

LUMBER

TCDL

BCLL

BCDI

TOP CHORD 2 X 6 SPF No.2 BOT CHORD 2 X 8 SPF No.2 WFBS 2 X 3 SPF Stud

10.0

0.0

10.0

BRACING

(Matrix)

Code IBC2009/TPI2007

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 7-2-13 oc bracing. BOT CHORD

REACTIONS (lb/size) 1=744/0-3-8 (min. 0-1-8), 7=742/0-3-8 (min. 0-1-8)

Max Horz 1=-308(LC 7)
Max Uplift1=-372(LC 9), 7=-369(LC 10)

Max Grav 1=836(LC 2), 7=832(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension

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1-2=-509/210, 2-3=-672/332, 3-12=-603/297, 4-12=-439/304, 4-13=-455/309, 5-13=-610/302, 5-6=-644/291, 6-7=-437/214 1-9=-146/374, 9-10=-135/364, 10-11=-135/364, 8-11=-135/364, 7-8=-131/376 TOP CHORD

20.0

0.0

20.0

BOT CHORD

15.0

0.0

TCDI

BCLL

BCDI

REQUIRED FIELD JOINT CONNECTIONS - Maximum Compression (lb)/ Maximum Tension (lb)/ Maximum Shear (lb)/ Maximum Moment (lb-in)

- 1) Wind: ASCE 7-05; 120mph @24in o.c.; TCDL=3.0psf; BCDL=3.0psf; (Alt. 147mph @16in o.c.; TCDL=4.5psf); (Alt. 150mph @12in o.c.; TCDL=6.0psf; BCDL=3.0psf; BCDL=3
- 2) TCLL: ASCE 7-05; Pg=55.0 psf (ground snow); Ps=42.3 psf (roof snow); Category II; Exp C; Partially Exp.; Ct=1.1
- 3) Roof design snow load has been reduced to account for slope.
 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 6) All plates are MT20 plates unless otherwise indicated.
- 7) See BEH18 DETAILS for plate placement.
- 8) Provisions must be made to prevent lateral movement of hinged member(s) during transportation
- 9) All additional member connections shall be provided by others for forces as indicated.10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 11) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 372 lb uplift at joint 1 and 369 lb uplift at joint 7.
- 13) This truss has been designed in accordance with the 2009 IBC Section 2303.4.6, 2009 IRC Section 802.10.2.

 14) Take precaution to keep the chords in plane, any bending or twisting of the hinge plate must be repaired before the building is put into service.

 15) If shown, field installed members are an integral part of this design. To ensure proper performance, all field installed members must be installed prior to
- applying any loading to the truss.

 16) Revision of P799901; updated code.



Weight: 65 lb

[P]

FT = 0%



WARNING - Verify design parameters and READ NOTES

2801 EAST BELTLINE RD, NE Universal Forest Products, Inc. PHONE (616)-364-6161 FAX (616)-365-0060 GRAND RAPIDS MI 49525 This building component has only been designed for the loads noted on this drawing. Construction and lifting forces have not been considered. The builder is responsible

for lifting methods and system design. Builder responsibilities are defined under TPI1. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult BCSI 1-06 from the Wood Truss Council of America and Truss Plate Institute Recommendation available from WTCA, 6300 Enterprise LN, Madison, WI 53719 J:\support\MitekSupp\templates\ufp.tpe@ copyright 2011 by: Universal Forest Products, Inc.

