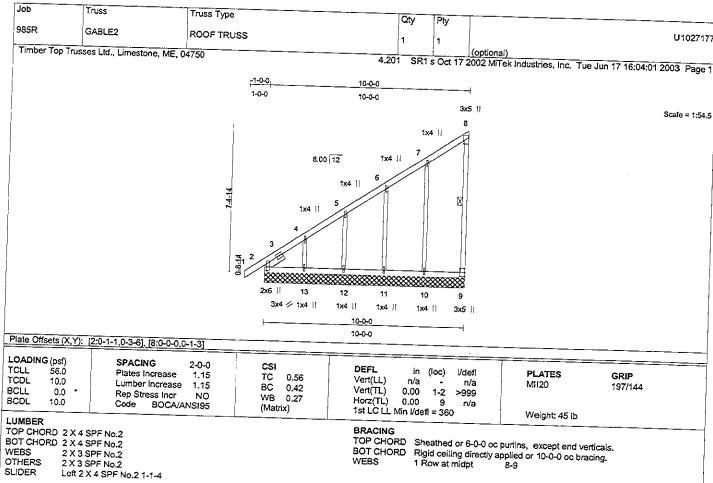
MITEK CANADA, INC. GENERAL SPECIFICATIONS (U.S.A.) DATED APRIL 1, 1997 FORM AN INTEGRAL PART OF THIS DESIGN.



MiTek Canada, Inc.

100 Industrial Rd., P.O. Box 1329 Bradford, Ontario, L3Z 2B7





Left 2 X 4 SPF No.2 1-1-4

WEBS

Sheathed or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Row at midpt

8-9

REACTIONS (lb/size) 9=121/10-0-0, 2=295/10-0-0, 10=320/10-0-0, 11=302/10-0-0, 12=307/10-0-0, 13=292/10-0-0

Max Horz2=482(load case 5)

Max Uplift9=98(load case 5), 2=-108(load case 4), 10=-66(load case 5), 11=-87(load case 5), 12=-51(load case 5), 13=-171(load case 5)

Max Grav9=143(load case 2), 2=354(load case 2), 10=379(load case 2), 11=358(load case 2), 12=364(load case 2), 13=344(load case 2)

FORCES (ib) - First Load Case Only
TOP CHORD 1-2-32, 2-3=-111, 3-4=65, 4-5=73, 5-6=71, 6-7=72, 7-8=42, 8-9=-103
BOT CHORD 2-13=1, 12-13=1, 11-12=1, 10-11=1, 9-10=1
7-10=-279, 6-11=-262, 5-12=-267, 4-13=-250

NOTES

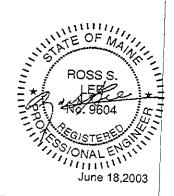
1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category III, condition I enclosed building, If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

Cable End Detail*

3) Gable requires continuous based at the design of the trust only.

Gable End Detail*
3) Gable requires continuous bottom chord bearing.
4) Gable studs spaced at 2-0-0 oc.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 98 ib uplift at joint 9, 108 ib uplift at joint 10, 87 ib uplift at joint 11, 51 ib uplift at joint 12 and 171 ib uplift at joint 13.
7) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard

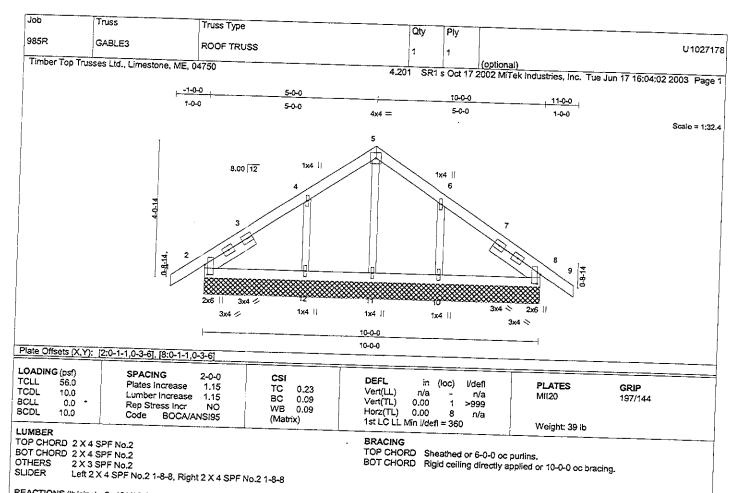


MITEK CANADA, INC. GENERAL SPECIFICATIONS (U.S.A.) DATED APRIL 1, 1997 FORM AN INTEGRAL PART OF THIS DESIGN.



100 Industrial Rd., P.O. Box 1329 Bradford, Ontario, L3Z 2B7





REACTIONS (lb/size) 2=461/10-0-0, 8=461/10-0-0, 11=76/10-0-0, 12=393/10-0-0, 10=393/10-0-0

(Ib/size) 2=451/10-0-0, 0=461/10-0-0, 1=1-0,

FORCES (ib) - First Load Case Only
TOP CHORD 1-2=32, 2-3=-313, 3-4=-161, 4-5=-242, 5-6=-242, 6-7=-161, 7-8=-313, 8-9=32
BOT CHORD 2-12=134, 11-12=134, 10-11=134, 8-10=134
WEBS 5-11=-57, 4-12=-322, 6-10=-322

NOTES

NOTES

1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from humicane oceanline, on an occupancy category III, condition I enclosed building, if porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

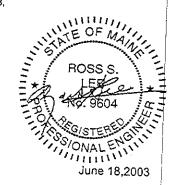
Gable End Detail*

2) Child Turns designed for wind loads in the plane of the truss only. For study exposed to wind (normal to the face), see MiTek "Standard"

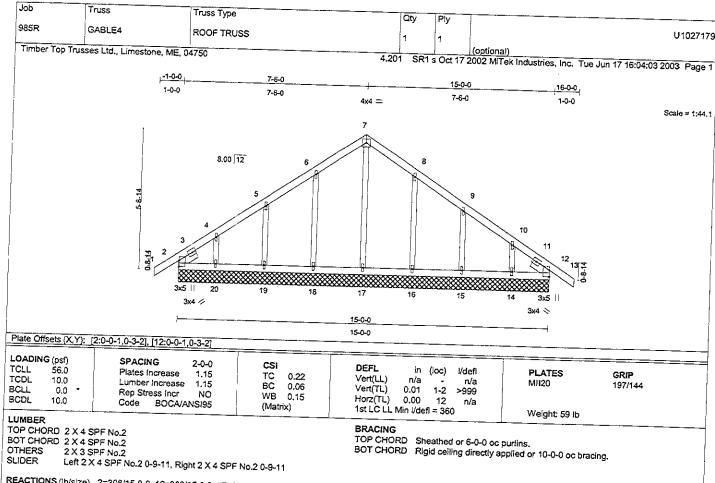
3) Gable requires continuous bottom chord bearing.

3) Gable requires continuous bottom chord bearing.
4) Gable studes spaced at 2-0-0 oc.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 111 lb uplift at joint 2, 111 lb uplift at joint 12 and 106 lb uplift at joint 10.
7) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard







REACTIONS (lb/size) 2=308/15-0-0, 12=308/15-0-0, 17=252/15-0-0, 18=299/15-0-0, 19=320/15-0-0, 20=218/15-0-0, 16=299/15-0-0, 15=320/15-0-0, 14=218/15-0-0

FORCES (lb) - First Load Case Only
TOP CHORD 1-2=32, 2-3=-146, 3-4=-77, 4-5=-117, 5-6=-124, 6-7=-122, 7-8=-122, 8-9=-124, 9-10=-117, 10-11=-20, 11-12=-146,
12-13=32
BOT CHORD 2-20=41, 19-20=41, 18-19=41, 17-18=41, 16-17=41, 15-16=41, 14-15=41, 12-14=41
WEBS 7-17=-212, 6-18=-260, 5-19=-276, 4-20=-198, 8-16=-260, 9-15=-276, 10-14=-198

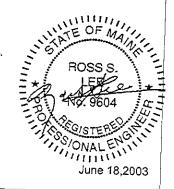
- NOTES

 1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category !!!, condition i enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard 3) All plates are 1v4 Mil20 unless otherwise indicated.
- 3) All plates are 1x4 MII20 unless otherwise indicated.
 4) Gable requires continuous bottom chord bearing.

- 4) Gable requires continuous bottom chore bearing.
 5) Gable studs spaced at 2-0-0 oc.
 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between
- 6) This truss has been designed for a live load of 20.0pst on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.
 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 2, 46 lb uplift at joint 12, 75 lb uplift at joint 18, 79 lb uplift at joint 19, 97 lb uplift at joint 20, 72 lb uplift at joint 16, 79 lb uplift at joint 15 and 82 lb uplift at joint 14.
 8) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard



MITEK CANADA, INC. GENERAL SPECIFICATIONS (U.S.A.) DATED APRIL 1, 1997 FORM AN INTEGRAL PART OF THIS DESIGN.



MiTek Canada, Inc. 100 Industrial Rd., P.O. Box 1329 Bradford, Ontario, L3Z 2B7



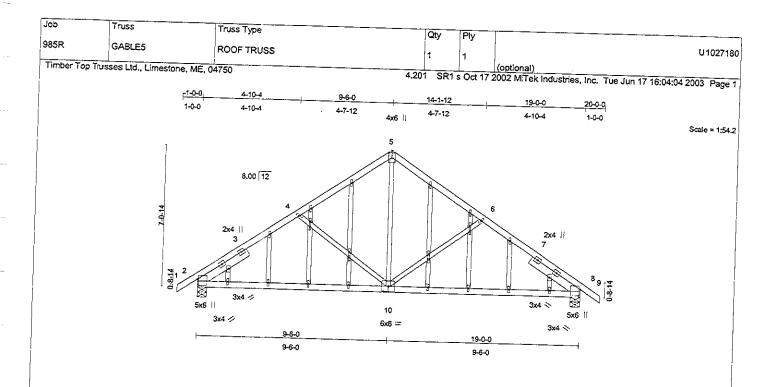


Plate Offsets (X,Y): [2:0-0-7,0-3-14], [8:0-0	-7,0-3-14]					
LOADING (psf) SPACING	e 1.15	CSI TC 0.65 BC 0.60 WB 0.54 (Matrix)	DEFL in (loc) I/defl Vert(LL) -0.06 10 >999 Vert(TL) -0.17 8-10 >999 Horz(TL) 0.04 8 n/a 1st LC LL Min I/defl = 360	PLATES Mil20 Weight: 93 jb	GRIP 197/144	

BRACING

LUMBER
TOP CHORD 2 X 4 SPF No.2
BOT CHORD 2 X 4 SPF No.2
WEBS 2 X 3 SPF No.2
OTHERS 2 X 3 SPF No.2
SLIDER Left 2 X 5 SPF 1650F 1.5E 2-10-10, Right 2 X 5 SPF 1650F 1.5E 2-10-10 TOP CHORD Sheathed or 4-5-3 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

FORCES (ib) - First Load Case Only
TOP CHORD 1-2=32, 2-3=-1930, 3-4=-1629, 4-5=-1445, 5-6=-1445, 6-7=-1628, 7-8=-1930, 8-9=32
BOT CHORD 2-10=1436, 8-10=1436
WEBS 4-10=-464, 5-10=763, 6-10=-464

NOTES

- 1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category III, condition I enclosed building, If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

 Sable End Detail*

 2) Design tend to the face), see MiTek "Standard."

- Gable End Detail*

 3) Design load is based on 56.0 psf specified roof snow load.

 4) Unbalanced snow loads have been considered for this design.

 5) All plates are 1x4 Mil20 unless otherwise indicated.

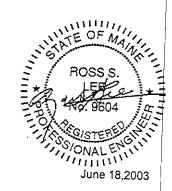
 6) Gable studs spaced at 2-0-0 oc.

 7) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.

 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 243 lb uplift at joint 2 and 243 lb uplift at ioint 8.
- joint 8.

 9) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard

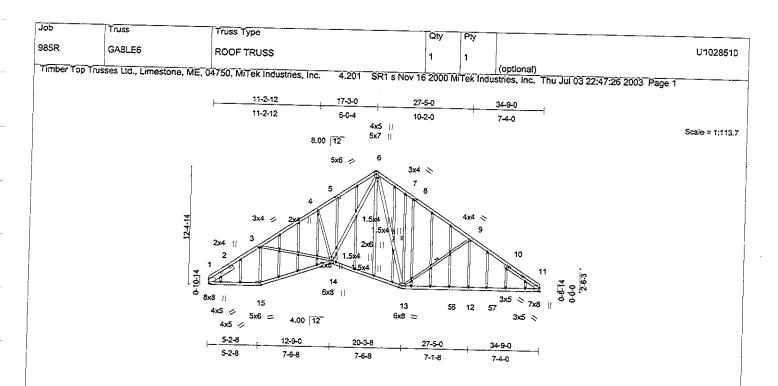


MITEK CANADA, INC. GENERAL SPECIFICATIONS (U.S.A.) DATED APRIL 1, 1997 FORM AN INTEGRAL PART OF THIS DESIGN.



MiTek Canada, Inc. 100 Industrial Rd., P.O. Box 1329 Bradford, Ontario, L3Z 2B7





LOADING TCLL TCDL BCLL BCDL	(psf) 56.0 10.0 0.0 *	[1:0-0-1,0-5-15], [3:0-1-1 SPACING Plates Increase Lumber Increase Rep Stress Incr Code BOCA/AN	2-0-0 1.15 1.15 YES	CSI TC 0.97 BC 0.99 WB 0.96 (Matrix)	DEFL Vert(LL) -0 Vert(TL) -0	in (loc) 1.28 14-15 2.42 14-15 2.24 11	l/defl >999 >984 n/a	PLATES MII20	GRIP 197/144
LUMBER TOP CHORI	D 2X45	SPF 2100F 1.8E *Except*		V	BRACING			Weight: 229 lb	_
BOT CHORE	6-82X 2	4 SPF 1650F 1.5E SPF No.2 *Except* X 4 SPF 1650F 1.5E			TOP CHORD BOT CHORD	Sheathed or 2 Rigid ceiling of 8-8-9 oc braci	directly appli	rfins. ed or 10-0-0 oc bracing	. Except:

2 X 4 SPF No.2 "Except" 1-152 X 4 SPF 1650F 1.5E 2 X 3 SPF No.2 "Except" 6-14 2 X 4 SPF No.2, 6-13 2 X 4 SPF No.2 2 X 3 SPF No.2 "Except" 6-16 2 X 4 SPF No.2

OTHERS SLIDER

Left 2 X 6 SPF 1650F 1.5E 3-0-7, Right 2 X 4 SPF No.2 4-4-8

(lb/size) 1=2677/Mechanical, 11=2777/Mechanical Max Horz 1=-343(load case 4) Max Uplift 1=-342(load case 6), 11=-342(load case 6) REACTIONS

FORCES (lb) - First Load Case Only TOP CHORD 1-2=3946, 2-3=-374

1-2=-3946, 2-3=-3744, 3-4= -4429, 4-5=-4182, 5-6=-3897, 6-7=-2951, 7-8=-2615, 8-9=-3080,

1-2=-3946, 2-3=-3/44, 3-4=-44.29, 4-5=-4162, 5-6=-3169, 12-57=3161, 11-57=3161, 11-57=3161, 11-57=3161, 11-57=3161, 3-15=-896, 3-14=-574, 4-14=-643, 6-14=2655, 6-13=774, 7-13=-710, 9-13=-1029, 9-12=301

WEBS

NOTES

BOT CHORD

WEBS

NOTES

1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category III, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

2) Truss designed for wind loads in the plane of the truss only. For study exposed to wind (normal to the face), see MTek "Standard Gable End Detail"

3) Design load is based on 56.0 psf specified roof snow load.
4) Unbalanced snow loads have been considered for this design.
5) All plates are 1x4 Mil20 unless otherwise indicated.
6) Gable studs spaced at 2-0-0 oc.
7) *This truss has been designed for a line lead of 00 and 6 in the lead.

ntinued on page 2

7) "This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.

8) Refer to girder(s) for truss to truss connections.

MITEK CANADA, INC. GENERAL SPECIFICATIONS (U.S.A.) DATED APRIL 1, 1997 FORM AN INTEGRAL PART OF THIS DESIGN.



MiTek Canada, Inc.

100 Industrial Rd., P.O. Box 1329 Bradford, Ontario, L3Z 2B7



LOADING AND DIMENSIONS SPECIFIED BY FABRICATOR, SUBJECT TO VERIFICATION BY AUTHORITIES IN JURISDICTION.

ATE OF MA ROSS S SO ONAL EN WALENT

6-13, 7-13, 9-13

July 3,2003

Job	Truss	Truss Type			
985R	GABLE6	ROOF TRUSS	Qty	Ply	
Timber Ton True			1	1	U102851
NOTES	ises Etd., Crinestone, ME, 0	4750, MiTek Industries, Inc. 4.201 SR1 s Nov 16	2000 Mi	lek Indu	stries, Inc. Thu Jul 03 22:47:26 2003 Page 2

NOTES
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 342 lb uplift at joint 1 and 342 lb uplift at joint 11.
10) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard



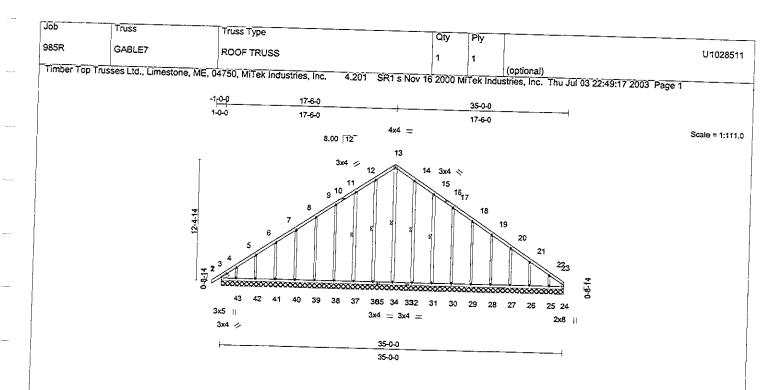


Plate Offsets	(X,Y):	[2:0-0-1,0-3-2], [24:0-1-0,0-2-12]			<u> </u>		
TCLL TCDL BCLL BCDL	(psf) 56.0 10.0 0.0	SPACING 2-0-0 Plates Increase 1.15 Lumber Increase 1.15 Rep Stress Incr NO Code 80CA/ANSI95	CSI TC 0.36 BC 0.18 WB 0.51 (Matrix)	DEFL in (loc Vert(LL) n/a Vert(TL) 0.01 1- Horz(TL) -0.02 1st LC LL Min I/deft = 360	- n/a 2 >999 2 n/a	PLATES MII20 Weight: 193 lb	GRIP 197/144
LUMBER TOP CHORD BOT CHORD WEBS OTHERS	2 X 4 S 2 X 3 S 2 X 3 S 13-34 2 11-37 2 15-31 2	PF No.2 PF No.2 PF No.2 PF No.2 "Except" X 4 SPF No.2, 12-36 2 X 4 SPF No X 4 SPF No.2, 14-32 2 X 4 SPF No X 4 SPF No.2 4 SPF No.2	.2 2	DOT CHORD Rigia ce	ed or 6-0-0 oc p siling directly ap at midpt	ourlins, except end verti pplied or 10-0-0 oc bracin 13-34, 12-36, 11-3	na.
REACTIONS	(lb/size) Max Horz Max Uplifi	24=112/35-0-0, 2=302/35-0-0, 35 38=300/35-0-0, 39=306/35-0-0, 4 43=221/35-0-0, 32=290/35-0-0, 3 28=304/35-0-0, 27=302/35-0-0, 2 24=382(load case 5), 2=-69(load 38=-72(load case 5), 39=-75(load 42=-71(load case 6), 43=-134(load 30=-72(load case 4), 29=-76(load 26=64(load case 6), 25=-220(load	1=307/35-0-0, 41=307/ 1=310/35-0-0, 30=300/ 6=311/35-0-0, 25=269/ case 4), 36=-60(load c case 6), 40=-74(load c d case 5), 32=-67(load case 4), 38=-74(load c	35-0-0, 42=319/35-0-0, 35-0-0, 29=306/35-0-0, 35-0-0, 33=12/35-0-0 ase 5), 37=-36(load case 6), ase 6), 41=-77(load case 5),	5-0-0,		

2b=-64(load case 6), 25=-220(load case 4)
24=312(load case 4), 2=344(load case 2), 35=12(load case 3), 34=287(load case 6),
36=360(load case 2, 37=366(load case 2), 38=355(load case 2), 39=362(load case 2),
40=360(load case 2), 41=356(load case 2), 42=377(load case 2), 43=267(load case 2),
32=359(load case 3), 31=366(load case 3), 30=355(load case 3), 29=362(load case 3),
28=360(load case 3), 27=358(load case 3), 26=368(load case 3), 25=326(load case 3),
33=12(load case 3)

FORCES (ib) - First Load Case Only
TOP CHORD

1-2=32, 2-3=-138, 3-4=-13, 4-5=-110, 5-6=-116, 6-7=-114, 7-8=-114, 8-9=-115, 9-10=-113, 10-11=33, 11-12=-116, 12-13=-114, 13-14=-114, 14-15=-116, 15-16=33, 16-17=-113, 17-18=-115, 18-19=-114, 19-20=-115, 20-21=-115, 21-22=-112, 22-23=-83, 23-24=-98

BOT CHORD

2-43=34, 42-43=34, 41-42=34, 40-41=34, 39-40=34, 38-39=34, 37-38=34, 36-37=34, 35-36=34, 34-35=34, 33-34=34, 32-33=34, 31-32=34, 30-31=34, 29-30=34, 28-29=34, 27-28=34, 26-27=34, 25-26=34, 24-25=34

WERS

WERS

13-34=-292, 12-36=-260, 11-37=-269, 9-38=-260, 8-39=-266, 7-40=-264, 6-41=-262, 5-42=-275,

25-26-34, 24-25-34 13-34-222, 12-36-260, 11-37-269, 9-38-260, 8-39-266, 7-40-264, 6-41-262, 5-42-275, 4-43-200, 14-32-260, 15-31-269, 17-30-260, 18-29-266, 19-28-264, 20-27-263, 21-26-270, 22-25=-237

Continued on page 2



MiTek Canada, Inc. 100 Industrial Rd., P.O. Box 1329

Bradford, Ontario, L3Z 2B7



LOADING AND DIMENSIONS SPECIFIED BY FABRICATION, SUBJECT TO VERIFICATION BY AUTHORITIES IN JURISDICTION.

SS/ONAL ENGINE

ROSS S

	Job	Truss	Truss Type	Qty	Ply	
	985R	GABLE7	ROOF TRUSS	1	,	U1028511
	Timber Top Trus	ses Ltd., Limestone, ME, 0	4750, MiTek Industries, Inc. 4.201 SR1 s Nov 16	2000 Mi	Tek Indus	(optional) stries, Inc. Thu Jul 03 22:49:17 2003 Page 2
۱	NOTEC					The term of the following the

NOTES

- 1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category III, condition I enclosed building, of dimensions 45 ft by 24 exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

 Detail*

 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End

- Detail*

 3) All plates are 1x4 Mil20 unless otherwise indicated.

 4) Gable requires continuous bottom chord bearing.

 5) Gable studs spaced at 2-0-0 oc.

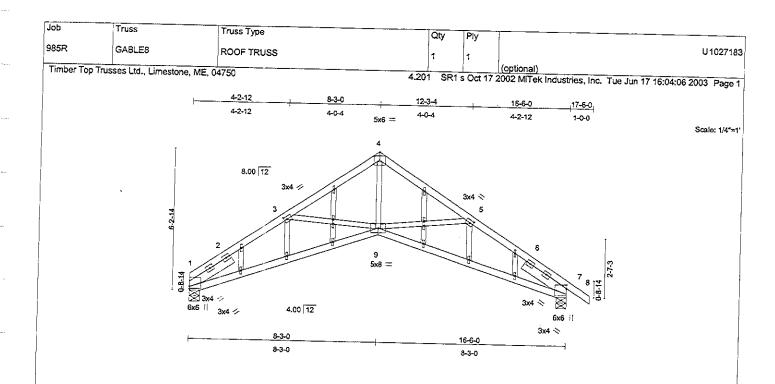
 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.

 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 113 lb uplift at joint 24, 69 lb uplift at joint 2, 60 lb uplift at joint 36, 86 lb uplift at joint 37, 72 lb uplift at joint 38, 75 lb uplift at joint 39, 74 lb uplift at joint 40, 77 lb uplift at joint 41, 71 lb uplift at joint 42, 134 27, 64 lb uplift at joint 25 and 220 lb uplift at joint 25.

 8) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard





LOADING (psf) TCLL 56.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING 2-0-0 Plates Increase 1.15 Lumber Increase 1.15 Rep Stress Incr YES Code BOCA/ANSI95	CSI TC 0.77 BC 0.76 WB 0.50 (Matrix)	DEFL in (loc) I/defi Vert(LL) -0.15 9 >999 Vert(TL) -0.21 1-9 >932 Horz(TL) 0.19 7 n/a 1st LC LL Min I/defi = 360	PLATES GRIP MII20 197/14 Weight: 68 lb	
--	--	--	---	--	--

LUMBER
TOP CHORD 2 X 4 SPF No.2
BOT CHORD 2 X 4 SPF No.2
WEBS 2 X 3 SPF No.2
OTHERS 2 X 3 SPF No.2
SLIDER Left 2 X 5 SPF 1650F 1.5E 2-3-0, Right 2 X 5 SPF 1650F 1.5E 2-3-0

REACTIONS (lb/size) 1=1235/0-5-8, 7=1389/0-5-8 Max Horz 1=-182(load case 4) Max Uplift1=-158(load case 6), 7=-224(load case 6)

FORCES (ib) - First Load Case Only
TOP CHORD 1-2=-2579, 2-3=-2431, 3-4=-2017, 4-5=-2016, 5-6=-2411, 6-7=-2563, 7-8=18
BOT CHORD 1-9=2007, 7-9=1981
WEBS 3-9=-272, 4-9=1452, 5-9=-247

NOTES

- 1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category !!!, condition I enclosed building, If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

 Gable End Detail"

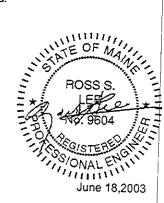
 3 Designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard"

- 3) Design load is based on 56.0 psf specified roof snow load.
 4) Unbalanced snow loads have been considered for this design.
 5) All plates are 1x4 Mil20 unless otherwise indicated.

- 5) All plates are 1x4 MII20 unless otherwise indicated.
 6) Gable studs spaced at 2-0-0 oc.
 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.
 8) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1-1995 angle to grain formula. Building designer should verify capacity of bearing surface.
 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 158 lb uplift at joint 1 and 224 lb uplift at loint 7.

10) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard



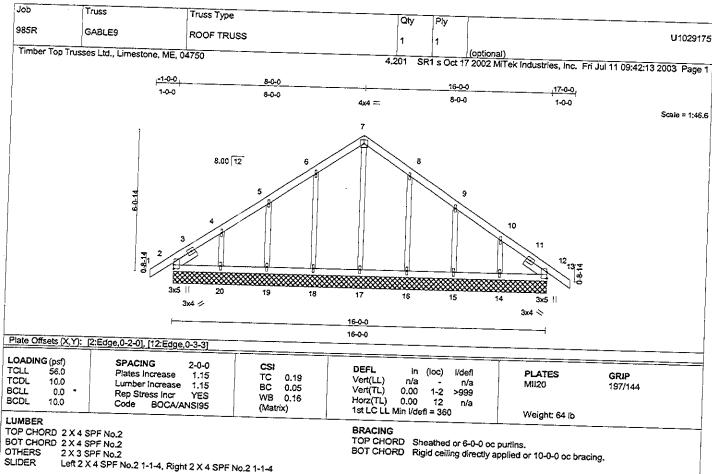
MITEK CANADA, INC. GENERAL SPECIFICATIONS (U.S.A.) DATED APRIL 1, 1997 FORM AN INTEGRAL PART OF THIS DESIGN.





TOP CHORD Sheathed or 2-10-9 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.



Left 2 X 4 SPF No.2 1-1-4, Right 2 X 4 SPF No.2 1-1-4

REACTIONS (lb/size) 2=344/16-0-0, 12=344/16-0-0, 17=234/16-0-0, 18=300/16-0-0, 19=311/16-0-0, 20=276/16-0-0, 16=300/16-0-0, 15=311/16-0-0, 14=276/16-0-0 Max Horz2=-172(load case 4)

Max Uplift2=-83(load case 4), 12=-43(load case 6), 18=-76(load case 5), 19=-77(load case 6), 20=-106(load case 5), 16=-71(load case 4), 15=-77(load case 6), 16=-71(load case 6), 16=-71(load

14=-95(load case 4)
Max Grav2=387(load case 2), 12=387(load case 3), 17=234(load case 1), 18=372(load case 2), 19=365(load case 2), 20=334(load case 2), 16=372(load case 3), 15=365(load case 3), 14=334(load case 3)

FORCES (lb) - First Load Case Only
TOP CHORD 1-2=32, 2-3=-181, 3-4=-23, 4-5=-137, 5-6=-141, 6-7=-139, 7-8=-139, 8-9=-141, 9-10=-137, 10-11=-21, 11-12=-181, 12-13=-32

BOT CHORD 2-20=55, 19-20=55, 18-19=55, 17-18=55, 16-17=55, 15-16=55, 14-15=55, 12-14=55
WEBS 7-17=-194, 6-18=-260, 5-19=-271, 4-20=-237, 8-16=-260, 9-15=-271, 10-14=-237

NOTES

10) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category III, condition I enclosed building, If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

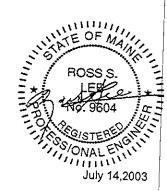
Cable End Detail*

3) All plates are 1/4 MICO unloss at particular in the plane of the truss only.

Gable End Detail*
3) All plates are 1x4 Mili20 unless otherwise indicated.
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 83 lb uplift at joint 2, 43 lb uplift at joint 12, 76 lb uplift at joint 18, 77 lb uplift at joint 19, 106 lb uplift at joint 20, 71 lb uplift at joint 16, 77 lb uplift at joint 15 and 93 lb uplift at joint 14.

8) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard



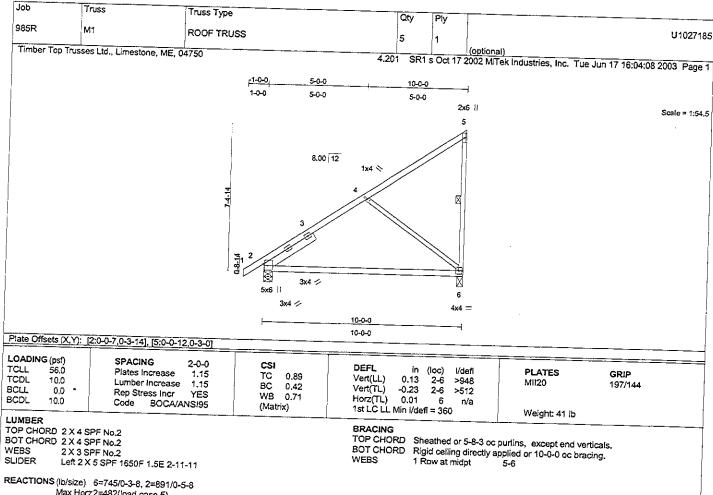
MITEK CANADA, INC. GENERAL SPECIFICATIONS (U.S.A.) DATED APRIL 1, 19 FORM AN INTEGRAL PART OF THIS DESIGN.



MiTek Canada, Inc. 100 Industrial Rd., P.O. Box 1329 Bradford, Ontario, L3Z 2B7



LOADING AND DIMENSIONS SPECIFIED BY FABRICATOR



REACTIONS (lb/size) 6=745/0-3-8, 2=891/0-5-8

Max Horz2=482(load case 5)

Max Uplif6=-259(load case 5), 2=-67(load case 4)

Max Grav6=883(load case 2), 2=1059(load case 2)

FORCES (lb) - First Load Case Only TOP CHORD 1-2=32, 2-3=-797, 3-4=-644, 4-5=124, 5-6=-261 BOT CHORD 2-6=545 WEBS 4-6=-652

NOTES

1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category III, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANSI95 if end verticals or cantilevers exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

2) Design load is based on 56.0 psf specified roof snow load.

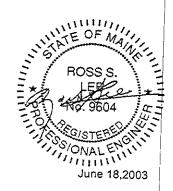
3) Unbalanced snow loads have been considered for this design.

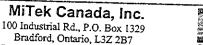
4) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 259 lb uplift at Joint 6 and 67 lb uplift at joint 2.

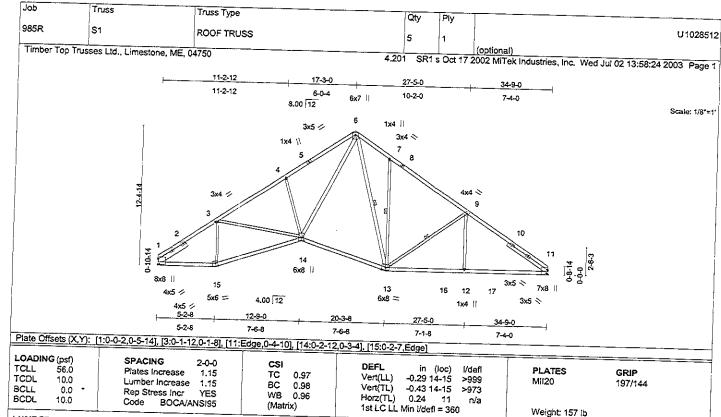
6) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard









BRACING TOP CHORD BOT CHORD

WEBS

LUMBER
TOP CHORD 2 X 4 SPF 2100F 1.8E "Except"
6-8 2 X 4 SPF 1650F 1.5E
BOT CHORD 2 X 4 SPF No.2 "Except"
1-15 2 X 4 SPF 1650F 1.5E
WEBS 2 X 3 SPF No.2 "Except"
6-14 2 X 4 SPF No.2, 6-13 2 X 4 SPF No.2
1 off 2 X 6 SPF 1650F 1.5E 3-0-7, Right 2 X

Left 2 X 6 SPF 1650F 1.5E 3-0-7, Right 2 X 4 SPF No.2 4-4-8

REACTIONS (lb/size) 1=2677/Mechanical, 11=2777/Mechanical

Max Horz1=-347(load case 4)
Max Uplift1=-342(load case 6), 11=-342(load case 6)

FORCES (ib) - First Load Case Only
TOP CHORD 1-2=-3944, 2-3=-3741, 3-4=-4441, 4-5=-4238, 5-6=-3995, 6-7=-2940, 7-8=-2615, 8-9=-3080, 9-10=-3646, 10-11=-4093
BOT CHORD 1-15=2969, 14-15=3137, 13-14=2288, 13-16=3170, 12-16=3170, 12-17=3163, 11-17=3163
WEBS 3-15=-892, 3-14=599, 4-14=-785, 6-14=2765, 6-13=760, 7-13=-689, 9-13=-1032, 9-12=301

NOTES

1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category III, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANSI95 If end verticals or cantilevers exist, they are exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

2) Design load is based on 56.0 psf specified roof snow load.

3) Unbalanced snow loads have been considered for this design.

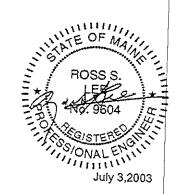
4) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 342 lb uplift at joint 1 and 342 lb uplift at joint 1.

7) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard



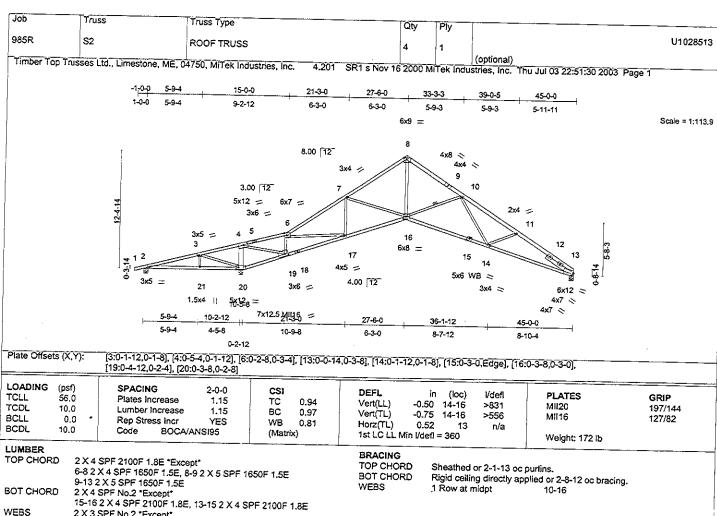
MITEK CANADA, INC. GENERAL SPECIFICATIONS (U.S.A.) DATED APRIL 1, 19 FORM AN INTEGRAL PART OF THIS DESIGN



100 Industrial Rd., P.O. Box 1329 Bradford, Ontario, L3Z 2B7



Sheathed or 2-3-11 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing. Except: 8-8-13 oc bracing: 14-15.
1 Row at midpt 6-13, 7-13, 9-13



WEBS

BOT CHORD

2 X 3 SPF No.2 'Except' 4-20 2 X 6 SPF 1650F 1.5E, 8-16 2 X 4 SPF No.2 4-19 2 X 3 SPF 2100F 1.8E

SLIDER Right 2 X 5 SPF 1650F 1.5E 3-5-1

REACTIONS (lb/size) 2=-58/0-5-8, 20=4662/0-5-8, 13=2350/0-5-8

Max Horz 2=360(load case 5)
Max Uplift 2=-227(load case 4), 20=-579(load case 6), 13=-309(load case 6) Max Grav 2=109(load case 2), 20=4662(load case 1), 13=2351(load case 3)

FORCES (ib) TOP CHORD

1-2=29, 2-3=2350, 3-4=3883, 4-5=-1168, 5-6=-1031, 6-7=-3971, 7-8=-3691, 8-9=-3305, 9-10=-3694, 10-11=-5129, 11-12=-5542, 12-13=-5767
2-21=-2174, 20-21=-2161, 19-20=-3902, 18-19=1226, 17-18=1257, 16-17=3323, 15-16=4191, 14-15=4150, 42-4450

14-15=4150, 13-14=4673

WE8\$ 3-21=137, 3-20=1620, 4-20=2886, 6-19=-2593, 6-17=1958, 7-17=-600, 7-16=-266, 8-16=3048, 10-16=-1147, 10-14=477, 11-14=-271, 4-19=4894

NOTES

BOT CHORD

- NOTES

 1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category III, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCAVANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

 2) Design load is based on 56.0 psf specified roof snow load.

 3) Unbalanced snow loads have been considered for this design.

 4) All plates are Mil20 plates unless otherwise indicated.

- Unbalanced snow loads have been considered for this design.
 All plates are Mil20 plates unless otherwise indicated.
 *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.
 Bearing at joint(s) 20, 13 considers parallel to grain value using ANSI/TPI 1-1995 angle to grain formula. Building designer should verify capacity of bearing surface.
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 227 lb uplift at joint 2, 579 lb uplift at joint 20 and 309 lb uplift at joint 13.
 This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard



MiTek Canada, Inc. 100 Industrial Rd., P.O. Box 1329 Bradford, Ontario, L3Z 2B7



Rigid ceiling directly applied or 2-8-12 oc bracing.

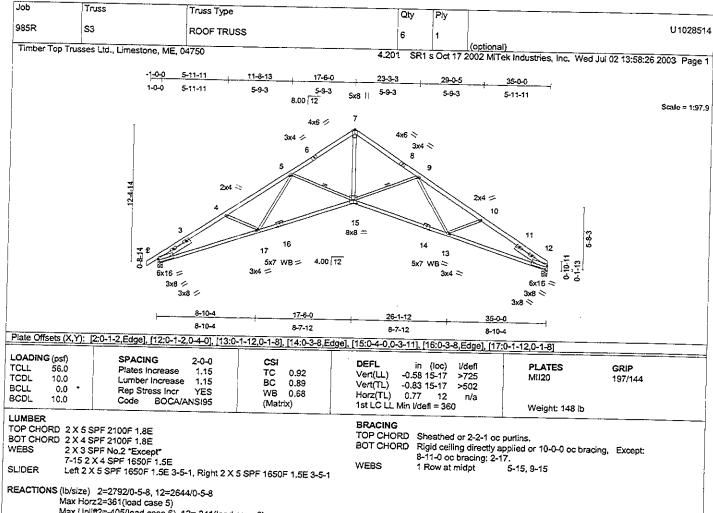
1 Row at midpt 10-16

LOADING AND DIMENSIONS SPECIFIED BY FABRICATOR SUBJECT TO VERIFICATION BY AUTHORITIES IN JURISDICTION.

SONAL ENG WHITE IN

July 3,2003

BOS BOS



REACTIONS (lb/size) 2=2792/0-5-8, 12=2644/0-5-8 Max Horz 2=361(load case 5) Max Uplift2=-405(load case 6), 12=-341(load case 6)

FORCES (lb) - First Load Case Only
TOP CHORD 1-2=24, 2-3=-6564, 3-4=-6337, 4-5=-5976, 5-6=-4672, 6-7=-4450, 7-8=-4450, 8-9=-4671, 9-10=-5993, 10-11=-6361,
11-12=-6586
BOT CHORD 2-17=5326, 16-17=4962, 15-16=5004, 14-15=5010, 13-14=4968, 12-13=5353
WEBS 4-17=-168, 5-17=358, 5-15=-1100, 7-15=4187, 9-15=-1106, 9-13=373, 10-13=-182

NOTES

NOTES

1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 ml from hurricane oceanline, on an occupancy category III, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

2) Design load is based on 56.0 psf specified roof snow load.

3) Unbalanced snow loads have been considered for this design.

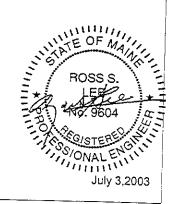
4) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.

5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1-1995 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 405 lb uplift at joint 2 and 341 lb uplift at joint 12.

7) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard

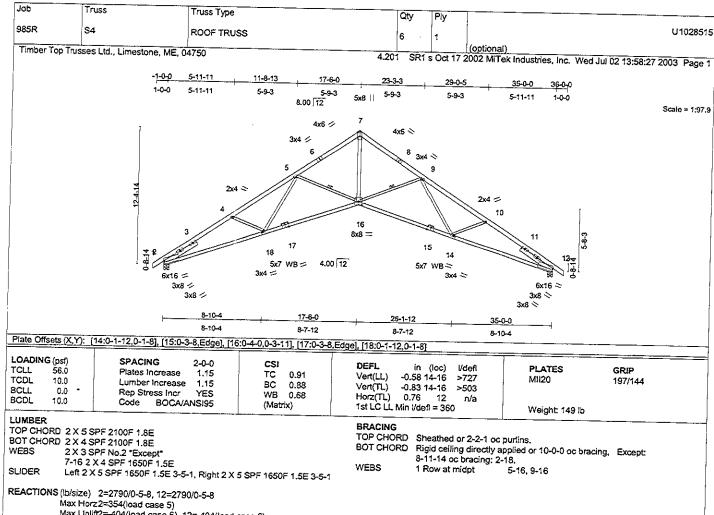


MITEK CANADA, INC. GENERAL SPECIFICATIONS (U.S.A.) DATED APRIL 1, 1997 FORM AN INTEGRAL PART OF THIS DESIGN.



100 Industrial Rd., P.O. Box 1329 Bradford, Ontario, L3Z 2B7





1 Row at midpt

REACTIONS (lb/size) 2=2790/0-5-8, 12=2790/0-5-8 Max Horz2=354(load case 5) Max Uplift2=-404(load case 6), 12=-404(load case 6)

FORCES (lb) - First Load Case Only
TOP CHORD 1-2=24, 2-3=-6557, 3-4=-6330, 4-5=-5969, 5-6=-4664, 6-7=-4442, 7-8=-4442, 8-9=-4664, 9-10=-5969, 10-11=-6330,
11-12=-6557, 12-13=24

BOT CHORD 2-18=5321, 17-18=4956, 16-17=4998, 15-16=4998, 14-15=4956, 12-14=5321

WEBS 4-18=-168, 5-18=358, 5-16=-1100, 7-16=4180, 9-16=-1100, 9-14=358, 10-14=-168

NOTES

NOTES

1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceaniline, on an occupancy category iil, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANSI95 if end verticals or cantilevers exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

2) Design load is based on 56.0 psf specified roof snow load.

3) Unbalanced snow loads have been considered for this design.

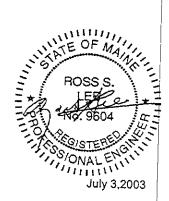
4) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.

5) Bearing at joint(s) 2, 12 considers parallel to grain value using ANSI/TPI 1-1995 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 404 lb uplift at joint 2 and 404 lb uplift at joint 12.

7) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard



MITEK CANADA, INC. GENERAL SPECIFICATIONS (U.S.A.) DATED APRIL 1, 1997 FORM AN INTEGRAL PART OF THIS DESIGN.

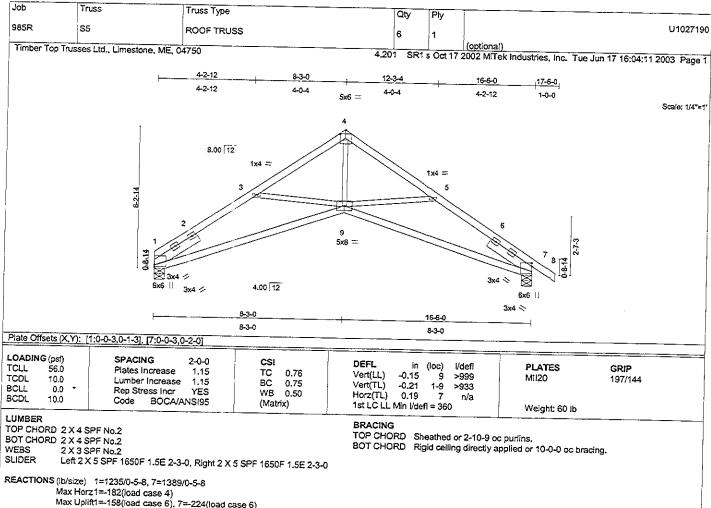


MiTek Canada, Inc. 100 Industrial Rd., P.O. Box 1329 Bradford, Ontario, L3Z 2B7



Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 8-11-14 oc bracing: 2-18.

5-16, 9-16



Max Horz1=-182(load case 4)
Max Uplift1=-158(load case 6), 7=-224(load case 6)

FORCES (lb) - First Load Case Only
TOP CHORD 1-2=-2581, 2-3=-2432, 3-4=-2018, 4-5=-2017, 5-6=-2413, 6-7=-2564, 7-8=18
BOT CHORD 1-9=2008, 7-9=1983
WEBS 3-9=-273, 4-9=1453, 5-9=-248

NOTES

- NOTES

 1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category III, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANSI95 If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

 2) Design load is based on 56.0 psf specified roof snow load.

 3) Unbalanced snow loads have been considered for this design.

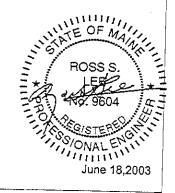
 4) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.

 5) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1-1995 angle to grain formula. Building designer should verify capacity of bearing surface.

 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 158 lb uplift at joint 1 and 224 lb uplift at joint 7.

 7) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard

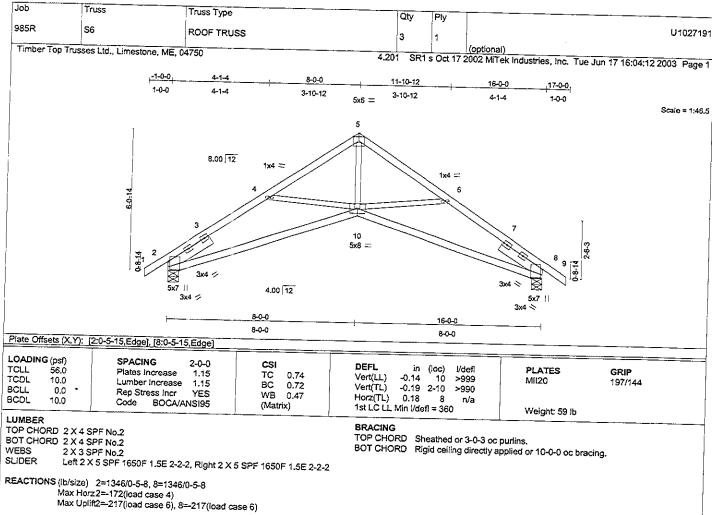


MITEK CANADA, INC. GENERAL SPECIFICATIONS (U.S.A.) DATED APRIL 1, 1997 FORM AN INTEGRAL PART OF THIS DESIGN.



MiTek Canada, Inc. 100 Industrial Rd., P.O. Box 1329 Bradford, Ontario, L3Z 2B7





REACTIONS (lb/size) 2=1346/0-5-8, 8=1346/0-5-8 Max Horz 2=-172(load case 4) Max Uplift2=-217(load case 6), 8=-217(load case 6)

FORCES (lb) - First Load Case Only
TOP CHORD 1-2=18, 2-3=-2450, 3-4=-2303, 4-5=-1927, 5-6=-1927, 6-7=-2303, 7-8=-2449, 8-9=18
BOT CHORD 2-10=1886, 8-10=1886
WEBS 4-10=-229, 5-10=1371, 6-10=-229

NOTES

- NOTES

 1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category III, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANSI95 if end verticals or cantilevers exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

 2) Design load is based on 56.0 psf specified roof snow load.

 3) Unbalanced snow loads have been considered for this design.

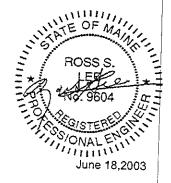
 4) *This truss has been designed for a live load of 20.0 psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.

 5) Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1-1995 angle to grain formula. Building designer should verify capacity of bearing surface.

 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 217 lb uplift at joint 2 and 217 lb uplift at joint 8.

- joint 8.
 7) This truss has been designed with ANSI/TPI 1-1995 criteria.

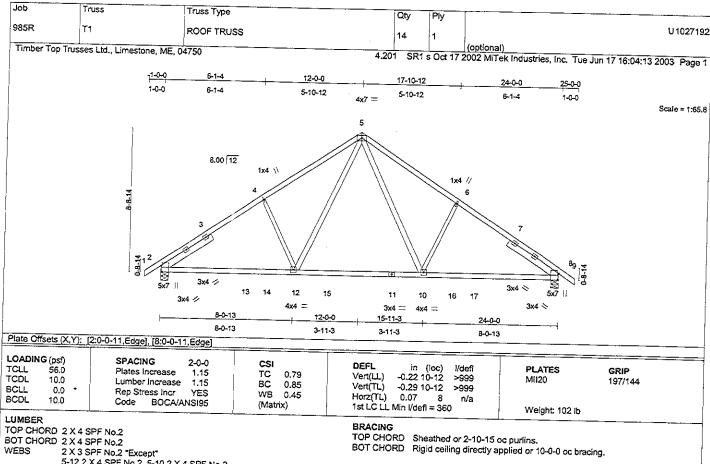
LOAD CASE(S) Standard











TOP CHORD 2 X 4 SPF No.2

BOT CHORD 2 X 4 SPF No.2

BOT CHORD 2 X 4 SPF No.2

WEBS 2 X 3 SPF No.2 *Except*

5-12 2 X 4 SPF No.2, 5-10 2 X 4 SPF No.2

SLIDER Left 2 X 5 SPF 1650F 1.5E 3-7-10, Right 2 X 5 SPF 1650F 1.5E 3-7-10

REACTIONS (lb/size) 2=2088/0-5-8, 8=2088/0-5-8 Max Horz 2=249(load case 5) Max Uplift2=-292(load case 6), 8=-292(load case 6)

FORCES (lb) - First Load Case Only
TOP CHORD 1-2=32, 2-3=-2738, 3-4=-2359, 4-5=-2405, 5-6=-2405, 6-7=-2359, 7-8=-2738, 8-9=32
BOT CHORD 2-13=2068, 13-14=2068, 12-14=2068, 12-15=1464, 11-15=1464, 10-11=1464, 10-16=2069, 16-17=2069, 8-17=2069
WEBS 4-12=-571, 5-12=891, 5-10=891, 6-10=-571

NOTES

1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category III, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

2) Design load is based on 56.0 psf specified roof snow load.

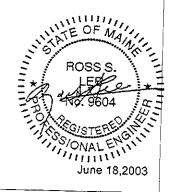
3) Unbalanced snow loads have been considered for this design.

4) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 292 lb uplift at joint 2 and 292 lb uplift at joint 8.

joint 8.
6) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(\$) Standard

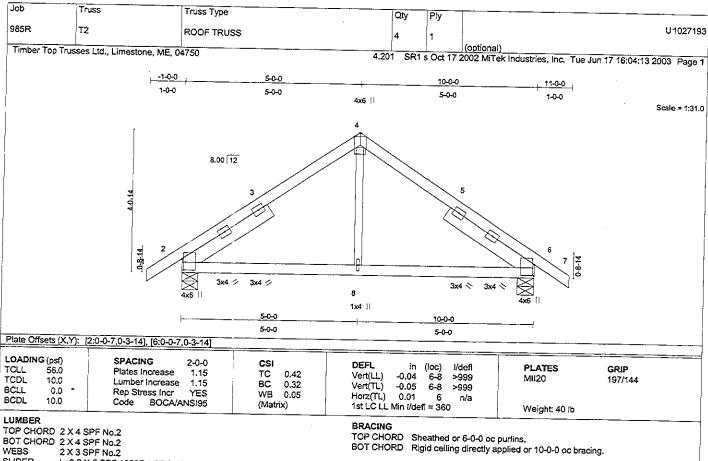


MITEK CANADA, INC. GENERAL SPECIFICATIONS (U.S.A.) DATED APRIL 1, 19 FORM AN INTEGRAL PART OF THIS DESIGN









TOP CHORD 2 X 4 SPF No.2 BOT CHORD 2 X 4 SPF No.2 WEBS 2 X 3 SPF No.2 WEBS SLIDER

Left 2 X 5 SPF 1650F 1.5E 2-10-14, Right 2 X 5 SPF 1650F 1.5E 2-10-14 REACTIONS (lb/size) 2=892/0-5-8, 6=892/0-5-8

Max Horz2=-115(load case 4)
Max Uplift2=-155(load case 6), 6=-155(load case 6)
Max Grav2=892(load case 2), 6=892(load case 1)

FORCES (lb) - First Load Case Only TOP CHORD 1-2=32, 2-3=-909, 3-4=-689, 4-5=-689, 5-6=-909, 6-7=32 BOT CHORD 2-8=573, 6-8=573 WEBS 4-8=139

NOTES

- NOTES

 1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category III, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANSI95 If end verticals or cantilevers exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

 2) Design load is based on 56.0 psf specified roof snow load.

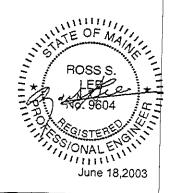
 3) Unbalanced snow loads have been considered for this design.

 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.

 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 155 lb uplift at joint 2 and 155 lb uplift at joint 6.

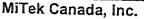
joint 6.
6) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard



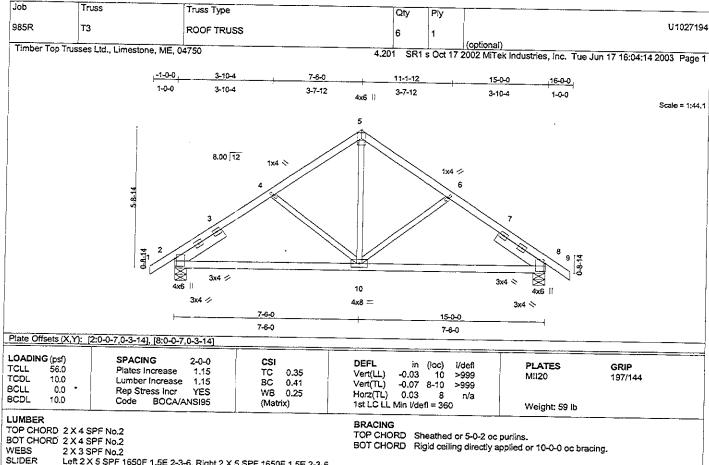
MITEK CANADA, INC. GENERAL SPECIFICATIONS (U.S.A.) DATED APRIL 1, 1997 FORM AN INTEGRAL PART OF THIS DESIGN.





100 Industrial Rd., P.O. Box 1329 Bradford, Ontario, L3Z 2B7





TOP CHORD 2 X 4 SPF No.2 BOT CHORD 2 X 4 SPF No.2 WEBS

2 X 3 SPF No.2 2 X 3 SPF No.2 Left 2 X 5 SPF 1650F 1.5E 2-3-6, Right 2 X 5 SPF 1650F 1.5E 2-3-6 SLIDER

REACTIONS (lb/size) 2=1272/0-5-8, 8=1272/0-5-8 Max Horz2=163(load case 5) Max Upiif(2=-204(load case 6), 8=-204(load case 6)

FORCES (lb) - First Load Case Only
TOP CHORD 1-2=32, 2-3=-1485, 3-4=-1362, 4-5=-1113, 5-6=-1113, 6-7=-1241, 7-8=-1484, 8-9=32
BOT CHORD 2-10=1083, 8-10=1083
WEBS 4-10=-329, 5-10=562, 6-10=-329

NOTES

- NOTES

 1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category III, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANSI95 if end verticals or cantilevers exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

 2) Design load is based on 56.0 psf specified roof snow load.

 3) Unbalanced snow loads have been considered for this design.

 4) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.

 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 204 lb uplift at joint 2 and 204 lb uplift at joint 8.

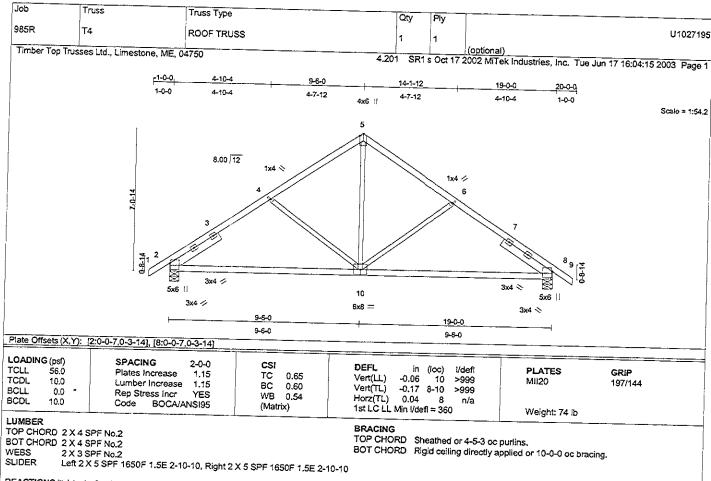
- 6) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard









REACTIONS (lb/size) 2=1576/0-5-8, 8=1576/0-5-8 Max Horz2=-201 (load case 4) Max Uplif(2=-243 (load case 6), 8=-243 (load case 6)

FORCES (lb) - First Load Case Only TOP CHORD 1-2=32, 2-3=-1930, 3-4=-1629, 4-5=-1445, 5-6=-1445, 6-7=-1628, 7-8=-1930, 8-9=32 BOT CHORD 2-10=1436, 8-10=1436 WEBS 4-10=-464, 5-10=763, 6-10=-464

- NOTES

 1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category III, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANSI95 If end verticals or cantilevers exist, they are not exposed to wind. The lumber OOL increase is 1.33, and the plate grip increase is 1.33.

 2) Design load is based on 56.0 psf specified roof snow load.

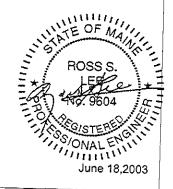
 3) Unbalanced snow loads have been considered for this design.

 4) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.

 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 243 lb uplift at joint 2 and 243 ib uplift at joint 8.

- joint 8.
 6) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard

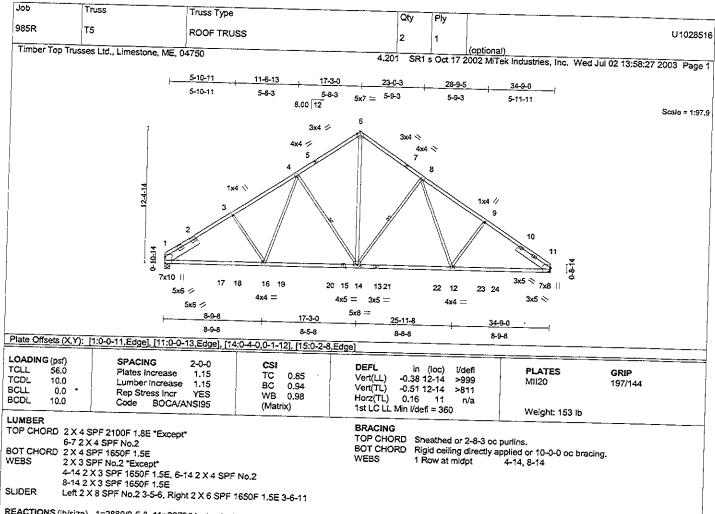


MITEK CANADA, INC. GENERAL SPECIFICATIONS (U.S.A.) DATED APRIL 1, 1997 FORM AN INTEGRAL PART OF THIS DESIGN.



MiTek Canada, Inc. 100 Industrial Rd., P.O. Box 1329 Bradford, Ontario, L3Z 2B7





Left 2 X 8 SPF No.2 3-5-6, Right 2 X 6 SPF 1650F 1.5E 3-6-11

REACTIONS (lb/size) 1=2880/0-5-8, 11=2873/Mechanical

Max Horz1=-347(load case 4)
Max Uplift1=-342(load case 6), 11=-342(load case 6)

FORCES (ib) - First Load Case Only
TOP CHORD 1-2=-4217, 2-3=-3987, 3-4=-3754, 4-5=-2815, 5-6=-2441, 6-7=-2439, 7-8=-2819, 8-9=-3863, 9-10=-4089, 10-11=-4273
BOT CHORD 1-17=3182, 17-18=3182, 16-19=3182, 16-19=2771, 19-20=2771, 15-20=2771, 14-15=2771, 13-14=2807, 13-21=2807, 21-22=2807, 12-22=2807, 12-23=3312, 23-24=3312, 11-24=3312
3-16=-381, 4-16=649, 4-14=-1069, 6-14=2147, 8-14=-1118, 8-12=748, 9-12=-470

NOTES

1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category III, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANSI95 If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

2) Design load is based on 56.0 psf specified roof snow load.

3) Unbalanced snow loads have been considered for this design.

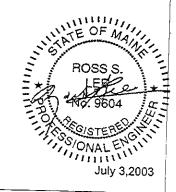
1) "This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 342 lb uplift at joint 1 and 342 lb uplift at joint 1.

7) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard



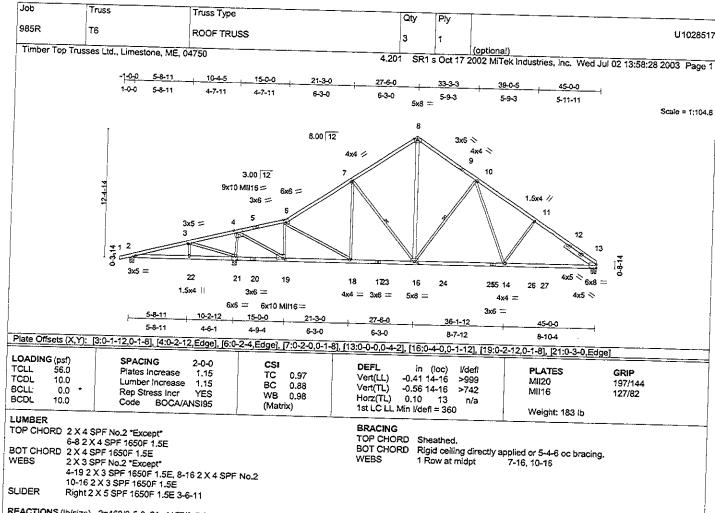
MITEK CANADA, INC. GENERAL SPECIFICATIONS (U.S.A.) DATED APRIL 1, 1997 FORM AN INTEGRAL PART OF THIS DESIGN.







TOP CHORD
Sheathed or 2-8-3 oc purlins.
BOT CHORD
Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS
1 Row at midpt
4-14, 8-14



REACTIONS (lb/size) 2=468/0-5-8, 21=4157/0-5-8, 13=2702/0-5-8

Max Horz2=362(load case 5)

Max Uplift2=207(load case 4), 21=-515(load case 6), 13=-323(load case 6)

Max Grav2=519(load case 2), 21=4157(load case 1), 13=2702(load case 1)

FORCES (ib) - First Load Case Only
TOP CHORD 1-2=29, 2-3=337, 3-4=1814, 4-5=-2086, 5-6=-1952, 6-7=-3049, 7-8=-2537, 8-9=-2133, 9-10=-2513, 10-11=-3577,
11-12=-3807, 12-13=-3991
BOT CHORD 2-22=-231, 21-22=-220, 20-21=-1690, 19-20=-1690, 18-19=2007, 17-18=2362, 17-23=2362, 16-23=2362, 16-24=2561,
24-25=2561, 15-25=2561, 14-15=2561, 14-26=3087, 26-27=3087, 13-27=3087
3-22=119, 3-21=-1541, 4-21=-3577, 4-19=4196, 6-19=-2000, 6-18=413, 7-18=-6, 7-16=-717, 8-16=1766, 10-16=-1124,
10-14=777, 11-14=-494

NOTES

NOTES

1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category III, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANSI95 if end verticals or cantilevers exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

2) Design load is based on 56.0 psf specified roof snow load.

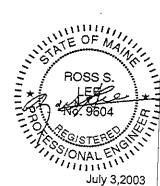
3) Unbalanced snow loads have been considered for this design.

4) All plates are MII20 plates unless otherwise indicated.

5) * This truss has been designed for a live load of 20 Options to the history.

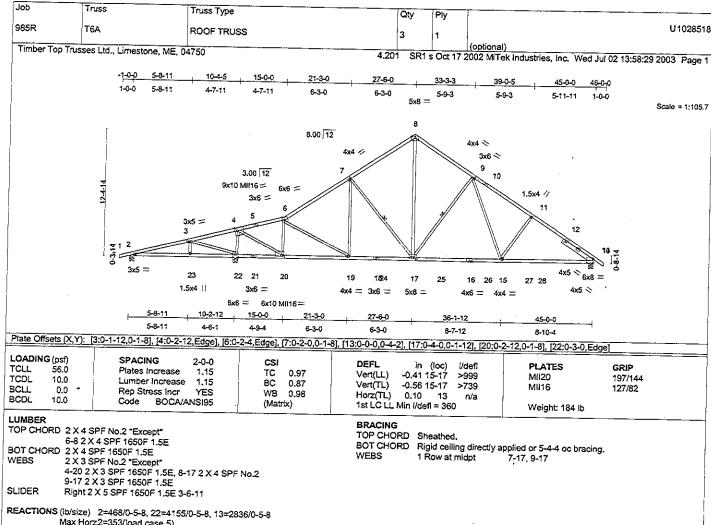
4) Air plates are Mil20 plates unless otherwise indicated.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.
6) WARNING: Required bearing size at joint(s) 21 greater than input bearing size.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 207 lb uplift at joint 2, 515 lb uplift at joint 21.
8) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard









1 Row at midpt

7-17, 9-17

REACTIONS (lb/size) 2=468/0-5-8, 22=4155/0-5-8, 13=2836/0-5-8

Max Horz2=353(load case 5)

Max Uplift2=-209(load case 4), 22=-511(load case 6), 13=-381(load case 6)

Max Grav2=519(load case 2), 22=4155(load case 1), 13=2836(load case 1)

FORCES (ib) - First Load Case Only
TOP CHORD 1-2=29, 2-3=338, 3-4=1815, 4-5=-2083, 5-6=-1949, 6-7=-3045, 7-8=-2534, 8-9=-2510, 9-10=-3185, 10-11=-3565, 11-12=-3796, 12-13=-3981, 13-14=32
BOT CHORD 2-23=-232, 22-23=-221, 21-22=-1691, 20-21=-1691, 19-20=2004, 18-19=2359, 18-24=2359, 17-24=2359, 17-25=2556, 16-26=2556, 15-26=2556, 15-27=3073, 27-28=3073, 13-28=3073
WEBS 3-23=119, 3-22=-1541, 4-22=-3575, 4-20=4194, 6-20=-1999, 6-19=413, 7-19=-6, 7-17=-717, 8-17=1763, 9-17=-1119, 9-15=767, 11-15=-484

NOTES

1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category III, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33.

2) Design load is based on 56.0 psf specified roof snow load.

3) Unbalanced snow loads have been considered for this design.

4) All plates are MII20 plates unless otherwise indicated.

5) This truss has been designed for the vertical of th

4) All plates are Milzu plates unless otherwise indicated.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.
6) WARNING: Required bearing size at joint(s) 22 greater than input bearing size.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 209 lb uplift at joint 2, 511 lb uplift at joint 22 and 384 lb uplift at joint 13

and 381 lb uplift at joint 13.

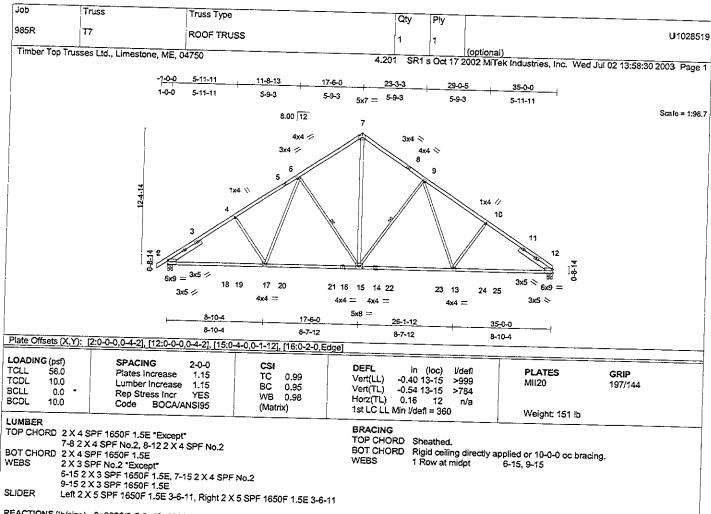
8) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard









REACTIONS (lb/size) 2=3026/0-5-8, 12=2890/0-5-8 Max Horz2=364(load case 5) Max Uplift2=-401(load case 6), 12=-343(load case 6)

FORCES (lb) - First Load Case Only
TOP CHORD 1-2=32, 2-3=-4293, 3-4=-4107, 4-5=-3877, 5-6=-3497, 6-7=-2849, 7-8=-2469, 8-9=-2849, 9-10=-3885, 10-11=-4115,
11-12=-4301
BOT CHORD 2-18=3324, 18-19=3324, 17-19=3324, 17-20=2828, 20-21=2828, 16-21=2828, 15-16=2828, 14-15=2831, 14-22=2831,
22-23=2831, 13-23=2831, 13-24=3336, 24-25=3336, 12-25=3336
WEBS 4-17=-465, 6-17=732, 6-15=-1108, 7-15=2170, 9-15=-1113, 9-13=742, 10-13=-476

NOTES

1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category III, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANSI95 If end verticals or cantilevers exist, they are exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

2) Design load is based on 56.0 psf specified roof snow load.

3) Unbalanced snow loads have been considered for this design.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.

the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 401 lb uplift at joint 2 and 343 lb uplift at

joint 12.
6) This truss has been designed with ANSI/TP! 1-1995 criteria.

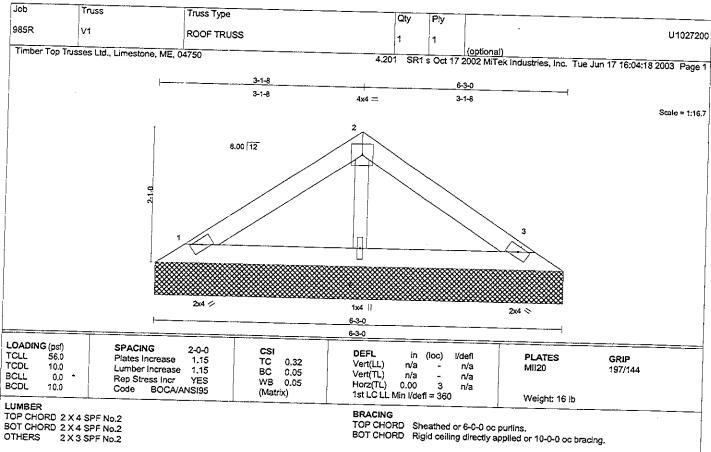
LOAD CASE(S) Standard

ROSS S S/ONAL ENGLISH









REACTIONS (lb/size) 1=235/6-3-0, 3=235/6-3-0, 4=334/6-3-0

Max Horz 1=51 (load case 5)

Max Uplift1=-45(load case 6), 3=-45(load case 6), 4=-14(load case 6)

Max Grav1=259(load case 2), 3=259(load case 3), 4=334(load case 1)

FORCES (ib) - First Load Case Only TOP CHORD 1-2=-151, 2-3=-151 BOT CHORD 1-4=61, 3-4=61 WEBS 2-4=-268

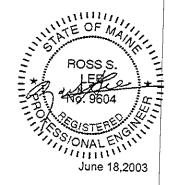
- NOTES

 1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category III, condition I enclosed building, If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

 3 *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.

 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 45 ib uplift at joint 1, 45 ib uplift at joint 3 and 14 lb uplift at joint 4.

LOAD CASE(S) Standard

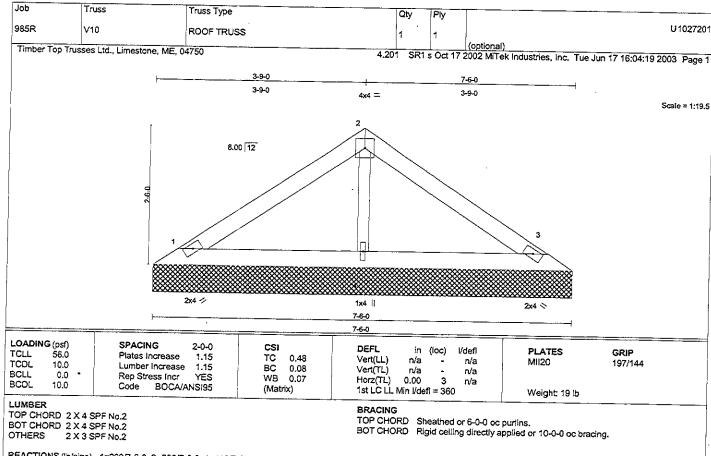


MITEK CANADA, INC. GENERAL SPECIFICATIONS (U.S.A.) DATED APRIL 1, 1997 FORM AN INTEGRAL PART OF THIS DESIGN.



MiTek Canada, Inc. 100 Industrial Rd., P.O. Box 1329 Bradford, Ontario, L3Z 2B7





REACTIONS (lb/size) 1=290/7-6-0, 3=290/7-6-0, 4=413/7-6-0

Max Horz1=-63(load case 4)

Max Uplift1=-56(load case 6), 3=-56(load case 6), 4=-17(load case 6)

Max Grav1=320(load case 2), 3=320(load case 3), 4=413(load case 1)

FORCES (lb) - First Load Case Only TOP CHORD 1-2=-186, 2-3=-186 BOT CHORD 1-4=75, 3-4=75 WEBS 2-4=-331

NOTES

NOTES

1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category III, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33.

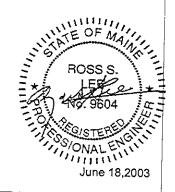
2) Gable requires continuous bottom chord bearing.

3) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 56 lb uplift at joint 1, 56 lb uplift at joint 3 and 17 lb uplift at joint 4.

5) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard

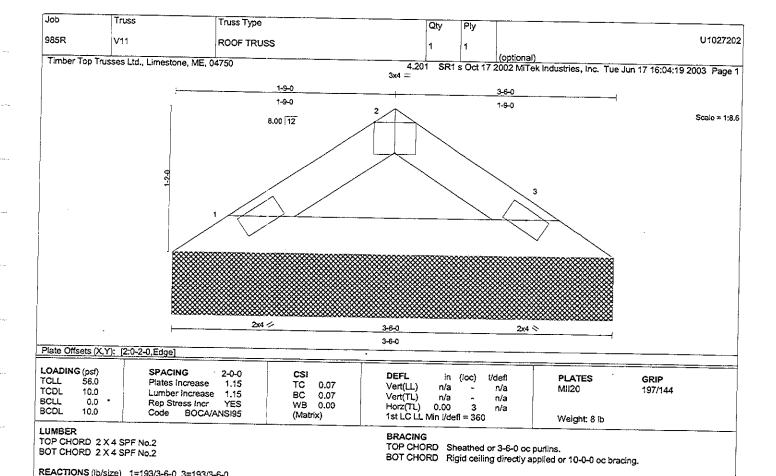


MITEK CANADA, INC. GENERAL SPECIFICATIONS (U.S.A.) DATED APRIL 1, 1997 FORM AN INTEGRAL PART OF THIS DESIGN.



MiTek Canada, Inc. 100 Industrial Rd., P.O. Box 1329 Bradford, Ontario, L3Z 2B7





REACTIONS (lb/size) 1=193/3-6-0, 3=193/3-6-0 Max Horz1=24(load case 5) Max Uplift1=-25(load case 6), 3=-25(load case 6)

FORCES (lb) - First Load Case Only TOP CHORD 1-2=-188, 2-3=-188 BOT CHORD 1-3=126

NOTES

- NOTES

 1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category lit, condition 1 enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

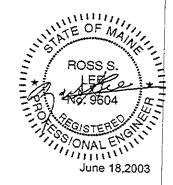
 2) Gable requires continuous bottom chord bearing.

 3) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.

 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 25 lb uplift at joint 1 and 25 lb uplift at joint 3.

- 5) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard

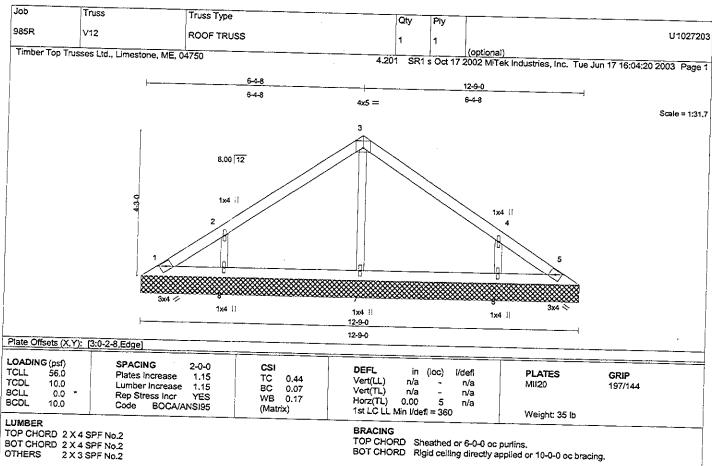


MITEK CANADA, INC. GENERAL SPECIFICATIONS (U.S.A.) DATED APRIL 1, 1997 FORM AN INTEGRAL PART OF THIS DESIGN.



MiTek Canada, Inc. 100 Industrial Rd., P.O. Box 1329 Bradford, Ontario, L3Z 2B7





TOP CHORD Sheathed or 6-0-0 oc purlins.

BOT CHORD Rigid celling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=134/12-9-0, 5=134/12-9-0, 7=506/12-9-0, 8=509/12-9-0, 6=509/12-9-0

Max Horz1=113(load case 5)

Max Uplift=-35(load case 4), 5=-14(load case 5), 8=-140(load case 5), 6=-139(load case 4)

Max Grav1=134(load case 1), 5=134(load case 1), 7=506(load case 1), 8=655(load case 2), 6=655(load case 3)

FORCES (Ib) - First Load Case Only TOP CHORD 1-2=-143, 2-3=-264, 3-4=-264, 4-5=-143 BOT CHORD 1-8=105, 7-8=105, 6-7=105, 5-6=105 WEBS 3-7=-420, 2-8=-443, 4-6=-443

NOTES

NOTES

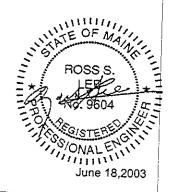
1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 ml from hurricane oceanline, on an occupancy category III, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANSi95 if end verticals or cantilevers exist, they are exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

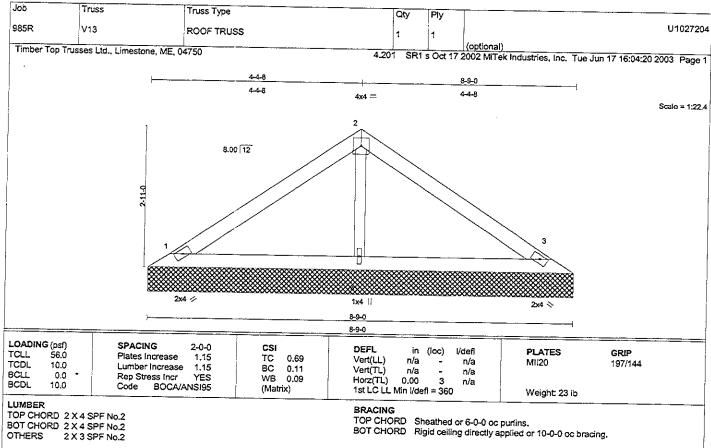
2) Sable requires continuous bottom chord bearing.

3) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 1, 14 lb uplift at joint 5, 14 lb uplift at joint 6.

LOAD CASE(S) Standard





TOP CHORD 2 X 4 SPF No.2 BOT CHORD 2 X 4 SPF No.2 OTHERS 2 X 3 SPF No.2

REACTIONS (lb/size) 1=346/8-9-0, 3=346/8-9-0, 4=492/8-9-0

(losize) = 34076-9-0, 3-34076-9-0, 4-3216 6-0 Max Horz1=75(load case 5) Max Uplift1=-66(load case 6), 3=-66(load case 6), 4=-21(load case 6) Max Grav1=381(load case 2), 3=381(load case 3), 4=492(load case 1)

FORCES (lb) - First Load Case Only TOP CHORD 1-2=-222, 2-3=-222 BOT CHORD 1-4=90, 3-4=90 WEBS 2-4=-394

NOTES

NOTES

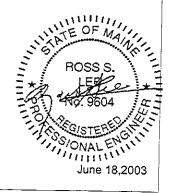
1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category III, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

2) Gable requires continuous bottom chord bearing.

3) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 66 lb uplift at joint 1, 66 lb uplift at joint 3 and 21 lb uplift at joint 4.

LOAD CASE(S) Standard



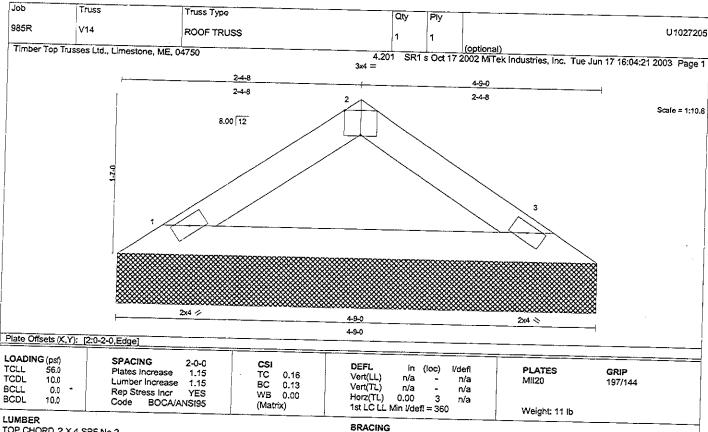
MITEK CANADA, INC. GENERAL SPECIFICATIONS (U.S.A.) DATED APRIL 1, 1997 FORM AN INTEGRAL PART OF THIS DESIGN.





Bradford, Ontario, L3Z 2B7





LUMBER

TOP CHORD 2 X 4 SPF No.2 BOT CHORD 2 X 4 SPF No.2

REACTIONS (b/size) 1=288/4-9-0, 3=288/4-9-0

Max Horz1=36(load case 4)
Max Uplift1=37(load case 6), 3=-37(load case 6)

FORCES (ib) - First Load Case Only TOP CHORD 1-2=-281, 2-3=-281 BOT CHORD 1-3=187

NOTES

- NOTES

 1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 ml from hurricane oceanline, on an occupancy category III, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANS!95 If end verticals or cantilevers exist, they are exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

 2) Gable requires continuous bottom chord bearing.

 3) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.

 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 ib uplift at joint 1 and 37 ib uplift at joint 3.

- 3.
 5) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard

ATE OF MA ROSS S SONAL ENGINE June 18,2003

MITEK CANADA, INC. GENERAL SPECIFICATIONS (U.S.A.) DATED APRIL 1, 1997 FORM AN INTEGRAL PART OF THIS DESIGN.

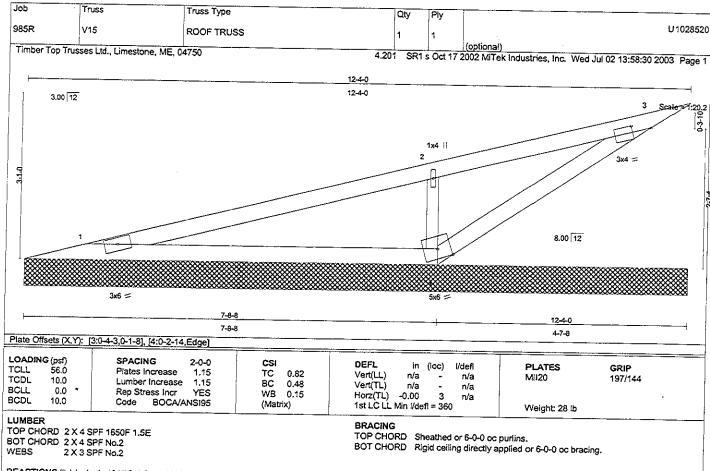






TOP CHORD Sheathed or 4-9-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.



REACTIONS (lb/size) 1=404/12-4-0, 4=1037/12-4-0, 3=144/12-4-0 Max Horz1=123(load case 4) Max Upliff1=-55(load case 4), 4=-186(load case 4), 3=-39(load case 4)

FORCES (ib) - First Load Case Only TOP CHORD 1-2=83, 2-3=66 BOT CHORD 1-4=28, 3-4=-78 WEBS 2-4=-817

NOTES

- NOTES

 1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 ml from hurricane oceanline, on an occupancy category III, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33.
- If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate gnp increase is 1.33.

 3) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.

 4) Bearing at joint(s) 3 considers parallel to grain value using ANSI/TPI 1-1995 angle to grain formula. Building designer should verify capacity of bearing surface.

 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 55 lb uplift at joint 1, 186 lb uplift at joint 4 and 39 lb unlift at loint 3.
- and 39 to uplift at joint 3.

 6) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard

ATE OF MAIN SONAL ENTIN July 3,2003

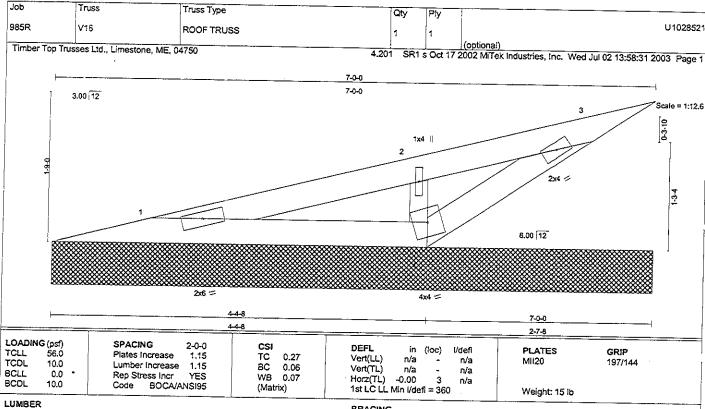
MITEK CANADA, INC. GENERAL SPECIFICATIONS (U.S.A.) DATED APRIL 1, 19 FORM AN INTEGRAL PART OF THIS DESIGN.



MiTek Canada, Inc. 100 Industrial Rd., P.O. Box 1329 Bradford, Ontario, L3Z 2B7



LOADING AND DIMENSIONS SPECIFIED BY FABRICATION, SUBJECT TO VERIFICATION BY AUTHORITIES IN JURISDICTION.



LUMBER
TOP CHORD 2 X 4 SPF No.2
BOT CHORD 2 X 4 SPF No.2
WEBS 2 X 3 SPF No.2

BRACING

TOP CHORD Sheathed or 6-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=203/7-0-0, 4=492/7-0-0, 3=79/7-0-0 Max Horz 1=61(load case 4) Max Uplift1=-24(load case 4), 4=-98(load case 4), 3=-15(load case 4)

FORCES (lb) - First Load Case Only TOP CHORD 1-2=32, 2-3=45 BOT CHORD 1-4=26, 3-4=-47 WEBS 2-4=-424

NOTES

NOTES
 This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load, 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category III, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
 This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.
 Bearing at joint(s) 3 considers parallel to grain value using ANSI/TPI 1-1995 angle to grain formula. Building designer should verify capacity of bearing surface.

capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 24 lb uplift at joint 1, 98 lb uplift at joint 4 and 15 ib uplift at joint 3.

6) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard

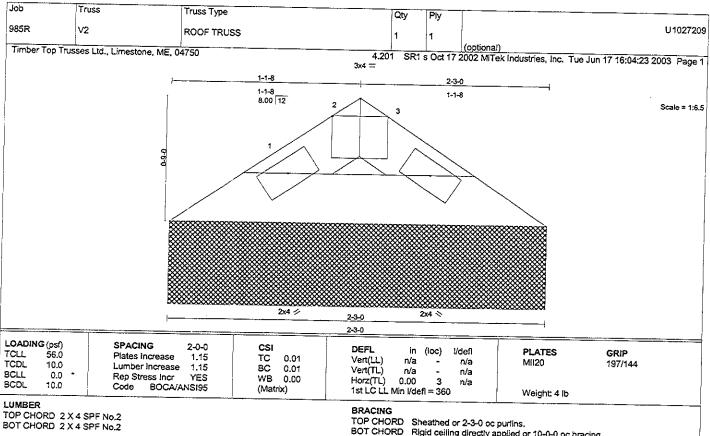


MITEK CANADA, INC. GENERAL SPECIFICATIONS (U.S.A.) DATED APRIL 1, 1997 FORM AN INTEGRAL PART OF THIS DESIGN.



MiTek Canada, Inc. 100 Industrial Rd., P.O. Box 1329 Bradford, Ontario, L3Z 2B7





BRACING

TOP CHORD Sheathed or 2-3-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=58/2-3-0, 3=58/2-3-0 Max Horz1=12(load case 5) Max Uplift1=-7(load case 6), 3=-7(load case 6)

FORCES (ib) - First Load Case Only TOP CHORD 1-2=-49, 2-3=-49 BOT CHORD 1-3=22

NOTES

- NOTES

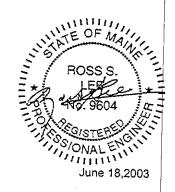
 1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category III, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANSI95 if end verticals or cantillevers exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

 2) Gable requires continuous bottom chord bearing.

 3) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.

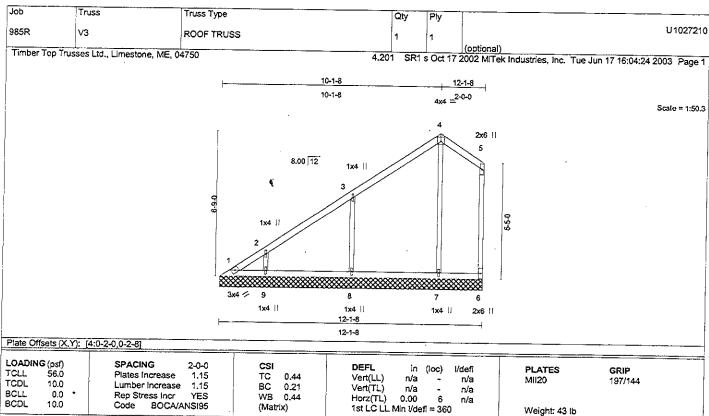
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 7 lb uplift at joint 1 and 7 lb uplift at joint 3.

LOAD CASE(S) Standard









LUMBER
TOP CHORD 2 X 4 SPF No.2
BOT CHORD 2 X 4 SPF No.2
WEBS 2 X 3 SPF No.2
OTHERS 2 X 3 SPF No.2

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=35/12-1-8, 6=74/12-1-8, 7=495/12-1-8, 8=650/12-1-8, 9=499/12-1-8

Max Horz1=379(load case 5). Max Uplift1=-59(load case 4), 6=-99(load case 4), 7=-142(load case 5), 8=-164(load case 5), 9=-122(load case 6). Max Grav1=197(load case 5), 6=151(load case 3), 7=495(load case 1), 8=777(load case 2), 9=589(load case 2)

FORCES (lb) - First Load Case Only TOP CHORD 1-2=105, 2-3=154, 3-4=134, 4-5=102, 5-6=-63 BOT CHORD 1-9=2, 8-9=2, 7-8=2, 6-7=2 WEBS 4-7=-431, 3-8=-564, 2-9=-440

NOTES

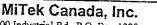
1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 ml from hurricane oceanline, on an occupancy category III, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33 (2) Gable requires continuous bottom chord bearing.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.

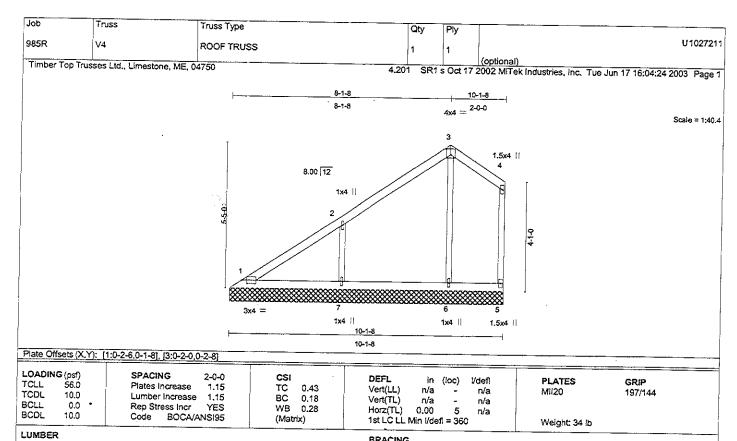
3) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 59 lb uplift at joint 1, 99 lb uplift at joint 6, 142 lb uplift at joint 7, 164 lb uplift at joint 8 and 122 lb uplift at joint 9.
5) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard

ATE OF O ONAL ENGIN MALE THE June 18,2003







TOP CHORD 2 X 4 SPF No.2 BOT CHORD 2 X 4 SPF No.2 WEBS 2 X 3 SPF No.2 OTHERS 2 X 3 SPF No.2

BRACING

TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=212/10-1-8, 5=77/10-1-8, 6=487/10-1-8, 7=673/10-1-8

Max Horz 1=291(load case 5), 5=84(load case 4), 6=-120(load case 5), 7=-169(load case 5)
Max Grav1=247(load case 2), 5=155(load case 3), 6=487(load case 1), 7=804(load case 2)

FORCES (lb) - First Load Case Only TOP CHORD 1-2=150, 2-3=134, 3-4=102, 4-5=-63 BOT CHORD 1-7=3, 6-7=3, 5-6=3 WEBS 3-6=-433, 2-7=-557

NOTES

NOTES

1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category III, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. The iumber DOL increase is 1.33, and the plate grip increase is 1.33

2) Gable requires continuous bottom chord bearing.

3) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 47 lb uplift at joint 1, 84 lb uplift at joint 5, 120 lb uplift at joint 6 and 169 lb uplift at joint 7.

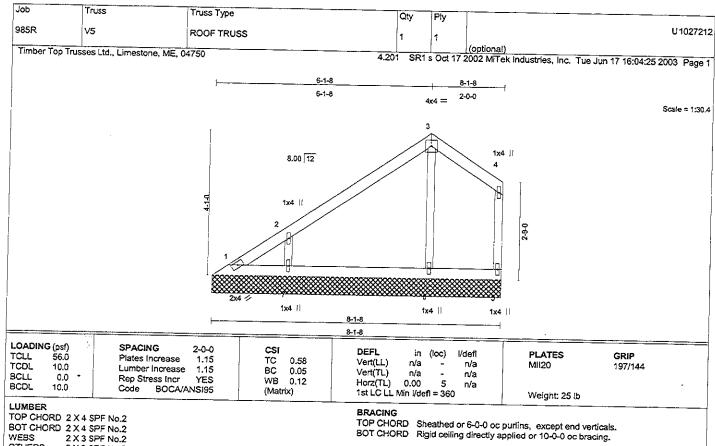
LOAD CASE(S) Standard



•







TOP CHORD 2 X 4 SPF No.2
BOT CHORD 2 X 4 SPF No.2
WEBS 2 X 3 SPF No.2
OTHERS 2 X 3 SPF No.2

REACTIONS (lb/size) 1=-10/8-1-8, 5=132/8-1-8, 6=410/8-1-8, 7=613/8-1-8

Max Horz 1=202(load case 5)

Max Uplift1=-41(load case 4), 5=-78(load case 4), 6=-73(load case 5), 7=-159(load case 5)

Max Grav1=118(load case 5), 5=159(load case 3), 6=410(load case 1), 7=728(load case 2)

FORCES (ib) - First Load Case Only TOP CHORD 1-2=127, 2-3=90, 3-4=-56, 4-5=-125 BOT CHORD 1-7=0, 6-7=0, 5-6=0 WEBS 3-6=-339, 2-7=-543

NOTES

- NOTES

 1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category III, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

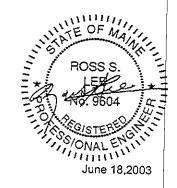
 2) Gable requires continuous bottom chord bearing.

 3) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.

 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 41 lb uplift at joint 1, 78 lb uplift at joint 5, 73 lb uplift at joint 7.

 5) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard

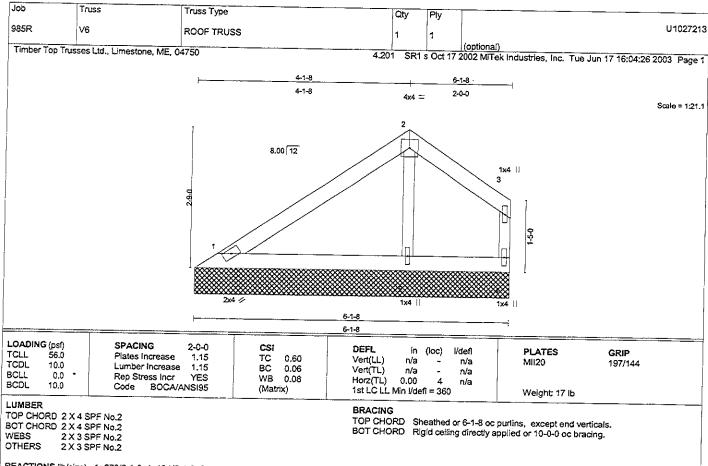


MITEK CANADA, INC. GENERAL SPECIFICATIONS (U.S.A.) DATED APRIL 1, 1997 FORM AN INTEGRAL PART OF THIS DESIGN.









REACTIONS (lb/size) 1=270/6-1-8, 4=131/6-1-8, 5=441/6-1-8 Max Horz1=114(load case 5) Max Uplift1=-30(load case 6), 4=-59(load case 4), 5=-56(load case 5) Max Grav1=321(load case 2), 4=157(load case 3), 5=441(load case 1)

FORCES (Ib) - First Load Case Only TOP CHORD 1-2=107, 2-3=-56, 3-4=-125 BOT CHORD 1-5=0, 4-5=0 WEBS 2-5=-366

NOTES

1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category III, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANSi95 if end verticals or cantilevers exist, they are exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

2) Gable requires continuous bottom chord bearing.

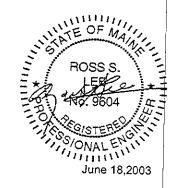
3) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 30 lb uplift at joint 1, 59 lb uplift at joint 4 and 56 lb uplift at joint 5.

and 56 lb uplift at joint 5.

5) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard



MITEK CANADA, INC. GENERAL SPECIFICATIONS (U.S.A.) DATED APRIL 1, 1997 FORM AN INTEGRAL PART OF THIS DESIGN.

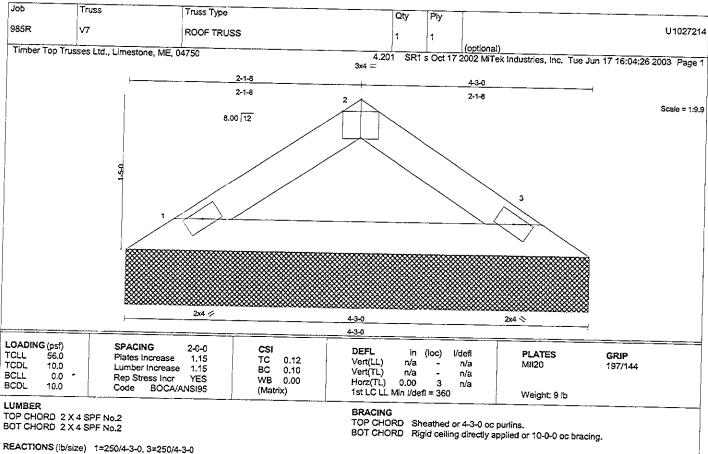


MiTek Canada, Inc.

100 Industrial Rd., P.O. Box 1329 Bradford, Ontario, L3Z 2B7



LOADING AND DIMENSIONS SPECIFIED BY FABRICATOR, SUBJECT TO VERIFICATION BY AUTHORITIES IN JURISDICTION.



TOP CHORD 2 X 4 SPF No.2 BOT CHORD 2 X 4 SPF No.2

REACTIONS (lb/size) 1=250/4-3-0, 3=250/4-3-0 Max Horz1=-31(load case 4) Max Uplift1=-32(load case 6), 3=-32(load case 6)

FORCES (Ib) - First Load Case Only TOP CHORD 1-2=-244, 2-3=-244 BOT CHORD 1-3=163

NOTES

- NOTES

 1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category III, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

 2) Gable requires continuous bottom chord bearing.

 3) "This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.

 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 32 lb uplift at joint 1 and 32 lb uplift at joint 3.

5) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard

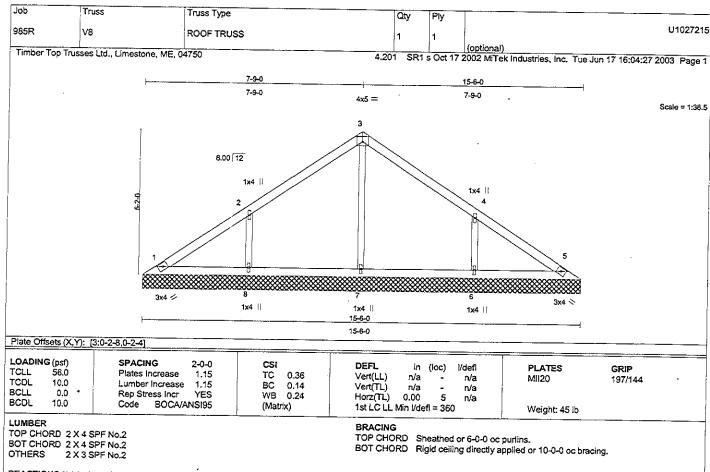
ATE OF MAIN ROSS S S/ONAL ENG June 18,2003

MITEK CANADA, INC. GENERAL SPECIFICATIONS (U.S.A.) DATED APRIL 1, 1997 FORM AN INTEGRAL PART OF THIS DESIGN.



MiTek Canada, Inc.





TOP CHORD Sheathed or 6-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=252/15-6-0, 5=252/15-6-0, 7=477/15-6-0, 8=615/15-6-0, 6=615/15-6-0

Max Horz 1=-139(load case 4)

Max Uplift1=-31(load case 4), 5=-9(load case 6), 8=-161(load case 5), 6=-161(load case 4)

Max Grav 1=258(load case 2), 5=258(load case 3), 7=477(load case 1), 8=765(load case 2), 6=765(load case 3)

FORCES (Ib) - First Load Case Only TOP CHORD 1-2=21, 2-3=-261, 3-4=-261, 4-5=-218 BOT CHORD 1-8=98, 7-8=98, 6-7=98, 5-6=98 WEBS 3-7=-407, 2-8=-510, 4-6=-510

NOTES

NOTES

1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 ml from hurricane oceanline, on an occupancy category Ill, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANSI95 If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

2) Gable requires continuous bottom chord bearing.

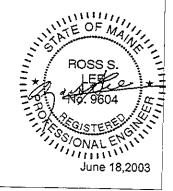
3) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 31 lb uplift at joint 1, 9 lb uplift at joint 5, 161 lb uplift at joint 8 and 161 lb uplift at joint 6.

5) This truss has been designed with ANSI/TPI 1-1995 criteria.

5) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard



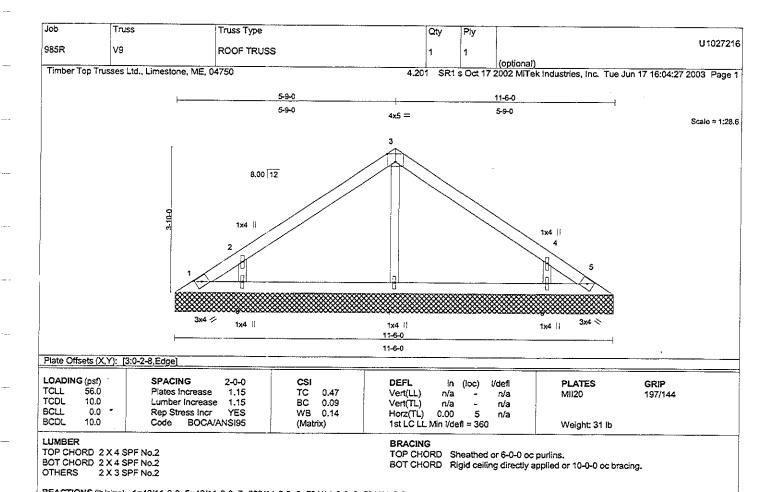
MITEK CANADA, INC. GENERAL SPECIFICATIONS (U.S.A.) DATED APRIL 1, 1997 FORM AN INTEGRAL PART OF THIS DESIGN.



MiTek Canada, Inc. 100 Industrial Rd., P.O. Box 1329 Bradford, Ontario, L3Z 2B7



LOADING AND DIMENSIONS SPECIFIED BY FABRICATOR, SUBJECT TO VERIFICATION BY AUTHORITIES IN JURISDICTION.



REACTIONS (lb/size) 1=48/11-6-0, 5=48/11-6-0, 7=503/11-6-0, 8=501/11-6-0, 6=501/11-6-0

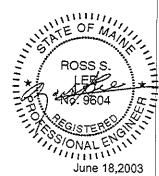
(Ib/S/Ze) 1=46/11-6-0, 3=46/11-6-0, 7=303/11-6-0, 0-30/11-0-0, 0-30/11-0-0, 0-30/11-0-0 Max Horz1=101(load case 5) Max Uplift1=-39(load case 4), 5=-20(load case 5), 7=-11(load case 6), 8=-142(load case 5), 6=-141(load case 4) Max Grav1=72(load case 3), 5=72(load case 2), 7=503(load case 1), 8=660(load case 2), 6=660(load case 3)

FORCES (ib) - First Load Case Only TOP CHORD 1-2=-108, 2-3=-266, 3-4=-266, 4-5=-108 BOT CHORD 1-8=107, 7-8=107, 6-7=107, 5-6=107 WEBS 3-7=-416, 2-8=-452, 4-6=-452

NOTES

- NOTES
 This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category III, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per BOCA/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL Increase is 1.33, and the plate grip increase is 1.33
 Gable requires continuous bottom chord bearing.
 This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 3-6-0 between the bottom chord and any other members.
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 39 lb uplift at joint 1, 20 lb uplift at joint 5, 11 lb uplift at joint 7, 142 lb uplift at joint 8 and 141 lb uplift at joint 6.
 This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard



MITEK CANADA, INC. GENERAL SPECIFICATIONS (U.S.A.) DATED APRIL 1, 1997 FORM AN INTEGRAL PART OF THIS DESIGN.



MiTek Canada, Inc. 100 Industrial Rd., P.O. Box 1329 Bradford, Ontario, L3Z 2B7



LOADING AND DIMENSIONS SPECIFIED BY FABRICATION, SUBJECT TO VERIFICATION BY AUTHORITIES IN JURISDICTION.

PLUMBING APPLICATION Department of Human Sciences Division of Health Engineering PROPERTY ADDRESS Town or Plantation Subdivision Lot # it Domilizad PROPERTY OWNERS NAME 8586 TOWN COPY 01810/1 Applicant Name: Mailing Address of Owner/Applicant (If Different) Owner/Applicant Statement I certify that the information submitted is correct to the best of my Caution: Inspection Required knowledge and understand that any falsification is reason for the Local I have inspected the installation authorized above and found it to be in Plumbing Inspectors to deny a Permit. compliance with the Maine Plumbing Rules. 7/12 Signature of Owner/Applicant Date Local Plumbing Inspector Signature Date Approved PERMIT INFORMATION This Application is for Type of Structure To Be Served: Plumbing To Be Installed By: 1.

NEW PLUMBING 1. SINGLE FAMILY DWELLING 2.
RELOCATED PLUMBING 2.

MODULAR OR MOBILE HOME 2.

OIL BURNERMAN 3.

MULTIPLE FAMILY DWELLING 3. 🗆 MFG'D. HOUSING DEALER/MECHANIC 4.

OTHER - SPECIFY 4.
PUBLIC UTILITY EMPLOYEE 5.
PROPERTY OWNER LICENSE # 0.7.0,9 Hook-Up & Piping Relocation Maximum of 1 Hook-Up Column 2 Number Type of Fixture Number Type of Fixture HOOK-UP: to public sewer in those cases where the connectior is not regulated and inspected by the local Sanitary District. Hosebibb / Sillcock Bathtub (and Shower) Floor Drain Shower (Separate) OR. Urinal Sink HOOK-UP: to an existing subsurface wastewater disposal system. **Drinking Fountain** Wash Basin Indirect Waste <u>PIPING RELOCATION:</u> of sanitary lines, drains, and piping without new fixtures. Water Closet (Toilet) Water Treatment Softener, Filter, etc. Clothes Washer Grease / Oil Separator Dish Washer Dental Cuspidor Garbage Disposal OR **Bidet** Laundry Tub Other: Water Heater TRANSFER FEE Fixtures (Subtotal) [\$6.00] Fixtures (Subtotal) Column 2 Column 1 Fixtures (Subtotal) SEE PERMIT FEE SCHEDULE Column 2 FOR CALCULATING FEE Total Fixtures Fixture Fee Transfer Fee Hook-Up & Relocation Fee Page 1 of 1 HHE-211 Rev. 6:94 Permit Fee (Total) TOWN COPY

JUN-16-2003 MON 09:32 AM Timber Top Trusses LTD.

FAX NO. 508 473 5729

P. 01/01



THE TOP TRUSSES LTD

MEMO

To: Joff Madin & Kevin Hackett c/o Hammond Lumber / Auburn

From: Steve F. Toner, P.Ung. RE: Engineered Shop drawings

Date: 2003 05 16 REE: P.O. # 40323983

Please note that this is an order confirmation with a scheduled delivery date of July 7, 2003. We will produce a set of scaled shop drawings by a registered professional engineer for the state of Maine and have them sent to you within the next 7 days.

Should you have any questions concerning this matter you can contact me at $(800)\ 810-5722\ \mathrm{ext}\ 3$.

Sincerely,

Steve F. Toner, P. Bug.

DEPT. OF BUILDING INSPECTION
CITY OF PORTLAND. 1/E

JUN 1 8 2003

E G E V E

Cox 7819, Grand-SoulVFelts, NB 1232 3128

Box 466, Limestone Maine. 04750

Tot.:(506) 473-5722 Fax:(506) 473-5729 G-mait: sales@(imbertop.nb.ca

Tel.:(800) 810-5722 Fax:(877) 810-5729 www.timbertop.nb.ca Amendment to Building Permit Application Jeff and Kirsten Martin 131 Hope Ave, Portland June 18, 2003

Attn: Tammy Munson

Note: numbered responses correspond to numbers on questionnaire

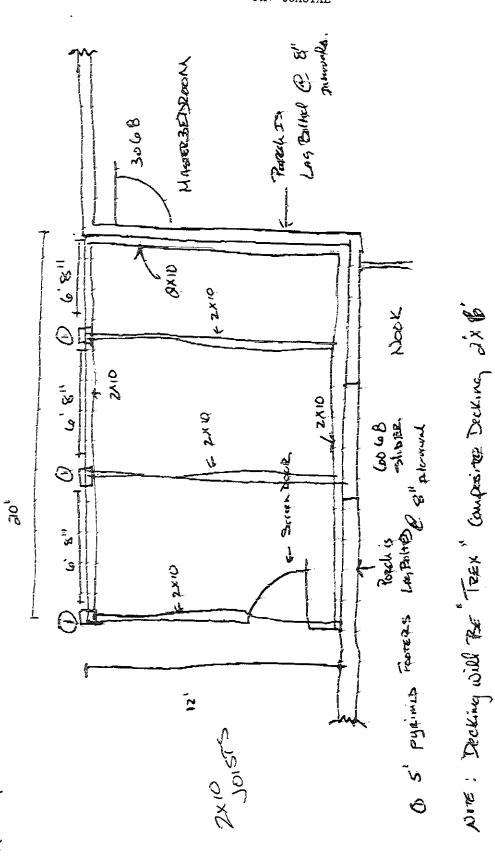
- 1. See attachment 1 for engineer's review of center girder.
- 2. See attachment 2 for engineer's review of floor and roof truss plan.
- 3. Windows above bathtub in master bath will be tempered.
- 4. The attic access in the laundry room will be $24" \times 30"$.
- 5. There will be a minimum 2" clearance around chimney from any flammable material
- 6. There are no interior bearing walls. Please see roof truss plan for specs on support. In the master bedroom area, LVL will be used on corner. 4 2x6 post will support all girder trusses in attic.





12:53 FAX 7973688

06/19/03



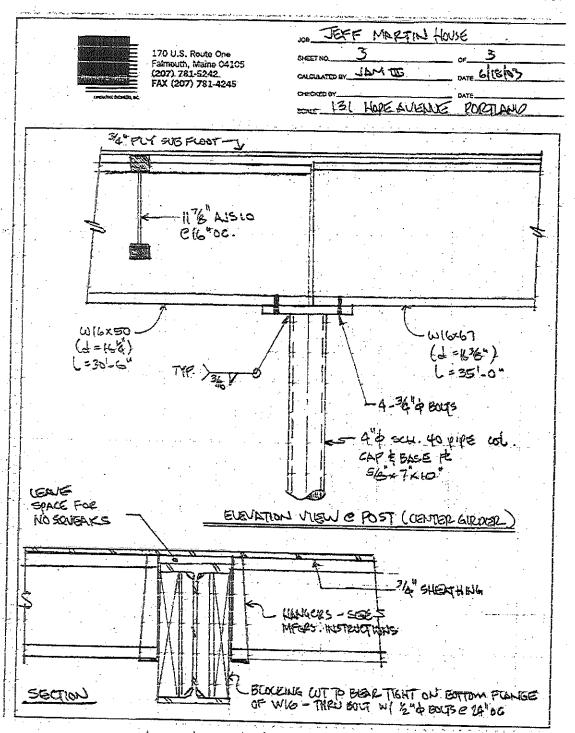
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	100 Jeff Mostin House	iG.
170 U.S. Route One Falmouth, Maine 04105	SHEET NO.	or 3
PINKHAIS & GREER (207) 781-5242 FAX (207) 781-4245	CALCULATED BY JAM 3	DATE 6/18/03
COMBULTING Enclant 2015, INC	SCALE 131 Hope Avenue	Parland
1 2 3 4 2 5 5 6 7 6 7 6 7 6 7 6 7 6 7 6 7 6 7 6 7		7 4 7 4 8 9 9 6 7 6 3 4 5
© Center Groders: L =	3016 (SAX 31') and 35'	
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170 U.S. Route One	SHEET NO	110-000	3
Falmouth, Maine 04105 (207) 781-5242	CALCULATED BY JAMES		6/18/03
FAX (207) 781-4245	CHECKED BY	DAT.	
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SIDE GIRDERS: L=161 \$ 18-6" UNS W8X31

PRODUCT 207



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	CONSULTING ENGINEERS, III	

170 U.S. Route One Falmouth, Maine 04105 (207) 781-5242 FAX (207) 781-4245

JOB_ Jeff Martin	The second secon
SHEET NO.	OF
CALCULATED BY JAM 3	DATE 6/18/03
CHECKED BY	DATE
SCALE 131 Hope Avenue	Portland

■ Center Givdors: L = 30-6 (SAY 31') in 35'

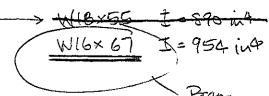
 $7218W = \frac{33}{2} = 16.5$ W = 10+40 (16.5) = 825pH (660pH we war)

 $\frac{L=35'}{M=825(35)^{2}/8=126.3 \text{ k'}} \longrightarrow W16x36 M_{Y}=155 (Gr.50)$

4360 I≥5(600)(35)A(1728)(360) . I=448 in4

I = 5(660)(35) + (1728)(360) = 659 in4 W16x36 NG. -> W16x50 (659) A = 1:17" W16x46

IF U/480 TO MARCH JOISTS: I = 878 in A WIBX55 (See BOISE SHOP DWG) WI6×67



 $\frac{L = 31'}{43000} = 457 \text{ in}^4 \longrightarrow \text{W16x36} (\frac{1}{360})$

[= 457 (48/36) = 610 int > 18x40 (4/480)

W16x50

Bram

CENTER POST LOAD: P = 35+31 (825) = 27.2 x

しい=81 -> 4"からの、32"中 STD. 3"中 STPOEPT. OF BUILDING INSPECTION
TRY 7×10 た fo = 386psi
N=3
UN 1 8 2003

N=3 t=0.62" %中min. thickness

25/19/02/107	MANUFACTION TO PROVIDE
X 11/13	HAM & GREER -
***************************************	cheste damento de pristante
	CONSULTING ENGINEERS, INC

170 U.S. Route One Falmouth, Maine 04105 (207) 781-5242 FAX (207) 781-4245

JOB_ JEF1	MARTIN		
SHEET NO.	2		OF
CALCULATED BY			DATE 6/18/03
CHECKED BY		· 	DATE
SCALE			

E SIDE BEAMS

$$L = 16' \quad \text{TRUB} = \frac{28.5}{2} = 14.25'$$

$$W = 50(14.25) = 712.5 \text{ pk} [570]$$

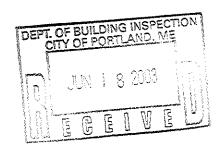
$$M = 22,800 \text{ ph.16.}$$

I ≥ 54" (: 4/360)
72 (4/80)

W8x31 Mr = 76 (504si) 54 (364si) I = 110 in4

CONLOUSE WBXZ1 I = 75 Mys 50

USE W8 x31 (1/420)



JEFF MARTIN 170 U.S. Route One SHEET NO. Falmouth, Maine 04105 6/18/03 (207) 781-5242 FAX (207) 781-4245 自 MAL PINKHAM & GREER CHECKED BY 34" PLY SUB FLOOR -J -11% AJS10 C16"0C. M(6x50 10x61W (9 = 10/2) (d=1636") l=351-0" L=301-6" -4-34" & BOUS 4" \$ SCH. 40 PIPE COL.
CAP & BASE PE
5/8" x 7" x 10" CERVE SPACE FOR ELEVATION VIEW @ POST NO SQUEAKS 34" SHEATHING MAYGUES - SEE DEPT. OF BUILDING INSPECTION CITY OF PORTLAND, ME MEGRS. INSTRUCTIONS JUN 1 8 2003 BLOCKING CUT TO BEAR TIGHT ON BOTTOM FLANGE OF WIG - THRU BOLT WI 2" & BOLTS @ 24" OC SECTION

BLOCKING WIT P BOAR TIGHT ON COTTON FLANGE OF WIG - THRU BOY W | END BOYSE IN 30 HOIDES 2401 PLATEM : CADTM 4-48-6- CSB/44/1-0 -3/4 SHEATHING EVENTION VIEW @ POST (CENTERGINDER) NO SQUEAKS SOACE FOR シュタシ of the sett to pipe of. shop photo-b-~0-15E=7 1=39-10E=7 (n9/9=P) (d = 16 kg) roxolm. osx91M C (6"0C. 01514 8/11 - 7000月日1007 70月 1年 COLE 131 HOPE AVENUE , DOLTUMOS FAX (207) 781-4245

Falmouth, Maine 04105 (207) 781-5242 170 U.S. Route One

1000 8 June

PRODUCT 207

50/31/2

LIEFF MARTIN

DSE M8X31

19-181 \$ 191=7 : SOBOTATE BOIS

SHEET NO.

SHEET NO.

SHEET NO.

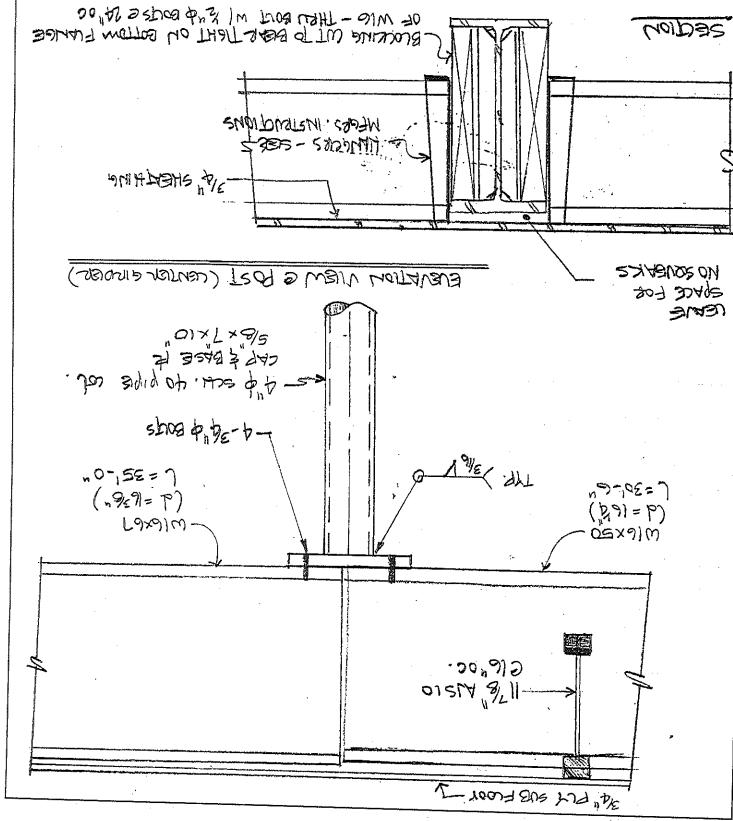
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CHECKED BY

CHECKE

170 U.S. Route One Falmouth, Maine 04105 (207) 781-5242 FAX (207) 781-4245





TOS TOUGORY

TEPT, OF BUILDING 1/15 CITY OF PORTLAND.

TEX8W 350 "9-181 \$ 191=7:509 asls 3015

City of Portland, M.	aine - Building or Use Permi	it	ľ	Permit No:	Date Applied For:	CBL:
-	4101 Tel: (207) 874-8703, Fax:		1- 8716	03-0621	06/05/2003	392 B025001
Location of Construction:	ocation of Construction: Owner Name: O		Ow	ner Address:		Phone:
131 Hope Ave	Goldeneye Corp	Goldeneye Corp 2		6 Falmouth Rd		207-846-6667
Business Name:	Contractor Name:		Con	ntractor Address:		Phone
	no contractor / self		P	ortland		
Lessee/Buyer's Name	Phone:		Per	mit Type:		
			S	ingle Family		
Proposed Use:		1	Proposed P	roject Description:		
build single family 45' x 05/07/03	75' - foundation permit 030336 issu	1	build sing 05/07/03	•	75' - foundation perm	ait 030336 issued
Dept: Zoning	Status: Approved with Condition	ns Revi	iewer: T	Tammy Munson	Approval D	_
Note:					Approval D	ate: 06/11/2003 Ok to Issue: ✓
Note:	Status: Approved with Conditional and a under permit 030336 are applicable.				Approval D	_
Note: 1) All conditions applied		le to this pe	ermit as w	vell.	Approval Da	_
Note: 1) All conditions applied 2) Separate permits shall	d under permit 030336 are applicab	le to this pe	ermit as w	vell. ges.		Ok to Issue: 🔽
Note: 1) All conditions applied 2) Separate permits shall 3) This property shall reapproval.	d under permit 030336 are applicable l be required for future decks, sheds	le to this pe s, pools, and change of t	ermit as w d/or garag use shall	vell. ges. require a separat	e permit application	Ok to Issue:
Note: 1) All conditions applied 2) Separate permits shall 3) This property shall reapproval. 4) This permit is being a	d under permit 030336 are applicable l be required for future decks, sheds main a single family dwelling. Any	le to this pe s, pools, and change of t itted. Any	ermit as w d/or garag use shall: deviation	vell. ges. require a separat	e permit application	Ok to Issue:
Note: 1) All conditions applied 2) Separate permits shall 3) This property shall reapproval. 4) This permit is being a work.	d under permit 030336 are applicable of the required for future decks, sheds main a single family dwelling. Any approved on the basis of plans subm	le to this pe s, pools, and change of t itted. Any	ermit as w d/or garag use shall: deviation	vell. ges. require a separat ns shall require a	e permit application separate approval be Approval Da	Ok to Issue:

•

Form # P 04

DISPLAY THIS CARD ON PRINCIPAL FRONTAGE OF WORK

Please Read Application And Notes, If Any, Attached	BUILDING LINSPECTIO PERMIT	
This is to certify that Goldeneye Corp /no co	ontract self	
has permission tobuild single family 45'	x 75' - 10 Indation permit 03 136 issued 06	/07/03
AT 131 Hope Ave	GE GE	392 B025001
provided that the person or person of the provisions of the Statutes the construction, maintenance at this department.	nd use of buildings and sixuat	oting this permit shall comply with all ses of the City of Portland regulating sures, and of the application on file in
Apply to Public Works for street line and grade if nature of work requires such information.	No ication inspect in must give and within permission procubers this building of at thereoday dor constitutions. H. R. NOTICE IS REQUIRED.	A certificate of occupancy must be procured by owner before this building or part thereof is occupied.
OTHER REQUIRED APPROVALS Fire Dept		
Appeal Board		Ch

PENALTY FOR REMOVING THIS CARD

Permit # 030-336

Pumit# 03-0621

All Purpose Building Permit Application

If you or the property owner owes real estate or personal property taxes or user charges on any property within the City, payment arrangements must be made before permits of any kind are accepted. Portland, avenue, 131 Hope Location/Address of Construction: 0 4103 Total Square Footage of Proposed Structure Square Footage of Lot 250/ Tax Assessor's Chart, Block & Lot Owner: Jeffrey and Kirsten Telephone: Chart# Block# Lot# Martin 846-6667 34% B Lessee/Buyer's Name (if Applicable) Applicant name, address & telephone: Kirskn Martin Work: \$ HESSEL CT Yarmouth, the 04106 Current use: W foundation If the location is currently vacant, what was prior use: Approximately how long has it been vacant: $\mathcal{A}^{\mathcal{O}^+}$ $\mathcal{A}^{\mathcal{O}^+}$ Proposed use: Project description: Contractor's name, address & telephone: 1,50-6196 Kirsten & Jeff Martin Who should we contact when the permit is ready: Mailing address: We will contact you by phone when the permit is ready. You must come in and pick up the permit and review the requirements before starting any work, with a Plan Reviewer. A stop work order will be issued and a \$100.00 fee if any work starts before the permit is picked up. PHONE: 846-6667 IF THE REQUIRED INFORMATION IS NOT INCLUDED IN THE SUBMISSIONS THE PERMIT WILL BE AUTOMATIC DENIED AT THE DISCRETION OF THE BUILDING/PLANNING DEPARTMENT, WE MAY REQUIRE ADDITIONAL. INFORMATION IN ORDER TO APROVE THIS PERMIT.

This is NOT a permit, you may not commence ANY work until the permit is issued. If you are in a Historic District you may be subject to additional permitting and fees with the Planning Department on the 4th floor of City Hall

Signature of applicant:

I hereby certify that I am the Owner of record of the named property, or that the owner of record authorizes the 2rdp certify and that I

have been authorized by the owner to make this application as his/her authorized agent. I agree to conform to all applicable lows of this jurisdiction. In addition, if a permit for work described in this application is issued, I certify that the code Official's authorized representative shall have the authority to enter all areas covered by this permit at any reasonable hour to enforce the provision for the group of t

Date: 5/28

Martin - 131 Hope Que

HIDS THERESON		A Company of the Comp
Estimate #93,000 for building X now	1 X 700 = "651.00	[[i]] MAY I 6 2003 [[i]]
		THE CONTRACT AND ADDRESS OF THE PARTY OF THE
Soil type/Presumptive Load Value (Table 401.4.1)	1 500	
Component	Market Control	Inspection/Date/Findings
STRUCTURAL	SEE PIRM HOME OVERNEY	Plan Reviewer
Footing Dimensions/Depth (Table 403.1.1 & 403.1.1(1), Section 403.1.2)		
Foundation Drainage Dampproofing (Section 406)	SEE PIAN	Prior permit
Yentilation (Section 409.1) Crawls Space ONLY	& N	
AnchorBolts/Straps (Section 403.1.4)	6 Specung, 12 diameter, 10" Deep	
Lally Column Type, Spacing and footing sizes (Table 502.3.4(2))	STEADARD STEEL COLUMN W/CEMENT FILER	
Built-Up Wood Center Girder Dimension/Type	SEE PIMPO WIGKSG STEELBEGM W 8 YSI STEELBEMM	Held specs
(Table 502,3,4(2))	1	
)imesions	2 XL PrescueTreated WFOOM Sill Sealer	OK_
First Floor Joist Species Dimensions and Spacing (Table 503.3.1(1) & Table 503.3.2(1))	I-Justs -see floor Plan Layout	No Colon
Second Floor Joist Species Dimensions and Spacing Table(503.3.1(1) & Table 503.3.2(1))	No second Floor - N/A	N/A

Tempered Needin Unable to read Mud spects 5126 3plans ARBANFII PLOOR. HONGATEZ 4'X8' 3/4" ROW: TRANSAFERIN WALL-ADVANTEZ 4'YB' 1/2" GATTE 4'YB' 3/6" WALL-ADVANTEZ 4'YB' 1/2". 5/8 4x8 Free Res SHEETPOCK 90 min Thurmather Fire Dage # 5,0 SEE SCHEDLLE-ATTRCHMENT / Chimney Cap -1 But Sheuthing where root and Chimney meet see attachnent 5 Per Buch Psuilding Conle 448' 5/8 Advented Sheather 30 X46 Double Hung Architectual Shingles SEE ATTACHMENT 3 13/4" elevation (sill) In taumeliey Rown 15# Let Proch SET Plan SEE PLAN Section 309 and Section 407 1999 BOCA)
Living Space? Notwayspace above
(Above or beside) Lunny Space Rear Rightof Fabore Dimensions and Spacing(Table 802.4.2 or 503.3.1(1) & Table 503.3.2(1)) Attic or additional Floor Joist Species Roof Rafter; Pitch, Span, Spacing& Fire rating of doors to living space Door Sill elevation (407.5 BOCA) Sheathing; Floor, Wall and roof Draft Stopping around chimney Egress Windows (Section 310) Dimension (Table 802.3.2(7) Safety Glazing (Section 308) Attic Access (BOCA 1211.1) Private Garage (Table 602.3(1) & (2)) astener Schedule Table 503.2.1(1) Fire separation Roof Covering (Chapter 9)

w

(b)

Header Schedule	SCE MTTACHMONT 4	OK
Type of Heating System	Forced Not Worke / Bass Bravel	oK
Stairs Number of Stairways		
Interior 1 - to Busynant		
Exterior 1- Deck - 3 stars double Hard Equard and	Spin	10K
Treads and Risers (Section 314)	10/6" Tread 7 3/4 maximum Rizer	
Width 4'	7 7	- ok
Headroom	8. MINUMIN	10 K
Guardrails and Handrails (Section 315)	- Interior 1/2" Hand rand -3/1" mey prayching thank and -36" mey prayching the same of and they At	¥
Smoke Detectors Location and type/Interconnected	See Plan . IN MI Bodrooms + Hallways Hard word W Rathery Buckey	Ne
Plan Reviewer Signature		
See Chimney Summary Checklist 8" X 10" Concrete Block - Interna	where the there interest	

the Back on extend 2' above any roof within 10' & Chinney. OK

Deck framing & setbacks. Interior headers in bearing walls-

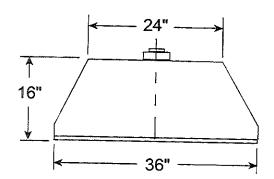
Attachment 4

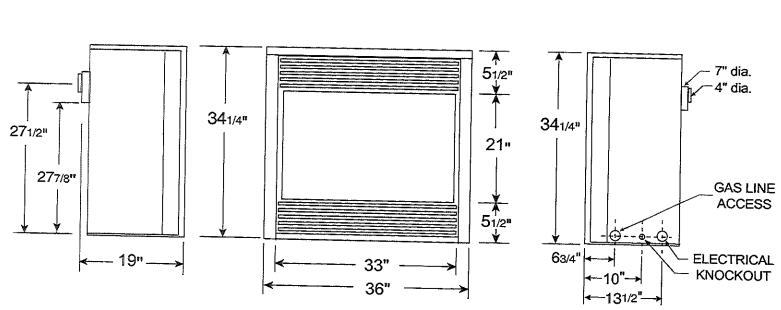
Header Schecdule

Width	Qty	Construction
3' (Window Header) 7'6"(Window Header) 9' (Window Header)	20 1 2	2-2X10 - Gable end beation 2-2X10 - Gable end beation 2-2X10 port bearing gable end
2'6"(Door Header) 3'(Door Header) 6' (Door Header) 9' (Door Header)	6 12 8 2	2-2X10 2-2X10 2-2X10- gable ends 2-2X10- garage dooks- not bearing
all headers double lamina	Led 2×10'5	

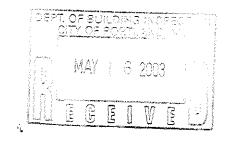


36BDVR SPECS AND DIMENSIONS





NOTE: For further information on specifications consult the appropriate Installation Instructions.

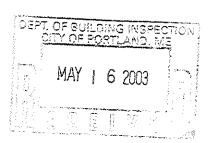


Attachment 1

Window Schedule

Number	Qty	Floor	Size	
W01	1	1	76X98/76X38 *	
W02	17	1	30X46	
W03	1	1	30X46-2	
W04	2	1	30X32	
W05	2	1	9x90 TRANSOM	

^{*}Custom Double Hung with Half round right stacked All windows sized in feet

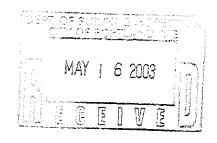


Attachment 2

Door Schedule

Number	Qty	Floor	Size		
Exterior					
D01	1	1	60X68 SI	LIDER	
D02	1	1	60X68 (front w elliptical trans)		
D14	Newsons .		30X68		
D15	Within	· Parameter and a second	60X68		
D16	Parameter	1	90X80 GARAGE		
D17	1	1	30X68 FIRE DOOR		
Interior					
D04	6	1	26X68 6 PANEL		
D 05	3		30X68	"	
D 06	3	1	30x68	44	
D 07	1	1	40X68	"BIFOLD	
D 08	3	1	30X68	"POCKT	
D09	1	1	60X68	FRENCH	
D 10		1	30X68	"POCKT	
D11	1	1	60X68	"POCKT	
D12		1	60X68	"POCKT	
D13	2	1	60x68	"BIFOLD	

All interior doors are masonite 6 pannel except french Doors Doors are sized in feet



Hammond Lumber Auburn 282 Poland Road Auburn, ME 04210 PHONE #: (207)784-4009 FAX #: (207)782-7780

DESCRIPTION

ESTIMATE FOR: (1050245) KIRSTEN MARTIN 165 ELDERBERRY DR. SOUTH PORTLAND, ME 04106 PHONE #: (207)799-4455 FAX #:

FILE NAME: martin8

CONTACT : KEVIN HACKETT x125

SETUP DATE: 05/12/03

CELL #:

START DATE: 05/12/03 UN BY: kch l N

415/03 REPRINT

Expiration Date: 05/20/03

HOME #: JOB #:

This estimate may contain forest products whose prices must be updated on the above * expiration date.

age: 1

WHERE USED QTY U/M PRICE U/M EXTENDED Revision R1 to this quote created 5/7/03
***BUILDER TO CONSULT WITH TRUSS MANUFACTORER PRIOR TO
CONSTRUCTION.ALL STEEL BEAMS ARE TO BE ENGINEERED. NO BEAMS INCLUDED IN QUOTE!!*** ** Foundation/Basement ** - MISC ADS FLEX PIPE - PERF. 01 RL 79.990 RL 250 LF PER ROLL 79.99 N 10" ANCHOR BOLT W/NUT & WASHER 45 EA 0.530 EA 3 1/2"X 8' COLUMN W/CAP & BASE 23.85 N 09 EA 19.500 EA 175.50 N * Bulkhead Steps/Custom Door ** 2X4-8' K.D. SPRUCE 2X4-8' PRESSURE TREATED 2X12-12' K.D. SPRUCE S 08 EA 1.940 EA 15.52 N 02 EA 2KS 3.020 EA 6.04 N 02 EA 2X10-16' K.D. SPRUCE 14.240 EA 6KS 28.48 N 01 EA 13.180 EA 1X6 V/M KNOTTY PINE 13.18 N 120 LF **GBLUE** 2"X2'X8' T&G STYROFOAM 25 PSI 0.589 LF 70.68 N 02 EA 12.170 EA R 10.0 24.34 N BILCO C BSMT DOOR 55X72X191/2 01 EA 312.750 EA 06 RL 4.500 RI ILL FOAM SILL SEALER 5 1/2X50' 312.75 N 4.500 RL 27.00 N



Hammond Lumber Auburn Page: 2

ESTIMATE FOR:(1050245) KIRSTEN MARTIN

FILE NAME: martin8

:M	DESCRIPTION 2X6 PRESSURE TREATED RANDOM	WHERE USED	1\U YTQ	4 PRICE II/M	CVTTNDCD	
Ψ'.	2X6 PRESSURE TREATED RANDOM LENGTH		262 LF	0.708 LF	185.50 N	
	** First Floor Assembly **					
OORTRUSS NG KS dvantech	FLOOR TRUSS PACKAGE FM HANGER OPTION 2X10 K.D. SPRUCE RANDOM LENGTH 4X8 3/4" T&G ADVANTECH FLOORING 28 OZ PL400 HD SUBFLOOR ADHEST		01 PKG 140 EA 263 LF 82 EA	4200.000 PKG 2.250 EA 0.808 LF 18.950 EA	4200.00 N 315.00 N 212.50 N 1553.90 N	* *
14KS 14KS 350	2X12-14' K.D. SPRUCE 2X10-14' K.D. SPRUCE WM231 HAND RAIL 1 1/2" X 1 3/4 DP57-1050 B/B HAND RAIL BRACKE		22 EA 03 EA 04 EA 14 LF 03 EA	4.310 EA 16.610 EA 12.330 EA 1.341 LF 1.340 EA	94.82 N 49.83 N 49.32 N 18.77 N 4.02 N	
S (S 3KS LOKS L2KS L6KS LOKS ER VANTECH	2X6 K.D. SPRUCE RANDOM LENGTH 2X6 K.D. SPRUCE 2X10-8' K.D. SPRUCE 2X10-10' K.D. SPRUCE 2X10-12' K.D. SPRUCE 2X10-16' K.D. SPRUCE 2X10-16' K.D. SPRUCE 2X10-10' K.D. SPRUCE 4X8 7/16" OSB PANEL 24/16 4'X8' 1/2" ADVANTECH SHEATHING		786 LF 284 EA 04 EA 17 EA 08 EA 02 EA 04 EA 70 EA	0.413 LF 3.360 EA 6.830 EA 8.530 EA 9.880 EA 13.180 EA 8.530 EA 7.920 EA 13.880 EA	324.62 N 954.24 N 27.32 N 145.01 N 79.04 N 26.36 N 34.12 N 554.40 N	OPTION
S S 2KS SKS 2KS SKS KS KS	2X4-8' K.D. SPRUCE 2x4 K.D. SPRUCE RANDOM LENGTH 2X6-8' K.D. SPRUCE 2X6 K.D. SPRUCE RANDOM LENGTH 2X10-12' K.D. SPRUCE 2X10-8' K.D. SPRUCE 2X10-10' K.D. SPRUCE 2X10-10' K.D. SPRUCE 2X10-16' K.D. SPRUCE 2X10-10' K.D. SPRUCE		383 EA 1064 LF 52 EA 124 LF 01 EA 10 EA 07 EA 04 EA 02 EA 02 EA 02 EA	1.940 EA 0.275 LF 3.360 EA 0.413 LF 9.880 EA 6.830 EA 9.880 EA 13.180 EA 8.530 EA 8.530 EA 9.880 EA	743.02 N 292.60 N 174.72 N 51.21 N 9.88 N 68.30 N 59.71 N 39.52 N 26.36 N 17.06 N 17.06 N	

ESTIMATE FOR:(1050245) KIRSTEN MARTIN

FILE NAME: martin8

Hammond Lumber Auburn Page: 3

.M	DESCRIPTION	WHERE USED	QTY U/M	PRICE U/M	EXTENDED	
	GARAGE EXTERIOR WALLS					
S OKS 10KS 12KS 16KS 12KS 12KS ER DVANTECH	2X6 K.D. SPRUCE RANDOM LENGTH 2X6-10' K.D. SPRUCE 2X10-10' K.D. SPRUCE 2X10-12' K.D. SPRUCE 2X10-16' K.D. SPRUCE 2X10-12' K.D. SPRUCE 2X10-12' K.D. SPRUCE 2X10-12' K.D. SPRUCE 4X8 7/16" OSB PANEL 24/16 4'X8' 1/2" ADVANTECH SHEATHING		114 EA 01 EA 01 EA 01 EA 02 FA	0.413 LF 3.990 EA 8.530 EA 9.880 EA 13.180 EA 9.880 EA 9.880 EA 7.920 EA 13.880 EA	145.38 N 454.86 N 8.53 N 9.88 N 13.18 N 19.76 N 19.76 N 300.96 N	OPTION
	GARAGE COMMON WALL				OE) . 14	OFITUM
S DKS 12KS	2X6 K.D. SPRUCE RANDOM LENGTH 2X6-10' K.D. SPRUCE 2X10-12' K.D. SPRUCE		90 LF 29 EA 01 EA		37.17 N 115.71 N 9.88 N	
	ROOF TRUSSES					
TRUSS PKG	ROOF TRUSSES COMPLETE PACKAGE		01 PKG 8	3395.000 PKG	8395.00 N	*
	TSB2-24 TRUSS SPACER/BRACER SIMPSON 50/BOX		200 EA	1.040 EA	208.00 N	
,	2x4 K.D. SPRUCE RANDOM LENGTH		510 LF	0.275 LF	140.25 N	
	ROOF FRAMING				- 	
	2X6 K.D. SPRUCE RANDOM LENGTH GABLES		1686 LF	0.413 LF	696.32 N	
R VANTECH ks KS KS KS ks	4X8 7/16" OSB PANEL 24/16 4'X8' 1/2" ADVANTECH SHEATHING 2X6-10' K.D. SPRUCE 2X6-12' K.D. SPRUCE 2X6-14' K.D. SPRUCE 2X6-16' K.D. SPRUCE 2X6-18' K.D. SPRUCE		04 EA	7.920 EA 13.880 EA 3.990 EA 4.580 EA 5.390 EA 6.110 EA 9.540 EA	18.32 N	OPTION

Hammond Lumber Auburn

ESTIMATE FOR:(1050245)

RIRSTEN MARTIN

FILE NAME: martin8

EM	DESCRIPTION	WHERE USED		PRICE U/M	EXTENDED	
gadvantech	4X8 5/8" T&G ADVANTECH SHEATHING		164 EA	15.750 EA		
)EW	8"X10" WHITE GALV DRIP EDGE STD 20/BOY			3.950 EA		
!S	15# FELT PAPER 432 SQ/FT ROLL GRACE ICE/WATERSHIELD 36"X75" 225 SQ FT		14 RL 07 RL	14.990 RL 84.950 RL	209.86 N 594.65 N	
.М30НВ Т2	CT LANDMARK 30* HEATHERBLEND 4 BDL/SQ 30 YEAR SHINGLE		204 BDL	11.790 BDL		
, 2	4' SHINGLE VENT RIDGEVENT SHINGLE OVER RIDGEVENT BLACK		32 EA	10.990 EA	351.68 N	
	** Windows **					
tgardrtrans	PRIMED WOOD SSB GLAS		02 EA	275.000 EA	550.00 N	;
DERSENWINDOW	ANDERSEN WINDOW QUOTE 100236 INCLUDES PATIO DOOR W/GRLLS		01 PKG10	903.670 PKG	10903.67 N	,
	EXTERIOR DOORS					
103068s1 _K	CLASSIC CRAFT CC-10 3068 14" CC-2020SL		01 EA 11	199.000 EA	1199.00 N	*
	SCHLAGE ENTRANCE LOCK F51 BEL 605 16-211 10-063		06 EA	20.660 EA	123.96 N	
01186R	3/0X6/8 SMTH STAR#118RH 6 9/16 908-P BNL SELF SEALING SILL		01 EA 3	329.000 EA	329.00 N	*
210db1	SMOOTH STAR 210 DOUBLE DOOR 6068		01 EA 5	19.000 EA	519.00 N	*
)210L	3/0X6/8 SMTH STAR#210 LH 49/16 908-P BNL SELF SEALING SILL		01 EA 2	47.000 EA	247.00 N	*
.0KDLH	3/0X6/8 PREM#510 FIRE DOOR ADJ STL FRAME 4 1/2-7 1/2 WALL LH		01 EA 2	19.000 EA	219.00 N	*
2S	9X8 #62 WHT SOLID INSULATED R-PANEL HOLMES GARAGE DOOR		02 EA 3	99.000 EA	798.00 N	*
1P12	1/2 HP DOOR OPENER MODEL CG40 CHAMBERLAIN CHAIN DRIVE		02 EA 14	49.950 EA	299.90 N	
	EXTERIOR TRIM					
:	1X6 D4S #4 PINE 1X3 D4S #4 PINE WM180 BRICK MOULD CLEAR		700 LF 700 LF 52 LF	0.340 LF 0.147 LF 1.575 LF	238.00 N 102.90 N 81.90 N	

ESTIMATE FOR: (1050245) KIRSTEN MARTIN

FILE NAME: martin8

Hammond Lumber Auburn Page: 5

.M DESCRIPTION WHERE USED QTY U/M PRICE U/M ------EXTENDED 1 1/4"X 2" ,2 1X8 #2 PINE 52 LF 0.797 LF 41.44 N ** Vinyl Siding and Accessories ** 10' VINYL STARTER STRIP 35 PC 50PC/BOX 3.650 PC 127.75 N WHITE VINYL PVC TRIM COIL 06 EA 75.590 EA 24"X50' 453.54 N WHITE UNDERSILL TRIM 12.6" 25 PC 3.880 PC 40PC/BOX 97.00 N ID VINYL SIDING SOLID SOFFIT 22 EA 8.410 EA USE FOR QUOTES ONLY 185.02 N VINYL SIDING PERF SOFFIT 19 EA 8.410 EA USE FOR QUOTES ONLY 159.79 N F-CHANNEL CHATEAU SOLID COLORS 38 EA 4.370 EA USE FOR QUOTES ONLY 166.06 N J-CHANNEL CHATEAU SOLID COLORS 72 EA 3.650 EA USE FOR QUOTES ONLY 262.80 N)LOR CHATEAU SOLID COLOR VINYL SDG 38 SQ 55.990 SQ USE FOR QUOTES ONLY 2127.62 N 'AR TYPAR BARRIER SHEETING 9'X100' 05 RL 82.790 RL 413.95 N INSULATION 6 1/4X15X93 KRAFT BATT R-19 48.44SF 5 PC/PKG #900266 40 BAG 16.120 BAG 644.80 N K 12X16X48 KRAFT BATT R-38 56 BAG 22.790 BAG 32.00SF 6 PC/PKG #900125 1276.24 N 12X24X48 KRAFT BATT R-38 48.00 SF 6 PC/PKG #900135 RAFT-R-MATE 75PC/BOX 60 BAG 33.990 BAG 2039.40 N 150 EA 1.200 EA ATTIC RAFTER VENT 180.00 N 1X3 D4S #4 PINE 2400 LF 0.147 LF 352.80 N DRYWALL SR. 1/2 4X12 SHEETROCK 180 EA 9.790 EA 5/8 4X8 FIRE RES SHEETROCK 1762.20 N 0.250 SF 8.900 EA 1/2 4X8 MOISTURE RES SHEETROCK 92 sf 23.00 N 30 EA DRYWALL ALLOWANCE 267.00 N 01 EA 400.000 EA 400.00 N

ESTIMATE FOR:(1050245) KIRSTEN MARTIN

FILE NAME: martin8

Hammond Lumber Auburn Page: 6

EM	DESCRIPTION	WHERE USED	QTY U/M	PRICE	U/M	EXTENDED
	INTERIOR TRIM	, *********				
.0	COLONITAL CACTUC		1100 LF	0.599	LF	658.90 N
2 RKT	WM662 BASE COL 9/16" X 3 1/2" 1X6 #2 PINE LWM233 CLOSET POLE 1 1/4" SHELF & ROD BRACKET 193000 1X12 #2 PINE 1X3 #2 PINE		200 LF 64 LF 10 EA 64 LF	0.599 0.891 3.140 1.395	LF LF EA LF	632.00 N 119.80 N 57.02 N 31.40 N 89.28 N 37.08 N
	** Interior Doors **					
SCCULH 3MD 8MD 925 3MDULH 4068	2/6X6/8 LH CRF FJ SPLT P8710 2/6X6/8 6 PNL WG MLD DOOR ONLY 3/0X6/8 6 PNL WG MLD DOOR ONLY POCKET DOOR FRAME SET 1/4X6/8 LH MLD FJ SPLT P8710 4/0X6/8 6PNL PRM MOULDED BFLD TEXTURED	,	13 EA 03 EA 03 EA 06 EA 01 EA 01 EA	121.430 47.660 51.260 62.060 76.460 83.660	EA EA EA EA EA	1578.59 N 142.98 N 153.78 N 372.36 N 76.46 N 83.66 N
5068	5/0X6/8 6PNL PRM MOULDED BFLD		02 EA	91.760	EA	183.52 N
5068	6/0X6/8 6PNL PRM MOULDED BFLD TEXTURED		01 EA	101.660	ĒΑ	101.66 N
1db8606r	DBL 3068 H/C MOLDED SMOOTH BROSCO		01 EA	302.570	EA	302.57 N
_KBV	SCHLAGE PASSAGE LOCK F10V BEL 605 TRIPLE LATCH SCHLAGE PRIVACY LOCK		05 EA	12.560	EA	62.80 N
_KBV	SCHLAGE PRIVACY LOCK F40V BEL 605 TRIPLE LATCH		09 EA	13.460	EA	121.14 N
	** Porches/Decks **					
\ 	5' TAPERED CONCRETE POST 8"X50' .013 ALUMINUM FLASHING STANDARD - FULL ROLLS ONLY		01 RL	14.720	EA RL	269.76 N 14.72 N
:PT	2X8-12' PRESSURE TREATED 2X8 LUS28 SINGLE JOIST HANGER FACE MOUNT 50/BOX		17 EA 48 EA	11.320 0.710	EA EA	192.44 N 34.08 N
	2X8 PRESSURE TREATED RANDOM LENGTH					20.90 N
.T	2X8-8' PRESSURE TREATED 2X8 PRESSURE TREATED RANDOM		09 EA 42 LF	6.980 1.045	EA LF	62.82 N 43.89 N

ESTIMATE FOR:(1050245) . KIRSTEN MARTIN

Hammond Lumber Auburn

Page: 7

EM	DESCRIPTION	WHERE USED	QTY U/M	PRICE U/	M EXTENDED		
STWG 3PT L2PT	LENGTH 5/4X6 WINCH GRAY TREX DECKING RANDOM LENGTH 4X4-8' PRESSURE TREATED 4X4-12' PRESSURE TREATED		06 EA	1.690 L	A 43.14 N		*
/KP =GS	1X6 V/M KNOTTY PINE 72" CHARCOAL FIBERGLASS SCREENING		01 EA 770 LF 48 LF	10.850 E, 0.589 L! 1.620 L!	F 453.53 N		
ISD	3/0X6/9X1 1/8 WOOD SCREEN DOOR ALLOWANCE		01 EA	49.460 E	49.46 N		
.OWK .OWB .OWN OWFL .OWPS CDSG5	KITCHEN ALLOWANCE BATH ALLOWANCE NAIL & MISC HARDWARE ALLOWANCE FLOORING ALLOWANCE PAINT & STAIN ALLOWANCE 5# 2 1/2" COMPOSITE DECK SCREW GRAY - SQUARE DRIVE - TRAPEASE 5"X5"X8' COLONIAL PORCH POST YARDCRFTRS-LOAD BEARING 5000LB			0.000 EA 0.000 EA 0.000 EA 0.000 EA 26.990 PR	A A A A KG 107.96 N	0.00 N 0.00 N 0.00 N 0.00 N 0.00 N	OPTION OPTION OPTION OPTION OPTION

SALES TAX NOT INCL NET 63229.69

Net toxl

FILE NAME: martin8

****** TERMS OF ESTIMATE ******

Upon request, your Hammond Lumber Company salesperson will provide an updated quotation after the estimate date has expired.

Due to various methods of construction and building practices, we recommend that you supply your own material list to be priced. Material lists created by Hammond Lumber represent only our best estimate of the required quantities needed to complete your project. Hammond Lumber Company assumes no responsibility for the accuracy of the quantities.

If allowance figures have been used in this estimate, they represent approximate costs based only on past experience with similar projects. Hammond Lumber Company assumes no responsibility for the accuracy of allowance estimates.

Items priced out by the LF in this quote may include 10'. 12', 14' & 16' lengths only.

ATTACHMENT 3

from BOCA 99 Table 2305.2

FASTNER SCHEDULE

CONNECTION	NAILING
1. Joist to sill or girder, toenail	3-8d (1)
2. Bridging to joist, toenail each end	2-8d
3. 1" x 6" (25 mm x 152 mm) subfloor or less to each joist, face nail	2-8d
4. Wider than 1" x 6" (25 mm x 152 mm) subfloor to each joist, face nail	3-8d
5. 2" (51 mm) subfloor to joist or girder, blind and face nail	2-16d
6. Sole plate to joist or blocking, typical face nail Sole plate to joist or blocking at braced wall panels	16d @ 16" (406 mm) o.c. 3-16d per 16" (406 mm) o.c.
7. Top plate to stud, end nail	2-16d
8. Stud to sole plate	4-8d toenail, or 2-16d end nail
9. Doubled studs, face nail	16d @ 24" (610 mm) o.c.
10. Doubled top plates, typical face nail	16d @ 16" (406 mm) o.c.
Double top plates, lap splice	8-16d
11. Blocking between joists or rafters to top plate, toenail	3-8d
12. Rim joist to top plate, toenail	8d @ 6" (152 mm) o.c.
13. Top plates, laps and intersections, face nail	2-16d
14. Continuous header, two pieces	16d at 16" (406 mm) o.c. along each edge.
15. Ceiling joists to plate, toenail	3-8d
16. Continuous header to stud, toenail	4-8d
17. Ceiling joists, laps over partitions, face nail	3-16d
18.Ceiling joists to parallel rafters, face nail	3-16d



19.Rafter to plate, toenail	3-8d
20. 1" (25 mm) brace to each stud and plate, face nail	2-8d
21. 1" x 8" (25 mm x 203 mm) sheathing or less to each bearing, face nail	2-8d
22. Wider than 1" x 8" (25 mm x 203 mm) sheathing to each bearing, face nail	3-8d
23. Built-up corner studs	16d @ 24" (610 mm)o.c.
24. Built-up girder and beams	20d @ 32" (813 mm)o.c. at top and bottom and staggered, 2-20d at ends and at each splice.
25. 2" (51 mm) planks	2-16d at each bearing
26. Wood structural panels and particleboard:	
Subfloor, roof and wall sheathing (to framing): (1 inch = 25.4 mm)	
1/2" and less 19/32" - 3/4" 7/8" - 1" 1 1/8" - 1 1/4"	6d 8d or 6d 8d 10d or 8d
Combination subfloor-underlayment (to framing): (1 inch = 25.4 mm)	
3/4" and less 7/8" - 1" 1 1/8" - 1 1/4"	6d 8d 10d or 8d
27. Panel siding (to framing): 1/2" (13 mm) or less 5/8" (16 mm)	6d 8d
28. Fiberboard sheathing: (7) 1/2" (13 mm) thickness	No. 11 ga. 6d No. 16 ga.
25/32" (20 mm) thickness	No. 11 ga. 8d No. 16 ga.
29. Interior paneling 1/4" thickness 3/8" thickness	6d 8d





Land Surveying Land Planning

133 Gray Road Falmouth, Maine 04105-2029 (207) 797-9199 Fax (207) 878-3142

Bath (207) 442-7799 New Gloucester (207) 926-4699

June 4, 2003

Jonathan Reed City of Portland 389 Congress Street Portland, ME 04101

VIA FAX: (207) 874-8716

re: Presumpscot River Place III

Dear Mr. Reed:

This letter will confirm that Titcomb Associates has located the foundation forms as installed for the building on Lot 25 at the Presumpscot River Place III Subdivision. The said forms are in accordance with the locations shown on the development plans prepared by Gorrill-Palmer Consulting Engineers. The forms for the garage portion have not been installed as of this date. This letter of confirmation is for the dwelling portion of the building only.

The house location is in compliance with the City of Portland Zoning Ordinance and Setback Requirements. The building location is based on established property pins.

Please call if you have any further questions.

Sincerely,

David E. Titcomb, PLS
President, Titcomb Associates

cc: Jeff Martin [via fax: (207)878-6338]



Titcomb Associates

DATE: JUNE 6, 2003

Land Surveying Land Planning

8 Portland North Business Park Falmouth, Maine 04105 (207) 797-9199

Bath (207) 442-7799 . New Gloucester (207) 926-4699

FAX COVER SHEET

TO: JONATHAN ROED			
COMPANY NAME: Cidy of Populano FAX NO. 874-8716			******
FAX NO. 874-8716			
NUMBER OF PAGES (INCLUDING COVER SHEET)_	2		
MESSAGE: REGRITE IGNORE FIRST	Letter	1+ hAD	WYENG
Lot Number Ref.			
FROM: Rex CroteAU			



Land Surveying Land Planning

133 Gray Road Falmouth, Maine 04105-2029 (207) 797-9199 Fax (207) 878-3142

Bath (207) 442-7799 New Gloucester (207) 926-4699

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David E. Treomb, PLS
President, Titcomb Associates

cc: Jeff Martin [via fax: (207)878-6338]

Earm # P 04

Please Read

DISPLAY THIS CARD ON PRINCIPAL FRONTAGE FOR WORK CITY OF PORTLAND

Other MAY 0 7 2003 Department Name	PENALTY FOR REMOV	INGTHIS CARD	Director - Building & Inspection Services
Fire Dept. PERMIT SSUED Health Dept. Appeal Board			
OTHER REQUIRED APPROVALS			- /
Apply to Public Works for street line and grade if nature of work requires such information.	g and writen permis	t thereo p losed-in.	A certificate of occupancy must be procured by owner before this building or part thereof is occupied.
the construction, maintenanc this department.	e and use of buildings a	and situatures, an	d of the application on file i
of the provisions of the Statu			
provided that the person or pe	ersons, firm or corpor at	ion accepting this	s permit shall comply with a
AT Lot 25 Hope Ave		GL 389 G00	3001
has permission to Single family found	dation onl		
This is to certify that Goldeneye Corp/P	.A. Ricci I wavating	<u> </u>	
Notes, If Any, Attached	PERM		Permit Number: 030336

City of Portland, Maine - Buil	lding or Use Permi	t	Permit No:	Date Applied For:	CBL:	
389 Congress Street, 04101 Tel: (.6 03-0336	04/15/2003	389 G003001	
Location of Construction:	Owner Name:		Owner Address:		Phone:	
Lot 25 Hope Ave (131)	Goldeneye Corp				207-773-5650	
Business Name:	Contractor Name:	ne: Contractor Address: Phone			Phone	
	P.A. Ricci Excavating	;	40 Chambers Ave South Portland (207) 767-4338			
Lessee/Buyer's Name	Phone:		Permit Type:			
			Foundation Only/	Residential		
Proposed Use: Proposed Project Description:						
Lot 25 Presumpscot River Place III: Single family foundation only Single family foundation only						
•	,			omy		
Dept: Zoning Status: A	parayod with Condition		N. O.1. 1	1		
•	pproved with Condition		r: Marge Schmucka	T 1		
Note: 4/29/03 Actually on hold - I t	alked to Jeff Martin - I	need elevations	to determine height	and number of	Ok to Issue: 🗹	
stories for required setbacks - 05/01/03 received requested p	le will get to me by the	e end of the we	ek .			
		d				
,	_					
 All driveways shall be paved with 12' wide. Individual homeowners 	in six (6) months of the shall be responsible for	receipt of a ter paving of driv	nporary certificate of eways.	foccupancy and sha	ll be a minimum of	
3) "No parking" street signs shall be of occupancy (whether permanent	installed along the subo or temporary) for this s	division roadwa	ys PRIOR to the iss	suance of the first ho	ouse lot certificate	
4) No certificate of occupancy shall the entire length of Hope Avenue	be issued for any house	lot in the subdi	vision until the base	pavement has been	completed along	
5) This approval is for a FOUNDAT			required for the bui	llding construction.		
6) Separate permits shall be required						
o, soparato permito suan se regamen	for fature decks, stieds,	, pools, and/or	garages. No rear deci	ks are snown on the	submitted plans	
Dept: Building Status: A	pproved	Reviewe	: Tammy Munson	Approval D	Pate: 05/07/2003	
Note:			•	pp	Ok to Issue:	
					OR to issue:	
Comments:						
5/2/03-tmm: Spoke w/ Kirsten Martin bolts.	regarding info needed of	on plans - need	to show dampproofi	ng, drain tile, filter	fabric, and anchor	
5/6/03-tmm: rec'd requested info - ok	to issue.					

	Applicant: Jelf Martin Date: 4/29/03
	Address: Hope AUE (LITES) C-B-L: 392-A-25 PATA PRESUMPSCOT RIVER PLACE CHECK-LIST AGAINST ZONING ORDINANCE
	Date - New #03-0336
	Zone Location - R-Z
	Interior or corner lot - Proposed Use/Work - Construct New Sunfa fair with Attached garage Sougge Disposal - C. fr
	Proposed Use Work - Con Strick New Sungar TAL DU METHOD
	Doints 2 17 17 17 17 17 17 17 17 17 17 17 17 17
	Loi Street Frontage - 50/ - g _ 50/ + Show
	Front Yard - 25'in - D43' Scalad
	Rear Yard - 25 mm - 250'+ Show To min reg - 12' Show of Show
	Side Yard - Twanty to devoting the Side Yard -
	Projections - (Men) (9 - 10)
	Projections - (mitentry - No Very Olects Side) Width of Lot - 80 mi - 100' Scallad Height - 35' MAX - with for plans 24:25' scalad to ridge
	Height - 35 MAX - W2ND
	Lot Area - 10,000 mm 44,161 mstr plan
	(For Coverage Surface - 70 / N QQ 37 74 MX
()	Area per Family - 10,000k Off-street Parking - 2 Reg W24 will gAAG Show - 2 CAN Show
0)	Off-street Parking -
	Loading Bays - A
	Site Plan - # 2003-007/
	Shoreland Zoning/Stream Protection - N
	Flood Plains - PAvel 2 Zone X
ans Od	mers of Cot 25 shall Not impact wetlands at The
	card The lot.

From:

Marge Schmuckal

To:

Jay Reynolds

Date:

Tue, Apr 29, 2003 12:28 PM

Subject:

lot #25 Hope Ave.

Just to update you on this lot - I said that I would write a letter to the owner on this because his building is outside of the shown envelope. I can't do that because note # 3 does not include lot #25 in the building envelope area. and the notation above it actually states "side yards shown on plan are 14 feet and may be increased or decreased depending upon number of stories". So lot #25 is ok for this IF it is only one story.

I have talked to Jeff Martin. I require some elevations so that I can determine the height and number of stories so that I can determine required setbacks. He said that he could get that for me by the end of this week.

Marge

CC:

PENNY LITTELL; Sarah Hopkins

All Purpose Building Permit Application

	must be made before permits of any k	narges on any property withir kind are accepted.
Location/Address of Construction:	Appeare sub.	LOT# 23
Total Square Footage of Proposed Structu 2985	re Square Footage of Lot	acus or 47,161
Tax Assessor's Chart, Block & Lot Chart# 30 Block# 30 Jot# 00 Not available per assessor	Swner: Goldeneye Corp	Telephone:
Lessee/Buyer's Name (If Applicable)	Applicant name, address & telephone: Jeff Marhn	Cost Of 38,000 Work: \$_38,000
Jeft Martin	4 Essex CT. Yarmouth me 04091	
Current use: Vacant Land		Site Fee 30
If the location is currently vacant, what wo	J	(010 +
Approximately how long has it been vaca Proposed use: Single family Project description:		
Contractor's name, address & telephone:		
Who should we contact when the permit i Mailing address:	is ready: <u>Coastal Managlmen</u>	<u>+</u> 791.3688
We will contact you by phone when the preview the requirements before starting ar		

INFORMATION IN ORDER TO APROVE THIS PERMIT.

I hereby certify that I am the Owner of record of the named property, or that the owner of record authorizes the proposed work and that I have been authorized by the owner to make this application as his/her authorized agent. I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in this application is issued, I certify that the Code Official's authorized representative shall have the authority to enter all areas covered by this permit at any reasonable hour to enforce the provisions of the codes applicable to this permit.

Signature of applicant:	Date:	DEPT, OF BUILDING INSPECTION
This is NOT a permit, you may not commence ANY If you are in a Historic District you may be subject to add Planning Department on the 4 th floor	litional perm	itting and tees with then

CITY OF PORTLAND, MAINE DEVELOPMENT REVIEW APPLICATION PLANNING DEPARTMENT PROCESSING FORM DRC Copy

2003-0071

Application I. D. Number

Jeff & Kristen Martin 04/15/2003 Application Date Applicant 4 Essex Court, Yarmouth, ME 04096 Lot 25, Hope Ave., Presumpscot River Applicant's Mailing Address Project Name/Description 25 Hope Ave, Portland, Maine Consultant/Agent ess of Proposed Site Applicant Ph: (207) 000-0000 Agent Fax: G003001 Applicant or Agent Daytime Telephone, Fax Assessor's Reference: Chart-Block-Lot Proposed Development (check all that apply): 📝 New Building 🗌 Building Addition 📋 Change Of Use 📝 Residential 📋 Office 📋 Retail ☐ Manufacturing ☐ Warehouse/Distribution ☐ Parking Lot Other (specify) foundation only R2 Proposed Building square Feet or # of Units Acreage of Site Zoning Check Review Required: Site Plan Subdivision PAD Review 14-403 Streets Review (major/minor) # of lots ☐ Flood Hazard Shoreland DEP Local Certification Zoning Conditional Zoning Variance Other Use (ZBA/PB) Site Plan Fees Paid: \$50.00 Subdivision Engineer Review \$250.00 Date 04/15/2003 DRC Approval Status: Reviewer Jay Reynolds Approved Approved w/Conditions ___ Der See Attached Approval Date 05/01/2003 Approval Expiration 05/01/2004 Extension to Additional Sheets ✓ Condition Compliance Jay Reynolds 05/01/2003 signature date Performance Guarantee Required* Not Required * No building permit may be issued until a performance guarantee has been submitted as indicated below Performance Guarantee Accepted date amount expiration date inspection Fee Paid date amount Building Permit Issue date Performance Guarantee Reduced date remaining balance signature Temporary Certificate of Occupancy Conditions (See Attached) date expiration date Final Inspection date signature Certificate Of Occupancy date Performance Guarantee Released date signature Defect Guarantee Submitted submitted date amount expiration date Defect Guarantee Released date signature

CITY OF PORTLAND, MAINE DEVELOPMENT REVIEW APPLICATION PLANNING DEPARTMENT PROCESSING FORM

2003-0071

	ADDENDUM	Application I. D. Number
Jeff & Kristen Martin		04/15/2003
Applicant		Application Date
Essex Court, Yarmouth, ME 04096		Lot 25, Hope Ave., Presumpscot River
Applicant's Mailing Address		Project Name/Description
	25 - 25 Hope Ave,	Portland, Maine
Consultant/Agent	Address of Propose	d Site
Applicant Ph: (207) 000-0000 Agent Fax:	389 G003001	
Applicant or Agent Daytime Telephone, Fax	Assessor's Referen	ce: Chart-Block-Lot

Approval Conditions of DRC

- 1 NOTE: THE PROPOSED BUILDING FALLS OUTSIDE OF THE BUILDING ENVELOPE. LOT 25 IS NOT LISTED AS ONE THAT WOULD NEED A SUBDIVISION PLAT AMENDMENT. THIS IS ACCEPTABLE, BEING THAT IT MEETS THE CURRENT ZONING STANDARDS.
- 2 All damage to sidewalk, curb, street, or public utilities shall be repaired to City of Portland standards prior to issuance of a Certificate of Occupancy.
- 3 Two (2) City of Portland approved species and size trees must be planted on your street frontage prior to issuance of a Certificate of Occupancy.
- 4 Your new street address HAS NOT BEEN ESTABLISHED, the number must be displayed on the street frontage of your house prior to issuance of a Certificate of Occupancy.
- The Development Review Coordinator (874-8632) must be notified five (5) working days prior to date required for final site inspection. Please make allowances for completion of site plan requirements determined to be incomplete or defective during the inspection. This is essential as all site plan requirements must be completed and approved by the Development Review Coordinator prior to issuance of a Certificate of Occupancy. Please schedule any property closing with these requirements in mind.
- 6 A sewer permit is required for you project. Please contact Carol Merritt at 874-8300, ext . 8822. The Wastewater and Drainage section of Public Works must be notified five (5) working days prior to sewer connection to schedule an inspector for your site.
- As-built record information for sewer and stormwater service connections must be submitted to Public Works Engineering Section (55 Portland Street) and approved prior to issuance of a Certificate of Occupancy.
- 8 The site contractor shall establish finish grades at the foundation, bulkhead and basement windows to be in conformance with the first floor elevation (FFE) and sill elevation (SE) set by the building contractor to provide for positive drainage away from entire footprint of building.
- 9 The Development Review Coordinator reserves the right to require additional lot grading or other drainage improvements as necessary due to field conditions.

P.02

p. 1

PURCHASE AND SALE AGREEMENT - LAND ONLY

Feb. 3, 2003	
-	Effective Date is defined in Paragraph 20 of this Agreement
1 PARTIES: This Agreement is made between , Je	
(hereinafter called "Buyer") of	125 and Kirsten Martin
	hereinafter called "Seller") of
)[2]	Westernot WF 14002
State of Maine, located at Lat 2	after set forth. Seller agrees to sell and Buyer agrees to buy (atl
Acquisity of Decois Book(s) Page(s)	If "part of" see Other Conditions (personnel 22) for all the
3. CONSIDERATION: For such Deed and conveyance Buyer is of which	s to pay the sum of PRICE \$ 95000
13 included herewith as an exmediment	DEPOSITS 4000
of the Dee	od.
This Purchase and Sale Agroement is subject to the following en-	nditions:
4 EARNEST MONEY/ACCEDTANCE A #	c C 1 1
and act as escrow agent until closing, this offer shall be valid until	("Agency") shall hold said earnest money
that the Agency is made a new to seller's non-acceptance.	this earnest money shall be returned promptly to Rever to the
attorney's fees and costs which shall be assessed as court costs in	this earnest money shall be returned promptly to Buyer. In the event ting as escrow agent, Agency shall be entitled to recover reasonable favor of the providing party.
5. TITLE AND CLOSING: A dead	party.
the Maine Bar Association shall be delivered to Report and this	chantable title in accordance with the Standards of Title adopted by transaction shall be closed and Buyer shall pay the balance due and
exceed 30 days from the time c	this paragraph, then Seller thall have a recently it
carnest money and he relieved from all obligations. Seller here	s a merchantable title, Buyer may, at Buyer's option, withdraw said
• *****	s a merchantable title, Buyer may, at Buyer's option, withdraw said by agrees to make a good-faith effort to cure any title defect during
O. DEED the person shall be a second of	
continued current use of the account	deed, and shall be free and clear of all inicitions of record which do not materially and adversely affect the
7 POSSUSCION IN	and monetación succesión s
7. POSSESSION: Possession of premises shall be given to Buyer 8. RISK OF LOSS: Until the closing shall be given to Buyer	r immediately at closing unless otherwise named in which
E RISK OF LOSS: Until the closing, the risk of loss or demand	to closing for the purpose of determining that the purpose of determining the purpose of determining that the purpose of determining the purpose of determining the purpose of determining the purpose of the purpose
substantially to view the property within 24 hours prior	to closing for the purpose of determining that the premises are in
and the date of this Agreement.	or determining that the premises are in
9. PRORATIONS: The following items, where applicable shall	the prorated as of the date of closing; rent, association fees, (other)
fiscal year) Sall- is Real estate tax	res shall be provated as of the date of closing; rent, association fees, (other) rears. If the amount of said taxes is a said (based on municipality's
they shall be approximed on the best any unpaid taxes for prior y	cars. If the amount of said taxes is and it (based on municipality's
and valuation can be ascertained which large assessed for th	cars. If the amount of said taxes is not known at the time of closing, are preceding year with a reapportionment as soon as the new tax rate are closing. Durier and Seller will see the new tax rate
required by State of Maine.	re preceding year with a reapportionment as soon as the new tax rate arvive closing. Duyer and Seller will each pay their transfer tax as
10 PROPERTY DISCLOSURE FORM	and the second s
information from professionals regarding any specific issue or cont	ccipt of Seller's Property Disclosure Form and is encouraged to seek
INSPECTIONS. Buyer is encouraged to seek information of	
11 INSPECTIONS: Buyer is encouraged to seek information from Re 2002 Page 1 of 4 - Page 4.0 Buyer(a) Initials	professionals regarding any specific issue or concern
The state of the s	_ Seller(v) Initials _
	/1/~
If we would have the clos	Sellers maints

207 871 0680 TO 98748716 781 7193

P.03

Agent makes no warranties regarding the condition, permitted use or value of Scilers' real property. This Agreement is subject to the following contingencies, with results being satisfactory to Buyer

_	CONTINCENCY	YES	NO	Days for Completion	OBTAINED BY	TO BE PAID FOR BY
1	SURVEY					rok B1
	Purpose/_/	school	& pur	chares sale	corecment	
2.	SOILS TEST		Ø		-	
	Purpose:					-
3	LOCAL PERMITS		\boxtimes			·
	Purpose:			• • • • • • • • • • • • • • • • • • • •		
4	HAZARDOUS WASTE REPORTS		Ø.			
	Purpose;				* #	***************************************
5	SUB-DIVISION APPROVAL	A			- 	
	Purpose:	2 ap	provel	our amende	1 estes de	
6	DEPALURC APPROVATE	1 50	F}			
	PurposeAllas	hed f	purchas	e + soles =	greement.	
7.	ZONING VARIANCE		177			
	Purpose;					
8.	MDOT DRIVEWAY/ ENTRANCE PERMIT	Ø				,
	Purposo:					
)	OTHER		[X]			
	Purpose:					
Ų.	ther specifications regarding a	nv of the ah	011-			

Unless otherwise specified above, all of the above will be obtained and paid for by Buyer. If the result of any inspection or other condition specified herein is unsatisfactory to Buyer, Buyer will declare the Agreement null and void by notifying Seller in writing within the specified number of days, and any earnest money shall be returned to Buyer. If the result of any inspection or other condition specified herein is unsatisfactory to Buyer, and Buyer wishes to pursue remedies other than voiding the Agreement, Buyer must do so to full membring within the time avoid set forth above, otherwise this contingency is waited. If Buyer does not notify must do so to full resolution within the time period set forth above; otherwise this contingency is waived If Buyer does not notify Seller that an inspection is unsatisfactory within the time period set forth above, this contingency is waived by Buyer. In the absence of inspection(x) mentioned above. Buyer is relying completely upon fluyer's own opinion as to the condition of the property

Page 2 of 4 - Page 4 of 4 - Page 5 of 4 - Page 6 of 4 - Page 6 of 4 - Page 7 of 4 - Page 7

Page 2 of 4 - PAS-L (1 Anyonia) Initials

APR 15 2003 10:50 FR CREATIVE LENDING Mar 27 03 02:46p Bob Adam.

207 781 7193

p.3

	12 FUNANCING This Agreement is subject to Buyer obtaining an approved // mongage of % of
	the purchase price, at an interest rate not to exceed % and amortized over a period of years.
	information, is qualified for the loan requested within days from the Effective Date of the Appreciation of
λ	b Buyer to provide Seller with mortgage commitment letter from lender showing that Buyer has secured the loan commitment within
MY	If either of these conditions is not met within said time periods. Seller may terminate this Agreement and the earnest money shall be returned to Buyer.
M	d After (a) and (b) are met. Buyer is obligated to notify Seller in writing if the lender notifies Buyer that it is unable or unwilling to proceed with the financing. Any failure by Buyer to notify Seller within 48 hours of receipt by Buyer of notice from the lender shall be a default under this Agreement.
	c. Suyer agrees to pay no more than points. Seller agrees to pay S toward points and/or Buyer's closing costs.
	Neilker the seller acknowledge they have been advised of the following agency relationships. Neilker the seller of buyer have agency relationships. Of presents
	Listing Agent Of Agency represents
	Agency
	Selling Agent Agency represents
	If this transaction involves Disclosed Dual Agency, the Buyer and Seller acknowledge the limited fiduciary duties of the agents and hereby consent to this arrangement. In addition, the Buyer and Seller acknowledge prior receipt and signing of a Disclosed Dual Agency Consent Agreement. 14. MEDIATION: Any dispute or claim arising out of or relating to this Agreement or the property addressed in this Agreement shall be submitted to mediation in accordance with the Maine Residential Real Estate Mediation Rules of the American Arbitration Association. Buyer and Seller are bound to mediate in good faith and pay their respective mediation fees. If a party does not agree first to go to mediation, then that party will be liable for the other party's legal fees in any subsequent litigation regarding that same matter in which the party who refused to go to mediation loses in that subsequent litigation. This clause shall survive the closing of the transaction.
	15. DEFAULT. In the event of default by the Buyer, Seller may employ all legal and equitable remedies, including without limitation, termination of this Agreement and forfeiture by Buyer of the carnest money. In the event of a default by Seller, Buyer may employ all legal and equitable remedies, including without limitation, termination of this Agreement and return to Buyer of the earnest money. Agency acting as escrow agent has the option to require written releases from both parties prior to disbursing the earnest money to either Buyer or Seller.
	16. PRIOR STATEMENTS. Any representations, statements and agreements are not valid unless contained herein. This Agreement completely expresses the obligations of the parties.
	17. HEIRS/ASSIGNS. This Agreement shall extend to and be obligatory upon heirs, personal representatives, successors, and assigns of the Seller and the assigns of the Buyer.
	18 COUNTERPARTS: This Agreement may be signed on any number of identical counterparts, such as a faxed copy, with the same binding effect as if the signatures were on one instrument Original or faxed signatures are binding.
	14 ADDENDA Yes Explain: DEP Approval, Subdivision Approval 20 EFFECTIVE DATE: This Agreement is a binding contract when signed by both Department.
	20 EFFECTIVE DATE: This Agreement is a binding contract when signed by both Buyer and Seller and when that fact has been communicated to Buyer and Seller or to their agents. Agent is authorized to complete Effective Date on Page 1 of this Agreement The use of "by (date)" or "within days" shall refer to calendar days being counted from the Effective Date as noted on Page 1 of the Agreement, beginning with the first day after the Effective Date and ending at 5:00 p.m. Eastern Time on the last day counted.
	21. CONFIDENTIALITY: Buyer and Seller understand that the terms of this Agreement are confidential but authorize the disclosure of the information herein to the agents, attorneys, lenders, appraisers, inspectors and others involved in the transaction necessary for statement

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STATE OF MAINE DEPARTMENT OF ENVIRONMENTAL PROTECTION STATE HOUSE STATION 17 AUGUSTA, MAINE 04333

DEPARTMENT ORDER

IN THE MATTER OF

BURT WOLF & BOB ADAM

Portland & Falmouth, Cumberland County

PRESUMPSCOT RIVER PLACE,

FALMOUTH PHASE & PHASE 3

L-19486-L2-C-N (approval)

L-19486-L6-D-N

) SITE LOCATION OF DEVELOPMENT) NATURAL RESOURCES PROTECTION ACT) WATER QUALITY CERTIFICATION

) FINDINGS OF FACT AND ORDER

Pursuant to the provisions of 38 M.R.S.A. Sections 481 \underline{et} $\underline{seg.}$ and 480-A \underline{et} $\underline{seg.}$, and Section 401 of the Federal Water Pollution Control Act, the Department of Environmental Protection has considered the application of BURT WOLF AND BOB ADAM with the supportive data, agency review comments, and other related materials on file and FINDS THE FOLLOWING FACTS:

1. <u>PROJECT DESCRIPTION</u>:

A. History of Project: The applicants purchased the project parcel in the mid 1980's and have since developed several residential subdivisions. Presumpscot River Place - Phase 1 was developed in 1984 and consists of 27 lots. Presumpscot River Place - Phase 2 was developed in 1985 and consists of 27 lots. Alice Road and Hope Avenue were constructed in 1993 and consist of a total of 9 lots. Presumpscot River Place - Falmouth was developed in 1998 and consists of 22 lots on 45 acres.

The Falmouth phase obtained a stormwater management permit, DEP #L-19486-NI-B-N, and a Wetland Alteration permit, Tier 1 #98-503-S, for 6,500 square feet of forested freshwater wetland fill. Both permits are dated February 23, 1998. On May 12, 1998, the applicants increased the amount of wetland fill approved by 3,315 square feet, with DEP #98-554-S. DEP # 99-708-S, dated February 18, 1999, approved an additional 990 square feet of wetland fill on lot 4. DEP #99-839-S, dated October 4, 1999, approved an additional 3,078 square feet of freshwater wetland fill. Wetland impacts for the Falmouth Phase now total 13,883 square feet.

B. Summary: The applicants are seeking approval under the Site Location of Development Act (Site Law) for the existing Falmouth Phase of Presumpscot River Place and for a proposed Phase 3 located within the City of Portland. The earlier phases were recorded more than 5 years ago and do not need to be approved under the Site Law. Phase 3 will subdivide a 53.4 acre parcel of land into 29 house lots ranging in size from approximately 0.55 to 2.71 acres, and two lots 31 and 32, 1.46 and 22.41 acres, respectively. The applicants do not propose to develop lots 31 and 32 at this time. Phase 3 is shown on a set of plans, the first of which is entitled "Presumpscot River Place - Phase 3," prepared

by Gorrill-Palmer Consulting Engineers Inc., and dated November 2001, with a last revision date of August 22, 2002. The Falmouth Phase is shown on a set of plans the first of which is entitled, "Final Subdivision Plan," prepared by Stephen W. Tibbetts, P.E. and Owen Haskell, Inc., and dated September 16, 1997. The project site is located at the north end of Curtis Road, in the City of Portland, Maine and to the west of Stapleford Drive, in the Town of Falmouth.

The applicants are also seeking a Natural Resources Protection Act permit to cross three streams for the construction of the access road to Phase 3. Hope Avenue, and to place stormwater outfalls adjacent to the streams. The project will also fill approximately 13,276 square feet of forested freshwater wetlands for the construction of Hope Avenue and lots 6 and 7 in Phase 3.

C. Current Use of Site: The site of Phase 3 is currently undeveloped fields and woodland. There are no structures on the property. The Falmouth Phase is developed with single-family homes. Both phases contain a Central Maine Power Easement and are located adjacent to the Presumpscot River.

2. FINANCIAL CAPACITY:

The cost for the Falmouth Phase was \$400,000. Falmouth Phase has been completed and no improvements are required at this time. The total cost of Presumpscot River Place Phase 3 is estimated to be \$1,597,000. The applicants are in the process of selling a parcel of land along the Presumpscot River to the City of Portland. The applicants submitted a copy of the purchase and sales agreement with the City of Portland and a letter stating that they intend to use that money towards the construction of Phase 3. The applicants also submitted a letter from Peoples Bank, dated March 7, 2002 indicating that it intends to provide additional financing for this project.

The Department finds that the applicants have demonstrated adequate financial capacity to comply with Department standards.

3. <u>TECHNICAL ABILITY</u>:

The applicants provided resume information for key persons involved with the project and a list of projects successfully constructed by the applicant. The applicants also retained the services of Gorrill-palmer Consulting Engineers, Inc., a professional engineering firm, to assist in the design and engineering of the project.

The Department finds that the applicants have demonstrated adequate technical ability to comply with Department standards.

4. NOISE:

The Department finds that no regulated sources of noise have been identified.

5. <u>SCENIC CHARACTER</u>:

The proposed project is located adjacent to other residential subdivisions and undeveloped land that runs along the north and west property boundaries. The applicants are selling a portion of their property, which runs along the Presumpscot River to the City to permanently protect it from development. The project is located in the City of Portland's North Deering neighborhood, an area currently experiencing a high residential housing demand.

Based on the project's location and design, the Department finds that the proposed project will not have an unreasonable adverse effect on the scenic character of the surrounding area.

6. <u>WILDLIFE AND FISHERIES</u>:

The applicants propose to cross three streams for the construction of Hope Avenue. The proposed stream crossings are shown on plans entitled "Grading, Drainage & Erosion Control Detail Plan and Profile," prepared by Gorrill-Palmer Consulting Engineers and last revised June 21, 2002. The inlet and outlets of all proposed stream crossings will be protected with riprap aprons. The first stream crossing, located at station 28 along Hope Avenue, will be a 36-inch culvert and measure 103 feet in length. The second stream crossing, located at station 17.5, will be a 24-inch culvert and measure 115 feet in length. The third stream crossing, located at station 14.5, will be an 18-inch culvert and measure 145 feet in length.

The Maine Department of Inland Fisheries & Wildlife (MDIFW) reviewed the proposed project. In its comments, MDIFW stated that it found no records of any essential or significant wildlife habitats, or other wildlife habitats of special concern associated with this site. IF&W stated that the project could potentially impact fisheries on the Presumpscot River. Subsequently, IF&W fisheries biologists and the applicants' consultant agreed that permanently protecting the proposed stream buffers with deed restrictions will ensure that fisheries on the Presumpscot River are not likely to be impacted by the project. The applicants revised the set of plans referenced in Finding 1 to reflect this agreement. The applicants propose to provide stream buffers that are more than 100 feet wide on lots 5 and 6; a minimum of 100 foot wide on lots 2, 3, 4 and 8; 75 feet wide on lots 1, 9, 10, 15, 16 and 22; and 50 feet wide on lot 30. IF&W also stated that the stormwater drainage system should be designed to minimize water quality impacts to the small streams. The applicants revised the stormwater management plan to move all stormwater outlets a minimum of 25 feet away from the streams. These changes are reflected in the grading, drainage and erosion control plans referenced earlier in this Finding.

The Department finds that the applicants have made adequate provision for the protection of wildlife and fisheries.

7. HISTORIC SITES AND UNUSUAL NATURAL AREAS:

The Maine Historic Preservation Commission (MHPC) reviewed the proposed project. At the request of MHPC, the applicant conducted both Phase I and Phase II archeological surveys. Upon reviewing the results of those surveys, MHPC stated that the project will not have an effect upon any structure or site of historic, architectural, or archaeological significance as defined by the National Historic Preservation Act of 1966.

The Maine Natural Areas Program database does not contain any records documenting the existence of rare or unique botanical features on the project site and, as discussed in Finding 6, MDIFW did not identify any unusual wildlife habitats located on the project site. The applicants' consultant surveyed the proposed project site and confirmed that no unusual features exist on-site.

The Department finds that the proposed development will not have an adverse effect on the preservation of historic sites or unusual natural areas either on or near the development site.

8. <u>BUFFER STRIPS</u>:

The applicants propose to protect several small streams that flow through the project site with undisturbed buffers as discussed in Findings 6 and 19.

The Department finds that the applicants have made adequate provision for buffer strips.

9. <u>SURFACE WATER OUALITY</u>:

The proposed project is not located within the watershed of a lake or great pond. No discharges to surface waters are proposed other than stormwater.

The proposed project includes 4.55 acres of impervious area and is located within the watershed of the Presumpscot River. Because of the project's location and size, stormwater runoff from the project site must be treated to meet the sliding scale total suspended solids (TSS) standard outline in Chapter 500 of the Department Rules. The applicants propose to remove 40 per cent of TSS from the project's stormwater runoff by installing two Vortechnics Stormwater Treatment units, Model #5000 and Model #11000, as well as Casco Traps on all catch basins. The locations of the Vortechnics units, labeled as WQU1 and WQU2, are shown on the plan prepared by Gorrill-Palmer Consulting Engineers Inc., entitled "Grading, Drainage and Erosion Control Plan and Profile," last revised June 21, 2002.

As discussed in Finding 11, the applicants' proposed stormwater management system was reviewed by, and revised in response to, comments from the Division of Watershed Management of the Bureau of Land and

Water Quality (DWM). Specific aspects of the system, including measures to protect water quality, are further discussed in Finding 11.

Based on the stormwater management system's design and the comments discussed above, the Department finds that the applicants have made adequate provision to ensure that the proposed project will meet the stormwater quality standards contained in Department Rules, Chapter 500 and to ensure that the project will not have an unreasonable adverse impact on surface water quality.

10. <u>SOILS</u>:

The applicants submitted a soil survey map and report based on the soils found at the project site. This report was prepared by a certified soil scientist and reviewed by staff from the Division of Environmental Assessment of the Bureau of Land and Water Quality (DEA).

The Department finds that, based on this report and DEA's review the soils on the project site present no limitations to the proposed project that cannot be overcome through standard engineering practices.

11. STORMWATER MANAGEMENT:

The applicants are not proposing a formal stormwater management system to detain stormwater from 24-hour storms of 2-, 10-, and 25-year frequency. Instead, since the project site is located adjacent to the Presumpscot River, the applicants request a waiver from the peak flow standard pursuant to Department Rules, Chapter 500(3)(A)(1).

The stormwater management system proposed by the applicants was reviewed by, and revised in response to, comments from the Division of Watershed Management of the Bureau of Land and Water Quality (DWM). In its comments, DWM stated that the proposed system complies with Department standards for stormwater management and the waiver may be granted.

Based on the system's design and these comments, the Department finds that the applicants have made adequate provision to ensure that the proposed project will meet the stormwater quantity standards for: (1) peak flow from the site and peak flow of the receiving waters; (2) grading or other construction activity; (3) channel limits and runoff areas; (4) maintenance; (5) discharge to freshwater wetlands; and (6) level spreaders.

12. MAINTENANCE OF COMMON FACILITIES:

The applicants will be responsible for the maintenance of all common facilities including the road and stormwater management system, which maintenance will include, but not be limited to, any necessary erosion and sedimentation control measures, and the long-term maintenance of the stormwater management system as outlined in Section 13 of the application.

13. <u>EROSION AND SEDIMENTATION CONTROL</u>:

The applicants submitted an Erosion and Sedimentation Control Plan as Section 24 of the application. This plan and plan sheets containing erosion control details were reviewed by, and revised in response to the comments of DWM. Erosion control details will be included on the final construction plans and the erosion control narrative will be included in the project specifications to be provided to the construction contractor.

The Department finds that the applicants have made adequate provision to control erosion and sedimentation.

14. **GROUNDWATER:**

The project site is not located over a mapped sand and gravel aquifer. The project does not propose any withdrawal from, or discharge to, the groundwater except for the subsurface wastewater disposal systems in the Falmouth Phase. DEA reviewed the proposed project and commented that there will not be an impact to groundwater.

The Department finds that the proposed project will not have an unreasonable adverse effect on ground water quality or quantity.

15. WATER SUPPLY:

When completed, the proposed project is anticipated to use 19,080 gallons of water per day. The Portland Water District will supply the water. The applicants submitted a letter from the District, dated January 17, 2002, indicating that it will be capable of servicing this project.

The Department finds that the applicants have made adequate provision for securing and maintaining a sufficient and healthful water supply.

16. WASTEWATER DISPOSAL:

When completed, Phase 3 of the proposed project is anticipated to discharge 10,800 gallons of wastewater per day to the City of Portland's Portland Water District wastewater treatment facility. The applicants and the City of Portland agreed to extend the sewer lines and construct a pump station to serve this project. The applicants submitted a letter from the City of Portland's Public Works Department stating that it will accept these flows. This project was reviewed by the Division of Engineering, Compliance and Technical Assistance of the Bureau of Land and Water Quality (DECTA), which commented that the City of Portland's Portland Water District wastewater treatment facility has the capacity to treat these flows and is operating in compliance with the water quality laws of the State of Maine.

Based on DECTA's comments, the Department finds that the applicants have made adequate provision for Phase 3's wastewater disposal at a facility that has the capacity to ensure satisfactory treatment.

Wastewater for the Falmouth Phase is currently being disposed of by individual subsurface wastewater disposal system on each lot. The applicants submitted the soil survey map and report discussed in Finding 14. Each individual system was designed to meet the requirements of the Maine State Plumbing Code. This information was reviewed by, and revised in response to comments from DEA.

Based on DEA's comments, the Department finds that the wastewater disposal systems were built on suitable soil types.

17. SOLID WASTE:

When completed, the proposed project is anticipated to generate 450 cubic yards of household solid waste per year. All general solid wastes from the proposed project will be disposed of at Regional Waste Systems, which is currently in substantial compliance with the Solid Waste Management Regulations of the State of Maine.

The proposed project will generate approximately 5,880 cubic yards of stumps and grubbings. All stumps and grubbings generated will be disposed of on site, either chipped or burned, with the remainder to be worked into the soil, in compliance with Solid Waste Management Regulations of the State of Maine or will be disposed of at the Jolly Farmer facility in Poland, which is currently in substantial compliance with the Solid Waste Management Regulations of the State of Maine.

The proposed project will generate approximately 400 cubic yards of construction debris and demolition debris. All construction and demolition debris generated will be disposed of at Maine Energy Recycling Company, which is currently in substantial compliance with the Solid Waste Management Regulations of the State of Maine.

Based on the above information, the Department finds that the applicants have made adequate provision for solid waste disposal.

18. FLOODING:

The proposed project is not located within the 100-year floodway of any river or stream.

The Department finds that the proposed project is unlikely to cause or increase flooding or cause an unreasonable flood hazard to any structure.

19. <u>WETLAND IMPACTS</u>:

The applicants propose to alter 13,276 square feet of forested freshwater wetland to construct the Phase 3 access road and to place fill on Lots 6 & 7. The Falmouth Phase previously altered 13,883 square feet of freshwater wetlands as outlined in Finding 1. The cumulative impact on freshwater wetlands for this project totals 27,159 square feet.

The Wetland Protection Rules, Chapter 310 requires that the applicants meet the following standards:

- a. Avoidance. No activity, which would cause a loss in wetland area, functions and values, will be permitted if there is a practicable alternative to the project that will be less damaging to the environment. The applicants submitted an alternative analysis for the proposed project. There are no other alternatives that would impact less wetlands.
- b. Minimal Alteration. The applicants are required to minimize the amount of wetland alteration while meeting the project's purpose. The applicants designed the project to minimize wetland impacts. Building windows have been changed to avoid wetland impacts to the greatest extent practicable.
- c. Compensation. The applicants have not submitted a functional assessment. Department staff visited the site on several occasions and determined that a functional assessment was not necessary. The applicants propose to protect 7.6 acres of streams and forested uplands with a deed restriction. The protected area is shown on a plan entitled, "Wetland Permitting Plan Presumpscot River Place," prepared by Gorrill-Palmer Consulting Engineers Inc., and last revised August 22, 2002. Prior to the start of construction, a copy of the recorded deed restriction must be submitted to the Bureau of Land and Water Quality.

The Department finds that the applicants have avoided and minimized wetland impacts to the greatest extent practicable, and that the proposed project represents the least environmentally damaging alternative that meets the project's purpose.

BASED on the above findings of fact, and subject to the conditions listed below, the Department makes the following conclusions pursuant to 38 M.R.S.A. Sections 480-A et seq. and Section 401 of the Federal Water Pollution Control Act:

- A. The proposed activity will not unreasonably interfere with existing scenic, aesthetic, recreational, or navigational uses.
- B. The proposed activity will not cause unreasonable erosion of soil or sediment.
- C. The proposed activity will not unreasonably inhibit the natural transfer of soil from the terrestrial to the marine or freshwater environment.
- D. The proposed activity will not unreasonably harm any significant wildlife habitat, freshwater wetland plant habitat, threatened or endangered plant habitat, aquatic habitat, travel corridor, freshwater, estuarine, or marine fisheries or other aquatic life.
- E. The proposed activity will not unreasonably interfere with the natural flow of any surface or subsurface waters.

- F. The proposed activity will not violate any state water quality law including those governing the classifications of the State's waters.
- G. The proposed activity will not unreasonably cause or increase the flooding of the alteration area or adjacent properties.
- H. The proposed activity is not on or adjacent to a sand dune.
- The proposed activity is not on an outstanding river segment as noted in 38 M.R.S.A. Section 480-P.

BASED on the above findings of fact, and subject to the conditions listed below, the Department makes the following conclusions pursuant to 38 M.R.S.A. Sections 481 et seg.:

- A. The applicants have provided adequate evidence of financial capacity and technical ability to develop the project in a manner consistent with state environmental standards.
- B. The applicants have made adequate provision for fitting the development harmoniously into the existing natural environment and the development will not adversely affect existing uses, scenic character, air quality, water quality or other natural resources in the municipality or in neighboring municipalities provided a copy of the recorded deed restriction is submitted to the Bureau of Land and Water Quality as discussed in Finding 19.
- C. The proposed development will be built on soil types which are suitable to the nature of the undertaking and will not cause unreasonable erosion of soil or sediment nor inhibit the natural transfer of soil.
- D. The proposed development meets the standards for storm water management in Section 420-D and the standard for erosion and sedimentation control in Section 420-C.
- E. The proposed development will not pose an unreasonable risk that a discharge to a significant groundwater aquifer will occur.
- F. The applicants have made adequate provision of utilities, including water supplies, sewerage facilities, solid waste disposal and roadways required for the development and the development will not have an unreasonable adverse effect on the existing or proposed utilities and roadways in the municipality or area served by those services.
- G. The activity will not unreasonably cause or increase the flooding of the alteration area or adjacent properties nor create an unreasonable flood hazard to any structure.

THEREFORE, the Department APPROVES the application of BURT WOLF & BOB ADAM to construct Presumpscot River Place, Falmouth Phase and Phase 3, SUBJECT TO THE FOLLOWING CONDITIONS and all applicable standards and regulations:

1. The Standard Conditions of Approval, a copy attached.

- 2. In addition to any specific erosion control measures described in this or previous orders, the applicants shall take all necessary actions to ensure that their activities or those of their agents do not result in noticeable erosion of soils or fugitive dust emissions on the site during the construction and operation of the project covered by this approval.
- 3. The applicants shall include in all conveyances of subdivision lots deed restrictions making the conveyance subject to all terms and conditions of this Department permit and any applicable municipal approval. These terms and conditions may be incorporated by specific and prominent reference to the permit in the deed. All conveyances required by this approval to contain restrictions shall include in the restrictions the requirement that any subsequent conveyance shall specifically include the same restrictions.
- 4. The applicants shall give a copy of this permit, including the standard conditions, and a copy of the approved subdivision plan to each lot buyer at least 14 days prior to the date of closing on the sale or lease of the lot. The applicants also shall maintain a file containing signed and dated statements by lot buyers or lessees acknowledging that they have received and read their copy of this permit and the subdivision plan prior to the closing on their lot. The file shall also contain a copy of the signed and dated deed or lease containing the restrictive covenants required under this approval. The applicants shall make this file available for inspection upon request by the Department.
- 5. Prior to the start of construction, a copy of the recorded deed restriction shall be submitted to the Bureau of Land and Water Quality.

THIS APPROVAL DOES NOT CONSTITUTE OR SUBSTITUTE FOR ANY OTHER REQUIRED STATE, FEDERAL OR LOCAL APPROVALS NOR DOES IT VERIFY COMPLIANCE WITH ANY APPLICABLE SHORELAND ZONING ORDINANCES.

DONE AND DATED AT AUGUSTA, MAINE, THIS 23 DAY OF Agust

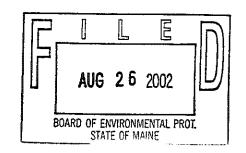
DEPARTMENT OF ENVIRONMENTAL PROTECTION

MARTHA G. KIRKPATRICK, COMMISSIONER

PLEASE NOTE THE ATTACHED SHEET FOR GUIDANCE ON APPEAL PROCEDURES...

Date of initial receipt of application 3/12/02 Date of application acceptance 3/19/02

Date filed with Board of Environmental Protection deh/L19486cn



STANDARD CONDITIONS

STRICT CONFORMANCE WITH THE STANDARD AND SPECIAL CONDITIONS OF THIS APPROVAL IS NECESSARY FOR THE PROJECT TO MEET THE STATUTORY CRITERIA FOR APPROVAL.

- 1. This approval is dependent upon and limited to the proposals and plans contained in the application and supporting documents submitted and affirmed to by the applicant. Any variation from the plans, proposals and supporting documents is subject to the review and approval of the Board prior to implementation. Further subdivision of proposed lots by the applicant or future owners is specifically prohibited, without prior approval by the Board of Environmental Protection, and the applicant shall include deed restrictions to this effect.
- 2. The applicant shall secure and comply with all applicable Federal, State and local licenses, permits, authorizations, conditions, agreements, and orders, prior to or during construction and operation as appropriate.
- 3. The applicant shall submit all reports and information requested by the Board or Department demonstrating that the applicant has complied or will comply with all conditions of this approval. All preconstruction terms and conditions must be met before construction begins.
- 4. Advertising relating to matters included in this application shall refer to this approval only if it notes that the approval has been granted WITH CONDITIONS, and indicates where copies of those conditions may be obtained.
- 5. Unless otherwise provided in this approval, the applicant shall not sell, lease, assign or otherwise transfer the development or any portion thereof without prior written approval of the Board where the purpose or consequence of the transfer is to transfer any of the obligations of the developer as incorporated in this approval. Such approval shall be granted only if the applicant or transferee demonstrates to the Board that the transferee has the technical capacity and financial ability to comply with conditions of this approval and the proposals and plans contained in the application and supporting documents submitted by the applicant.
- 6. If the construction or operation of the activity is not begun within two years, this approval shall lapse and the applicant shall reapply to the Board for a new approval. The applicant may not begin construction or operation of the development until a new approval is granted. Reapplications for approval shall state the reasons why the development was not begun within two years from the granting of the initial approval and the reasons why the applicant will be able to begin the activity within two years from the granting of a new approval, if granted. Reapplications for approval may include information submitted in the initial application by reference.
- 7. If the approved development is not completed within five years from the date of the granting of approval, the Board may reexamine its approval and impose additional terms or conditions or prescribe other necessary corrective action to respond to significant changes in circumstances which may have occurred during the five-year period.
- 8. A copy of this approval must be included in or attached to all contract bid specifications for the development.
- 9. Work done by a contractor pursuant to this approval shall not begin before the contractor has been shown by the developer a copy of this approval.

(2/81)/Revised November 1, 1979

STANDARD CONDITIONS

THE FOLLOWING STANDARD CONDITIONS SHALL APPLY TO ALL PERMITS GRANTED UNDER THE NATURAL RESOURCE PROTECTION ACT, TITLE 38, M.R.S.A. SECTION 480-A ET.SEQ. UNLESS OTHERWISE SPECIFICALLY STATED IN THE PERMIT.

- A. <u>Approval of Variations From Plans.</u> The granting of this permit is dependent upon and limited to the proposals and plans contained in the application and supporting documents submitted and affirmed to by the applicant. Any variation form these plans, proposals, and supporting documents is subject to review and approval prior to implementation.
- B. <u>Compliance With All Applicable Laws.</u> The applicant shall secure and comply with all applicable federal, state, and local licenses, permits, authorizations, conditions, agreements, and orders prior to or during construction and operation, as appropriate.
- C. <u>Erosion Control.</u> The applicant shall take all necessary measures to ensure that his activities or those of his agents do not result in measurable erosion of soils on the site during the construction and operation of the project covered by this Approval.
- D. <u>Compliance With Conditions.</u> Should the project be found, at any time, not to be in compliance with any of the Conditions of this Approval, or should the applicant construct or operate this development in any way other the specified in the Application or Supporting Documents, as modified by the Conditions of this Approval, then the terms of this Approval shall be considered to have been violated.
- E. <u>Initiation of Activity Within Two Years.</u> If construction or operation of the activity is not begun within two years, this permit shall lapse and the applicant shall reapply to the Board for a new permit. The applicant may not begin construction or operation of the activity until a new permit is granted. Reapplications for permits shall state the reasons why the applicant will be able to begin the activity within two years form the granting of a new permit, if so granted. Reapplications for permits may include information submitted in the initial application by reference.
- F. Reexamination After Five Years. If the approved activity is not completed within five years from the date of the granting of a permit, the Board may reexamine its permit approval and impose additional terms or conditions to respond to significant changes in circumstances which may have occurred during the five-year period.
- G. No Construction Equipment Below High Water. No construction equipment used in the undertaking of an approved activity is allowed below the mean high water line unless otherwise specified by this permit.
- H. Permit Included In Contract Bids. A copy of this permit must be included in or attached to all contract bid specifications for the approved activity.
- Permit Shown To Contractor. Work done by a contractor pursuant to this permit shall not begin before the contractor has been shown by the applicant a copy of this permit.

Revised (4/92)

Centulard Title

GOLDENEYE CORP. P.O. BOX 1382 PORTLAND, ME 04104

Robert Adam Tel. 207-781-3224 Burt Wolf Tel. 207-773-5650

BUYER CONDITIONS

The Buyer agrees that:

- 1. The Premise is subject to the conditions of the approved recording Plat;
- The Premise is subject to the Declaration of Restrictions to be recorded at the Cumberland County Registry of Deeds;
- Any structure that is to be built on the Premises must be approved by the Seller and/or his agent. No structure shall be erected on the Premises except one detached single family, residential dwelling of not more than two and one-half stories in height, and containing not less than 2000 square feet of enclosed space above the foundation, excluding garages and open porches, except in instances when the Seller shall give prior permission to the contrary in writing. All homes shall be required to include an attached garage containing not less than two bays;
- 4. There shall be no house trailer, business or commercial vehicle or vehicles or similar nature shall be brought upon, or maintained or be permitted to remain on the Premise except a business vehicle normally used by a lot owner in his or her occupation. No unregistered or inoperable motor vehicles or trailers of any nature may be kept upon the Premise unless such vehicle is stored in a garage or other enclosed structure. No recreational vehicles, camping trailers or similar vehicles shall be permitted or maintained on any lot unless the same are stored completely within a garage.
- No livestock, animals or poultry, other than dogs and cats shall be kept, maintained or allowed on the Premise.
- The Buyer is required to conserve or plant a minimum of two trees on their lot meeting the City of Portland's arboricultural specification and standard of practice and landscape design guidelines.

J- 10/24/02

MDEP

Buyer acknowledges receipt of a copy of the Maine Department of Environmental Protection (MDEP) Site Location of Development Act Permit for the project. It is the Buyer's responsibility to review the Permit as it may affect their proposed lot.

EUYER

DATE 1/24/02

To: BUYER

From: SELLER, GOLDENEYE COROPERATION

SELLER will be responsible for snow plowing and sanding Hope Avenue until said road is accepted by the City of Portland.

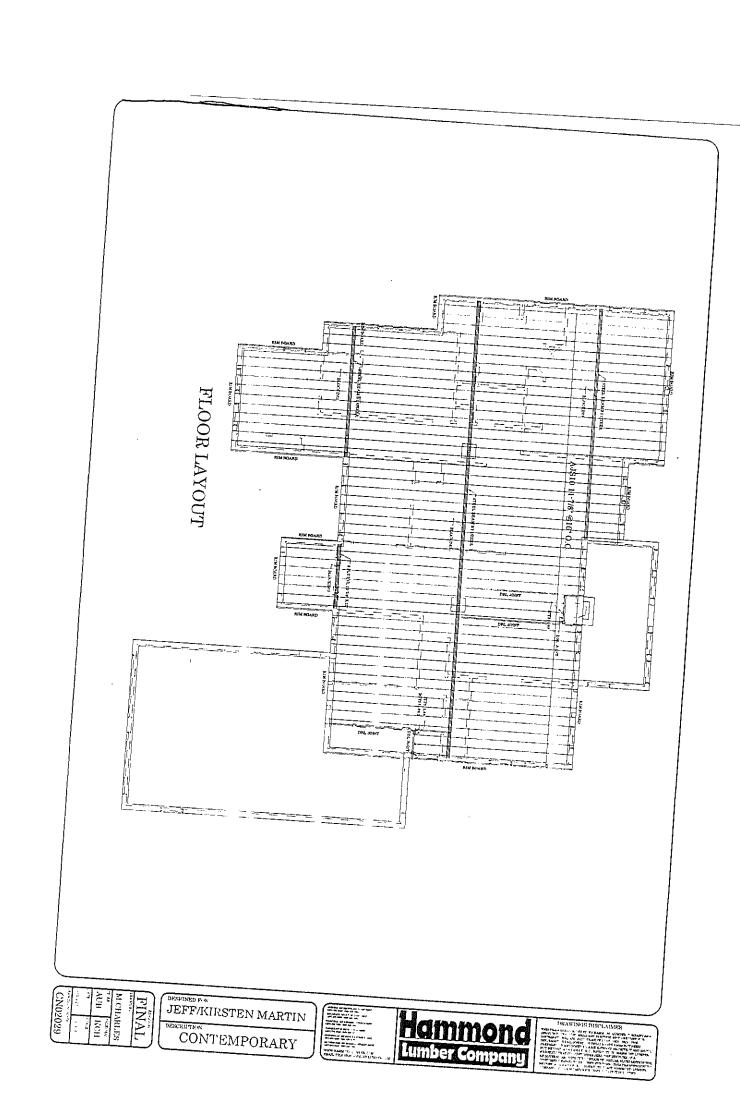
SELLER will be responsible to provide electricity to street light poles until the City accepts Hope Avenue

SELLER has provided BUYER with a "copy of sheets 7, 8, 9, & 14 from the Plan Set as stated in Item # 12 of the Notes

SELLER has provided BUYER with a copy of the Maine Department of Environmental Protection (MDEP) Site Location Permit for the project. It is the BUYER'S responsibility to review the Permit as it may affect their proposed lot.

BUYER is responsible for their own rubbish removal until the City accepts Hope Avenue.

Signature	10/24/02
	Date
Signature	Date



CITY OF PORTLAND, MAINE DEVELOPMENT REVIEW APPLICATION PLANNING DEPARTMENT PROCESSING FORM

Building Copy

		Building Copy	Application I. D. Number	
Jeff & Kristen Martin			4/15/2003	
Applicant 4 Essex Court, Yarmouth, ME 04	1000		Application Date	
Applicant's Mailing Address	1096		Lot 25, Hope Ave., Presu	ımpscot Riv
		25 - 25 Hone Av	Project Name/Description e, Portland, Maine	
Consultant/Agent Applicant Ph: (207) 000-0000	A	Address of Propo	sed Site	
Applicant or Agent Daytime Teleph	Agent Fax:	389 G003001		
Proposed Development (check all		Assessor's Refere	ence: Chart-Block-Lot	
Manufacturing Warehous		is Delicing Addition Char	ge Of Use 📝 Residential 🗌 Office	Retail
		47161	Other (specify) foundation only	
Proposed Building square Feet or #	f of Units	Acreage of Site	R2	
Check Review Required:			Zoning	
Site Plan (major/minor)	Subdivision # of lots	PAD Review	14-403 Streets I	Review
Flood Hazard	Shoreland			
Zoning Conditional		☐ HistoricPreserv	ation DEP Local Certi	fication
Use (ZBA/PB)	Zoning Variance		Other	
Fees Paid: Site Pla s	550.00 Subdivision			
		Engineer Review	\$250.00 Date 4/15/2003	
Building Approval Stat	us:	Reviewer		
Approved	Approved w/Condi	tions	enied	
Approval Date	Approval Expiration	F		
Condition Compliance	pprova Expiration	Extension to	Additional Sheet	s
	signature	date	Attached	
Performance Guarantee	Required*	Not Positive I		
* No building permit may be issued u		Not Required		
Performance Guarantee Accepte	d	erias been submitted as indicated b	elow	
	date			
Inspection Fee Paid		amou	expiration dat	e
To Monthly and the	date	amou	nt	
Building Permit Issue				
Performance Guarantee Reduced	date			
	date			
Temporary Certificate of Occupan		remaining b	Signature	
	date	Conditions (See	·	
Final Inspection			expiration date	•
Certificate Of Occupancy	date	signatu	е	
Gertificate Of Occupancy				
Performance Guarantee Released	date			
	date	cionat	2	
Defect Guarantee Submitted		signatur	#	
Defect Guarantee Released	submitted date	amount	expiration date	
- 5.000 Quarantee meleased	at - 2			
	date	signature		

SPACE AND BULK REQUIREMENTS - R-2 ZONE

MINIMUM LOT SIZE:

10,000 S.F.

MINIMUM FRONTAGE:

50 FT.

MINIMUM SETBACKS:

FRONT YARD 25 FT.
REAR YARD 25 FT.
SIDE YARD*
1 STORY 12 FT

1 STORY 12 FT. 1 1/2 STORY 12 FT. 2 STORY 14 FT. 2 1/2 STORY 16 FT.

MINIMUM LOT WIDTH:

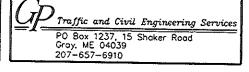
OTHER USES:

80 FT.

* THE WIDTH OF ONE (1) SIDE YARD MAY BE REDUCED ONE (1) FOOT FOR EVERY FOOT THAT THE OTHER SIDE YARD IS CORRESPONDINGLY INCREASED, BUT NO SIDE YARD SHALL BE LESS THAN TWELVE (12) FEET IN WIDTH.

THE SIDE YARDS SHOWN ON THE FOLLOWING FIGURES ARE BASED UPON A (1) ONE STORY STRUCTURE AND MAY BE INCREASED OR DECREASED DEPENDING UPON THE NUMBER OF STORIES.

Design: DER	Date: APR 03
Droft: SGB	Job No.: 759
Checked: AMP	Scole: NTS
File Nome: 759	==0 dwa



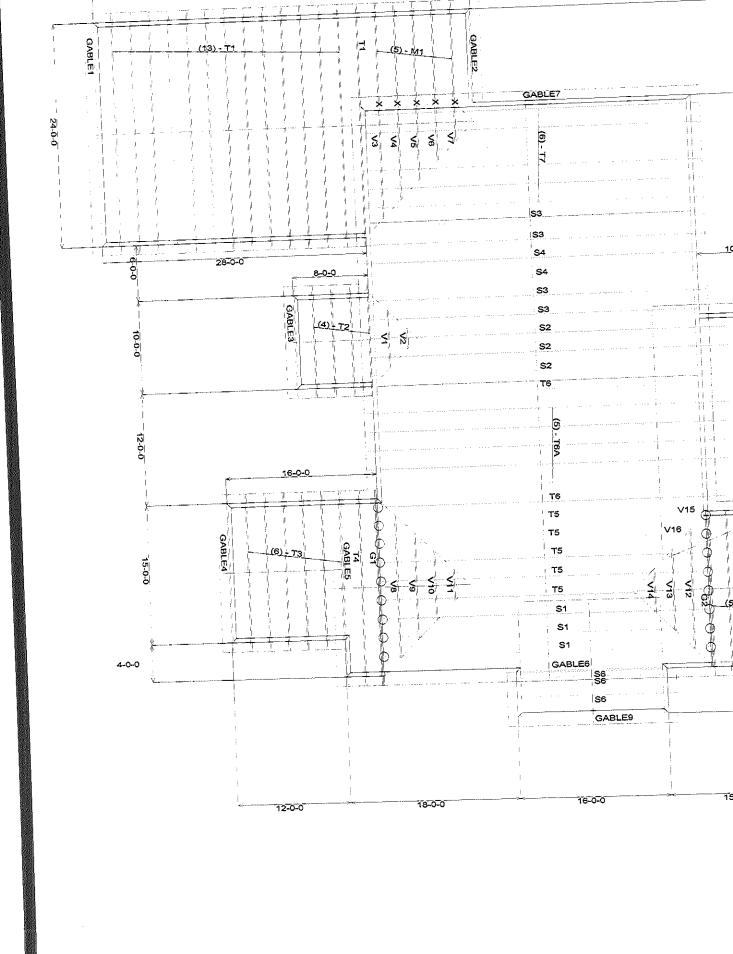
Drawing Name:
Space & Bulk Requirements
Project:
PRESUMPSCOT RIVER PLACE

P. A. Ricci Excavating, Inc. 40 Chambers Avenue South Portland, Maine 04106

Kirstine & Jeff Martin March 5,2003 165 Elderberry Drive H- 799-4455 C- 415-4745 South Portland, Me 04106 W- 797-3688 x 208 Terms: 1/2 down, 2/3 remaining balance when walls are poured. Balance upon completion Job Location: LOT 25 Presumpscot River Woods	Phone /6/-4338	Cell Phone
Kirstine & Jeff Martin 165 Elderberry Drive H-799-4455 C- 415-4745 South Portland, Me 04106 W-797-3688 x 208 Terms: 1/2 down, 2/3 remaining balance when walls are poured. Balance upon completion Job Location: LOT 25 Presumpscot River Woods Remove trees 20' around house and garage foundation Burry stumps and brush on lot Stock pile existing loom on lot Excavate for house foundation according to plans Form and pour house and garage according to plans with bulkhead and 4 windows Water plug and waterproof house wall All stone under house floor with radon Install inside and outside gravity foundation drain to stub at property line Pour 4" basement floor and finish Install sand inside garage foundation and compact Install gravel driveway approxamatly 12' x 75' x 24' wide at garage Install 4" sewer line from house to stub at property line Install 1 " water line from house to shut off at property line Grade entire area around house and garage Spread existing loom around house foundation and garage Excavate for electrical trench Price includes \$ 1500.00 for extra fill around house if needed Price includes all material and labor above Price does not include any extra loom or seeding Contract Price: \$38,745.00 Customer: Dated:	671-8343	
165 Elderberry Drive South Portland, Me 04106 W-797-3688 x 208 Terms: 1/2 down, 2/3 remaining balance when walls are poured. Balance upon completion Job Location: LOT 25 Presumpscot River Woods Remove trees 20' around house and garage foundation Burry stumps and brush on lot Stock pile existing loom on lot Excavate for house foundation according to plans Form and pour house and garage according to plans with bulkhead and 4 windows Water plug and waterproof house wall All stone under house floor with radon Install inside and outside gravity foundation drain to stub at property line Pour 4" basement floor and finish Install sand inside garage foundation and compact Install 4" concrete floor with fiber mesh and finish for garage Install 4" sewer line from house to stub at property line Install 1 " water line from house to stub at property line Grade entire area around house and garage Spread existing loom around house foundation and garage Excavate for electrical trench Price includes \$ 1500.00 for extra fill around house if needed Price includes all material and labor above Price does not include any extra loom or seeding Contract Price: \$38,745.00 Lif this contract is places with an attorney for collection, customer agrees to pay all attorney fees. Interest charged for late payment 1.5%. Customer: Dated:	Kirstine & Joff Mortin	e 未 a 不 a 我 a 我 a a a a a a a a a a a a a a
Terms: 1/2 down, 2/3 remaining balance when walls are poured. Balance upon completion Job Location: LOT 25 Presumpscot River Woods Remove trees 20' around house and garage foundation Burry stumps and brush on lot Stock pile existing loom on lot Excavate for house foundation according to plans Form and pour footings for house and garage Form and pour house and garage according to plans with bulkhead and 4 windows Water plug and waterproof house wall All stone under house floor with radon Install inside and outside gravity foundation drain to stub at property line Pour 4" basement floor and finish Install sand inside garage foundation and compact Install 4" concrete floor with fiber mesh and finish for garage Install gravel driveway approxamatly 12' x 75' x 24' wide at garage Install 1 " sewer line from house to stub at property line Grade entire area around house and garage Spread existing loom around house foundation and garage Excavate for electrical trench Price includes \$ 1500.00 for extra fill around house if needed Price includes all material and labor above Price does not include any extra loom or seeding Contract Price: \$38,745.00 If this contract is places with an attorney for collection, customer agrees to pay all attorney fees. Interest charged for late payment 1.5%. Customer: Dated:	165 Elderherry Drive	March 5,2003
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P. A. Ricci Excavating, Inc. 40 Chambers Avenue South Portland, Maine 04106

Phone 767-4338	Cell Phone
671-8343	Pager 741-1996
Kirstine & Jeff Martin	
	March 5,2003
South Portland, Me 04106	H- 799-4455 C- 415-4745 W- 797-3688 x 208
Terms: 1/2 down, 2/3 remaining balance w	hen walls are poured. Balance upon completion
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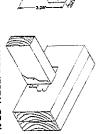


KT24 HANGER

Hammond Lumber Co. Auburn, Me



×



HEADER NAIL

QUOTE NUMBER	ILS ARE 3.6" C/WIRE (TOTAL OF 6) S ARE 1.6"- 9 GAUGE (TOTAL OF 2)
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JOB NUMBER

1773

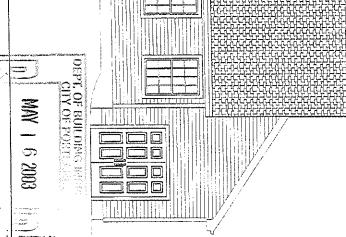
JOB DESCRIPTION DESIGNED BY: DATE: Jul 3,2003 TRUSS SPACING 24" o.c. 985

ROOF

STEVE PAGE, CET

FRONT ELEVATION

REAR ELEVATION



FINAL/8 05/01/03 1/1"=1" DRAWING NUMBER: AUB M.CHARLES DRAWN BY: APP.____ CN02029 КСН

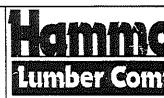
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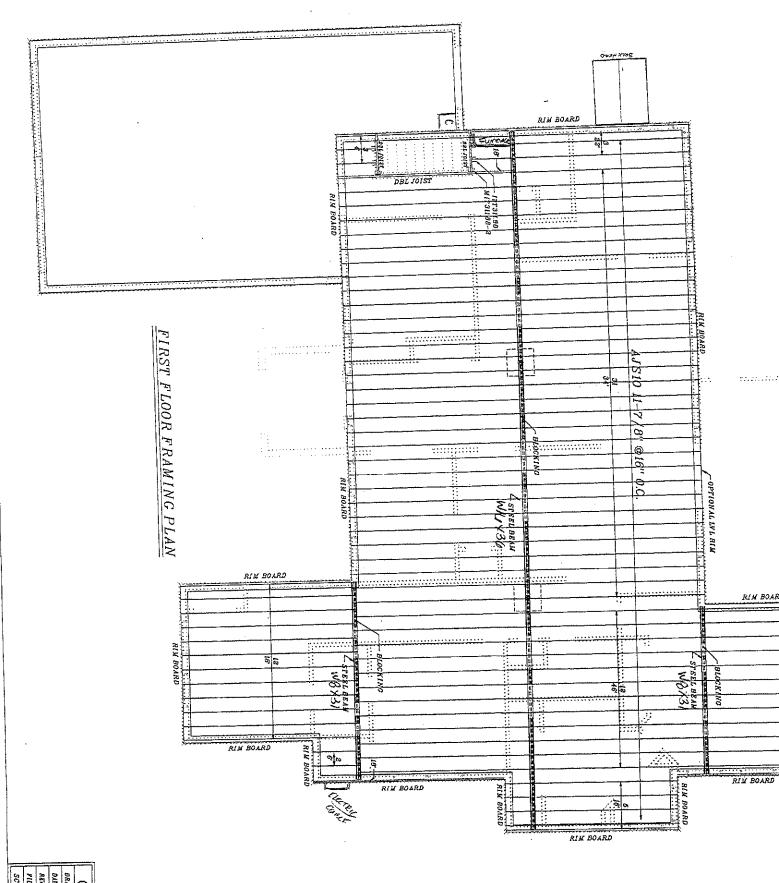
DESIGNED FOR:

JEFF/KIRSTEN MARTIN

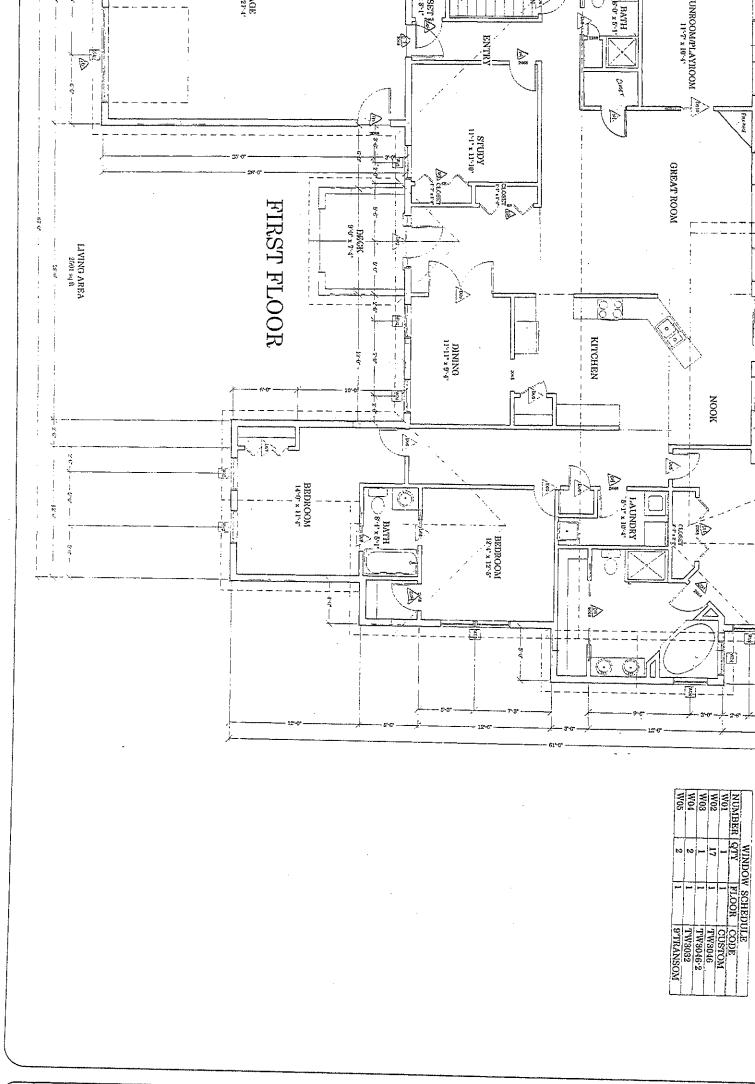
DESCRIPTION:

CONTEMPORARY





Boise Cascade	CUSTONER HANNOND LUMBER	UMBER
RATH BY: EK	CONTACT: KEYIN H.	
KE: 10/10/02	CONTACT 1:	
8715ED: 5/14/03	PROIECT: MARTIN RESIDENCE	ENCE
ILE NO. 3557(NEW)	SPECIAL NOTES:	<u></u>
CALE: 1/4" = 1'-0"		/



FINAL/8 AUB 05/01/03 1/4°=1' M.CHARLES CN02029 HYTESTIVE KCH

DESIGNED FOR:

JEFF/KIRSTEN MARTIN

DESCRIPTION:
CONTEMPORARY

Lumber Company

