



General Notes:

- Structural drawings shall be used in conjunction with the architectural, mechanical, electrical, site, shop drawings and specifications.
- Governing building code is the Maine State Building Code.

B. Shop Drawings
 Shop drawings, prepared by general contractor for reinforcing steel, concrete mix design, joint deck and formwork, shall be submitted to the architect for review and approval before construction can proceed. Erection shall be made from approved shop drawings only. Revised shop drawings can proceed. Reproduction of these contract drawings for use on a shop order shall be the responsibility of the contractor. Changes, substitutions, or omissions made by the general contractor or their subcontractors to the contract documents shall be submitted to the architect for approval. The submission shall include and approval procedure shall be considered not approved.

C. Soil Conditions and Structural Fill
 All footings shall be carried to the depths shown and deeper, if required, and shall rest on undisturbed soil or compacted structural fill having a net allowable soil bearing pressure of 2,000 SF. Subsurface soil conditions shall be as shown on the geotechnical report. Construction shall be in accordance with the geotechnical report.

D. Concrete
 1. All reinforcement to be billet steel deformed bars conforming to ASTM A615 grade 60.
 2. Welded wire fabric shall conform to Standard specifications for Welded Steel Wire Fabric for Concrete Reinforcement (ASTM A185) and shall be supplied in flat sheets. All bars shall be lap spliced with the geotechnical report.
 3. Corners, lapped or necessary splices, splices staggered and hooked at discontinuous laps shall be 49 diameters of the larger bar.
 4. Provide all four sides of openings in concrete walls, 1-#3 bar, each face, extending 2'-0" beyond openings.
 5. All reinforcing bars are to be cold bent.
 6. All concrete shall attain a minimum compressive strength of 3,000 psi at 28 days except where noted in the specifications.
 7. Concrete against the earth: 3"
 8. Formed concrete exposed to earth or weather #5 or smaller 1/2" #6 or larger: 2"
 9. Concrete not exposed to earth or weather #5 or smaller 1/2" #6 or larger: 1 1/2"
 10. All concrete work and detailing shall comply with the latest specifications and recommendations of the ACI.

E. Steel
 1. All W sections shall conform to ASTM A992 Grade 50. All other sections shall conform to ASTM A36.
 2. All exposed steel shall be hot dipped galvanized.
 3. Steel headers for lintels not labeled shall consist of the following: Masonry openings to 4'-0" Lx3x1/2" (for each 4" width of block). Masonry openings 4'-0" to 6'-0" Lx3x1/2" Lx3x1/2" (for each 4" width of block). Masonry openings greater than 6'-0" Lx3x1/2" Lx3x1/2" (for each 4" width of block). Masonry openings shall be made by high strength bolts or welding. All beam/beam and beam/column connections shall be designed for a reaction Wc/2L as described by the AISC manual.
 4. Shop and field connections shall be designed for a reaction Wc/2L as described by the AISC manual.
 5. Structural bolts shall be high strength bolts conforming to ASTM A325 bearing type, conforming to the provisions of the Specifications for Structural Joints Using ASTM A325 or A490 Bolts, latest edition.
 6. Welding shall be performed only by AWS certified welders and shall conform to provisions of the Structural Welding Code-Steel of the American Welding Society, latest edition.

F. Wood
 1. Framing members shall have the properties of Spruce-Pine-Fir #2 or better (Fb=875 psi, E=1,400,000 psi) except that wood posts shall have the properties of Spruce-Pine-Fir #1 or better (Fc=700 psi, E=1,300,000 psi).
 2. All engineered lumber shall be installed in accordance with the manufacturer's specifications and recommendations.
 3. Timberstrand shall have the following minimum properties:
 Timberstrand Fb=1700 psi E=1,300,000 psi
 Microlam Fb=2600 psi E=1,300,000 psi
 4. All other framing shall be as specified in the specifications.
 5. Use stainless steel or galvanized nails for all connections exposed to the weather.

G. Concrete Block
 1. Concrete block shall be normal or light weight, hollow load-bearing concrete masonry units conforming to ASTM C 90, Grade N with a minimum compressive strength of 3,000 psi.
 2. Mortar for masonry units shall comply with ASTM C 270 Type S.
 3. Mortar Mix (Per 100 lbs. of Mortar):
 Portland Cement: 3 (Vol. %)
 Hydrated Lime: 1/4 to 1/2 Max
 Shovel Count (Sand): 2 1/2 - 25 @ 2 1/4 to 28-34 @ 3
 4. Horizontal reinforcement shall be #3 times the span. Vertical reinforcement shall be #4 times the span. Ducts shall be #3 times the span. Ducts shall be #4 times the span.
 5. All openings in masonry walls shall be reinforced with (one) #6 vertical bar, extending 2'-0" above head height, fully grouted in the cells.
 6. Provide one vertical bar (#6) each side of all control joints, full height and doweled into foundation.
 7. Hook bond beam reinforcing 16" at ends (where applicable).

H. Open Web Steel Joists
 1. Joists and joist girders designated on the drawings shall conform to standard specifications for series K joists adopted by the Steel Joist Institute and the American Institute of Steel Construction.
 2. All joists to be pointed one end or primer, touch up all welds.
 3. Provide bottom chord extension at all columns. All other joists to have standard bottom chord extensions for approval.
 4. Joist manufacturer is to be a member of the Steel Joist Institute.

I. Design Loads
 1. Design Live Loads
 Roof 42 PSF + Dift where applicable (Ground Snow Load = 60 PSF)
 First Floor 100 psf
 Basic Wind speed = 90 MPH (3 Second Gust)
 Exposure Category: B
 Seismic Hazard Exposure Group: I
 Seismic Performance Category: C (Ss = 0.35, S1 = 0.1)
 Soil Profile Type: D
 Basic Seismic Force Resisting System: Reinforced masonry shear walls
 Response Modification Factor: R = 2 1/2
 Deflection Amplification Factor: Ca = 1 3/4
 Analysis Procedure Used: Simplified Analysis Procedure

J. Construction Site Visits
 1. Site visits to review construction progress will be required by this office at the following intervals, or a minimum:
 A. Before foundation placement.
 B. Before foundation wall placement.
 C. After the structural steel is erected and roof deck stretched.
 D. Order to view the construction, or 48 hours notification (including Saturdays, Sundays and Holidays) in order to view the construction.
 3. This office will not be responsible for delays in construction due to the failure to provide adequate notice of observation. In addition, should the contractor decide to continue work without the previous notice with the Design Drawings, will be unable to sign off that the previous work was done in conformance

Sheet Notes:
 1. Coordinate roof drains and sewer line locations with A1.1, P1.1 and Civil Drawings.
 2. Verticality and bathroom slopes slope to drain. Coordinate with the Architectural Drawings.
 3. Bottom of footing elevation noted as (EL-X'-X").
 4. C.I. denotes location of control joint.

MOESER & ASSOCIATES
 HARRARD, MA 978-456-6965
ARCHITECT

DRAWINGS/SPECIFICATIONS BY:
 ■ LANDLORDS CONSULTANT
 ■ LANDLORDS CONTRACTOR (TURKEY CONSTRUCTION)
 ■ ALL CONSTRUCTION WORK, UNLESS NOTED OTHERWISE BY:
 ■ WALGREENS CONTRACTOR

STRUCTURAL ENGINEER:
DWD ENGINEERING, INC.
 5 Michael Road
 E. Bridgewater, MA
 TEL. 508.378.9802
 FAX. 508.378.2922

| NO. | DATE | BY | DESCRIPTION | CONST |
|---|------|----|-------------|-------|
| REVISIONS | | | | |
| CERTIFICATION AND SEAL | | | | |
| 1. HERBERY CERTIFY THAT THE DRAWING WAS PREPARED BY ME OR UNDER MY DIRECT AM I A DULY REGISTERED ARCHITECT OR ENGINEER UNDER THE LAWS OF THE STATE OF MASSACHUSETTS AS SIGNIFIED BY MY HAND AND SEAL. | | | | |
| FISCAL 2007 CRITERIA - STORE # 12325 | | | | |
| WALGREENS (MWO) WASHINGTON & ALLEN AVES. PORTLAND, MAINE | | | | |
| DRAWING TITLE FOUNDATION PLAN | | | | |
| CADD PLOT: SCALE: AS NOTED DRAWING NO. | | | | |
| VOID PLOT: DRAWN BY: RGC | | | | |
| RELEASED TO CONSTRUCTION: DATE: 03/30/09 | | | | |
| REVIEWED BY: S1 | | | | |
| DWD | | | | |