

PART 1 - GENERAL

- 1.01 GENERAL
 - A. NO PROVISIONS HAVE BEEN MADE FOR ANY TEMPORARY CONDITIONS THAT MAY ARISE DURING CONSTRUCTION PRIOR TO THE COMPLETION OF THE STRUCTURE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ADEQUATE DESIGN AND CONSTRUCTION OF ALL TEMPORARY BRACING AND PROVISIONS THROUGHOUT THE PROGRESS OF THE PROJECT.
 - B. TEMPORAL BRACING THROUGHOUT THE FOUNDATION ARE NOT SHOWN ON THESE DRAWINGS. THE GENERAL CONTRACTOR SHALL OBTAIN THE PROVISIONS TO DETAIL THE BRACING SYSTEMS AS HE SHALL PROVIDE FOR ALL DESIGNERS AND SHALL VERIFY SIZE AND LOCATION OF ALL DESIGNERS WITH OTHER PROJECT REQUIREMENTS. ANY DEVIATION FROM THE DESIGNERS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE BROUGHT TO THE ENGINEER'S ATTENTION FOR APPROVAL.
 - C. ALTERNATE CONNECTION DETAILS MAY BE USED IF SUCH DETAILS ARE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW AND APPROVAL. ACCEPTANCE IS GRANTED. HOWEVER, THE STRUCTURAL ENGINEER SHALL BE THE SILE ADJUDICATOR OF ACCEPTABILITY AND THE CONTRACTOR'S SO SHALL ANTICIPATE THE USE OF THOSE SPECIFIC DETAILS SHOWN ON THE DRAWINGS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF ANY ALTERNATE DETAILS WHICH HE PROPOSES.
 - D. WORK NOT INDICATED ON A PART OF THE DRAWINGS, BUT REASONABLY IMPLIED TO BE SHOWN, TO THAT SHOWN AT CORRESPONDING PLACES SHALL BE INCLUDED.
 - E. THE CONTRACTOR SHALL BE COMPLETELY RESPONSIBLE FOR THE SAFETY OF ADVISORY STRUCTURES, PROPERTY, AND THE PUBLIC. THE CONTRACTOR SHALL COMPLY WITH ALL FEDERAL, STATE, AND LOCAL REQUIREMENTS.
 - F. ANY MODIFICATION OR ALTERATION OF THESE CONSTRUCTION DOCUMENTS OR CHANGES IN CONSTRUCTION FROM THE INTENT OF THESE DOCUMENTS BY THE CONTRACTOR WITHOUT WRITTEN APPROVAL OF THE ENGINEER SHALL REMOVE ALL PROFESSIONAL AND LIABLE RESPONSIBILITY ON THE PART OF THE ENGINEER.
 - G. ALL CONTRACTORS ARE REQUIRED TO EXAMINE THE DRAWINGS AND SPECIFICATIONS CAREFULLY, VISIT THE SITE AND FULLY INFORM THEMSELVES AS TO ALL EXISTING CONDITIONS AND LIMITATIONS, PRIOR TO SUBMITTING THE PROPOSAL. FAILURE TO VISIT THE SITE AND FAMILIARIZE THEMSELVES WITH THE EXISTING CONDITIONS AND LIMITATIONS WILL IN NO WAY RELIEVE THE SUCCESSOR, BIDDOR FROM FURNISHING ANY MATERIALS OR SPECIFICATIONS WITHOUT ACCORDANCE WITH DRAWINGS AND SPECIFICATIONS WITHOUT ADDITIONAL COST TO THE OWNER.
 - H. DO NOT SCALE FROM DRAWINGS.
 - I. SET SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.
 - J. CONTRACTOR TO VERIFY BUILDING DIMENSIONS AND/OR ARCHITECTURAL DRAWINGS.
 - K. BETWEEN FLOOR DOWN LOCATIONS AND OUTFALL TO BE DETERMINED BY OTHERS.

CONCRETE NOTES

- 1.01 GENERAL
 - A. ADHERE TO AND COLD WEATHER CONCRETE SPECIFICATIONS, WHEN APPLICABLE.
 - B. ALL DIMENSIONS AND CONDITIONS MUST BE VERIFIED IN THE FIELD. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK.
 - C. CONTRACTOR SHALL TAKE NECESSARY PRECAUTIONS TO MAINTAIN STABILITY AND PREVENT UNDERMINING OF EXISTING FOUNDATIONS AT ALL TIMES.
 - D. NO FOUNDATIONS SHALL BE PLACED IN WATER OR ON FROZEN GROUND.
 - E. ALL FOOTINGS ARE TO BE EXCAVATED USING A BACKHOE WITH A SMOOTH TOOTHLESS CUTTING EDGE. FOOTING EXCAVATIONS ARE TO BE FINISHED BY HAND FOR NOT LESS THAN THE LAST SIX INCHES.
 - F. ALL FINISHED FOUNDATION EXCAVATIONS SHALL BE INSPECTED AND APPROVED BY THE STRUCTURAL ENGINEER OR HIS DESIGNATED BEFORE ANY CONCRETE IS PLACED.
 - G. THE OWNER, THE STRUCTURAL ENGINEER AND THEIR CONSULTANTS ASSUME NO RESPONSIBILITY FOR THE VALIDITY OF THE SUBSURFACE CONDITIONS DESCRIBED ON THE DRAWINGS, SPECIFICATIONS, TEST BODIES OR TEST RITS.
 - H. DETAILING OF CONCRETE REINFORCEMENT AND ACCESSORIES SHALL BE IN ACCORDANCE WITH AIS 315 - "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES," LATEST EDITION.

PART 2 - PRODUCTS

2.01 MATERIAL

- A. REINFORCING:
 1. SHALL BE GRADE 60, HEAVY DEFORMED BARS AND SHALL CONFORM TO ASTM A615. ALL REINFORCING BARS TO BE WELDED SHALL CONFORM TO ASTM A770.
 2. REINFORCING BARS MAY NOT BE WELDED EXCEPT WHERE DESIGNATED BY THE STRUCTURAL ENGINEER.
 3. ALL WELDED WIRE FABRIC (W.W.F.) SHALL CONFORM TO ASTM 108.
 4. ALL LAPS IN WELDED SHALL BE ONE INCH PLUS TWO INCHES AT SPICES. WELDED SHALL BE (BAR/W/AM/1.4 (TYP. UNK)
 5. CONCRETE PROTECTION FOR REINFORCEMENT SHALL BE PROVIDED AS FOLLOWS:
 - A. SURFACES CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH - 3 INCHES (CLEAN)
 - B. FORMED SURFACES EXPOSED TO EARTH OR WEATHER

3.01 FINISHES

- A. ALL EXTERIOR SURFACES OF CONCRETE SHALL BE FINISHED TO PROVIDE:
 1. FINISHES TO BE SPECIFIED BY THE ARCHITECT.
 2. FINISHES TO BE SPECIFIED BY THE ARCHITECT.
 3. FINISHES TO BE SPECIFIED BY THE ARCHITECT.
 4. FINISHES TO BE SPECIFIED BY THE ARCHITECT.
- B. STRUCTURAL FILL SHALL BE COMPACTED WITH ASTM D1557.
- C. SUBGRADE TO CONCRET OF AT LEAST 12" OF COMPACTED SAND OR GRAVEL. THIS MATERIAL SHALL BE:

1	100%	100%
2	80% - 100%	100%
3	60% - 100%	100%
4	0 - 50%	100%
- D. DRAINAGE STONE SHALL CONSIST OF CLEAN ANGULAR FRAGMENTS OF QUARRIED ROCK WITH UNIFORM QUALITY AND BE GRADED AS FOLLOWS:

NO. 20	100%
NO. 30	100%
NO. 40	100%
NO. 50	100%
NO. 60	100%
NO. 70	100%
NO. 80	100%
NO. 90	100%
NO. 100	100%

- 3.02 PLACEMENT
 - A. CONCRETE SLAB ON GRADE SHALL BE PLACED IN ONE CONTINUOUS PLACEMENT, WITH NO COLD JOINTS. IF COLD JOINTS ARE DESIRED, CONTRACTOR MUST PROVIDE PLACEMENT JOINTS AND JOINT DETAIL FOR DESIGNERS APPROVAL PRIOR TO PLACEMENT.
 - B. APPROVAL OF THE CONTRACTOR SHALL BE OBTAINED PRIOR TO PLACEMENT. THE SLAB HAS BEEN CURED FOR 30 DAYS.
 - C. WOOD FORMS SHALL BE USED TO FORM THE SLAB AND TO PROVIDE A BARRIER TO RAIN PREVENTION.
 - D. VAPOR BARRIER:
 1. 6 MIL POLYETHYLENE
 2. ACCEPTABLE TEST METHOD AS PERMS DETERMINED IN ACCORDANCE WITH ASTM D 1978
 3. BARRIER SHOULD NOT BE FURNISHED DURING CONSTRUCTION ACTIVITIES.
 4. EDGES SHOULD BE LAPPED A MINIMUM OF 6" TYPICAL, AND ALL CONCRETE EXPOSED TO THE WEATHER SHALL CONTAIN 50% - 75% AIR ENTRAINMENT.
 - E. ALL CONCRETE SHOULD BE PLACED IN ONE CONTINUOUS PLACEMENT, WITH NO COLD JOINTS.
 - F. ALL FOOTINGS SHALL BE PLACED UNIDIRECTIONALLY.
 - G. PILES OR CONCRETE PLACED IN SLABS ON GRADE SHALL NOT BE PLACED CLOSER THAN 3 INCHES FROM CENTER AND SHALL HAVE AN OUTSIDE DIAMETER LESS THAN 1/3 OF THE SLAB THICKNESS. ALUMINUM CONCRETE PILES SHALL NOT BE PLACED IN CONCRETE. NO CONCRETE SHALL BE PLACED IN SLABS ON GROUND.
 - H. ALL EXPOSED EDGES OF FORMING SHALL BE PROTECTED ON LEGS, SURFACES IN ALL DIRECTIONS, THROUGHOUT CLEAN LEAVE SURFACE PRIOR TO PLACING CONCRETE.
 - I. WHERE FORMATION ELEMENTS ARE TO HAVE TILT, ON BOTH SIDES, EACH SIDE SHALL BE FIELDED SMALLER THAN 1/4" PER FOOT.
 - J. CONTRACTOR SHALL PROVIDE CONTINUOUS DRAINAGE BY CHANNEL METHOD TO CONTROL SURFACE AND UNDERGROUND WATER AS REQUIRED DURING CONSTRUCTION, SO THAT ALL EXCAVATIONS ARE DRY.
 - K. ALL LOCATIONS WHERE REDWOOD IS REMOVED SHALL BE FREE DRAINING SO THAT NO POCKETS OF CONCRETE REMAINS WATER COLLECT.
 - L. ALL EXPOSED EDGES OF CONCRETE WEATHERS SHALL BE CHAMFERED 3/4" MINIMUM.
 - M. INTERIOR CONCRETE SLABS SHALL BE MOST CURED CONTINUOUSLY FOR 7 DAYS.
 - N. CONCRETE SHALL BE MAINTAINED ABOVE 50 DEGREES F, AND IN MOST CASES ABOVE 60 DEGREES F, THROUGHOUT THE CURE PERIOD.
 - O. ALL DIMENSIONS IN CONCRETE INCLUDING ANCHOR BOLTS SHALL BE PLACED AS SHOWN ON THE DRAWINGS.
 - P. CONSOLIDATE ALL CONCRETE WITH A VIBRATOR OR OTHER MEANS RECOMMENDED BY A.S.T.M. UNDEVELOPED SURFACES WILL NOT BE PERMITTED.
 - Q. SEE ARCHITECTURAL DRAWINGS FOR DOOR AND WINDOW OPENINGS, DAMS, HORIZONTAL ANCHOR BOLTS, ETC. ALL DIMENSIONS SHALL BE AS SHOWN ON ARCHITECTURAL DRAWINGS FOR LOCATIONS OF DAMS, SLABS, ETC. UNLESS OTHERWISE NOTED.

- 3.03 CONTROL JOINTS
 - A. PLACE CONTROL JOINTS WHERE SHOWN ON THE PLANS. SLAB REINFORCING SHALL BE PLACED WITH CONTROL JOINTS SHOULD BE SQUARE OR REAR CUT JOINTS IN CONCRETE. AT EACH CONTROL JOINT LOCATION, AS SHOWN AS SLAB WILL SUPPORT THE WEIGHT OF THE SOFT-CUT SAW AND OPERATOR. (NOMINALLY WITHIN 2 HOURS AFTER FINISHING AT CONTROL JOINT LOCATION). THE DEPTH OF CUT SHALL BE 1" TO 1 1/2". USE 3/4" DIAMETER SERRATED CLOSED END BARS TO PREVENT SPALLING OF THE CONCRETE.
 - C. SEE SPECIFICATIONS FOR THE CONTROL JOINTS.
 - 3.04 CONCRETE TESTING
 - A. FOUR CONCRETE TEST CYLINDERS TO BE SET ASIDE FOR LABORATORY TESTING EITHER EVERY 30 CUBIC YARDS FOR ONE CONTINUOUS PLACEMENT OR EACH NEW DAY PLACEMENT, WHICH EVER PRODUCES THE MOST CYLINDERS.
 - B. THE TESTS FOR THE FOUR CONCRETE CHANGERS ARE TO CONSIST OF A COMPRESSIVE STRENGTH TEST AND (1) HOLD CURED, THE TESTS SHALL INCLUDE TEMPERATURE OF THE CONCRETE AND THE COMPRESSIVE STRENGTH OF EACH SPECIMEN.
 - C. ALL TESTING SHALL BE PERFORMED BY A LABORATORY IN COMPLIANCE WITH ASTM C896.

DESIGN LOADING

- PART 1 - LOADING
 - 1.01 DESIGN SOIL BEARING PRESSURE
 - A. THE DESIGN SOIL BEARING PRESSURE IS ASSUMED TO BE 2500 PSF.
 - 1.02 DEAD LOAD
 - A. 1ST FLOOR-10 PSF
 - B. 2ND FLOOR-10 PSF
 - C. ROOF-15 PSF
 - 1.03 LIVE LOAD
 - A. 1ST FLOOR-40 PSF
 - B. 2ND FLOOR-40 PSF

PART 1 - GENERAL

- 1.01 STANDARD SPECIFICATIONS:
 - A. FABRICATION, ERECTION AND WELDING IN ACCORDANCE WITH THE SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS, ALLOWABLE STRESS DESIGN AND PLASTIC DESIGN ADOPTED JUNE 1989, INCLUDING ALL PUBLISHED SUPPLEMENTS.
 - B. WELDING SHALL BE DONE IN ACCORDANCE WITH THE AMERICAN WELDING SOCIETY "STRUCTURAL WELDING CODE," (LATEST EDITION).
 - C. ALL WELDING SHALL BE MADE WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER.
 - D. NO CHANGE IN SIZE OR POSITION OF THE STRUCTURAL ELEMENTS SHALL BE MADE WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER.
- 1.02 SUBMITTALS
 - A. SUBMIT SHOP DRAWINGS FOR REVIEW.
- 1.03 PRODUCT HANDLING:
 - A. STORE STRUCTURAL STEEL MEMBERS AT THE PROJECT SITE ABOVE GROUND ON PLATFORMS, SHOCKS, OR OTHER SUPPORTS.
 - B. FINISHED STEEL FROM COMPOUND.

PART 2 - PRODUCTS

- 2.01 MATERIALS:
 - A. STEEL SHAPES, BARS, AND PLATES WILL BE ASTM A-36.
 - B. STRUCTURAL TUBING AND COLLARS WILL BE ASTM B20.
 - C. ANCHOR BOLTS WILL BE ASTM A-307, GRADE A.
 - D. HIGH STRENGTH BOLTS WILL BE ASTM A-325, TYPE 1 OR 2.
 - E. WELDING WILL BE PERFORMED WITH 7018 E7018 OR E8018 RODS.
 - F. SHOP PAINT TO BE THINCO 80 RED METAL PRIMER, OR EQUIVALENT APPROVED BY THE ENGINEER.
 - G. NON-SHOP GROUT SHALL BE 7000 PSI (MIN.) COMPRESSIVE STRENGTH.
 - H. PLATE NON-SHOP GROUT UNDER ALL COLUMN BASE PLATES BEING ADDED ANY VERTICAL LOADS.
 - I. ALL WORK AND MATERIALS PERTAINING TO METAL DECK SHALL CONFORM TO STEEL DECK INSTITUTE CODES, SPECIFICATIONS AND RECOMMENDATIONS. METAL DECK SHALL BE GALVANIZED.
- PART 3 - ERECTION
 - 3.01 FABRICATION:
 - A. FABRICATE STRUCTURAL STEEL IN ACCORDANCE WITH THE REQUIREMENTS OF THE DRAWINGS.
 - B. SHOP PAINT SURFACES OF ALL STEEL WORK WITH FABRICATOR'S STANDARD RUST INHIBITIVE PAINT.
 - C. UNLESS OTHERWISE NOTED, ALL STEEL SHALL BE MADE BY WELDING.
 - D. ALL STRUCTURAL STEEL SHALL BE SHOP FINISHED EXCEPT THAT STRUCTURAL STEEL TO BE PERFORMED SHALL NOT BE FINISHED.
 - E. CONNECTIONS SHOWN ON THESE DRAWINGS ARE GENERALLY CONNECTIONS. THEY ARE INTENDED TO DEFINE THE SPATIAL RELATIONSHIP OF THE FINISHED MEMBERS AND SHOW A FEASIBLE METHOD OF MAKING THE CONNECTION. A LICENSED PROFESSIONAL ENGINEER IN THE STATE OF ILLINOIS SHALL BE RETAINED BY THE FABRICATOR SHALL DESIGN ANY CONNECTION THAT IS NOT SHOWN, OR IS NOT COMPLETELY DETAILED ON THE STRUCTURAL DRAWINGS.
 - F. SUBSTITUTION, ALTERATIONS OF CONNECTIONS SHALL NOT BE MADE WITHOUT PRIOR WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER.
 - G. WELDING CONNECTION PLATE THICKNESS SHALL BE 1/4 INCH, UNLESS OTHERWISE NOTED.
 - H. FOR ALL NON-COLLAR BEAMS, THE BEAM-TO-BEAM AND BEAM-TO-COLUMN CONNECTION SHALL DEVELOP THE END BEHAVIOR OF THE CONNECTED BEAM. THE END BEHAVIOR OF THE CONNECTED BEAM SHALL BE ASSUMED EQUAL TO THE BEAM ASSUMING FULL LATERAL SUPPORT, AS GIVEN IN PART 2 (BEAMS AND GIRDERS) OF AISC MANUAL, 9TH EDITION. A MINIMUM SHEAR CAPACITY OF 12 KIPS SHALL BE PROVIDED FOR ALL BEAMS GREATER THAN 8" DEEP AND 8 INCHES FOR BEAMS 8" DEEP OR LESS.
 - I. HOLES, CORNERS, ETC. REQUIRED FOR WORK OF OTHER TRADES SHALL BE SHOWN ON THE SHOP DRAWINGS AND MADE IN THE SHOP. CUTS OR BURNING OF HOLES IN STRUCTURAL STEEL MEMBERS IN THE FIELD WILL NOT BE PERMITTED, UNLESS APPROVED IN WRITING BY THE STRUCTURAL ENGINEER.
 - J. ALL BEAMS AND COLLARS EXPOSED IN MASONRY OR CONCRETE SHALL BE COVERED WITH A COAT, TWO COATS, 1/8" THICK OR SHALL BE GALVANIZED.

PART 3 - ERECTION

- 3.02 ERECTION:
 - A. THE STRUCTURAL STEEL SHALL BE ERECTED PLUMB AND TRUE TO THE LINES AND ELEVATIONS INDICATED ON THE DRAWINGS.
 - B. ERECTION TRADESMEN SHALL BE TRAINED TO THE LATEST

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 - B. WELDING SHALL BE DONE IN ACCORDANCE WITH THE AMERICAN WELDING SOCIETY "STRUCTURAL WELDING CODE," (LATEST EDITION).
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PART 2 - PRODUCTS

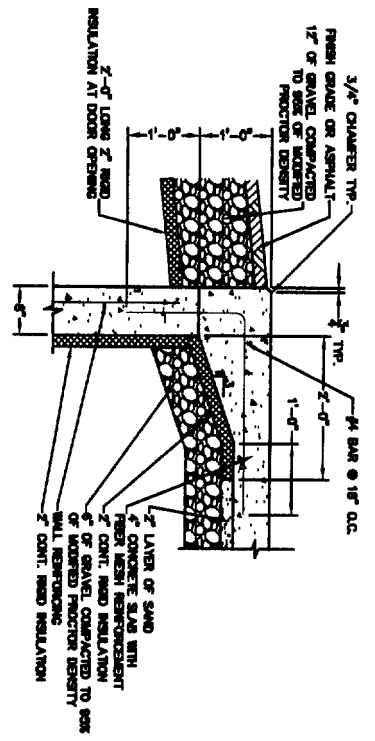
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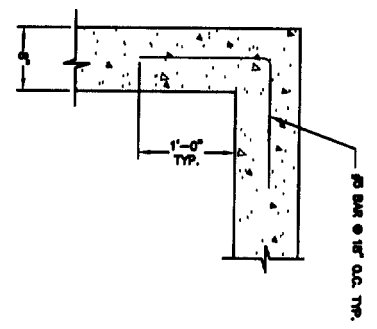
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WOOD

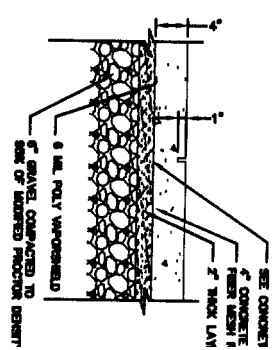
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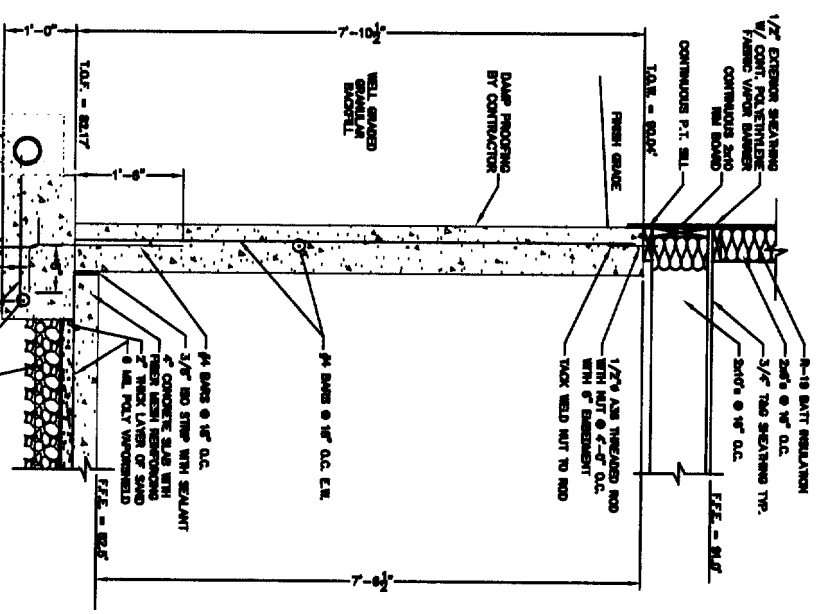
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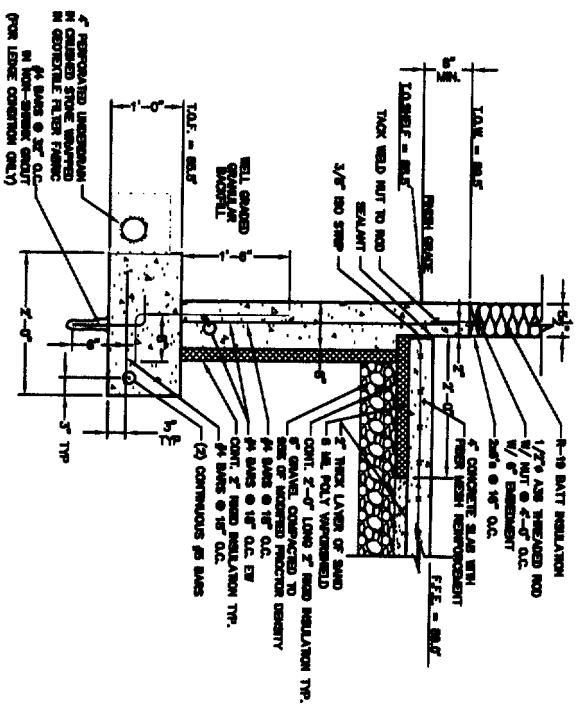
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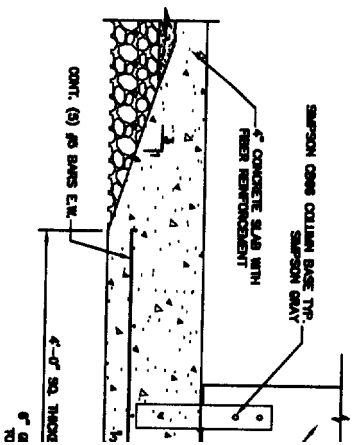
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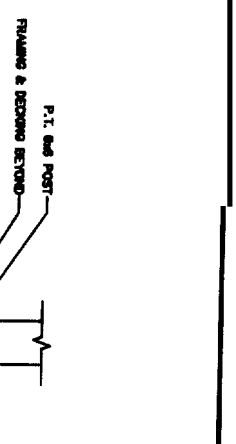
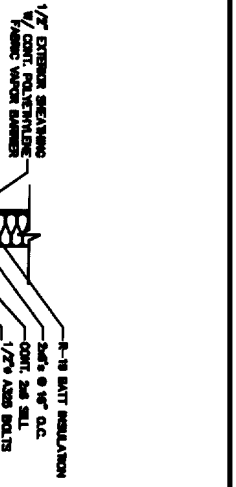
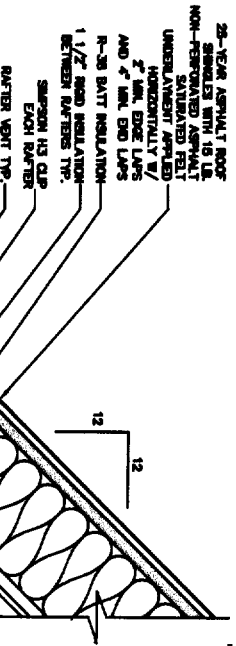
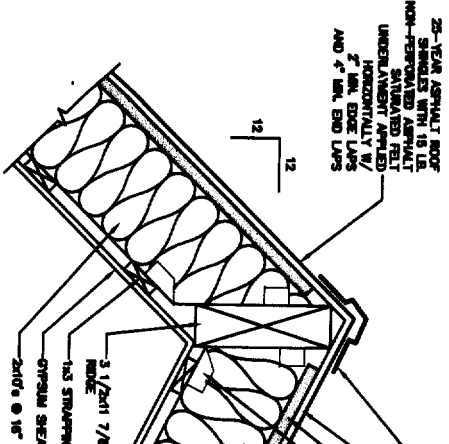
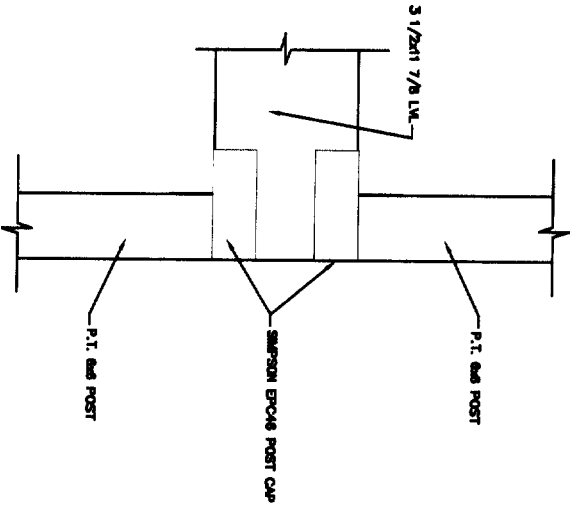
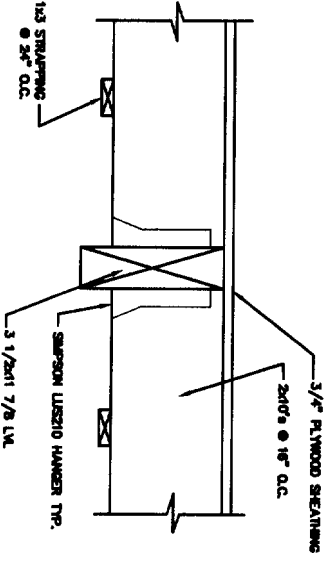
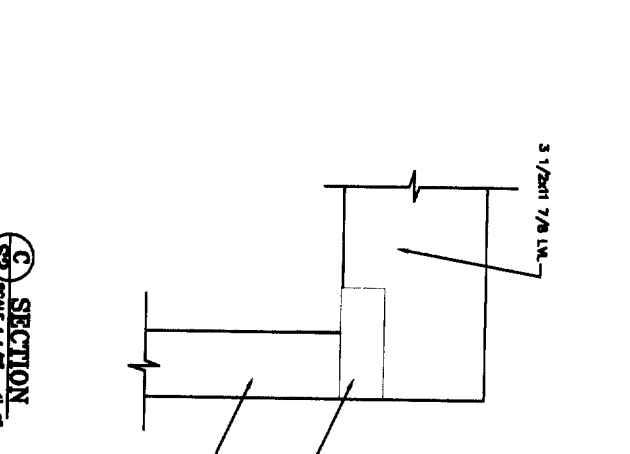
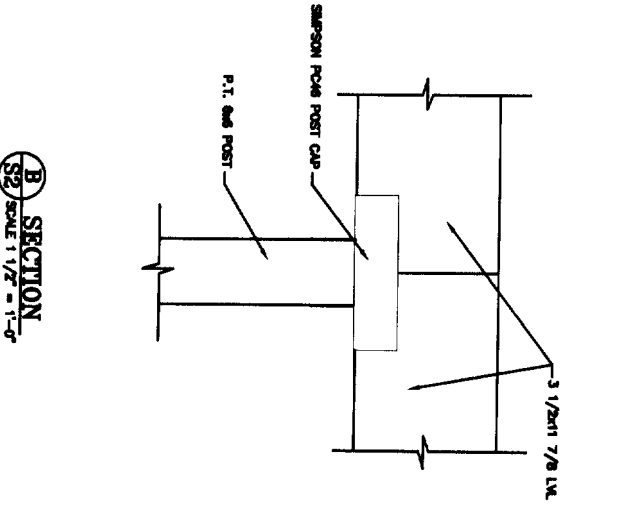
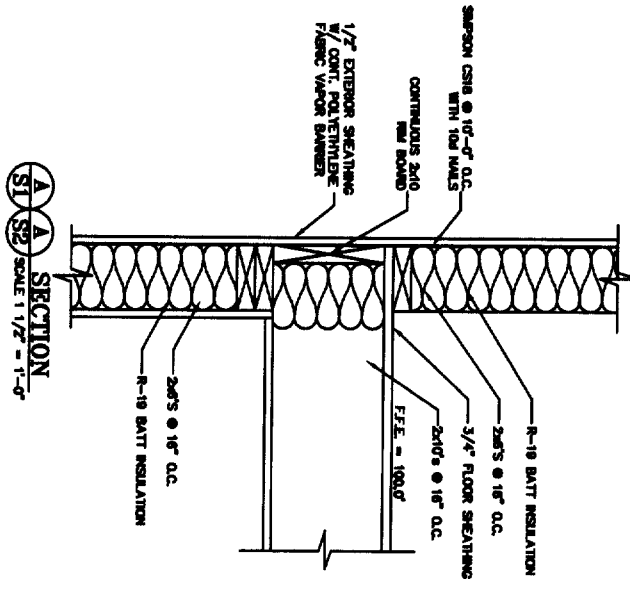


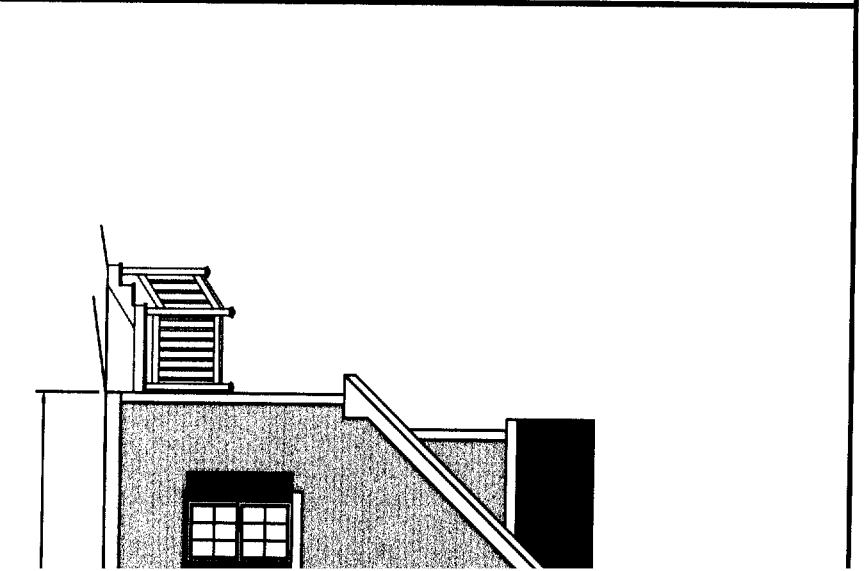
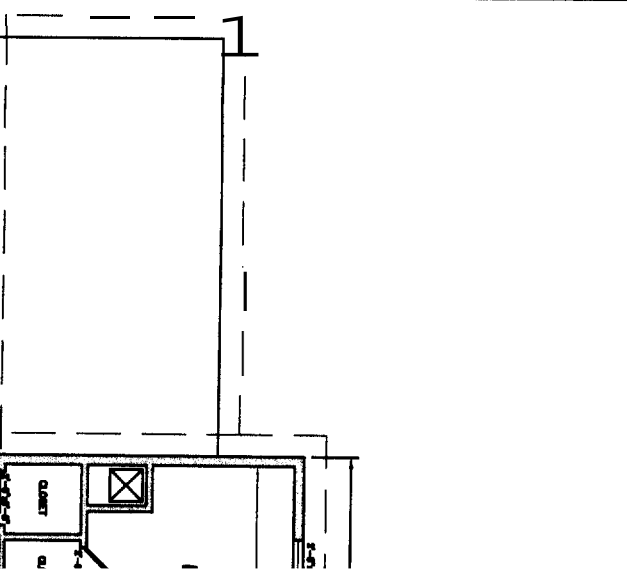
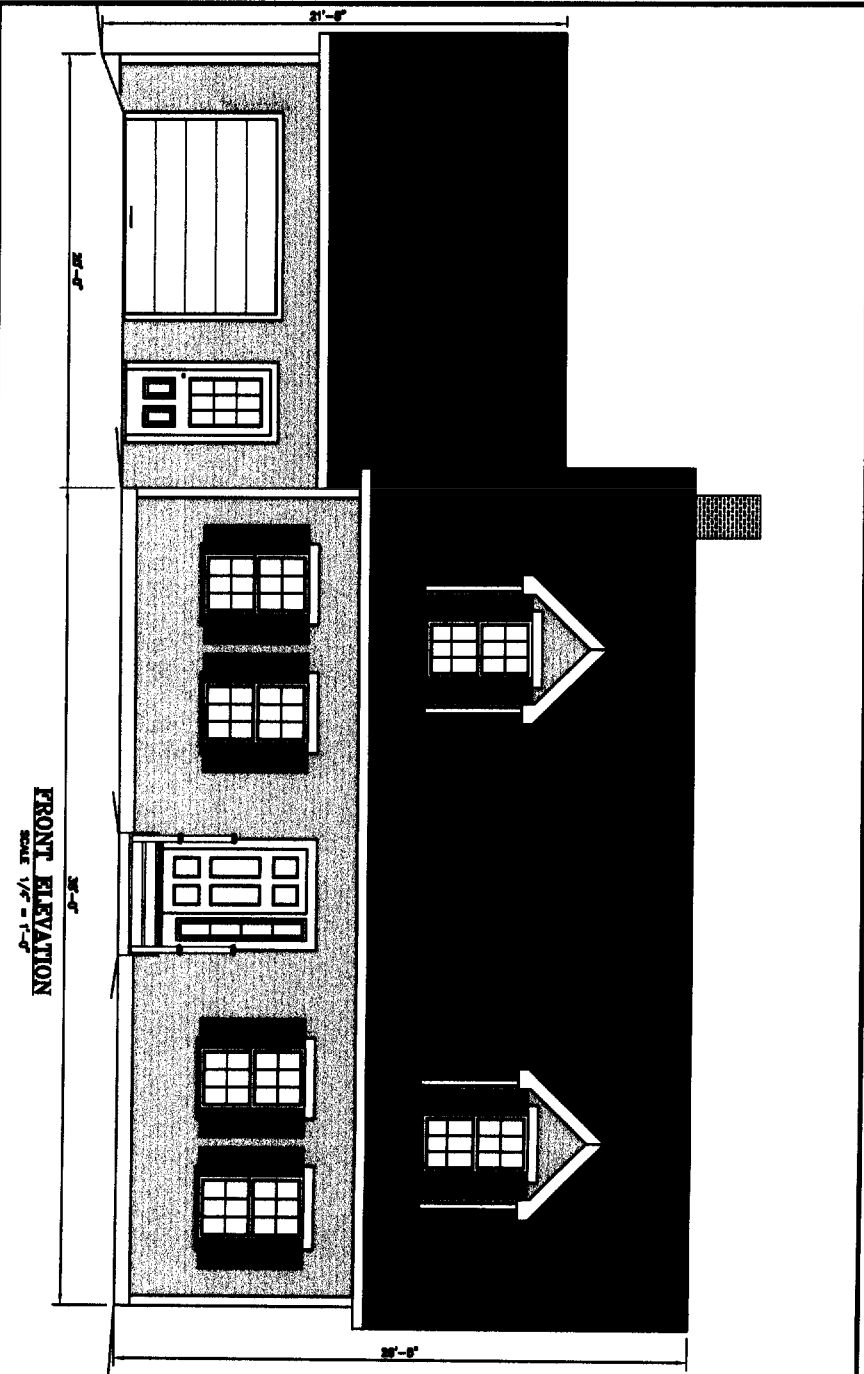
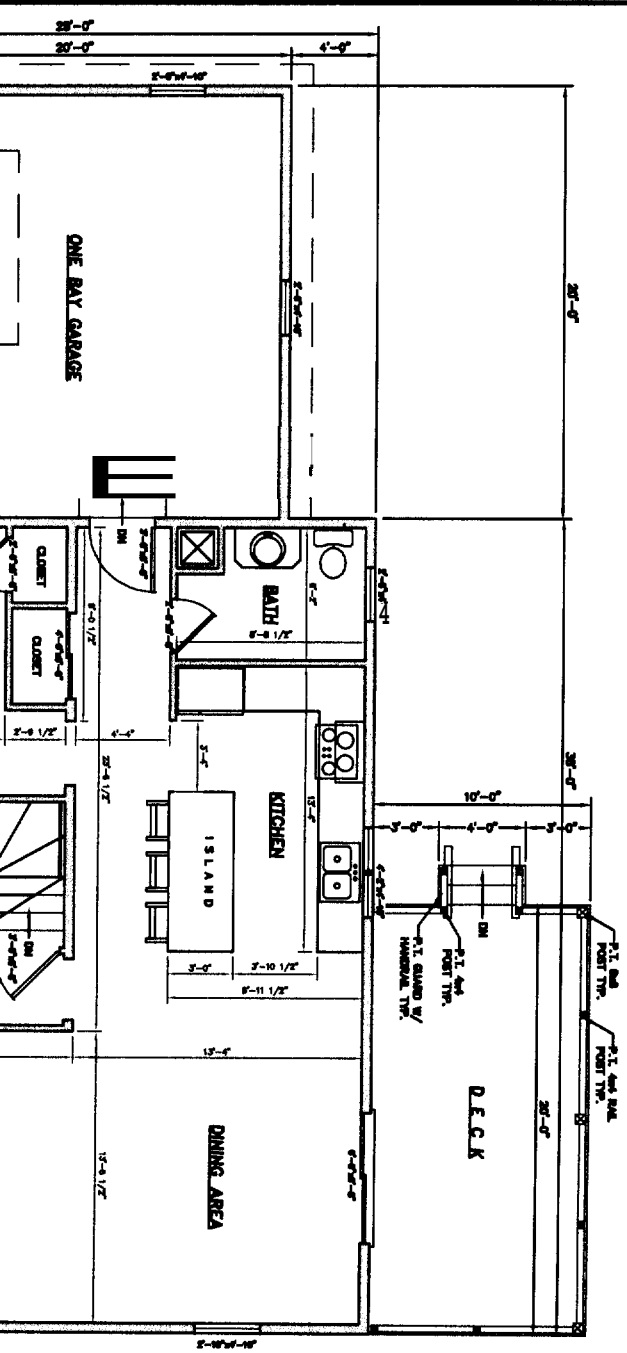
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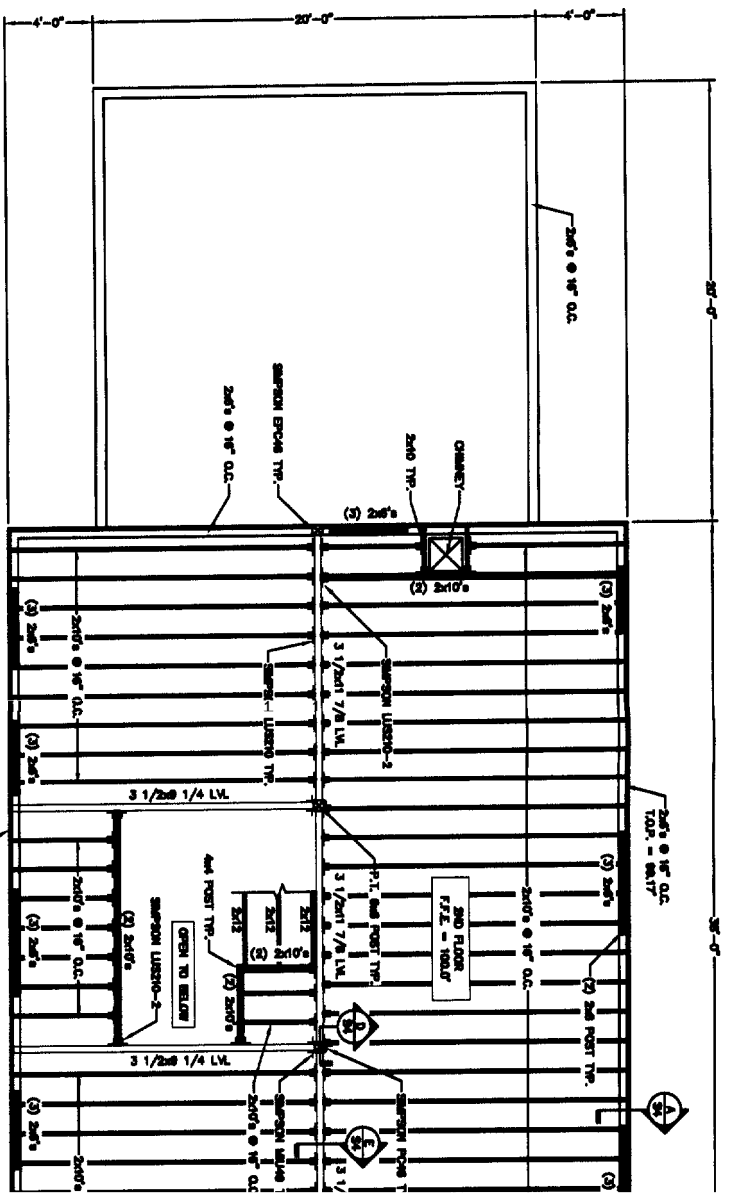


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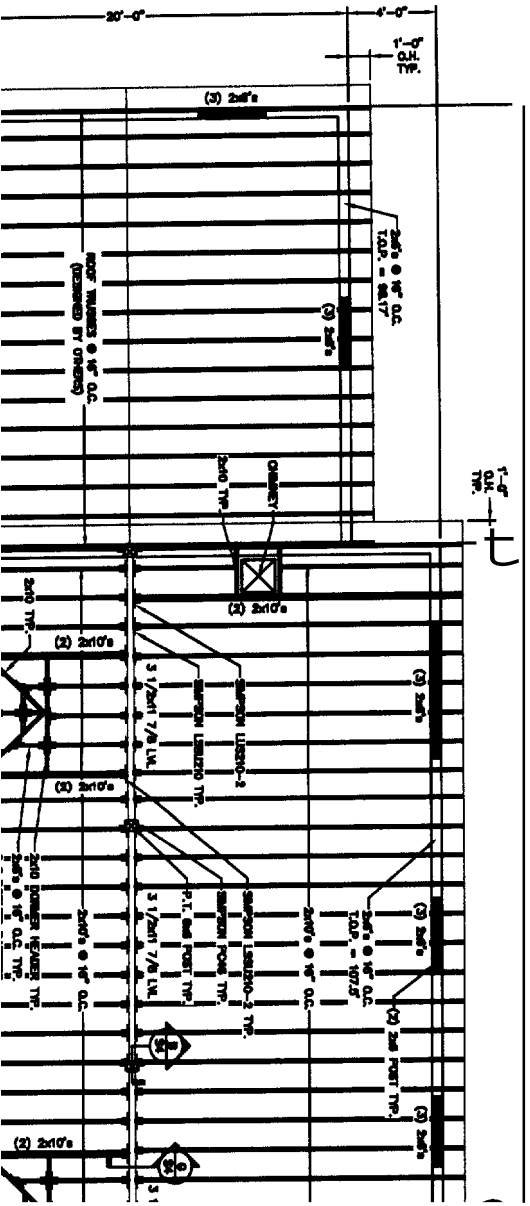


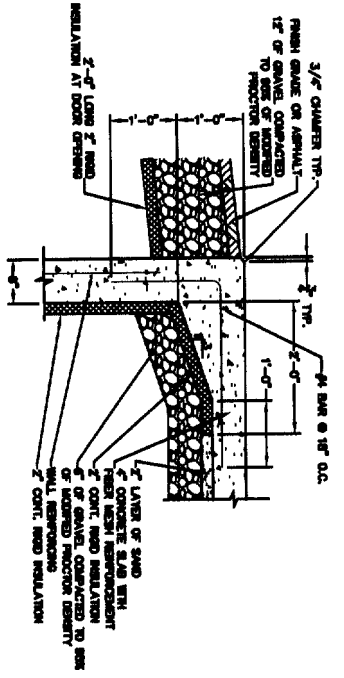




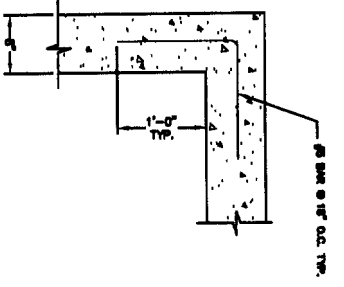
2ND FLOOR FRAMING PLAN

SCALE 1/8" = 1'-0"

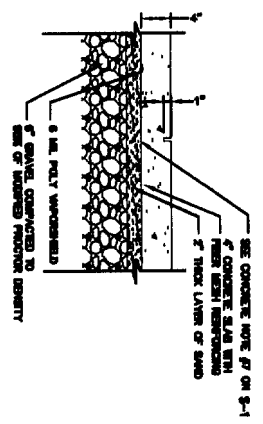




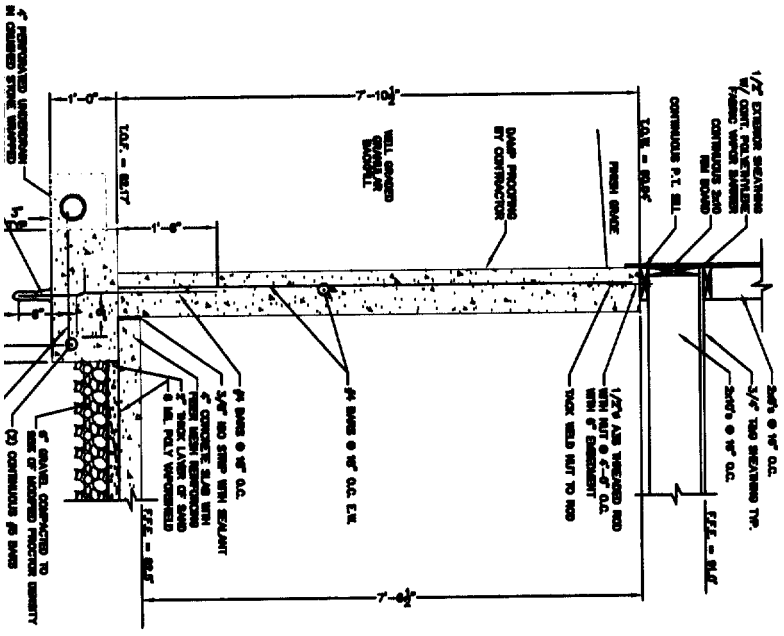
A SECTION
 SCALE 3/8" = 1'-0"



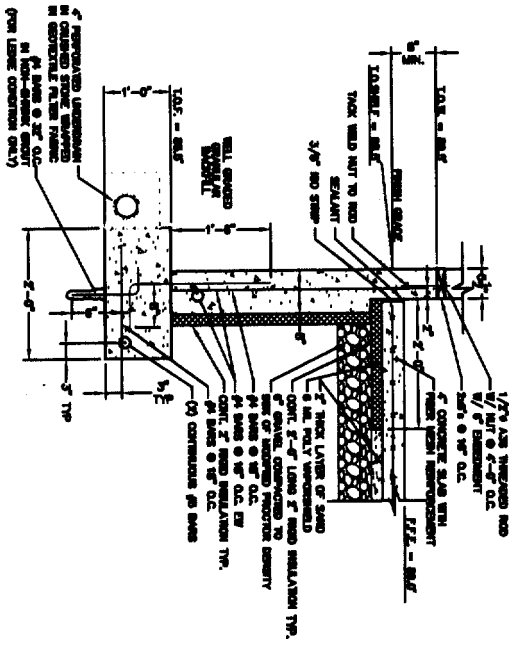
B SECTION
 SCALE 1" = 1'-0"



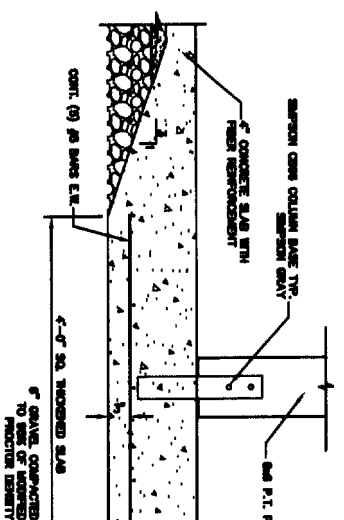
C SECTION
 SCALE 1" = 1'-0"



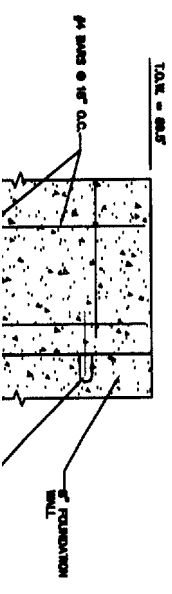
D SECTION
 SCALE 3/8" = 1'-0"



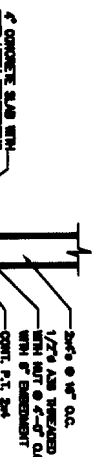
E SECTION
 SCALE 3/8" = 1'-0"



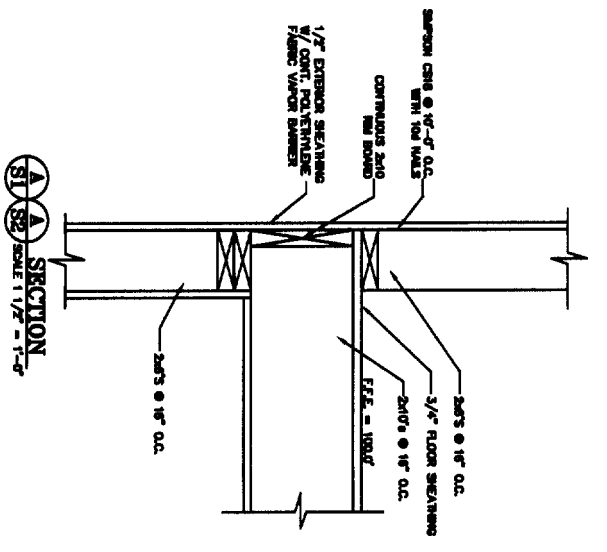
F SECTION
 SCALE 1" = 1'-0"



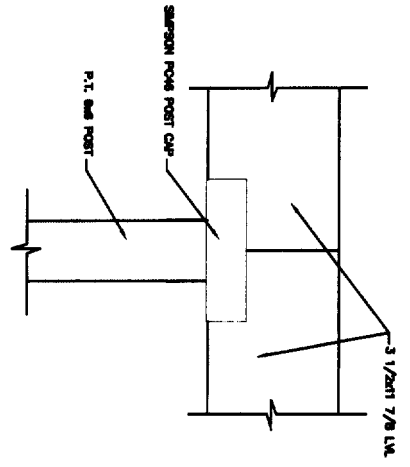
G SECTION
 SCALE 3/8" = 1'-0"



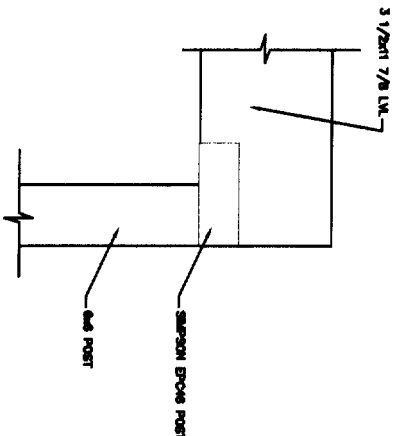
H SECTION
 SCALE 1" = 1'-0"



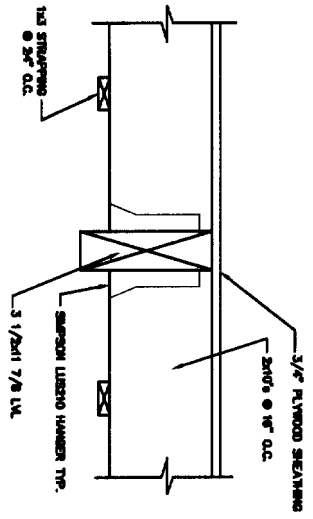
A SECTION
SCALE 1 1/2" = 1'-0"



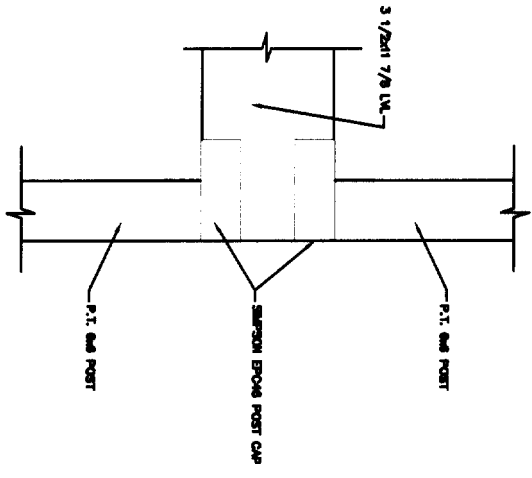
B SECTION
SCALE 1 1/2" = 1'-0"



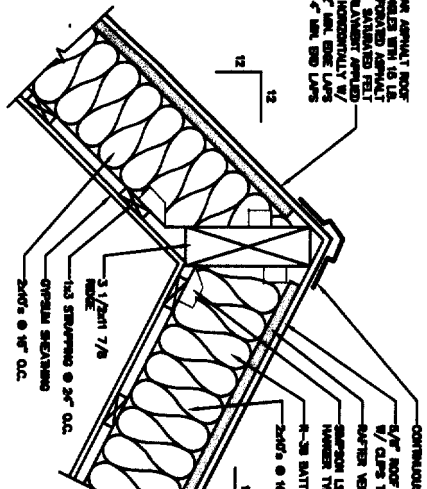
C SECTION
SCALE 1 1/2" = 1'-0"



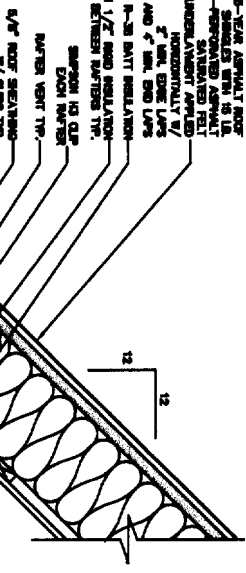
D SECTION
SCALE 1 1/2" = 1'-0"



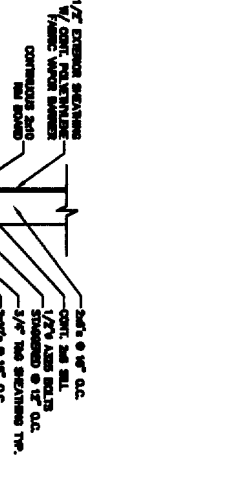
E SECTION
SCALE 1 1/2" = 1'-0"



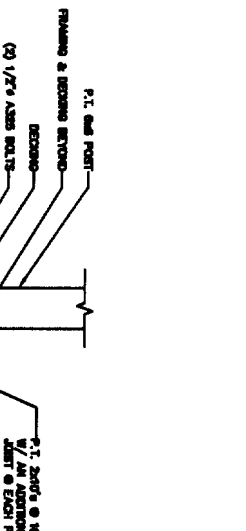
F SECTION
SCALE 1 1/2" = 1'-0"



G SECTION
SCALE 1 1/2" = 1'-0"



H SECTION
SCALE 1 1/2" = 1'-0"



I SECTION
SCALE 1 1/2" = 1'-0"