

DISPLAY THIS CARD ON PRINCIPAL FRONTAGE OF WORK

CITY OF PORTLAND

Please Read Application And Notes, if Any, Attached

PERMIT

PERMIT ISSUED
Permit Number: 051134
SEP 8 2005
CITY OF PORTLAND

This is to certify that Verrier Donald W &/Deane Cover
has permission to Change roof pitch/ interior and exterior renovation
AT 87 Alpine Rd

provided that the person or persons, firm or corporation accepting this permit shall comply with all of the provisions of the Statutes of Maine and of the Ordinances of the City of Portland regulating the construction, maintenance and use of buildings and structures, and of the application on file in this department.

Apply to Public Works for street line and grade if nature of work requires such information.

Notification of inspection must be given and when permit is procured before this building or part thereof is closed or enclosed-in. FOUR HOUR NOTICE IS REQUIRED.

A certificate of occupancy must be procured by owner before this building or part thereof is occupied.

OTHER REQUIRED APPROVALS
Fire Dept.
Health Dept.
Appeal Board
Other

James Bonke 9/8/05
Director - Building & Inspection Services

PENALTY FOR REMOVING THIS CARD

City of Portland, Maine - Building or Use Permit Application
 389 Congress Street, 04101 Tel: (207) 874-8703, Fax: (207) 874-8716

Permit No:	05-134	Issue Date:	SEP 8 2005	City:	380A 0005001
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Location of Construction: 87 Alpine Rd	Owner Name: Verrier Donald W &	Owner Address: 87 Alpine Rd	Phone:
Business Name:	Contractor Name: Deane Grover	Contractor Address: 22 Riverwoods Drive Scarborough	Phone: 2076713176
Lessee/Buyer's Name:	Phone:	Permit Type: Alterations - Dwellings	Zone: R3
Past Use: Single Family Home	Proposed Use: Single Family Home/ Change roof pitch/ interior and exterior renovations	Permit Fee: \$1,821.00	Cost of Work: \$200,000.00
Proposed Project Description: Change roof pitch/ interior and exterior renovations		FIRE DEPT: <input type="checkbox"/> Approved <input type="checkbox"/> Denied	INSPECTION: Use Group: R3 Type: SB IRL-2003 Signature: JMB 9/8/05
		PEDESTRIAN ACTIVITIES DISTRICT (P.A.D.):	Signature:
		Action: <input type="checkbox"/> Approved <input type="checkbox"/> Approved w/Conditions <input type="checkbox"/> Denied	Date:

Zoning Approval

1. This permit application does not preclude the Applicant(s) from meeting applicable State and Federal Rules. 2. Building permits do not include plumbing, septic or electrical work. 3. Building permits are void if work is not started within six (6) months of the date of issuance. False information may invalidate a building permit and stop all work..	Special Zone or Reviews <input type="checkbox"/> Shoreland <input type="checkbox"/> Wetland <input type="checkbox"/> Flood Zone <input type="checkbox"/> Subdivision <input type="checkbox"/> Site Plan Maj <input type="checkbox"/> Minor <input type="checkbox"/> MM <input type="checkbox"/> Date: JMB-9/8/05	Zoning Appeal <input type="checkbox"/> Variance <input type="checkbox"/> Miscellaneous <input type="checkbox"/> Conditional Use <input type="checkbox"/> Interpretation <input type="checkbox"/> Approved <input type="checkbox"/> Denied Date:	Historic Preservation <input checked="" type="checkbox"/> Not in District or Landmark <input type="checkbox"/> Does Not Require Review <input type="checkbox"/> Requires Review <input type="checkbox"/> Approved <input type="checkbox"/> Approved w/Conditions <input type="checkbox"/> Denied Date: JMB
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CERTIFICATION

I hereby certify that I am the owner of record of the named property, or that the proposed work is authorized by the owner of record and that I have been authorized by the owner to make this application as his authorized agent and I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in the application is issued, I certify that the code official's authorized representative shall have the authority to enter all areas covered by such permit at any reasonable hour to enforce the provision of the code(s) applicable to such permit.

SIGNATURE OF APPLICANT	ADDRESS	DATE	PHONE
RESPONSIBLE PERSON IN CHARGE OF WORK, TITLE		DATE	PHONE

10/5/05 Footing (in cavity footprint) OK MD

10/12/05 Backfill - MD

10/21/05 - Check Down tube depth - 4 rows some water in tubes - Contractor was pumping out while I was present - will make sure dry before filling w/cement - OK to pour when dry.

JM

1/18/06 Met Dean G. - Roof framing OK - received Truss design - still need the Truss Beam spec. OK to insulate.

Checked deck framing - still need to bolt ledger, will add a stiff arm post under Rim extension. Guard OK - needs handrail. Front steps are temporary - finish to be granite. JM

City of Portland, Maine - Building or Use Permit

389 Congress Street, 04101 Tel: (207) 874-8703, Fax: (207) 874-8716

Permit No: 05-1134	Date Applied For: 08/08/2005	CBL: 380A C005001
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Location of Construction: 87 Alpine Rd	Owner Name: Verrier Donald W &	Owner Address: 87 Alpine Rd	Phone:
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Business Name:	Contractor Name: Deane Grover	Contractor Address: 22 Riverwoods Drive Scarborough	Phone: (207) 671-3176
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Lessee/Buyer's Name	Phone:	Permit Type: Alterations - Dwellings
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Proposed Use: Single Family Home/ Change roof pitch/ interior and exterior renovations	Proposed Project Description: Change roof pitch/ interior and exterior renovations
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Dept: Zoning Status: Approved Reviewer: Jeanine Bourke Approval Date: 09/08/2005
 Note: Ok to Issue:

Dept: Building Status: Approved with Conditions Reviewer: Jeanine Bourke Approval Date: 09/07/2005
 Note: given to Marge by mistake - gave to Res T/J on 8/18/05 Ok to Issue:

- 8/25 left vm w/Donald V. For more details ie. Plot plan, floor plans, framing, headers, egress, attic access.
 Mrs. Verrier returned call and clarified some issues, will call contractor on 8/29
 9/7 I forgot to call the contractor and verified info today, ok to issue
- 1) A copy of the enclosed chimney disclosure must be submitted to this office upon completion of the permitted work or for the Certificate of Occupancy.
 - 2) Separate permits are required for any electrical, plumbing, or heating.
 - 3) Permit approved based on the plans submitted and reviewed w/owner/contractor, with additional information as agreed on and as noted on plans.
 - 4) There must be a 2" clearance maintained between the chimney and any combustible material, and fire blocking per code at each level
 - 5) The design load spec sheets for any engineered beam(s) must be submitted to this office.

All Purpose Building Permit Application

If you or the property owner owes real estate or personal property taxes or user charges on any property within the City, payment arrangements must be made before permits of any kind are accepted.

Location/Address of Construction: <u>87 Alpine Street Portland</u>		
Total Square Footage of Proposed Structure		Square Footage of Lot
Tax Assessor's Chart, Block & Lot Chart# <u>380</u> Block# <u>AC</u> Lot# <u>5</u>	Owner: <u>Donald UERRIER</u>	Telephone: <u>797 6636</u>
Lessee/Buyer's Name (if Applicable)	Applicant name, address & telephone: <u>Donald UERRIER</u> <u>87 Alpine street.</u>	Cost Of Work: \$ <u>220,000</u> Fee: \$ <u>1,821</u>
Current use: <u>Single family home</u>		
If the location is currently vacant, what was prior use: <u>NA</u>		
Approximately how long has it been vacant: <u>NA</u>		
Proposed use: Project description: <u>Chase Roof Pitch exterior & Interior Renovations</u>		
Contractor's name, address & telephone: <u>Deane Gower 22 Riverwoods DR</u> <u>Scar ME 04074 H 839-7330</u>		
Who should we contact when the permit is ready: <u>→</u> <u>C 671-3176</u>		
Mailing address:		
We will contact you by phone when the permit is ready. You must come in and pick up the permit and review the requirements before starting any work, with a Plan Reviewer. A stop work order will be issued and a \$100.00 fee if any work starts before the permit is picked up. PHONE:		

IF THE REQUIRED INFORMATION IS NOT INCLUDED IN THE SUBMISSIONS THE PERMIT WILL BE AUTOMATICALLY DENIED AT THE DISCRETION OF THE BUILDING/PLANNING DEPARTMENT, WE MAY REQUIRE ADDITIONAL INFORMATION IN ORDER TO APPROVE THIS PERMIT.

I hereby certify that I am the Owner of record of the named property, or that the owner of record authorizes the proposed work and that I have been authorized by the owner to make this application as his/her authorized agent. I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in this application is issued, I certify that the Code Official's authorized representative shall have the authority to enter all areas covered by this permit at any reasonable hour to enforce the provisions of the codes applicable to this permit.

Signature of applicant: Donald Uerrier Date: Aug 10 2005

**This is NOT a permit, you may not commence ANY work until the permit is issued.
If you are in a Historic District you may be subject to additional permitting and fees with the
Planning Department on the 4th floor of City Hall**

BUILDING PERMIT INSPECTION PROCEDURES

Please call 874-8703 or 874-8693 to schedule your inspections as agreed upon

Permits expire in 6 months, if the project is not started or ceases for 6 months.

The Owner or their designee is required to notify the inspections office for the following inspections and provide adequate notice. Notice must be called in 48-72 hours in advance in order to schedule an inspection:

By initializing at each inspection time, you are agreeing that you understand the inspection procedure and additional fees from a "Stop Work Order" and "Stop Work Order Release" will be incurred if the procedure is not followed as stated below.

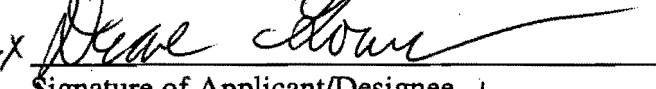
A Pre-construction Meeting will take place upon receipt of your building permit.

- Footing/Building Location Inspection: Prior to pouring concrete
- Re-Bar Schedule Inspection: Prior to pouring concrete
- Foundation Inspection: Prior to placing ANY backfill
- Framing/Rough Plumbing/Electrical: Prior to any insulating or drywalling
- Final/Certificate of Occupancy: Prior to any occupancy of the structure or use. NOTE: There is a \$75.00 fee per inspection at this point.

Certificate of Occupancy is not required for certain projects. Your inspector can advise you if your project requires a Certificate of Occupancy. All projects DO require a final inspection

DL If any of the inspections do not occur, the project cannot go on to the next phase, REGARDLESS OF THE NOTICE OR CIRCUMSTANCES.

DL CERTIFICATE OF OCCUPANCIES MUST BE ISSUED AND PAID FOR, BEFORE THE SPACE MAY BE OCCUPIED

<input checked="" type="checkbox"/> <u>DL</u> 	<u>9/16/05</u>
Signature of Applicant/Designee	Date
<u>Donna Martin Admin</u>	<u>9-16-05</u>
Signature of Inspections Official	Date
CBL: <u>380A-C-005</u>	Building Permit #: <u>05-1134</u>

This page contains a detailed description of the Parcel ID you selected. Press the **New Search** button at the bottom of the screen to submit a new query.

Current Owner Information

Card Number	1 of 1
Parcel ID	380A C005001
Location	87 ALPINE RD
Land Use	SINGLE FAMILY
Owner Address	VERRIER DONALD W & SIEGLINDE JTS 87 ALPINE RD PORTLAND ME 04103
Book/Page	
Legal	380A-C-5 ALPINE RD 87 10220 SF

Current Assessed Valuation For Fiscal Year 2006

Land	Building	Total
\$60,220	\$164,830	\$225,050

Estimated Assessed Valuation For Fiscal Year 2007*

Land	Building	Total
\$86,200	\$211,100	\$297,300

* Value subject to change based upon review of property status as of 4/1/06. The tax rate will be determined by City Council in May 2006.

Property Information

Year Built	Style	Story Height	Sq. Ft.	Total Acres	
1969	Raised Ranch	1	3074	0.235	
Bedrooms	Full Baths	Half Baths	Total Rooms	Attic	Basement
4	3		10	None	Full

Outbuildings

Type	Quantity	Year Built	Size	Grade	Condition
POOL-PLASTIC LINER	1	1980	20X44	C	A

Sales Information

Date	Type	Price	Book/Page
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Picture and Sketch

[Picture](#) [Sketch](#) [Tax Map](#)

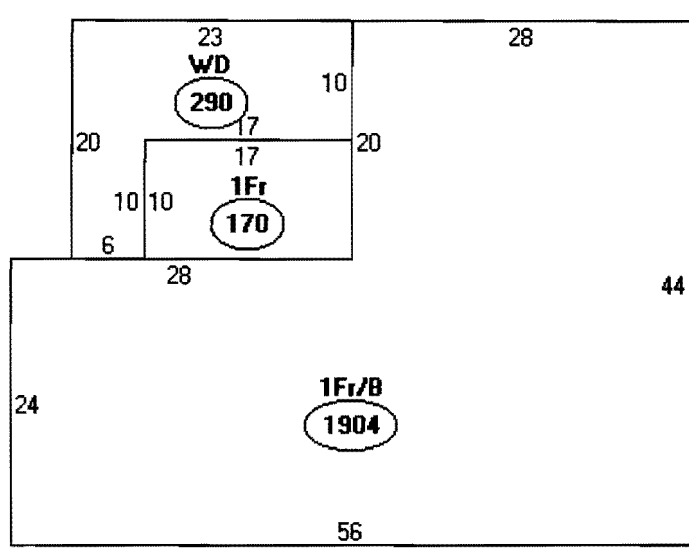
[Click here](#) to view Tax Roll Information.

Any information concerning tax payments should be directed to the Treasury office at 874-8490 or e-



<http://www.portlandassessor.com/images/pictures/02233301.jpg>

08/25/2005



Descriptor/Area
A: 1Fr/B
1904 sqft
B: 1Fr
170 sqft
C: WD
290 sqft

<http://www.portlandassessor.com/images/Sketches/02233301.jpg>

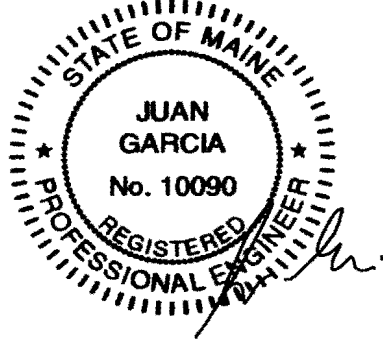
08/25/2005

Re: 421900
Hancock/Verrier Res/wd/11-2

The truss drawing(s) referenced below have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Wood Str Inc.

Pages or sheets covered by this seal: 19712367 thru 19712374

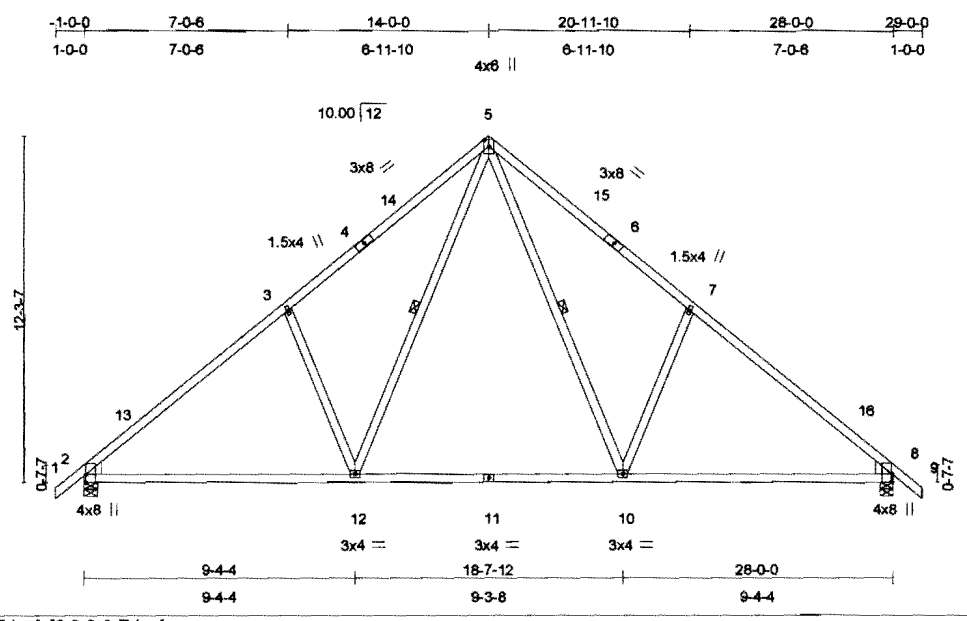
My license renewal date for the state of Maine is December 31, 2007.



January 19, 2006

Garcia, Juan

The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-2002 Chapter 2.



LOADING (psf)		SPACING		CSI		DEFL		PLATES		GRIP	
TCLL	50.0	Plates Increase	2-0-0	TC	0.90	in (loc)	1/defl	L/d	MT20	197/144	
(Roof Snow=50.0)		Lumber Increase	1.15	BC	0.60	Vert(LL)	-0.17	2-12	>999	180	
TCDL	10.0	Rep Stress Incr	YES	WB	0.42	Vert(TL)	-0.34	2-12	>965	180	
BCLL	0.0	Code	BOCA/ANSI95	(Matrix)		Horz(TL)	0.06	8	n/a	n/a	
BCDL	10.0										Weight: 126 lb

LUMBER
 TOP CHORD 2 X 4 SYP No 2 'Except'
 1-4 2 X 4 SPF 2100F 1.8E, 6-9 2 X 4 SPF 2100F 1.8E
 BOT CHORD 2 X 4 SPF 1650F 1.5E
 WEBS 2 X 4 SPF 1650F 1.5E
WEDGE
 Left: 2 X 6 SPF 1650F 1.5E, Right: 2 X 6 SPF 1650F 1.5E

BRACING
 TOP CHORD Sheathed or 3-6-10 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
 WEBS 1 Row at midpt 5-12, 5-10

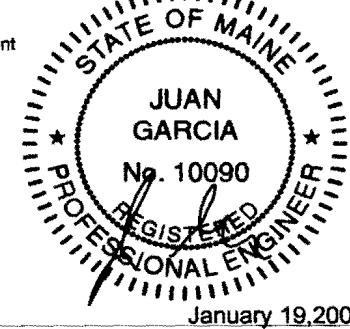
This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

REACTIONS (lb/size) 2=2075/0-5-8, 8=2075/0-5-8
 Max Horiz 2=405(load case 5)
 Max Uplift 2=324(load case 7), 8=324(load case 8)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/73, 2-13=2494/314, 3-13=2259/347, 3-4=2162/464, 4-14=1886/484, 5-14=1852/505, 5-15=1852/505,
 6-15=1886/484, 6-7=2162/464, 7-16=2259/347, 8-16=2494/314, 8-9=0/73
 BOT CHORD 2-12=250/1676, 11-12=65/1140, 10-11=65/1140, 8-10=136/1676
 WEBS 3-12=830/386, 5-12=296/1064, 5-10=296/1064, 7-10=830/386

NOTES (4)
 1) Wind: ASCE 7-02, 90mph; h=35ft; TCDL=5.0psf, BCDL=5.0psf, Category II; Exp C; enclosed; MWFRS gable end zone and C-C Exterior(2) - 1-0-0 to 2-0-0, Interior(1) 2-0-0 to 11-0-0, Exterior(2) 11-0-0 to 17-0-0, Interior(1) 17-0-0 to 26-0-0, Exterior(2) 26-0-0 to 29-0-0 zone, cantilever left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) Unbalanced snow loads have been considered for this design.
 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 324 lb uplift at joint 2 and 324 lb uplift at joint 8.
 4) Drawing prepared exclusively for manufacturing by Wood Structures Inc.

LOAD CASE(S) Standard



January 19, 2006

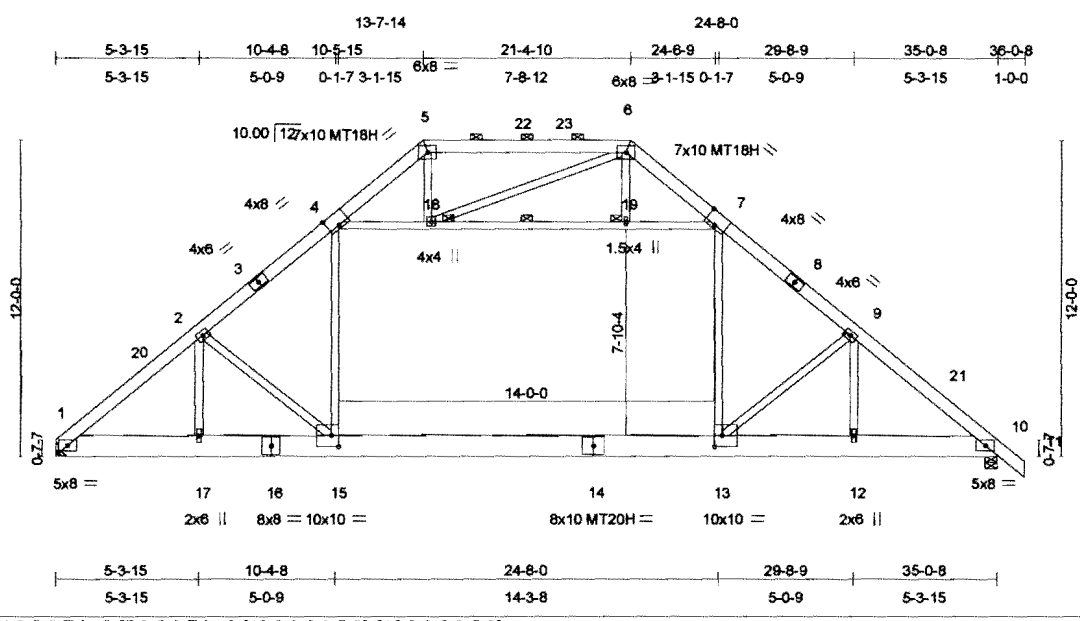


Plate Offsets (X,Y): [4-0-5-0, Edge], [7-0-5-0, Edge], [13-0-3-8-0-5-0], [15-0-3-8-0-5-0]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	Vdefl	L/d	PLATES	GRIP
TCLL 50.0	Plates Increase 1.15	TC 0.43	Vert(LL) -0.41	13-15	>999	360	MT20	197/144
(Roof Snow=50.0)	Lumber Increase 1.15	BC 0.68	Vert(TL) -0.48	13-15	>866	240	MT20H	187/143
TCDL 10.0	Rep Stress Incr YES	WB 0.40	Horz(TL) 0.05	10	n/a	n/a	MT18H	197/144
BCLL 0.0	Code BOCA/ANSI95	(Matrix)	Wnd(LL) 0.15	15	>999	240	Weight: 290 lb	
BCDL 10.0								

LUMBER
 TOP CHORD 2 X 6 SPF 1650F 1.5E
 BOT CHORD 2 X 10 SYP M 23
 WEBS 2 X 4 SPF 1650F 1.5E

BRACING
 TOP CHORD Sheathed or 4-0-14 oc purlins, except
 2-0-0 oc purlins (6-0-0 max.); 5-8.
 BOT CHORD Rigid ceiling directly applied or 5-11-10 oc bracing.
 WEBS 1 Row at midpt 18-19
 JOINTS 1 Brace at Jt(s): 18, 19

REACTIONS (lb/size) 1=3219/Mechanical, 10=3371/0-5-8
 Max Horz 1=-403(load case 5)
 Max Uplift 1=142(load case 7), 10=-209(load case 8)
 Max Grav 1=3229(load case 2), 10=3411(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-20=-4568/225, 2-20=-4204/242, 2-3=-4307/189, 3-4=-4095/218, 4-5=-1541/266, 6-7=-1578/268, 7-8=-4095/217,
 8-9=-4312/198, 9-21=-4263/217, 10-21=-4533/196, 10-11=0/105, 5-22=-1185/269, 22-23=-1185/269, 6-23=-1185/269
 BOT CHORD 1-17=-268/3305, 16-17=-268/3305, 15-16=-268/3305, 14-15=0/3146, 13-14=0/3146, 12-13=-63278, 10-12=-63278
 WEBS 4-18=-2034/136, 18-19=-1989/136, 7-18=-1989/135, 4-15=0/1491, 7-13=0/1506, 2-17=-859/209, 9-12=-877/217,
 2-15=-708/701, 9-13=-679/708, 5-16=-1294, 6-19=0/185, 6-18=-335/269

NOTES (10)
 1) Wind: ASCE 7-02; 90mph; h=35ft; TCDL=5.0psf; BCCL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Exterior(2) 0-2-12 to 3-8-13, Interior(1) 3-8-13 to 10-2-13, Exterior(2) 10-2-13 to 17-2-15, Interior(1) 17-2-15 to 17-9-9, Exterior(2) 17-9-9 to 24-9-11, Interior(1) 24-9-11 to 32-6-7, Exterior(2) 32-6-7 to 36-0-8 zone; cantilever left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) Unbalanced snow loads have been considered for this design.
 3) Provide adequate drainage to prevent water ponding.
 4) All plates are MT20 plates unless otherwise indicated.
 5) Ceiling dead load (5.0 psf) on member(s): 4-18, 18-19, 7-19; Wall dead load (5.0psf) on member(s): 4-15, 7-13
 6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room: 13-15
 7) Refer to girder(s) for truss to truss connections.
 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 142 lb uplift at joint 1 and 209 lb uplift at joint 10.
 9) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
 10) Drawing prepared exclusively for manufacturing by Wood Structures Inc.

LOAD CASE(S) Standard



January 19, 2006

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED NOTES REFERENCE PAGE 02-7473 BEFORE USE.
 Design valid for use only with Mittek connection. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANS/TP1 Quality Criteria, D58-89 and KCS1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

14515 N. Outer Forty,
 Suite 600,
 Chesterfield, MO 63017

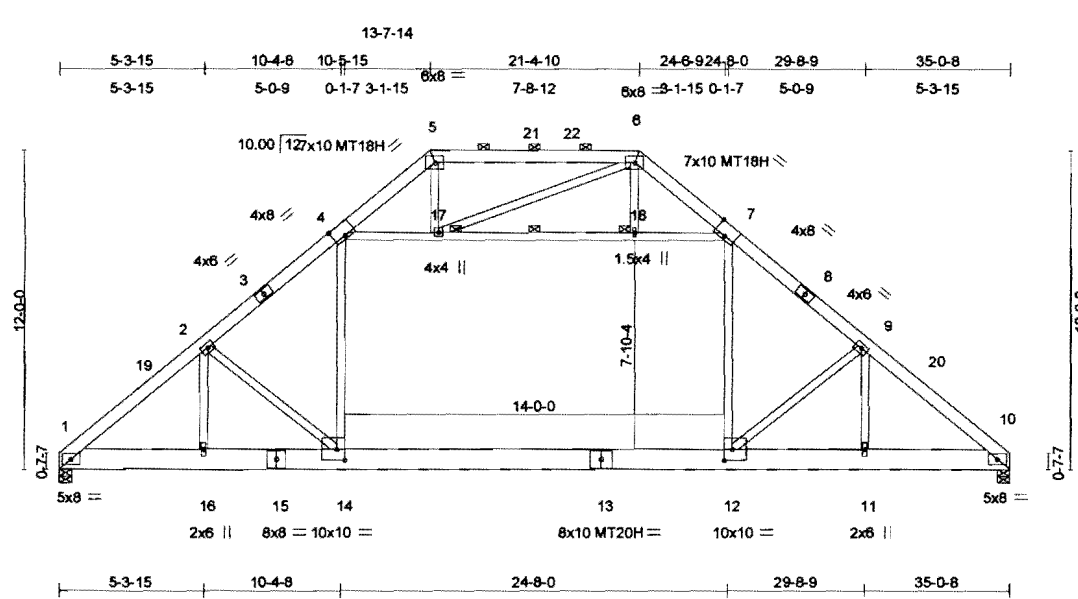


Plate Offsets (X, Y): [4-0-5-0, Edge], [7-0-5-0, Edge], [12-0-3-8-0-5-0], [14-0-3-8-0-5-0]

LOADING (psf)	SPACING	CS1	DEFL	in (loc)	l'depl	l'd	PLATES	GRIP
TCLL 50.0 (Roof Snow=50.0)	Lumber Increase 1.15	TC 0.43	Vert(LL) -0.41	12-14	>999	360	MT20	197/144
BCLL 0.0	Rep Stress Incr YES	BC 0.66	Vert(TL) -0.48	12-14	>867	240	MT20H	187/143
BCDL 10.0	Code BOCA/ANSI95	WB 0.40	Horz(TL) 0.05	10	n/a	n/a	MT18H	197/144
		(Matrix)	Wfnd(LL) 0.15	14	>999	240	Weight: 288 lb	

LUMBER
 TOP CHORD 2 X 6 SPF 1650F 1.5E
 BOT CHORD 2 X 10 SYP M 23
 WEBS 2 X 4 SPF 1650F 1.5E

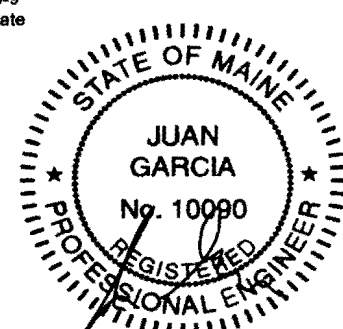
BRACING
 TOP CHORD Sheathed or 4-0-14 oc purlins, except
 2-0-0 oc purlins (6-0-0 max.); 5-6.
 BOT CHORD Rigid ceiling directly applied or 5-11-10 oc bracing.
 WEBS 1 Row at midpt
 17-18
 JOINTS 1 Brace at Jt(s): 17, 18

REACTIONS (lb/size) 1=3221/0-5-8, 10=3221/0-5-8
 Max Horz 1=383(load case 5)
 Max Uplift 1=143(load case 7), 10=143(load case 8)
 Max Grav 1=3229(load case 2), 10=3229(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-19=-458/228, 2-19=-4207/245, 2-3=-4313/197, 3-4=-4102/226, 4-5=-1541/266, 6-7=-1579/269, 7-8=-4102/226,
 8-9=-4319/197, 9-20=-4133/245, 10-20=-4566/228, 5-21=-1185/269, 21-22=-1185/269, 6-22=-1185/269
 BOT CHORD 1-16=-284/3305, 16-18=-284/3305, 14-15=-284/3305, 13-14=0/3151, 12-13=0/3151, 11-12=-83/3305, 10-11=-83/3305
 WEBS 4-17=-2040/140, 17-18=-1993/137, 7-18=-2003/136, 4-14=0/1485, 7-12=0/1507, 2-16=-863/210, 9-11=-872/213,
 2-14=-707/705, 9-12=-707/704, 5-17=-2296, 6-18=0/185, 6-17=-337/269

NOTES (9)
 1) Wind: ASCE 7-02; 90mph; h=35ft, TCFL=5.0psf, BCDL=5.0psf, Category II; Exp C; enclosed; MWFRS gable end zone and C-C
 Exterior(2) 0-2-12 to 3-8-13, Interior(1) 3-8-13 to 10-2-13, Exterior(2) 10-2-13 to 17-2-15, Interior(1) 17-2-15 to 17-9-9, Exterior(2) 17-9-9
 to 24-9-11, Interior(1) 24-9-11 to 31-3-11, Exterior(2) 31-3-11 to 34-9-12 zone, cantilever left and right exposed, Lumber DOL=1.60 plate
 grip DDL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) Unbalanced snow loads have been considered for this design.
 3) Provide adequate drainage to prevent water ponding.
 4) All plates are MT20 plates unless otherwise indicated.
 5) Ceiling dead load (5.0 psf) on member(s): 4-17, 17-18, 7-18. Wall dead load (5.0psf) on member(s): 4-14, 7-12
 6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room: 12-14
 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 143 lb uplift at joint 1 and 143 lb uplift at
 joint 10.
 8) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
 9) Drawing prepared exclusively for manufacturing by Wood Structures Inc.

LOAD CASE(S) Standard



January 19, 2006

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIE 7473 BEFORE USE.
 Design valid for use only with Mitek connection. This design is based only upon parameters shown, and is for an individual building component.
 Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown
 is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the
 erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding
 fabrication, quality control, storage, delivery, erection and bracing, consult ANS/TP1 Quality Criteria, D38-89 and BCSI Building Component
 Safety Information available from Truss Plate Institute, 383 D Onchick Drive, Woodson, WI 53719.

14515 N. Outer Forty,
 Suite 600
 Chesterfield, MO 63017



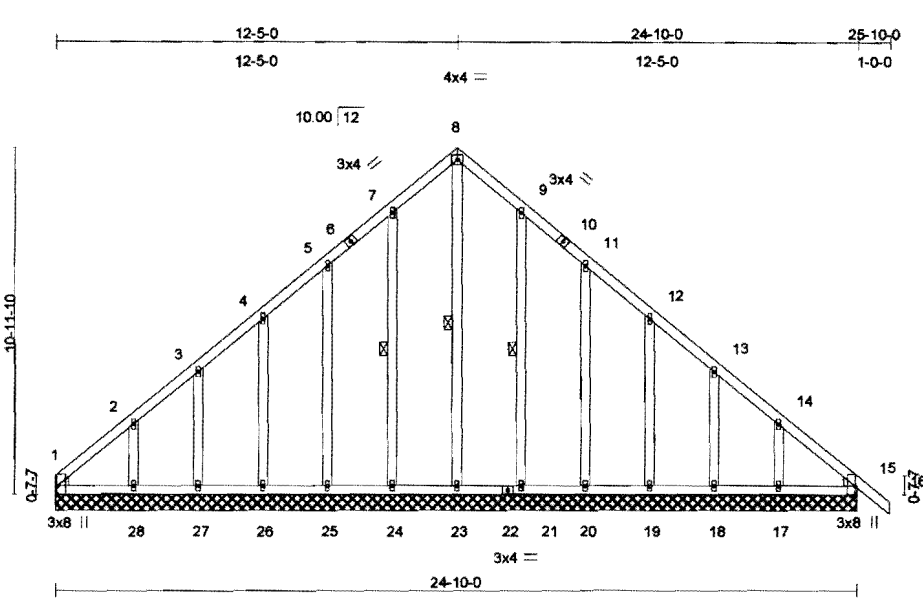


Plate Offsets (X,Y): [1:0-3-8,Edge], [10:0-0-0-0-0], [15:0-3-8,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 50.0	Plates Increase 1.15	TC 0.14	Ver(L) -0.00	16	n/r	180		MT20	197/144
(Roof Snow=50.0)	Lumber Increase 1.15	BC 0.08	Ver(TL) -0.00	16	n/r	80			
TCDL 10.0	Rep Stress Incr YES	WB 0.18	Horz(TL) 0.01	15	n/a	n/a			
BCLL 0.0	Code BOCA/ANSI95	(Matrix)							Weight: 145 lb
BCDL 10.0									

LUMBER
 TOP CHORD 2 X 4 SPF 1650F 1.5E *Except*
 6-8 2 X 4 SYP No.2, 8-10 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SPF 1650F 1.5E
 OTHERS 2 X 4 SPF 1650F 1.5E
WEBS
 Left: 2 X 4 SYP No.2, Right: 2 X 4 SYP No.2

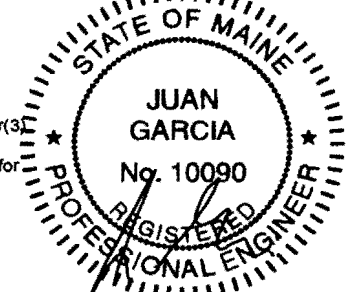
BRACING
 TOP CHORD Sheathed or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
 WEBS 1 Row at midpt 8-23, 7-24, 9-21

This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

REACTIONS (lb/size) 1=175/24-10-0, 23=217/24-10-0, 24=277/24-10-0, 25=280/24-10-0, 26=286/24-10-0, 27=255/24-10-0, 28=357/24-10-0, 21=277/24-10-0, 20=280/24-10-0, 19=283/24-10-0, 18=289/24-10-0, 15=327/24-10-0, 17=314/24-10-0
 Max Horiz=370(load case 5)
 Max Uplift=-123(load case 5), 24=-91(load case 7), 25=-119(load case 7), 26=-113(load case 7), 27=-99(load case 7), 28=-178(load case 7), 21=-87(load case 8), 20=-121(load case 8), 19=-111(load case 8), 18=-107(load case 8), 15=-54(load case 6), 17=-148(load case 8)
 Max Grav=233(load case 6), 23=279(load case 8), 24=341(load case 2), 25=328(load case 2), 26=337(load case 2), 27=301(load case 2), 28=423(load case 2), 21=340(load case 3), 20=329(load case 3), 19=333(load case 3), 18=317(load case 3), 15=371(load case 3), 17=371(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=353/221, 2-3=252/195, 3-4=190/188, 4-5=130/179, 5-6=130/219, 6-7=53/228, 7-8=-136/276, 8-9=-135/276, 9-10=-53/202, 10-11=-130/194, 11-12=-130/105, 12-13=-129/79, 13-14=-144/86, 14-15=-253/107, 15-16=0/70
 BOT CHORD 1-2=65/266, 27-28=65/266, 28-27=-65/266, 25-26=-65/266, 24-25=-65/266, 23-24=-65/266, 22-23=-65/266, 21-22=-65/266, 20-21=-65/266, 19-20=-65/266, 18-19=-65/266, 17-18=-65/266, 15-17=-65/266
 WEBS 8-23=259/0, 7-24=300/111, 5-25=-289/139, 4-26=-294/132, 3-27=-274/124, 2-28=-342/184, 9-21=-300/107, 11-20=-289/141, 12-19=-292/131, 13-18=-280/127, 14-17=-320/170

NOTES (8)
 1) Wind: ASCE 7-02; 90mph; h=35ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Corner(3) 0-0-0 to 3-0-0, Exterior(2) 3-0-0 to 9-5-0, Corner(3) 9-5-0 to 15-5-0, Exterior(2) 15-5-0 to 22-10-0, Corner(3) 22-10-0 to 25-10-0 zone; cantilever left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) Truss designed for wind loads in the plane of the truss only.
 3) Unbalanced snow loads have been considered for this design.
 4) All plates are 1.5x4 MT20 unless otherwise indicated.
 5) Gable requires continuous bottom chord bearing.
 6) Gable studs spaced at 2'-0" oc.



January 19, 2006

Continued on page 2

WARNING - Verify design parameters and READ NOTES OF THIS AND INCLUDED MITEK REFERENCE PAGE MIE 7473 BEFORE USE.
 Design valid for use only with MITek connection. The design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/AP1 Quality Criteria, D58-89 and BC31 Building Component Safety Information available from Truss Plate Institute, 363 D'Onofrio Drive, Madison, WI 53719.

14515 N. Outer Forty, Suite 600, Chesterfield, MO 63017
MITek

Job	Truss	Truss Type	Qty	Ply	Hancock/Verrier ResAwd/11-2	19712370
421900	005	GABLE	1	1		

Wood Structures, Inc., Biddeford, ME 04005 6:200 s Oct 18 2005 MITEK Industries, Inc. Wed Jan 18 15:13:03 2006 Page 2

Job Reference (optional)

NOTES (8)

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 123 lb uplift at joint 1, 91 lb uplift at joint 24, 119 lb uplift at joint 25, 113 lb uplift at joint 26, 99 lb uplift at joint 27, 176 lb uplift at joint 28, 87 lb uplift at joint 21, 121 lb uplift at joint 20, 111 lb uplift at joint 19, 107 lb uplift at joint 18, 54 lb uplift at joint 15 and 148 lb uplift at joint 17.

8) Drawing prepared exclusively for manufacturing by Wood Structures Inc.

LOAD CASE(S) Standard

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MEG-7473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing consult ANSL/ITI Quality Criteria, Q38-89 and QCS1 Building Component Safety Information available from Truss Plate Institute, 383 D'Onofrio Drive, Madison, WI 53719.

14515 N. Outer Forty,
 Suite 6300
 Chesterfield, MO 63017



Job 421900	Truss 006	Truss Type COMMON	Qty 2	Ply 1	Hancock/Verrier Res/wd/11-2	19712371
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Wood Structures, Inc., Biddeford, ME 04005 6.200 s Oct 18 2005 Mittek Industries, Inc. Wed Jan 18 15:13:03 2006 Page 1

Scale = 1/8"=1'-0"

Plate Offsets (X,Y): [1:0-3-8,Edge], [5:0-0-0-0-0], [7:0-3-8,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/def	L/d	PLATES	GRIP
TCLL 50.0 (Roof Snow=50.0)	Plates Increase 1.15	TC 0.88	Ver(LL) -0.15	1-11	>999	240		MT20	197/144
TCDL 10.0	Lumber Increase 1.15	BC 0.56	Ver(TL) -0.27	1-11	>999	180			
BCLL 0.0	Rep Stress Incr YES	WB 0.41	Horz(TL) 0.05	7	n/a	n/a			
BCDL 10.0	Code BOCA/ANSI95	(Main)							Weight: 111 lb

LUMBER
TOP CHORD 2 X 4 SPF 1650F 1.5E "Except"
3-4 2 X 4 SYP No.2, 4-5 2 X 4 SYP No.2
BOT CHORD 2 X 4 SPF 1650F 1.5E
WEBS 2 X 4 SPF 1650F 1.5E
WEDGE
Left: 2 X 6 SPF 1650F 1.5E, Right: 2 X 6 SPF 1650F 1.5E

BRACING
TOP CHORD Sheathed or 4-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

REACTIONS (lb/size) 1=1703/0-5-8, 7=1857/0-5-8
Max Horz1=370(load case 5)
Max Uplift1=228(load case 7), 7=295(load case 8)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-12=-2206/286, 2-12=-1963/307, 2-3=-1909/416, 3-13=-1666/432, 4-13=-1660/451, 4-14=-1649/444, 5-14=-1656/425, 5-6=-1698/408, 6-15=-1985/308, 7-15=-2196/280, 7-8=0/73
BOT CHORD 1-11=-222/1476, 10-11=-58/1007, 9-10=-58/1007, 7-9=-1177/1466
WEBS 2-11=-724/341, 4-11=-267/944, 4-9=-257/922, 6-9=-713/337

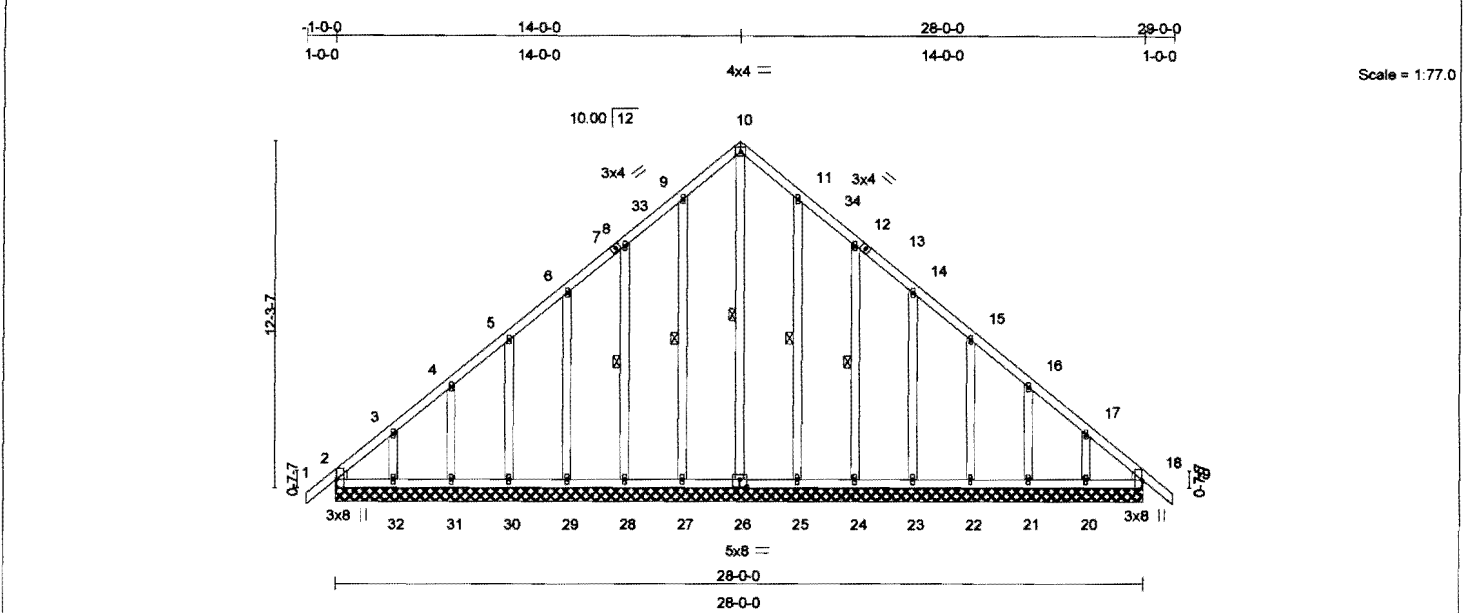
NOTES (4)
1) Wind: ASCE 7-02; 90mph; h=35ft; TCCL=5.0psf; BCCL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Exterior(2) 0-2-12 to 3-2-12, Interior(1) 3-2-12 to 9-5-0, Exterior(2) 9-5-0 to 15-5-0, Interior(1) 15-5-0 to 22-10-0, Exterior(2) 22-10-0 to 25-10-0 zone; cantilever left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Unbalanced snow loads have been considered for this design.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 228 lb uplift at joint 1 and 295 lb uplift at joint 7.
4) Drawing prepared exclusively for manufacturing by Wood Structures Inc.

LOAD CASE(S) Standard

January 19, 2006

WARNING: Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE 9473 BEFORE USE.
Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANS/ITF Quality Criteria, D58-87 and BCS1 Building Component Safety Information available from Truss Plate Institute, 263 D'Onofrio Drive, Madison, WI 53719.

14515 N. Overly Ferry, Suite #300, Chesterfield, MO 63017



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (oc)	U/defl	L/d	PLATES	GRIP
TCLL 50.0	Plates Increase	1.15	TC 0.14	Vert(LL)	-0.01	19	n/r	MT20	197/144
(Roof Snow=50.0)	Lumber Increase	1.15	BC 0.04	Vert(TL)	-0.01	19	n/r		
TCDL 10.0	Rep Stress Incr	YES	WB 0.20	Horz(TL)	0.01	18	n/a		
BCLL 0.0	Code	BOCA/ANSI95	(Matrix)						
BCLD 10.0									Weight: 176 lb

LUMBER
 TOP CHORD 2 X 4 SPF 1650F 1.5E "Except"
 7-10 2 X 4 SYP No.2, 10-13 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SPF 1650F 1.5E
 OTHERS 2 X 4 SPF 1650F 1.5E
 WEDGE Left: 2 X 4 SYP No.2, Right: 2 X 4 SYP No.2

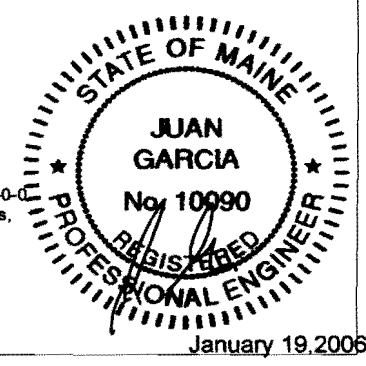
BRACING
 TOP CHORD Sheathed or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing
 WEBS 1 Row at midpt. 10-26, 9-27, 8-28, 11-25, 12-24

This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

REACTIONS (lb/size) 2=308/28-0-0, 18=308/28-0-0, 26=218/28-0-0, 27=276/28-0-0, 28=282/28-0-0, 29=280/28-0-0, 30=279/28-0-0, 31=283/28-0-0, 32=264/28-0-0, 25=276/28-0-0, 24=282/28-0-0, 23=280/28-0-0, 22=279/28-0-0, 21=283/28-0-0, 20=284/28-0-0
 Max Horz2=-405(load case 5)
 Max Uplift2=-162(load case 5), 18=-70(load case 6), 27=-86(load case 7), 28=-122(load case 7), 29=-110(load case 7), 30=-111(load case 7), 31=-113(load case 7), 32=-143(load case 7), 25=-80(load case 8), 24=-123(load case 8), 23=-109(load case 8), 22=-111(load case 8), 21=-113(load case 8), 20=-137(load case 8)
 Max Grav2=348(load case 2), 18=348(load case 3), 26=317(load case 6), 27=339(load case 2), 28=331(load case 2), 29=330(load case 2), 30=328(load case 2), 31=334(load case 2), 32=312(load case 2), 25=339(load case 3), 24=331(load case 3), 23=330(load case 3), 22=329(load case 3), 21=334(load case 3), 20=312(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/70, 2-3=-411/251, 3-4=-315/223, 4-5=-250/215, 5-6=-185/207, 6-7=-129/192, 7-8=-109/198, 8-33=-130/259, 9-33=-52/264, 9-10=-135/310, 10-11=-135/298, 11-34=-52/221, 12-34=-130/214, 12-13=-26/134, 13-14=-129/123, 14-15=-129/84, 15-16=-129/92, 16-17=-191/99, 17-18=-309/119, 18-19=0/70
 BOT CHORD 2-32=-73/297, 31-32=-73/297, 30-31=-73/297, 29-30=-73/297, 28-29=-73/297, 27-28=-73/297, 26-27=-73/297, 25-26=-73/297, 24-25=-73/297, 23-24=-73/297, 22-23=-73/297, 21-22=-73/297, 20-21=-73/297, 19-20=-73/297
 WEBS 10-26=-297/0, 9-27=-299/106, 8-28=-291/142, 6-29=-290/130, 5-30=-290/132, 4-31=-292/133, 3-32=-279/166, 11-25=-299/100, 12-24=-291/143, 14-23=-290/129, 15-22=-290/132, 16-21=-292/132, 17-20=-279/160

NOTES (6)
 1) Wind: ASCE 7-02; 90mph; h=35ft; TCDF=5.0psf; BCLD=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Exterior(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 11-0-0, Exterior(2) 11-0-0 to 17-0-0, Interior(1) 17-0-0 to 26-0-0, Exterior(2) 26-0-0 to 29-0-0 zone; cantilever left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) Truss designed for wind loads in the plane of the truss only.
 3) Unbalanced snow loads have been considered for this design.
 4) All plates are 1/8" MT20 unless otherwise indicated.
 5) Gable requires continuous bottom chord bearing.
 6) Gable ends braced at 2-0-0 oc.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MS-7473 BEFORE USE.
 Design valid for use only with Mittek connections. This design is based only upon parameters shown, and is for an individual building component.
 Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANS/TP1 Quality Criteria, D58-87 and BCS1 Building Component Safety Information available from Truss Plate Institute, 383 D'Onofrio Drive, Madison, WI 53719.

14515 N. Outer Forty
 Suite #300
 Chesterfield, MO 63017

MITTEK

Job	Truss	Truss Type	Qty	Ply	Hancock/Verrier Res/wd/11-2	18712372
421900	007	GABLE	1	1		


Wood Structures, Inc., Biddeford, ME 04005
 Job Reference (optional)
 8.200 s Oct 18 2005 Mittek Industries, Inc. Wed Jan 18 15:13:04 2006 Page 2

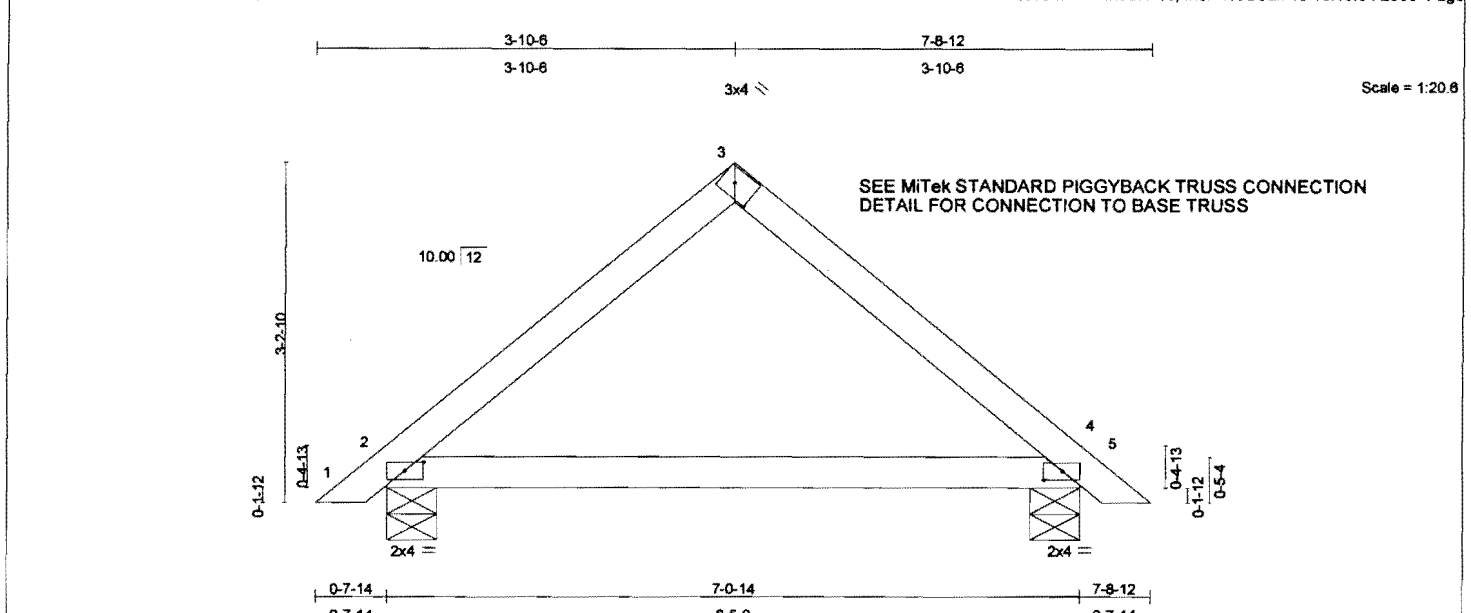
NOTES (8)

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 162 lb uplift at joint 2, 70 lb uplift at joint 18, 86 lb uplift at joint 27, 122 lb uplift at joint 28, 110 lb uplift at joint 29, 111 lb uplift at joint 30, 113 lb uplift at joint 31, 143 lb uplift at joint 32, 80 lb uplift at joint 25, 123 lb uplift at joint 24, 109 lb uplift at joint 23, 111 lb uplift at joint 22, 113 lb uplift at joint 21 and 137 lb uplift at joint 20.

8) Drawing prepared exclusively for manufacturing by Wood Structures Inc.

LOAD CASE(S) Standard

<p>WARNING - Verify design parameters and READ NOTES ON TRUS AND INCLUDED MITTEK REFERENCE PAGE 002-1473 BEFORE USE. Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANS/TP1 Quality Criteria, D58-87 and BCSI Building Component Safety Information available from Truss Plate Institute, 383 D Onaska Drive, Madison, WI 53717.</p>	<p>14515 N. Outer Forty, Suite #300 Chesterfield, MO 63017</p> 
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LOADING (psf)		SPACING		CSI		DEFL		PLATES		GRIP	
TCLL	50.0	Plates Increase	1.15	TC	0.34	in (loc)	l/defl	L/d	MT20	197/144	
(Roof Snow=50.0)		Lumber Increase	1.15	BC	0.80	Vert(LL)	-0.24	2-4	>304	240	
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Vert(TL)	-0.32	2-4	>221	180	
BCLL	0.0	Code	BOCA/ANSI95	(Matrix)		Horz(TL)	0.00	4	n/a	n/a	
BCDL	10.0									Weight: 22 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SPF 2100F 1.8E

BRACING
 TOP CHORD Sheathed or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

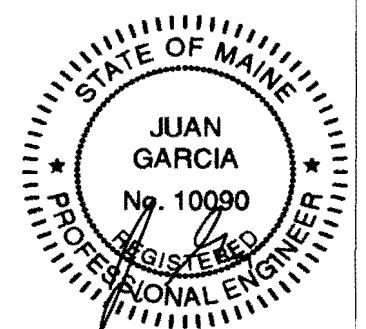
This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

REACTIONS (lb/size) 2=805/0-5-8, 4=805/0-5-8
 Max Horz 2=94 (load case 5)
 Max Uplift 2=49 (load case 7), 4=49 (load case 8)
 Max Grav 2=807 (load case 2), 4=807 (load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/39, 2-3=-395/77, 3-4=-395/77, 4-5=0/39
 BOT CHORD 2-4=-14/207

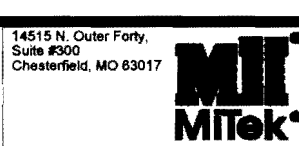
NOTES (6)
 1) Wind: ASCE 7-02; 90mph; h=35ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) Unbalanced snow loads have been considered for this design.
 3) Ceiling dead load (5.0 psf) on member(s), 2-3, 3-4
 4) Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room, 2-4
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 49 lb uplift at joint 2 and 49 lb uplift at joint 4.
 6) Drawing prepared exclusively for manufacturing by Wood Structures Inc.

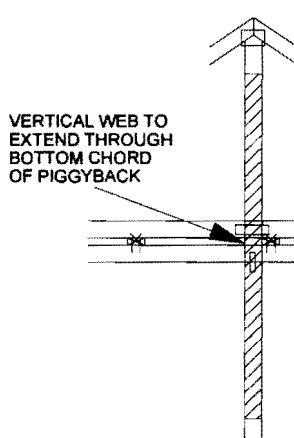
LOAD CASE(S) Standard



January 19, 2006


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE #27473 BEFORE USE.
 Design valid for use only with MITek connection. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing consult ANS/IFPI Quality Criteria, D58-89 and BCS1 Building Component Safety Information available from Truss Plate Institute, 383 D'Onofrio Drive, Madison, WI 53715.



OCT 29, 2004	STANDARD PIGGYBACK TRUSS CONNECTION DETAIL	ST-PIGGY
<p>★ 2 x 6 x 6'-0" SIZE TO MATCH TOP CHORD OF PIGGYBACK. ATTACHED TO ONE FACE OF TOP CHORD WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C.</p> <p>Mitek Industries, Inc.</p>	<p>PIGGYBACK TRUSS</p> <p>★ ATTACH PIGGYBACK TRUSS TO EACH PURLIN WITH 2 - 16d (0.131" X 3.5") NAILS TOENAILED.</p> <p>BASE TRUSS</p> <p>SPACE PURLINS ACCORDING TO THE MAXIMUM SPACING ON THE TOP CHORD OF THE BASE TRUSS (SPACING NOT TO EXCEED 24" O.C.). A PURLIN TO BE LOCATED AT EACH BASE TRUSS JOINT.</p> <p>ATTACH EACH PURLIN TO TOP CHORD OF BASE TRUSS WITH 2 - 16d (0.131" X 3.5") NAILS.</p> <p>★ FOR PIGGY BACK TRUSSES WITH SPANS < 12' SCAB MAY BE OMITTED PROVIDED THAT: ROOF SHEATHING TO BE CONTINUOUS OVER JOINT (SHEATHING TO OVERLAP MINIMUM 12" OVER JOINT)</p> <p>★ CAP CONNECTION IS MADE TO RESIST UPLIFT. SEE MAXIMUM CONNECTION CAPACITIES AND COMPARE WITH ENGINEERING DRAWING CONNECTION CAPACITIES FOR SCABS, PURLINS, AND SHEATHING MAY BE COMBINED WHEN DETERMINING OVERALL UPLIFT CAPACITY.</p>	<p>ALL VALUES SHOWN BELOW ARE BASED ON LOAD DURATION OF 1.33</p> <p>MAXIMUM UPLIFT SCAB CAPACITY USING (10) 10d (0.131" X 3") NAILS:</p> <p>SYP = 1409 LBS SPF = 1090 LBS DF = 1290 LBS HF = 1117 LBS SPF-S = 957 LBS</p> <p>MAXIMUM UPLIFT PURLIN CAPACITY USING (2) 16d (0.131" X 3.5") NAILS:</p> <p>SYP = 155 LBS SPF = 79 LBS DF = 122 LBS HF = 83 LBS SPF-S = 54 LBS</p> <p>MAXIMUM UPLIFT SHEATHING CAPACITY USING 1/2" SHEATHING AND (2) 8d (0.131" X 2.5") NAILS:</p> <p>SYP = 109 LBS SPF = 85 LBS DF = 85 LBS HF = 58 LBS SPF-S = 37 LBS</p>
<p>★ CONNECTION AS ABOVE</p>	<p>IF NO GAP EXISTS BETWEEN CAP TRUSS AND BASE TRUSS: REPLACE THE NAILING OF CAP TRUSS TO PURLINS WITH GUSSETS AS SHOWN, AND APPLY PURLINS TO LOWER EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS DESIGN DRAWING.</p> <p>★ 6" x 6" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT EACH BASE TRUSS JOINT. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)</p> <p>ADD PURLINS TO BOTTOM EDGE</p>	<p>MAXIMUM UPLIFT GUSSET CAPACITY USING 7/16" GUSSETS AND (6) 8d (0.113" X 2") NAILS:</p> <p>SYP = 399 LBS SPF = 367 LBS DF = 2580 LBS HF = 367 LBS SPF-S = 343 LBS</p>
<p>VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK</p> 	<p>FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:</p> <ol style="list-style-type: none"> 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL. 2) VERTICAL WEBS OF PIGGYBACK MUST RUN THROUGH BOTTOM CHORD SO THAT THERE IS FULL WOOD TO WOOD CONTACT BETWEEN WEB OF PIGGYBACK AND THE TOP CHORD OF THE BASE TRUSS. 3) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS. 4) ATTACH 2 x 6 x 6'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4) 5) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS. 6) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS. 	<p>MAXIMUM UPLIFT SCAB CAPACITY USING (20) 10d (0.131" X 3") NAILS:</p> <p>SYP = 2819 LBS SPF = 2181 LBS DF = 2580 LBS HF = 2234 LBS SPF-S = 1915 LBS</p>

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Wood Structures, Inc., Biddeford, ME 04005 6.200 s Oct 18 2005 MITek Industries, Inc. Wed Jan 18 15:13:05 2006 Page 1

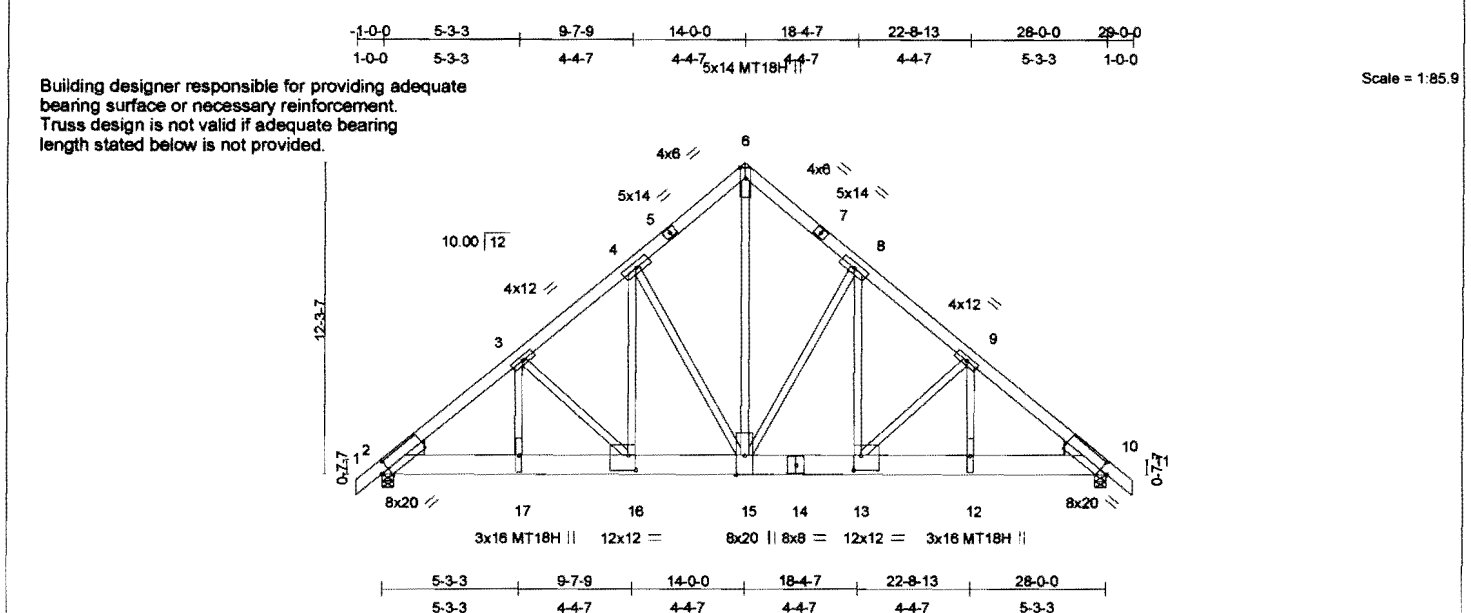


Plate Offsets (X,Y):		[2-0-3-12,0-4-8], [10-0-3-12,0-4-8], [13-0-3-8,0-7-0], [16-0-3-8,0-7-0]	
LOADING (psf)	SPACING	CSI	DEFL
TCLL 50.0	Plates Increase 1.15	TC 0.85	in (loc) Udefl L/d
(Roof Snow=50.0)	Lumber Increase 1.15	BC 0.65	Vert(LL) -0.02 15 >999 240
TCDL 10.0	Rep Stress Incr YES	WB 1.00	Vert(TL) -0.41 13-15 >807 180
BCLL 0.0	Code BOCA/ANSI95	(Matrix)	Horz(TL) 0.10 10 n/a n/a
BCDL 10.0			
			PLATES GRIP
			MT20 197/144
			MT18H 197/144
			Weight 767 lb

LUMBER
 TOP CHORD 2 X 6 SPF 1650F 1.5E
 BOT CHORD 2 X 10 SYP M 23
 WEBS 2 X 4 SPF 1650F 1.5E "Except"
 6-15 2 X 4 SPF 2100F 1.8E

BRACING
 TOP CHORD Sheathed or 4-4-9 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

WEDGE
 Left: 2 X 8 SYP No.2, Right: 2 X 8 SYP No.2

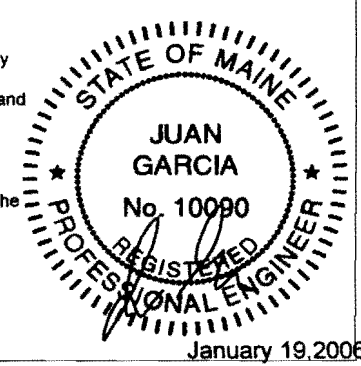
REACTIONS (lb/size) 2=24246/0-6-11, 10=24246/0-6-11
 Max Horz 2=395(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/105, 2-3=30411/0, 3-4=24504/0, 4-5=19181/0, 5-6=18679/0, 6-7=18879/0, 7-8=19181/0, 8-9=24504/0,
 9-10=30411/0, 10-11=0/105
 BOT CHORD 2-17=0/23010, 16-17=0/23011, 15-16=0/18655, 14-15=0/18655, 13-14=0/18655, 12-13=0/23011, 10-12=0/23010
 WEBS 3-17=0/7726, 4-16=0/10301, 6-15=0/23321, 8-13=0/10301, 9-12=0/7726, 3-16=6229/0, 4-15=8155/0, 8-15=8155/0,
 9-13=6229/0

- NOTES (8)**
- 3-ply truss to be connected together with 10d Common (148"x3") Nails as follows:
 Top chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
 Bottom chords connected as follows: 2 X 10 - 4 rows at 0-4-0 oc.
 Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
 - All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 - Wind: ASCE 7-02; 90mph; h=35ft; TCCL=5.0psf; BCCL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
 - Unbalanced snow loads have been considered for this design.
 - All plates are MT20 plates unless otherwise indicated.
 - n/a
 - Load case(s) 1, 2, 3, 4, 5, 6, 7, 8, 9, 10 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.
 - Drawing prepared exclusively for manufacturing by Wood Structures Inc.

LOAD CASE(S) Standard This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

Continued on page 2



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Job	Truss	Truss Type	Qty	Ply	Hancock/Verner Res/wd/11-2	19712374
421900	500	SPECIAL GIRDER	1	3		
Wood Structures, Inc., Biddeford, ME 04005					Job Reference (optional) 6.200 s Oct 18 2005 MITek Industries, Inc. Wed Jan 18 15:13:05 2006 Page 2	
LOAD CASE(S) Standard						
1) Snow: Lumber Increase=1.15, Plate Increase=1.15						
Uniform Loads (plf)						
Vert: 2-10=1630(F=1610), 1-6=120, 6-11=120						
2) Unbal.Snow-Left: Lumber Increase=1.15, Plate Increase=1.15						
Uniform Loads (plf)						
Vert: 2-10=1630(F=1610), 1-6=145, 6-11=20						
3) Unbal.Snow-Right: Lumber Increase=1.15, Plate Increase=1.15						
Uniform Loads (plf)						
Vert: 2-10=1630(F=1610), 1-6=20, 6-11=145						
4) MWFRS Wind Left: Lumber Increase=1.60, Plate Increase=1.60						
Uniform Loads (plf)						
Vert: 2-10=1620(F=1610), 1-2=7, 2-6=14, 6-10=20, 10-11=12						
Horz: 1-2=17, 2-6=4, 6-10=30, 10-11=22						
5) MWFRS Wind Right: Lumber Increase=1.60, Plate Increase=1.60						
Uniform Loads (plf)						
Vert: 2-10=1620(F=1610), 1-2=12, 2-6=20, 6-10=14, 10-11=7						
Horz: 1-2=22, 2-6=30, 6-10=4, 10-11=17						
6) MWFRS 1st Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60						
Uniform Loads (plf)						
Vert: 2-10=1620(F=1610), 1-2=7, 2-6=14, 6-10=11, 10-11=19						
Horz: 1-2=17, 2-6=4, 6-10=1, 10-11=9						
7) MWFRS 2nd Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60						
Uniform Loads (plf)						
Vert: 2-10=1620(F=1610), 1-2=19, 2-6=11, 6-10=14, 10-11=7						
Horz: 1-2=9, 2-6=1, 6-10=4, 10-11=17						
8) MWFRS 3rd Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60						
Uniform Loads (plf)						
Vert: 2-10=1620(F=1610), 1-2=33, 2-6=15, 6-10=12, 10-11=5						
Horz: 1-2=43, 2-6=25, 6-10=22, 10-11=15						
9) MWFRS 4th Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60						
Uniform Loads (plf)						
Vert: 2-10=1620(F=1610), 1-2=5, 2-6=12, 6-10=15, 10-11=33						
Horz: 1-2=15, 2-6=22, 6-10=25, 10-11=43						
10) Attic Floor: Lumber Increase=0.90, Plate Increase=0.90						
Uniform Loads (plf)						
Vert: 2-10=1630(F=1610), 1-6=20, 6-11=20						

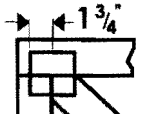
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 Safety Information available from Truss Plate Institute, 363 D Onofre Drive, Madison, WI 53719.

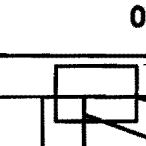
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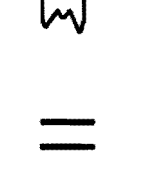


Symbols

PLATE LOCATION AND ORIENTATION

 *Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and securely seat.

 *For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.


 *This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MITek 20/20 software or upon request.

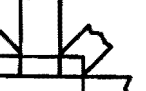
PLATE SIZE

4 x 4 The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

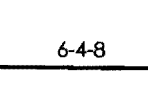
 Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

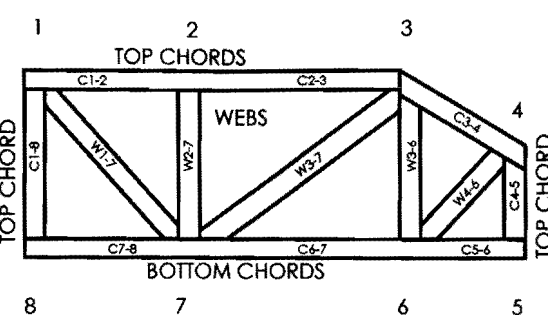
BEARING

 Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:
ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DS8-89: Design Standard for Bracing.
BCS11: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

 6-4-8 dimensions shown in ft-in-sixteenths



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 95-43, 96-20-1, 96-67, 84-32
ICBO	4922, 5243, 5363, 3907
SBCCI	9667, 9730, 9604B, 9511, 9432A



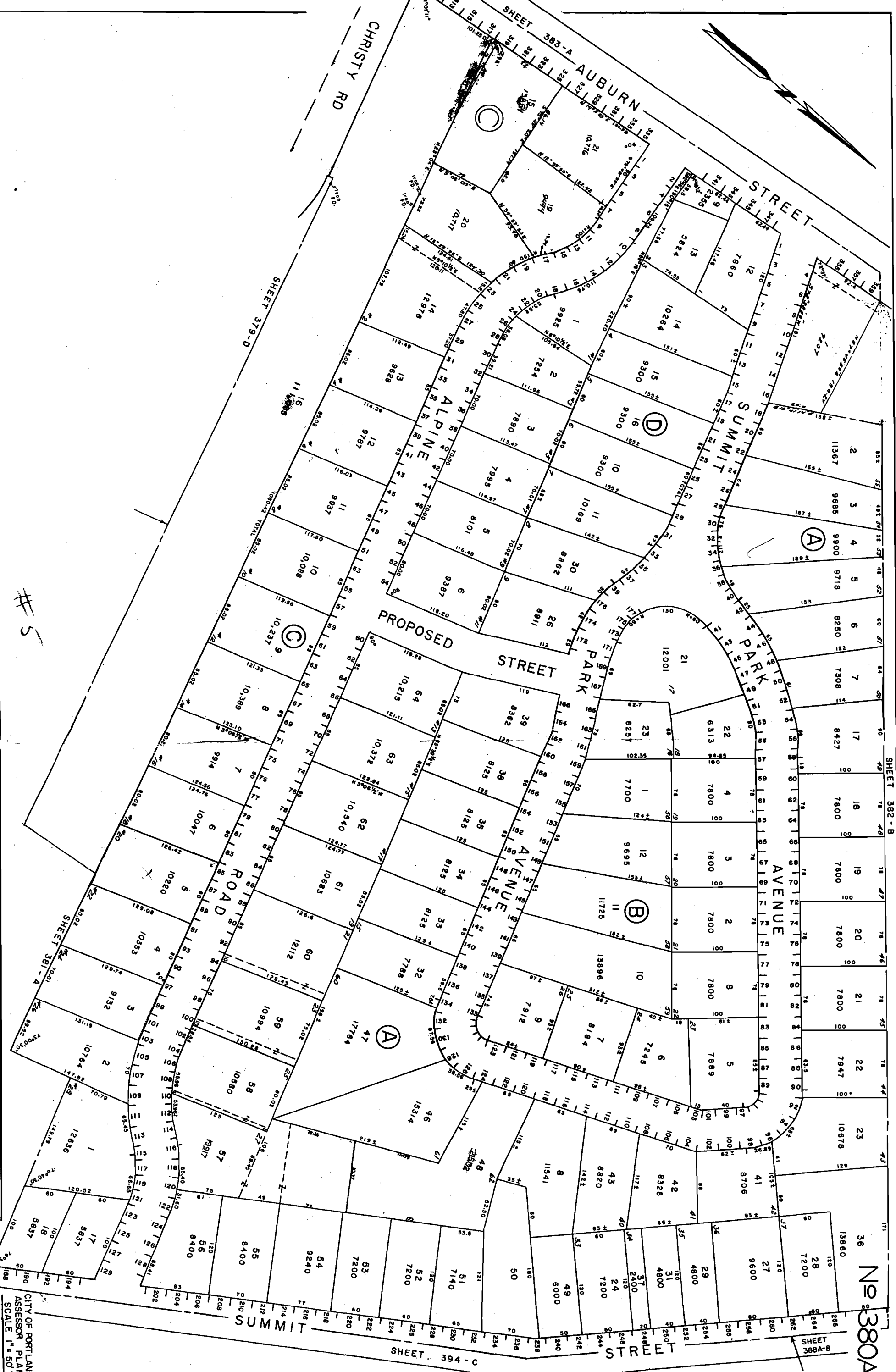
MITek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11.
2. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
3. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
4. Cut members to bear tightly against each other.
5. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI1.
6. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI1.
7. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
8. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
9. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
10. Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
11. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
12. Top chords must be sheathed or purlins provided at spacing shown on design.
13. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
14. Connections not shown are the responsibility of others.
15. Do not cut or alter truss member or plate without prior approval of a professional engineer.
16. Install and load vertically unless indicated otherwise.

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CITY OF PORTLAND
 ASSESSOR'S PLAN
 SCALE 1"=50'
 REVISED 10-78

#5

N 380A

SHEET 388A-B

SHEET 394-C

SHEET 379-D

SHEET 381-A

SHEET 382-B

SHEET 383-A

SHEET 384-B

SHEET 385-C

SHEET 386-D

SHEET 387-E

SHEET 388-F

SHEET 389-G

SHEET 390-H

SHEET 391-I

SHEET 392-J

SHEET 393-K

SHEET 394-L

SHEET 395-M

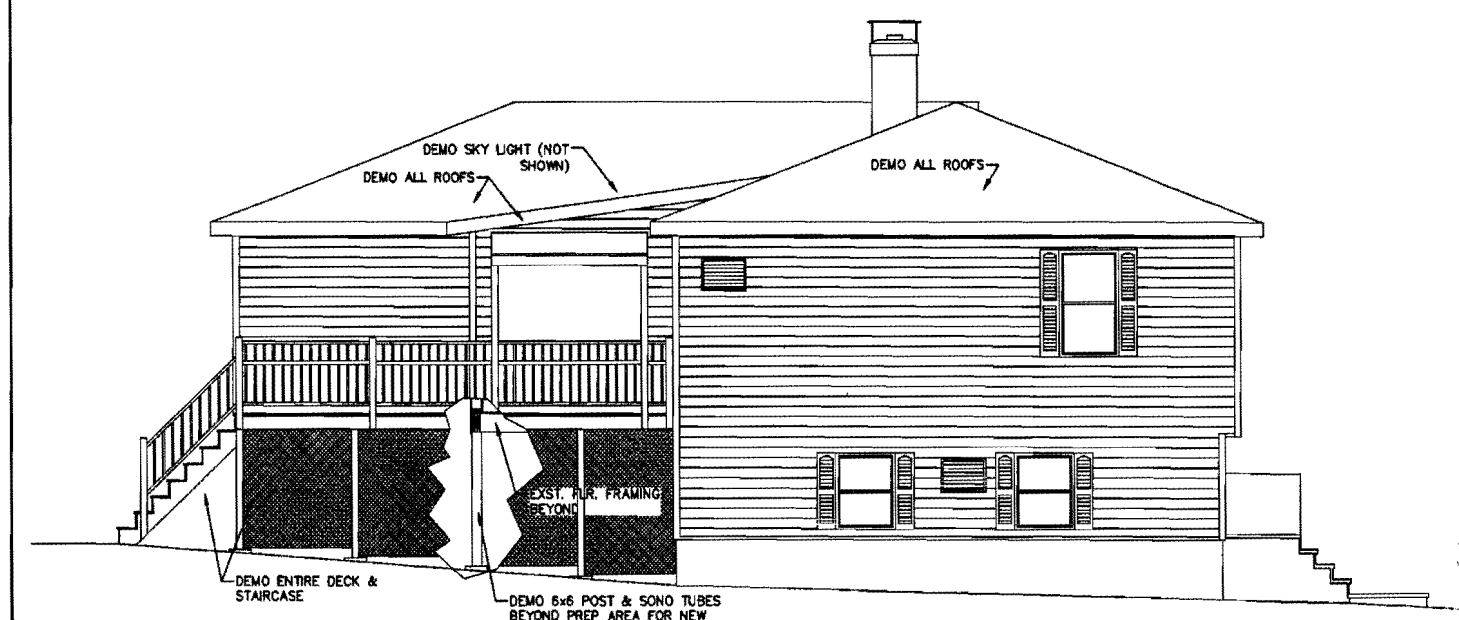
SHEET 396-N

SHEET 397-O

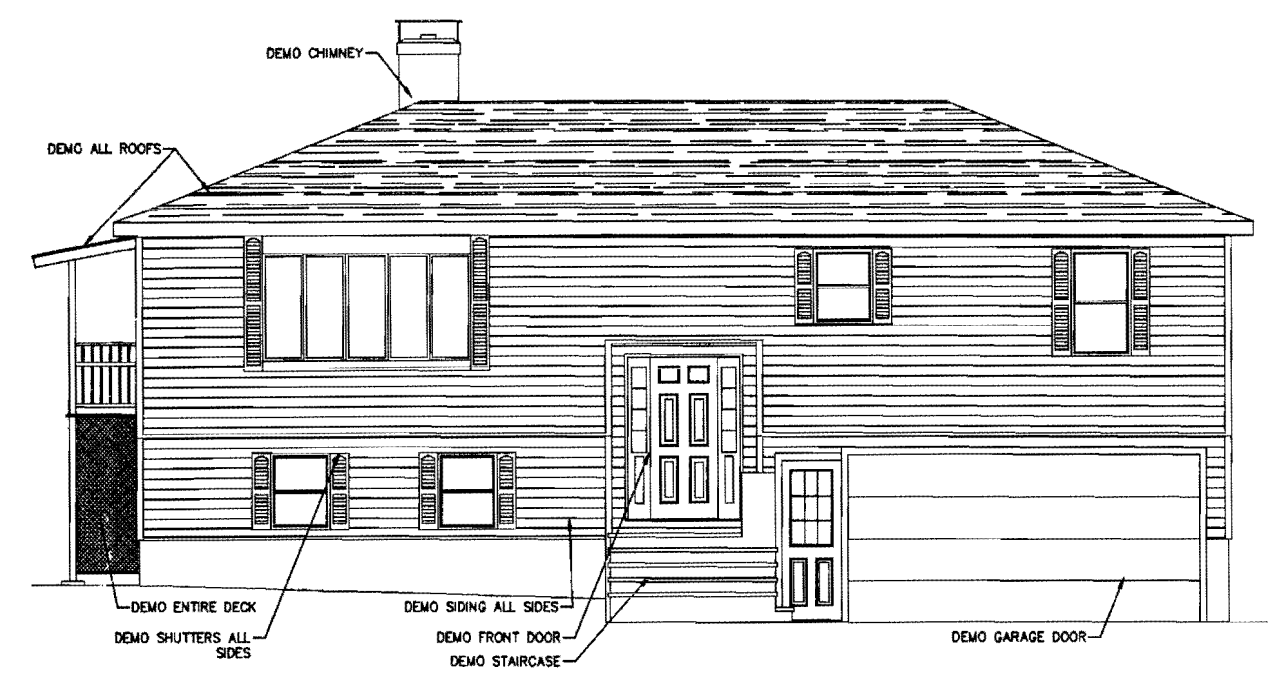
SHEET 398-P

SHEET 399-Q

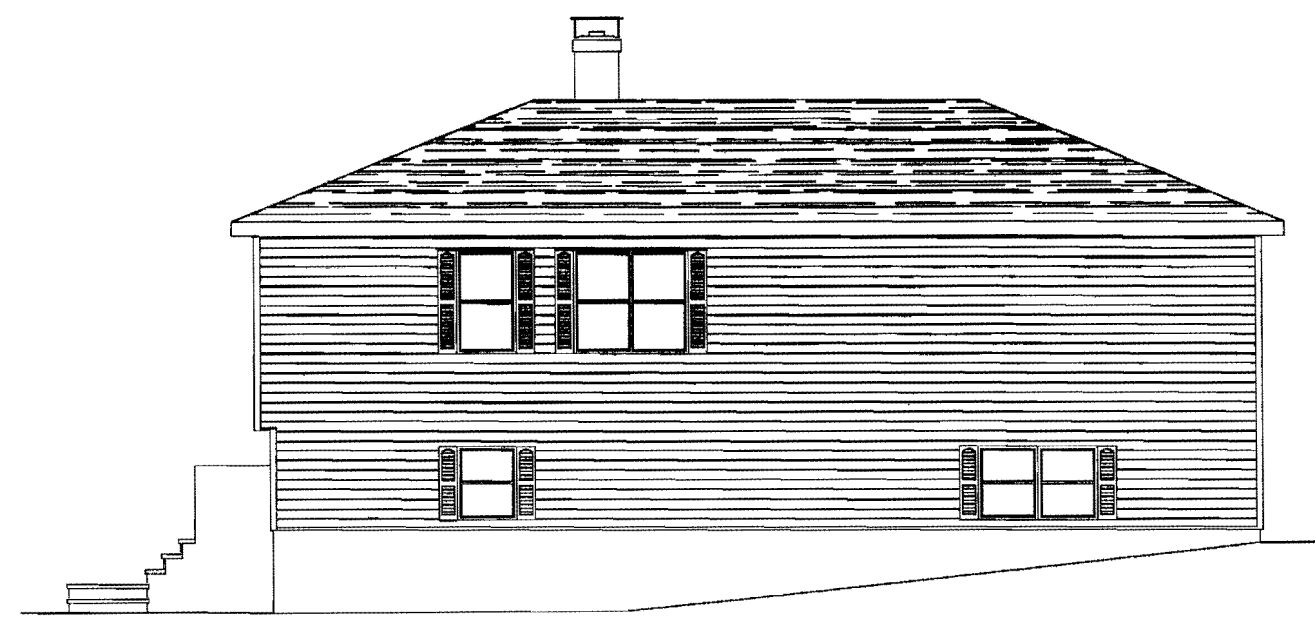
SHEET 400-R



2 LEFT SIDE ELEVATION / DEMO PLAN
1/4" = 1'-0"



1 FRONT ELEVATION / DEMO PLAN
1/4" = 1'-0"



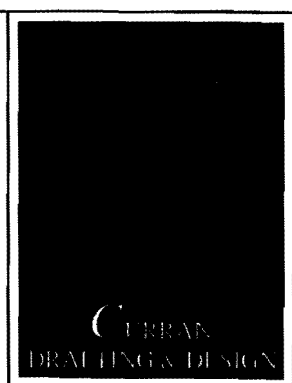
4 RIGHT SIDE ELEVATION / DEMO PLAN
1/4" = 1'-0"



3 REAR ELEVATION / DEMO PLAN
1/4" = 1'-0"

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DEMO PLAN

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DATE: 07/13/2005
FILE NUMBER 05-0031
REVISIONS:

A-1



2 LEFT SIDE ELEVATION
1/4" = 1'-0"



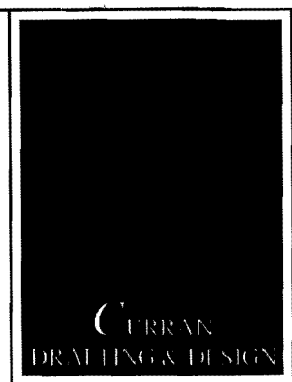
1 FRONT ELEVATION
1/4" = 1'-0"



4 RIGHT SIDE ELEVATION
1/4" = 1'-0"



3 REAR ELEVATION
1/4" = 1'-0"



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FILE NUMBER 05-0031

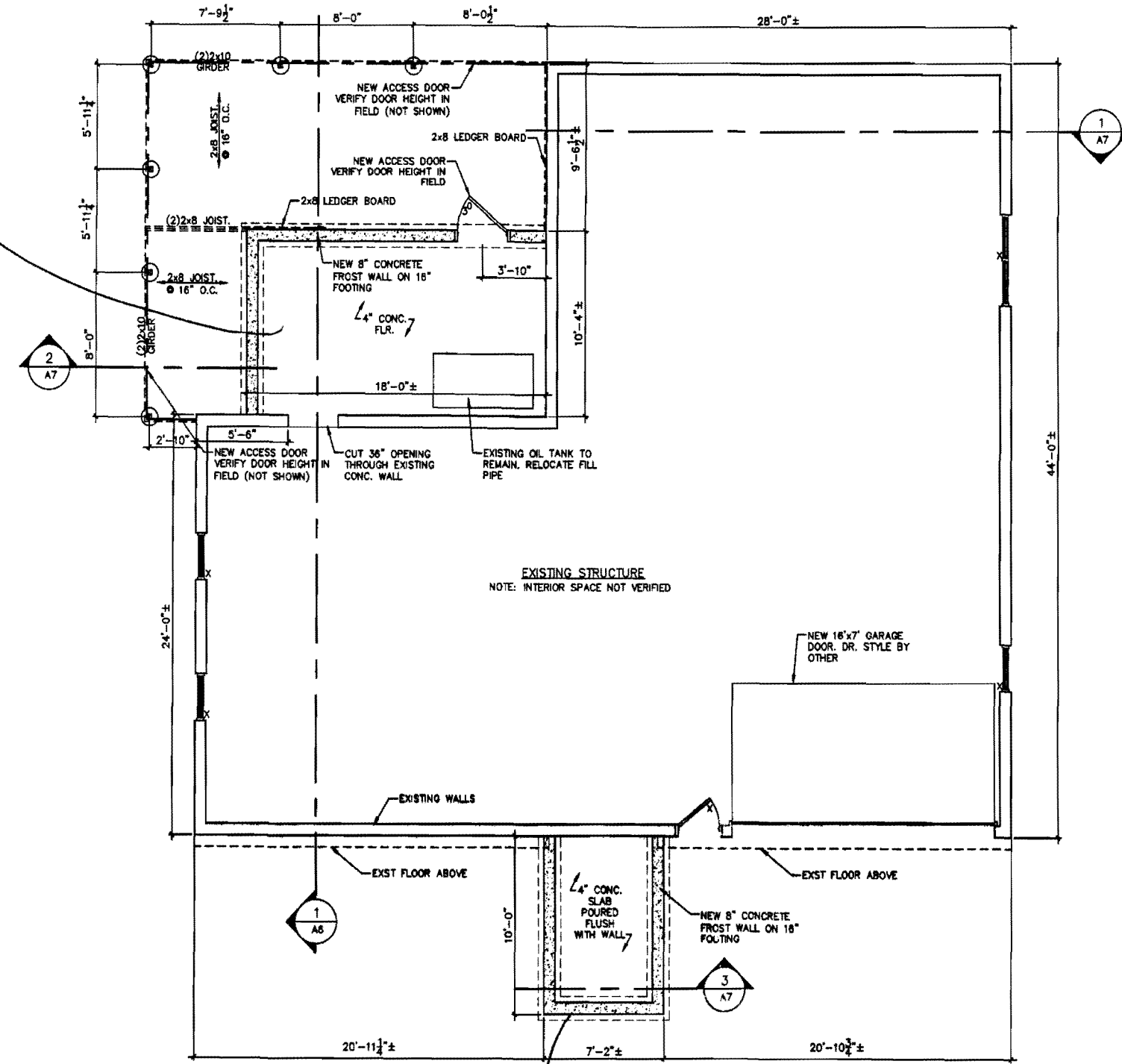
REVISIONS:
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CLIENT _____

A-2

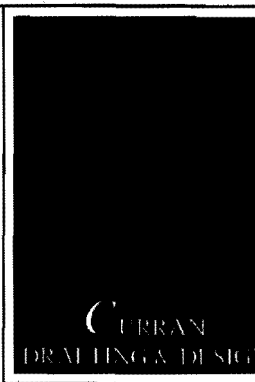
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Within existing Footprint



1 FOUNDATION / FIRST FLOOR PLAN
1/8" = 1'-0"

The same Footprint as existing



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BUILT BY

SHEET NAME

**FOUNDATION
/ FIRST FLR
PLAN**

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 (207) 831-1844 Td
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PROJECT NAME

**VERRIER
 RESIDENCE**

PROJECT LOCATION

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 PORTLAND ME. 04103

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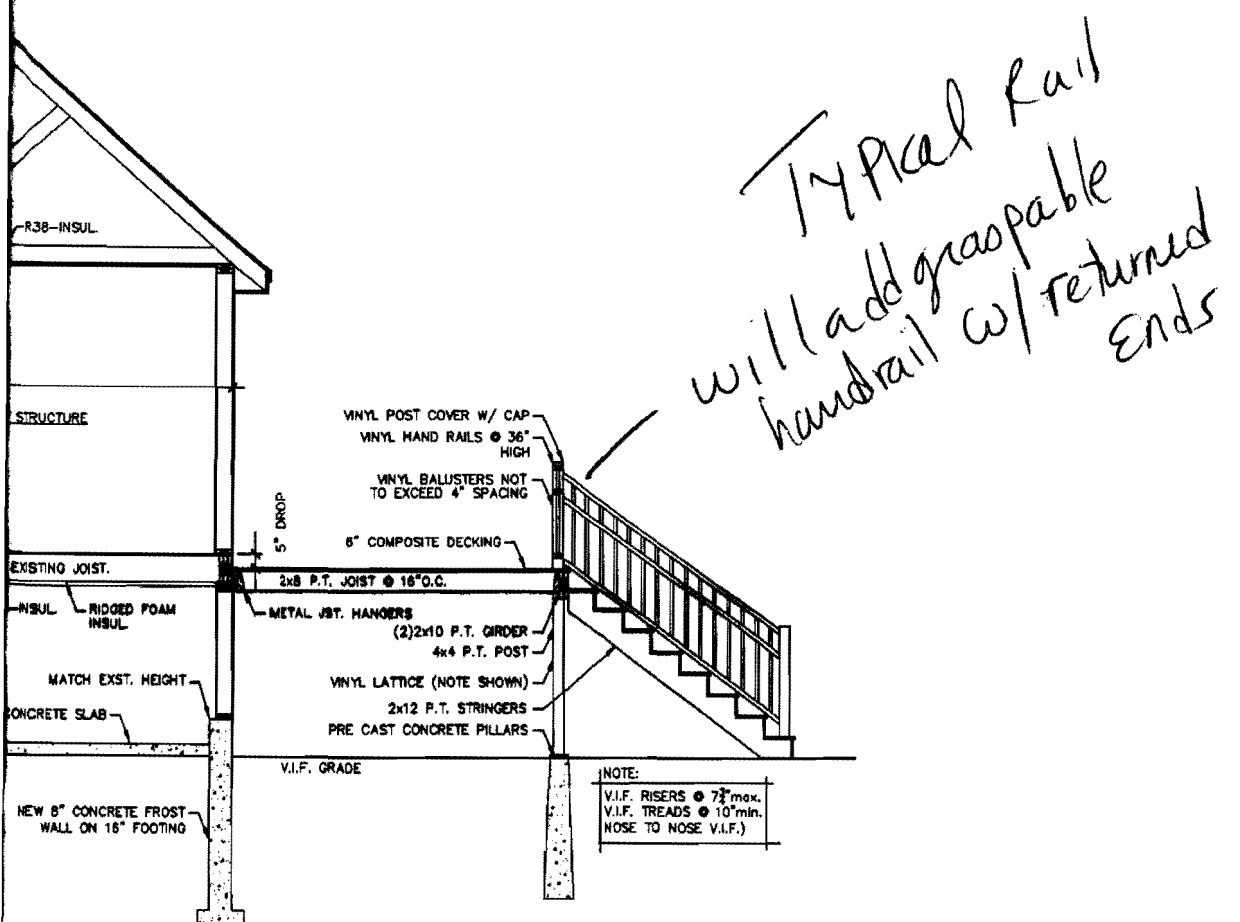
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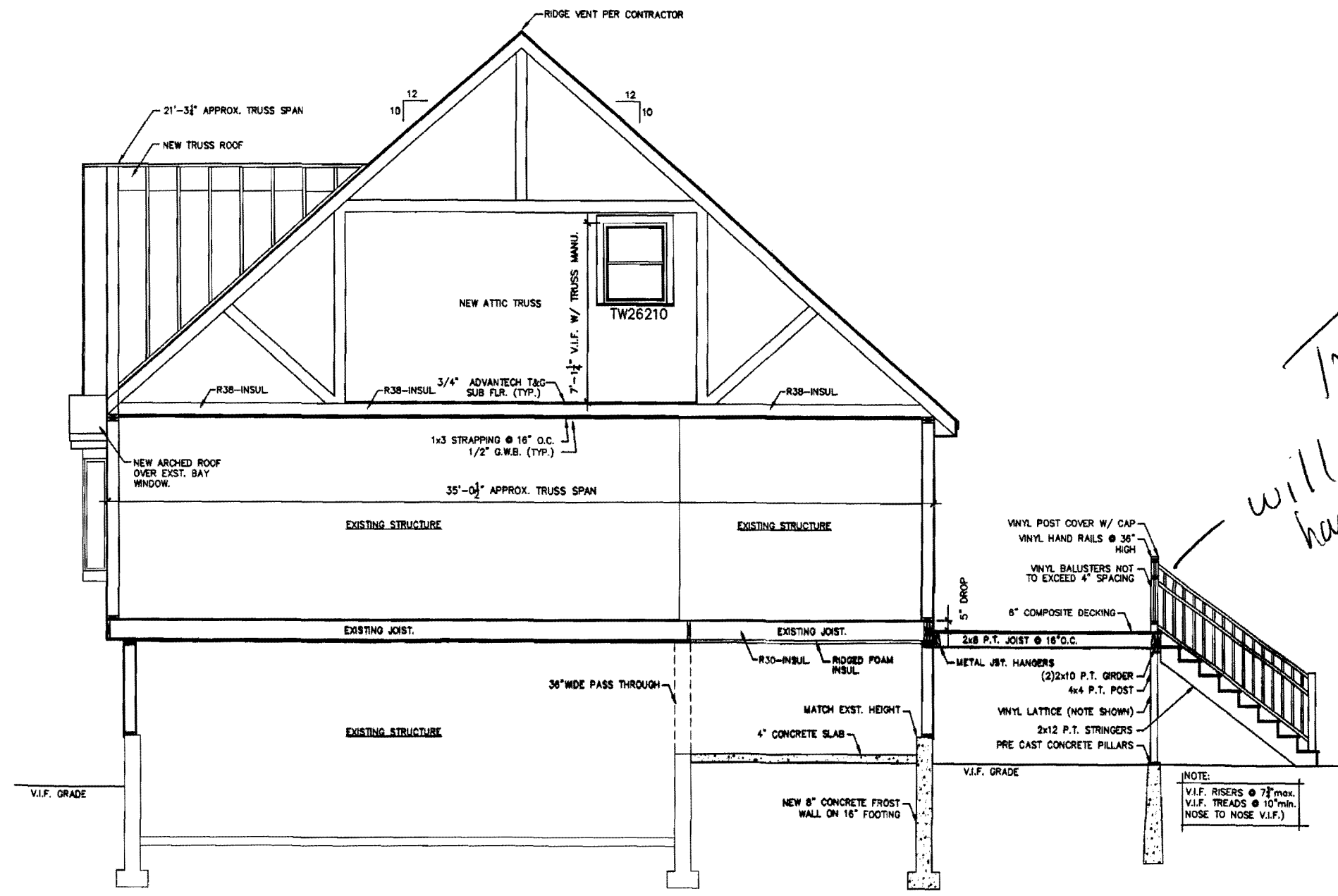
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A-6

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*Typical Rail
 will add graspable
 handrail w/ returned
 ends*



*Typical Rail
will add graspable
handrail w/ returned
ends*

1 CROSS SECTION
3/8" = 1'-0"

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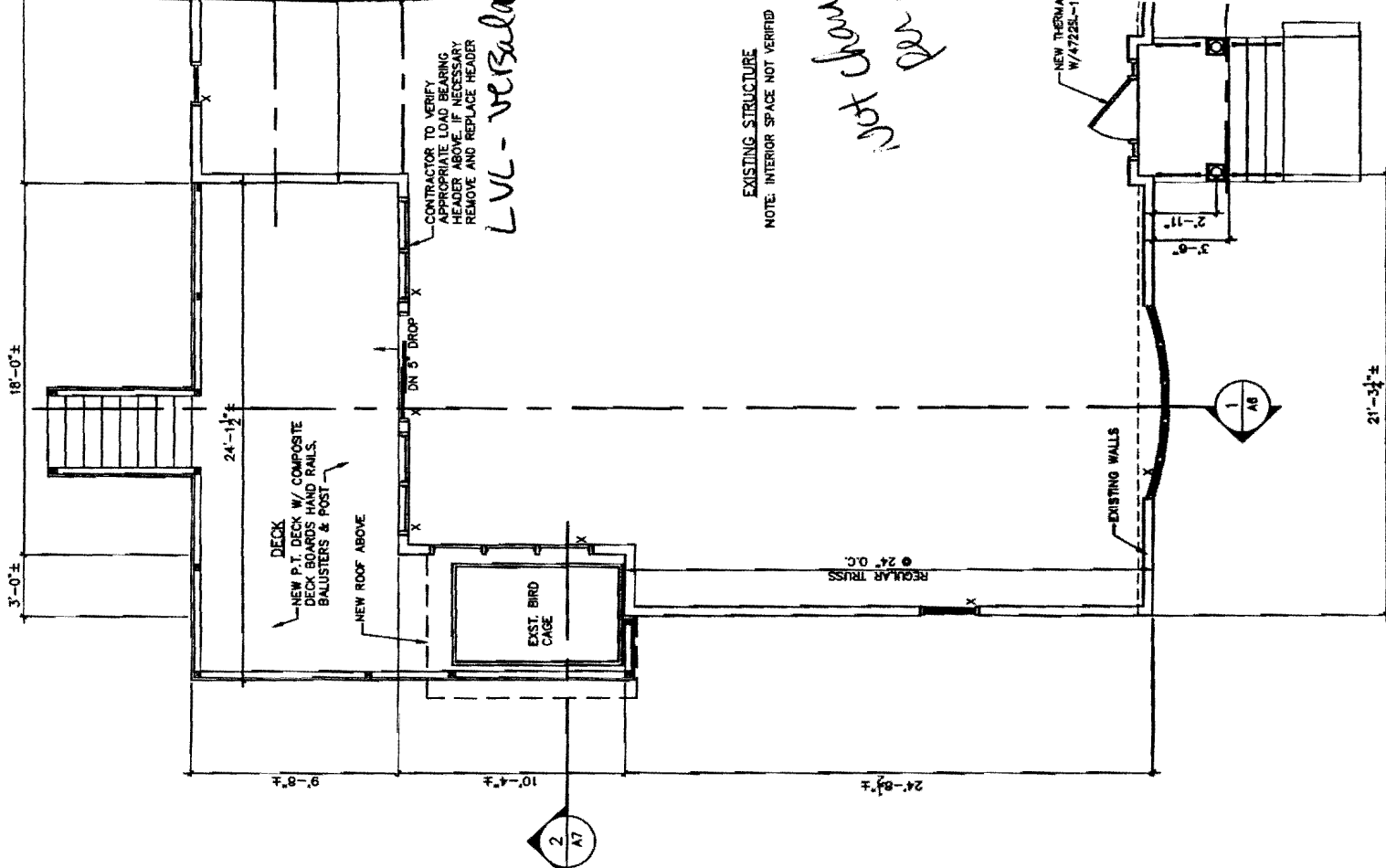
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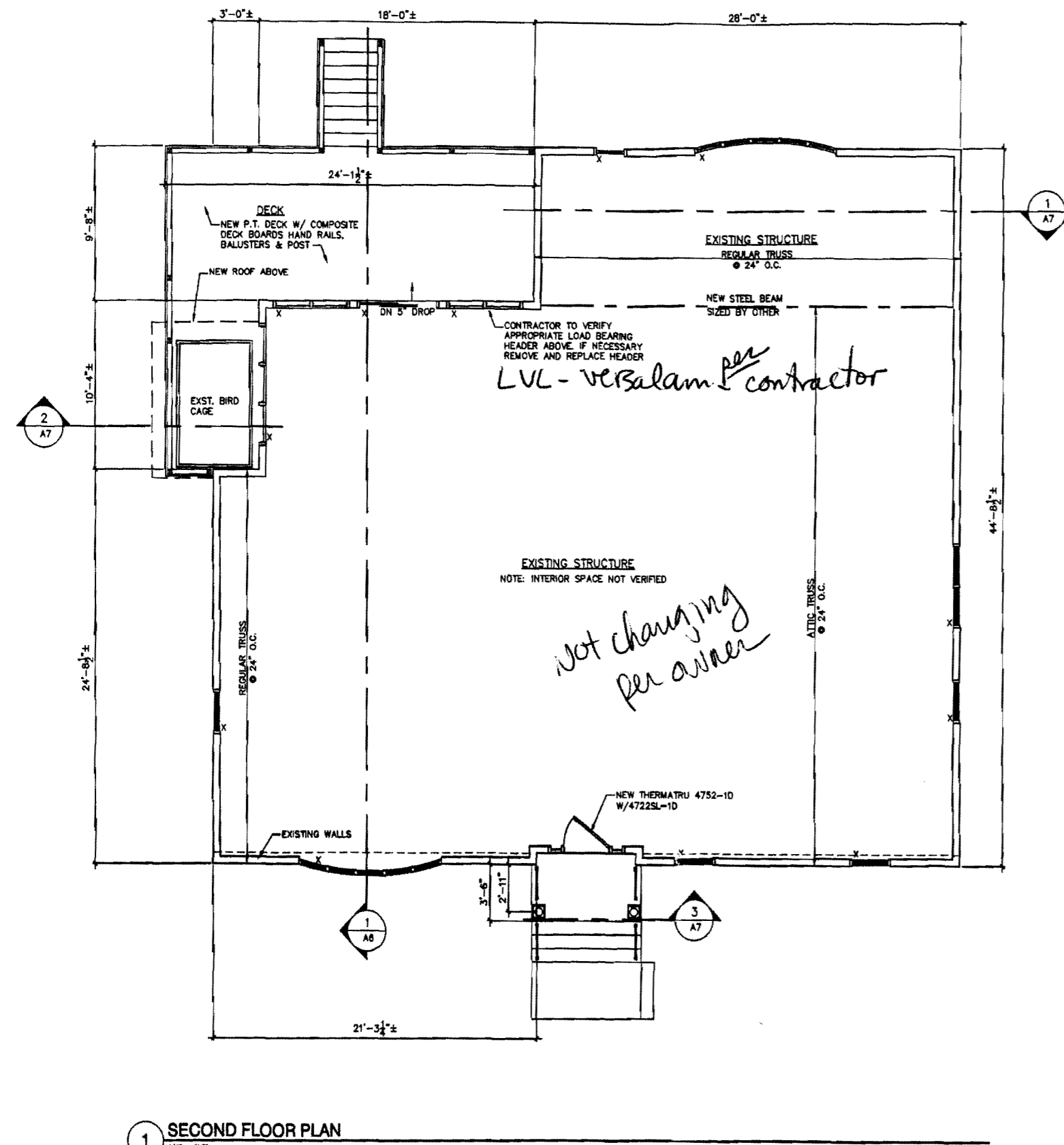
CONTRACTOR TO VERIFY
 ALL DIMENSIONS AND
 CONDITIONS OF WORK
 BEFORE BEGINNING WORK.
 REMOVE AND REPLACE HEADR
 LVL - VERBODEN

EXISTING STRUCTURE
 NOTE: INTERIOR SPACE NOT FINISHED

not shown per

1 SECOND FLOOR PLAN
 10-1-10

NOTES:
 1. ALL DIMENSIONS ARE TO FACE UNLESS OTHERWISE NOTED.
 2. ALL WORK SHALL BE IN ACCORDANCE WITH THE LATEST EDITIONS OF THE INTERNATIONAL BUILDING CODES AND ALL APPLICABLE LOCAL, STATE AND FEDERAL CODES.
 3. ALL WORK SHALL BE IN ACCORDANCE WITH THE LATEST EDITIONS OF THE INTERNATIONAL BUILDING CODES AND ALL APPLICABLE LOCAL, STATE AND FEDERAL CODES.
 4. ALL WORK SHALL BE IN ACCORDANCE WITH THE LATEST EDITIONS OF THE INTERNATIONAL BUILDING CODES AND ALL APPLICABLE LOCAL, STATE AND FEDERAL CODES.



1 SECOND FLOOR PLAN
1/4" = 1'-0"



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SECOND
FLOOR PLAN

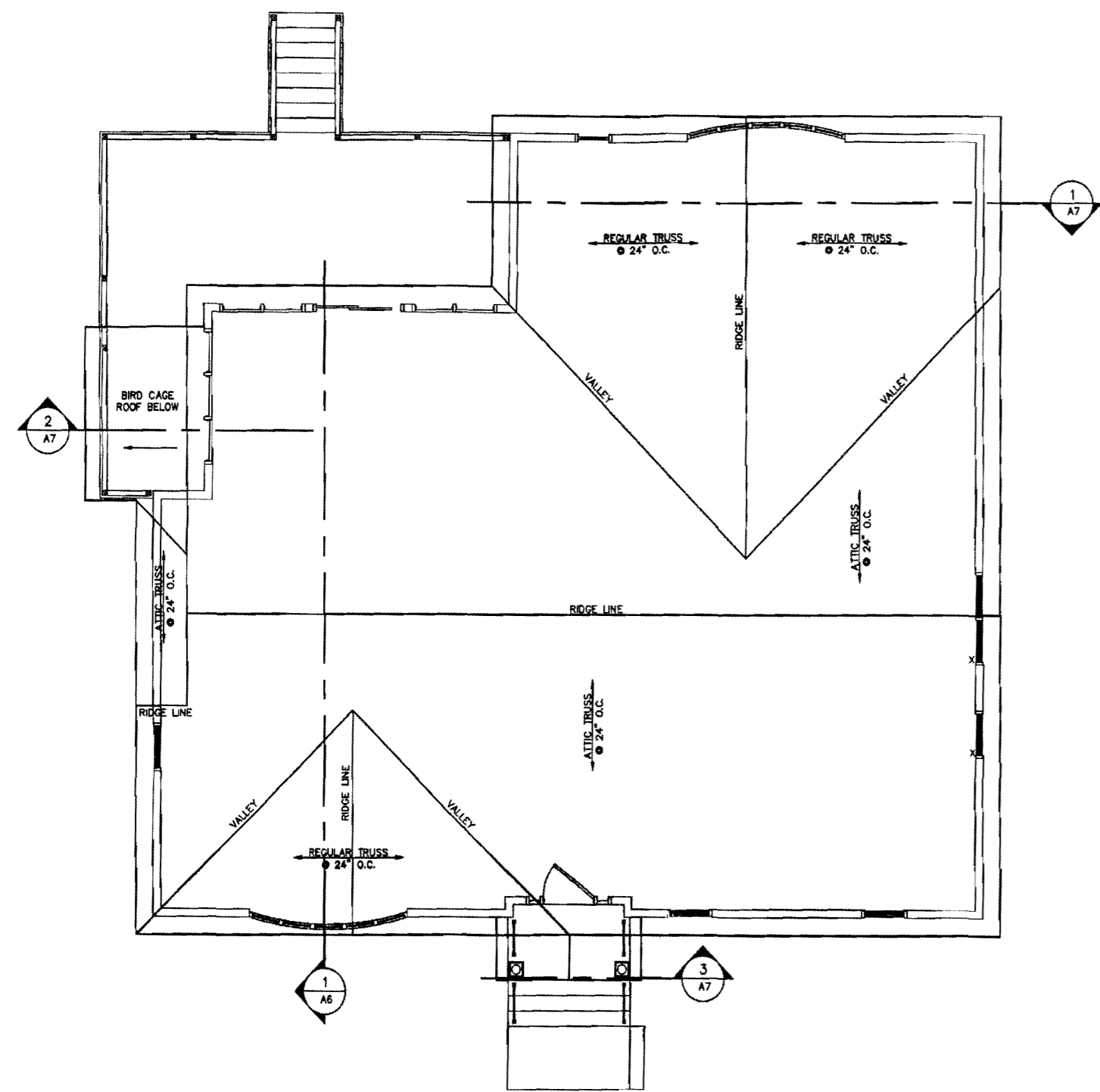
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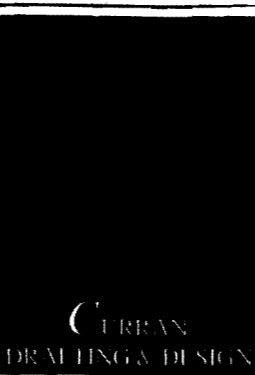
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1 ROOF PLAN
1/4" = 1'-0"



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ROOF PLAN

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