				DEDIN	17 LOOLUT		
				PERM	IT ISSUE	D	
City of Portland, Maine	- Building or Use	Permit Applicatio	n Per	mit No:	Issue Date:	CBL:	
389 C gress Street, 04101				02-0303APP	2002	333 K016001	
Location of Construction:	Owner Name:		Owner	Address:		Phone:	
159 Beverly St	Cpw Developr	ment Corporation	1 Per	cy Hawkes Rd	PORTI AI	ND	
Business Name:	Contractor Name	:	Contra	actor Address:	L VII L AI	Phone	
	John Ross Hea	ating	41 M	liddle Rd. Cumt	berland	2078294248	
Lessee/Buyer's Name	Phone:		Permit Type: Zone HVAC				
Past Use:	Proposed Use:		Permi	t Fee: Co	ost of Work:	CEO District:	
Single Family	Single Family			\$30.00	\$30.00	1	
			FIRE			ECTION: Group: P3 Type: Heating	
Proposed Project Description:						Maria	
Install Heating System w/ 1 2	75 Gallon Oil Tank		Signature: Signature: PEDESTRIAN ACTIVITIES DISTRICT (P.A.D.)				
			PEDE	STRIAN ACTIVI	TIES DISTRICT	(P.A.D.)	
			Action	n: Approved	Approved	w/Conditions Denied	
			Signat	ture:		Date:	
Permit Taken By:	Date Applied For:	Zoning Approval					
mjn	04/04/2002						
1. This permit application d	oes not preclude the	Special Zone or Revi	ews	Zoning A	Appeal	Historic Preservation	
Applicant(s) from meetin Federal Rules.	g applicable State and	Shoreland	Variance			Not in District or Landmark	
2. Building permits do not in septic or electrical work.	nclude plumbing,	Wetland	K	Miscellaneo	ous	Does Not Feedure Review	
<ol> <li>Building permits are void within six (6) months of t</li> </ol>		Flood Zone	I.	Conditional	l Use	Requires Review	
False information may in permit and stop all work.	validate a building	Subdivision	1	Interpretation		Approved	
		Site Plan		Approved		Approved w/Conditions	
		Maj 🛄 Minor 🛄 MM	1	Denied		Denied	
		Date:		Date:		Date:	

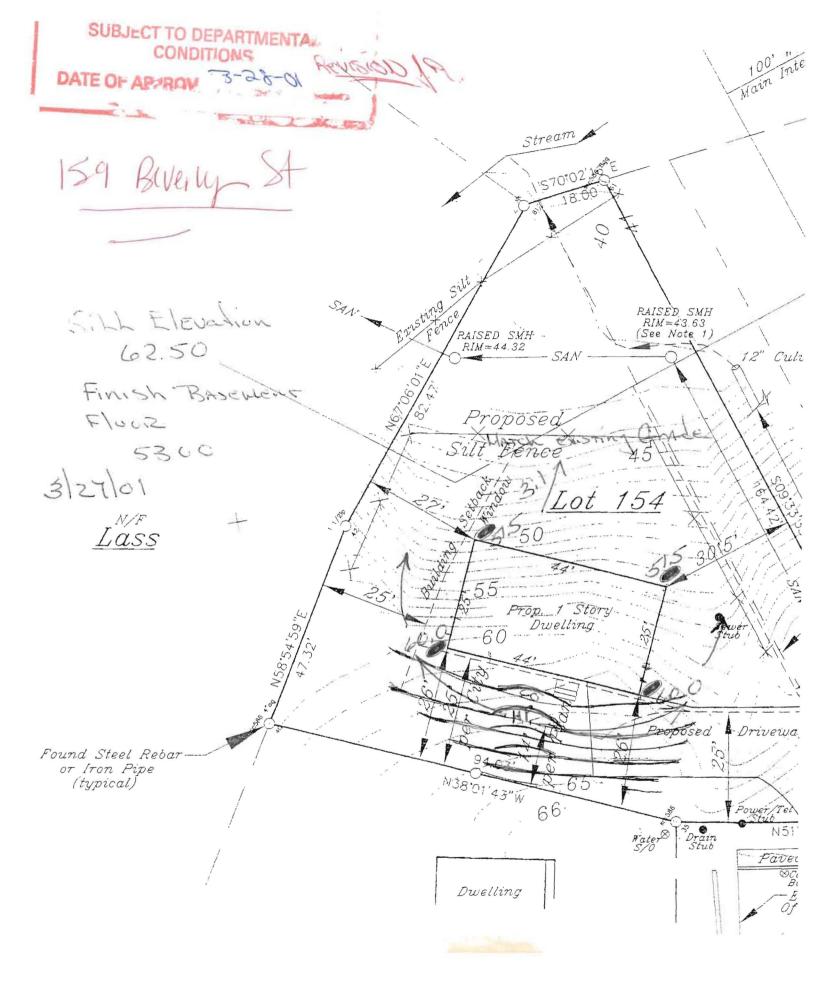
#### CERTIFICATION

I hereby certify that I am the owner of record of the named property, or that the proposed work is authorized by the owner of record and that I have been authorized by the owner to make this application as his authorized agent and I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in the application is issued, I certify that the code official's authorized representative shall have the authority to enter all areas covered by such permit at any reasonable hour to enforce the provision of the code(s) applicable to such permit.

SIGNATURE OF APPLICANT	ADDRESS	DATE	PHONE
		D	DUCONIC

	333 2016 02-0303 SIGN WITH INK
HEATING OR PO	APP
A	mont of Jub = # 1500 OF PORTLAND
To the INSPECTOR OF BUILDINGS, PORTLAND, ME.	all the following heating, cooking or power equipment in
Location <u>159</u> <u>Beverly Street</u> Us Name and address of owner of appliance <u>C. P. W.</u> Devel	e of Building Date <u>4-4-02</u>
Installer's name and address John Ross Heating 11 Middle Road, Cumberland	Me. 04021 Telephone 829-3066
Location of appliance:	Type of Chimney:
Basement 🖸 Floor	Masonry Lined
□ Attic □ Roof	Factory built
Type of Fuel:	Metal
🗆 Gas 🏹 Oil 🗅 Solid	Factory Built U.L. Listing #
Appliance Name:	Direct Vent
U.L. Approved X Yes D No	Type Power Venter UL#
Will appliance be installed in accordance with the manufacture's	Type of Fuel Tank
installation instructions? 💢 Yes 🛛 No	Dil
	Gas
IF <u>NO</u> Explain:	
	Size of Tank 275 gallen
The Type of License of Installer:	Number of Tanks
Master Plumber #	
🔊 Solid Fuel # <u>4585</u>	Distance from Tank to Center of Flame feet.
• Oil #	H
Gas #	420 2
• Other	30,00
Approved	Approved with Conditions
Fire:	See attached letter or requirement
Ele.:	
Bldg.:	
Signature of Installer	
	Pink - Applicant's Gold - Assessor's Copy

	- 1- 1- x X
	THE STORE AND TH
	CITY OF PORTLAND, MAINE Department of Building Inspections
4	<u>4/11</u> 20 Ob
	Received from CPW Quelipment Location of Work 159 Baveny St
	Cost of Construction \$ Permit Fee \$
1	Building (IL) Plumbing (I5) Electrical (I2) Site Plan (U2) Other
	CBL: 333-4016
	Check #: Total Collected \$ THIS IS NOT A PERMIT No work is to be started until PERMIT CARD is actually posted upon the premises. Acceptance of fee is no guarantee that permit will be granted. PRESERVE THIS RECEIPT. In case permit cannot be granted the amount of the fee will be refunded upon return of the
	receipt less \$10.00 or 10% whichever is greater. WHITE - Applicant's Copy YELLOW - Office Copy PINK - Permit Copy



MARK VEREILL CELL 415-4402 OFFICE 839-7603 Construction



L

• Geotechnical Engineering • Field & Laboratory Testing • Scientific & Environmental Consulting

GEOTECHNICAL ENGINEERING SERVICES PROPOSED RESIDENCE 154 BEVERLY STREET PORTLAND, MAINE

00-0573 S September 25, 2000

• Geotechnical Engineering • Field & Lab Testing • Scientific & Environmental Consulting

00-0573

September 25, 2000

CBW Development Attn: Tom Blackburn PO Box 4000 Windham ME 04062

**S.W.COLE** 

Subject: Geotechnical Engineering Services Proposed Residence 154 Beverly Street Portland, Maine

Dear Mr. Blackburn:

In accordance with our Service Contract dated August 25, 2000 and subsequent discussions, we have made an investigation at the site of the proposed residence on Lot 154 Beverly Street in Portland, Maine. The purpose of our investigation was to obtain subsurface information at the site of the proposed residence in order to evaluate global stability and to provide geotechnical parameters for your use in design of footings and basement walls. The investigation included the making of subsurface explorations, insitu soil testing and a geotechnical evaluation of the findings. The contents of this report are subject to the limitations set forth in Attachment A.

#### PROPOSED CONSTRUCTION

Based on information provided by Nadeau & Lodge (project surveyor), Shelley Engineering (project structural engineer) and CBW Development (general contractor), we understand the proposed residence will be a two-story wood-framed structure with a daylight basement and attached garage. The proposed structure will occupy a plan area of about 24 feet by 56 feet constructed on an existing 21/2H:1V slope. Based on our discussions with you, we understand the attached garage will have a structural parking deck slab with a full basement below.

Foundations will consist of spread footings with one row of interior columns, spaced about 8 feet on-center, along the long axis of the building. According to information provided by Shelley Engineering, column loads are anticipated to be 14 kips (dead plus live load) and perimeter wall loads are anticipated to be 2.8 kip per lineal foot of wall.

GRAY, ME OFFICE



00-0573 September 25, 2000

Based on our discussions with you, the basement floor will be at elevation 53 feet, which will require cuts on the order of 4 feet to 11 feet to establish footing grade and about 8½ feet to establish bottom of slab grade. Considering a design frost depth of 4 feet for the Portland area, we anticipate footings will be founded at about elevation 49 feet on the northerly side (downhill) of the proposed structure stepping up to about elevation 51 feet on the southerly side (uphill). Details of the proposed and existing site features are shown on the "Exploration Location Plan" attached as Sheet 1.

### SUBSURFACE CONDITIONS

Two test pits (TP-1 and TP-2) and one test boring (B-1) were made in the area of the proposed building at the approximate locations shown on Sheet 1. Beneath the topsoil and forest duff, the explorations generally encountered about 3 to 4½ feet of clayey fill soils overlying relatively stiff native brown silty clay underlain by a softer stratum of gray silty clay with shells. Test pits TP-1 and TP-2 were terminated in the relatively stiff stratum of brown silty clay at depths of 6 and 9½ feet, respectively. Test boring B-1 penetrated the upper stratum of brown silty clay at a depth of about 14 feet and was terminated at a depth of about 42 feet in the lower stratum of relatively soft gray silty clay. Free groundwater was not encountered during the short time period the explorations were open; however, the soils were observed to be wet below a depth of about 15 feet.

For a more detailed description of the subsurface conditions encountered see the logs attached as Sheets 2 through 4. A key to the notes and symbols used on the logs is attached as Sheet 5. Results of in-situ strength testing performed in test boring B-1 are shown on the log.

### DISCUSSION AND RECOMMENDATIONS

We have made an analysis of global stability for the proposed structure. Our analysis has been based on our understanding of the proposed construction and subsurface information obtained at the explorations. Additionally, we assumed that the interior columns would be supported on a reinforced concrete grade beam at least 2 feet wide. Based on these considerations, we estimate the factor of safety for the overall stability



00-0573 September 25, 2000

(deep rotational slope failure) of the site with proposed residence is on the order of 1.5. If seismic loads are considered, the factor of safety drops to about 1.2. Consequently, the site appears suitable for the proposed construction from a slope stability standpoint.

Wall and column footings should bear on at least 6 inches of compacted select fill placed upon undisturbed stiff brown silty clay. If subgrades are wet, the compacted select fill under footings should be replaced with a 6-inch layer of <sup>3</sup>/<sub>4</sub>-inch crushed stone wrapped in geotextile filter fabric. All wall footings should be at least 4 feet from freezing temperatures. Wall and column footings should be at least 2 feet in width and the interior columns should, ideally, be supported on a reinforced concrete grade beam running the entire length of the building. Footing and basement wall design should consider the following soil parameters:

- Allowable Bearing Pressure = 1.5 ksf (properly prepared subgrade, as noted)
- Design Frost Depth = 4.0 feet
- Base Friction Factor = 0.4 (compacted select fill)
- (K<sub>p</sub>) Passive Lateral Earth Pressure Coefficient = 3.0 (compacted select fill)
- (K<sub>o</sub>) At-Rest Lateral Earth Pressure Coefficient = 0.5 (compacted select fill)
- $(\gamma_T)$  Unit Weight of Backfill = 130 pcf (compacted select fill)

Wall design should also consider surcharge loads from vehicles within the driveway. We recommend that the wall be backfilled with compacted select fill. The select fill should be compacted to between 92 to 95 percent of ASTM D-1557. The select fill should be clean, well-drained granular fill meeting the following gradation:

SELECT FILL GRADATION					
Sieve Size	Percent Finer by Weight				
4 inch	100				
3 inch	90 - 100				
1/4 inch	25 - 90				
#40	0-30				
#200	0-5				



00-0573 September 25, 2000

An underdrain should be installed at footing grade around the perimeter wall footing. The underdrain should be perforated (perforations oriented downward) and have a positive gravity outlet. The underdrain should be surrounded with at least 12 inches of <sup>3</sup>/<sub>4</sub> inch crushed stone and the stone should be wrapped with geotextile fabric. Further, all below grade concrete walls should be damp-proofed and a layer of insulation should be installed adjacent to the exterior side of all basement walls. This will help reduce thermal conductivity and the potential for condensation.

### CLOSURE

We request that S.W.COLE ENGINEERING be retained to review the final design and specifications to determine that our foundation recommendations have been properly interpreted and implemented. During construction, an S.W.COLE ENGINEERING representative should be on-site to observe subgrade soils prior to fill or concrete placement. A soils and concrete testing program should be implemented to observe compliance with the design concepts, specifications, and recommendations and to allow design changes in the event that subsurface conditions are found to differ from those anticipated prior to the start of construction. S.W.COLE ENGINEERING is available to provide soil and concrete testing services.

We trust this letter meets your needs. Please do not hesitate to contact us if you have any questions.

Sincerely,

S. W. COLE ENGINEERING, INC. OF M PILING \* PROVIN 5 inothy J/Boyce, P. E. Geotechnical Engineer 

G:\Files\Projects\2000\00-0573\_CBW Development\_Portland\_154 Beverly St\00-0573 Report.doc

# ATTACHMENT A LIMITATIONS

This report has been prepared for the exclusive use of CBW Development for specific application to the proposed Residence on Lot 154 Beverly Street in Portland, Maine. S.W.COLE ENGINEERING, INC. has endeavored to conduct the work in accordance with generally accepted soil and foundation engineering practices. No other warranty, expressed or implied, is made.

The soil profiles described in the report are intended to convey general trends in subsurface conditions. The boundaries between strata are approximate and are based upon interpretation of exploration data and samples.

The analyses performed during this investigation and recommendations presented in this report are based in part upon the data obtained from subsurface explorations made at the site. Variations in subsurface conditions may occur between explorations and may not become evident until construction. If variations in subsurface conditions become evident after submission of this report, it will be necessary to evaluate their nature and to review the recommendations of this report.

Observations have been made during exploration work to assess site groundwater levels. Fluctuations in water levels will occur due to variations in rainfall, temperature, and other factors.

Recommendations contained in this report are based substantially upon information provided by others regarding the proposed project. In the event that any changes are made in the design, nature, or location of the proposed project, S.W.COLE ENGINEERING, INC. should review such changes as they relate to analyses associated with this report. Recommendations contained in this report shall not be considered valid unless S.W.COLE ENGINEERING, INC reviews the changes.

<u>S.W</u>				~~^	×					BOR	ING LOG	BORING NO .: SHEET:	1 OF 2
EOTECHN	ERI CAL C	NG, IN	NC.	$\sim \sim $		$\int$						PROJECT NO.:	
ROJECT /	CLIF	-NT·	154 BE		STREE				=NT			DATE START:	7/14/00
CATION		-		AND, N			T DLVL					DATE FINISH:	7/14/00
RILLING				HERN T		RINGS			 D	RILLER: MIKE NADEAU			
		-										ELEVATION:	62+/- '
				PE			HAMME	ER WT.	HAMM	ER FALL	,	SWC REP .:	RED
SING:		-		SA	41						N N	WATER LEVEL INFO	10000 NO 51000
MPLER:		-	. 8	is .	13	/8"	140		3	0*		Soils wet @ 1	5'
ORE BAR	REL:	-										·	
ISING		SAM	PLE		SAMP	LER BL	OWS P	ER 6F					
DER	10.	PEN.	REC.	DEPTH @ BOT	0-6	6-12	12-18	18-24	DEPTH		STRATA & TE	ST DATA	
									0.5'		TOPSOIL & ROOT	MATERIAL	
											BROWN SILTY C	AY (FILL)	
									4 <u>+</u> '				
											BROWN SILT	Y CLAY	q <sub>p</sub> = 5 ksf
1	ID	24"	18"	7.0	6	6	8	10			~VERY STIF		np o non
				ļ									
													q <sub>p</sub> = 4 ksf
				1									$q_p = 2 \text{ ksf}$
2	2D	24"	22"	12.0'	3	4	5	7			MEDIU	M~	L <sub>v</sub> = 0.75 ksf
				1					14 <u>+</u> '				
				1									L <sub>v</sub> = 0.2 ksf
3	3D	24"	24"	17.0'	3	2	2	4				0.01	
											GRAY SILTY WITH SHE		
		VANE		20.8'						S <sub>V</sub> = 0.54/0.09 ksf			
3.5	5"x7"	VANE		21.6'						S <sub>v</sub> = 0.54/0.08 ksf			
											~ MEDIU	M ~	L <sub>v</sub> = 0.2 ksf
	4D	24"	24"	25.0'	WOM	WOM	WOM	WOM					Ly - 0.2 K31
				1									
	5D	24"	24"	27.0'	WOM	WOM	WOM	WOM					
		VANE	·····	27.8'						$S_v = 0.50/0.11 \text{ ksf}$			
3.5	5"x7"	VANE		28.6'						S <sub>v</sub> = 0.54/0.11 ksf			
				1					31 <u>+</u> '				L <sub>v</sub> = 0.2 ksf
	6D	24"	24*	32.0'									
	70	24"	24"	34.0'	3	1	2	3		GRA	AY SILTY CLAY WITH AND SHE		
	8D	24"	24"	37.0'	WOM	WOM	WOM	WOM		$S_v = 0.38/0.01 \text{ ksf}$	~ MEDIU	M ~	
		VANE		37.8	ļ					S <sub>V</sub> = 0.54/0.02 ksf			
3.	5"x7"	VANE		38.4'	1								
AMPLES				SOIL	CLASSI	FIED BY	/:		REMAR	RKS:			
Descurrent and		menur			1					ATRATICATION			(2
SPLIT S						LER - V TECH.				STRATIFICATION LINES I		YPES	Ľ
=3" SHEL		TUBE		X	-	DRATO				AND THE TRANSITION M		BORING NO .:	B-1

S.V	Y,L	JUL	.C	^						BORING LOG	BORING NO .:	B-1
ENGIN	NEER	ING. I	NC.	$\sim$		[ <del></del>			<del></del>		SHEET:	2 OF 2
GEOTECH					V	<i>y</i>					PROJECT NO .:	00-0573
				VERLY		T/CB	W DEV	ELOPM	ENT		DATE START:	7/14/00
OCATIO				AND, N							DATE FINISH:	7/14/00
RILLING	GFIRM	A:	NORTI	HERN T	ESTBO	DRINGS	5		D	RILLER: MIKE NADEAU	ELEVATION:	62+/- '
			TY	'PE	SIZE	I.D.	HAMM	ER WT.	HAMME	R FALL	SWC REP.:	RED
ASING:	:			SA		/4"					TER LEVEL INFORM	
AMPLE	R:		, 5	SS	13	/8"	140	) lb	3	)″	Soils wet @ 15'	
ORE B	ARREL	.:										
ASING		CAL	IPLE	Marrie .	CALIF	LER BL	OW/S D	CD 61	C SALES			T Shield
LOWS PER	22.23	SAM		DEPTH	SAM	LER BL	.0445 P	ERD	DEPTH	STRATA & TES	T DATA	
FOOT	NO.	PEN.	REC.	@ BOT	0-6	6-12	12-18	18-24	140%			
	00	24"	24"	12.0'	MOH	WOH	WOH	WOH	42'	GRAY SILTY CLAY WITH FI		
	9D	24	24	42.0'	WOH	WUH	WUH	WUH	42	BOTTOM OF EXPLORA		
										BOTTOM OF EXPLORA		
			<u> </u>						1			
									1			
			1									
			ļ						}			
				+					-			
		<u> </u>				-						
		1	1	1					1			
									}			
		ļ		<u> </u>					{			
		+	+									
			1	1		1			1			
									]			
									4			
									1			
		-	1						1			
			1	1	1	1	1		1			
									1			
				+	ļ				-			
		+	+	+					1			
		+	+	+			1		1			
									1			
							ļ		1			
		+				+			-			
		+							1			
		1	+						1			
									-			
	L							L	DEMAN	ks:		· · · · · · · · ·
SAMPLI	ES:			SOIL	LASSI	FIED B	Υ:		REMA			(
)≈SPLI					-	LER - N				STRATIFICATION LINES REPRESENT THE		
C=3" SH		TUDE		X	1 001	TEOU	-VISUA	I I V		APPROXIMATE BOUNDARY BETWEEN SOIL TY		



PROJECT/CLIENT: 154 BEVERLY STREET / CBW DEVELOPMENT LOCATION: PORTLAND, MAINE

PROJECT NO. 00-0573

				TEST PIT	1		
		DATE:	7/6/00	SURFACE ELEVATION: 5	<u>3' +/-</u>	OCATION:	SEE SHEET 1
SAN	IPLE	DEPTH		STRATUM DESCRIPT	ON	14	TEST RESULTS
NO.	DEPTH	(FT)	Carline and the				
		1.0'		FOREST DUFF & TOPS			
				BROWN SILTY CLAY (FI	LL)		
		3.0'					
				BROWN SILTY CLAY			
S-1	5'						
		6.0'					
				BOTTOM OF EXPLORATION	@ 6.0'		
						{	
	I		L				
	C	OMPLETI	ON DEPTH:	6.0'	DEPTH TO WATER:		

			TEST PIT	2		
	DATE:	7/6/00	SURFACE ELEVATION	62'+/-	LOCATION:	SEE SHEET 1
SAMPL	221 10		STRATUM DESC	RIPTION		TEST RESULTS
NO. DE	PTH (FT)					
	0.5'	Sector Contraction	TOPSOIL & ROOT N	IATERIAL		
			BROWN SILTY CL	NY (FILL)		
	4.5'					
			BROWN SILTY	CLAY		
	9.5'		BOTTOM OF EXPLORA			
		l	BUTTOW OF EAFLURA	11011 @ 9.0		
	COMPLET	ION DEPTH:	9.5'	DEPTH	TO WATER: No Free Water	Observed
						4

## KEY TO THE NOTES & SYMBOLS Test Boring and Test Pit Explorations

All stratification lines represent the approximate boundary between soil types and the transition may be gradual.

# Key to Symbols Used:

w -	water content, percent (dry weight basis)
q <sub>u</sub> -	unconfined compressive strength, kips/sq. ft based on laboratory
	unconfined compressive test
S <sub>v</sub> -	field vane shear strength, kips/sq. ft.
	•
L <sub>v</sub> -	lab vane shear strength, kips/sq. ft.
q <sub>p</sub> -	unconfined compressive strength, kips/sq. ft. based on pocket
	penetrometer test
0 -	organic content, percent (dry weight basis)
W	liquid limit - Atterberg test
-	
W <sub>P</sub> -	plastic limit - Atterberg test
WOH -	advance by weight of hammer
WOM -	advance by weight of man
WOR -	advance by weight of rods
HYD -	advance by force of hydraulic piston on drill
RQD -	Rock Quality Designator - an index of the quality of a rock mass. RQD is
NGD -	
	computed from recovered core samples.
γτ <b>-</b>	total soil weight
γ <sub>B</sub> -	buoyant soil weight
10	·····

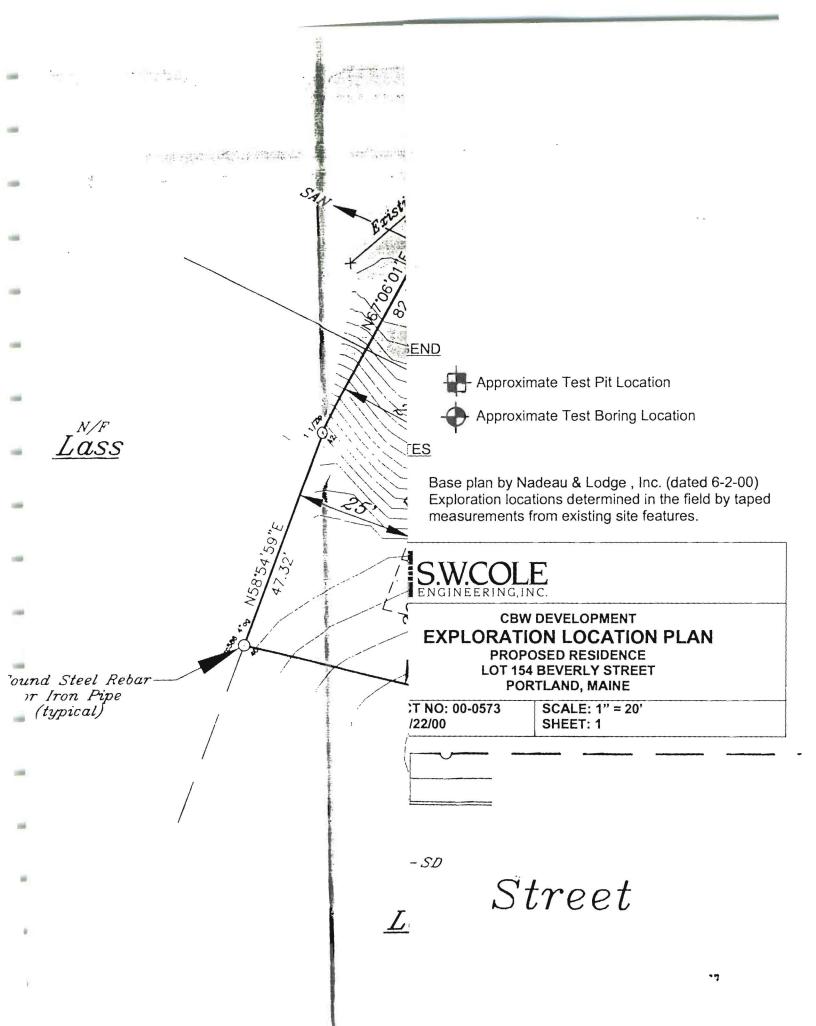
## **Description of Proportions:**

0 to 5% TRACE 5 to 12% SOME 12 to 35% "Y" 35+% AND

**REFUSAL:** <u>Test Boring Explorations</u> - Refusal depth indicates that depth at which, in the drill foreman's opinion, sufficient resistance to the advance of the casing, auger, probe rod or sampler was encountered to render further advance impossible or impracticable by the procedures and equipment being used.

**REFUSAL:** <u>Test Pit Explorations</u> - Refusal depth indicates that depth at which sufficient resistance to the advance of the backhoe bucket was encountered to render further advance impossible or impracticable by the procedures and equipment being used.

Although refusal may indicate the encountering of the bedrock surface, it may indicate the striking of large cobbles, boulders, very dense or cemented soil, or other buried natural or man-made objects or it may indicate the encountering of a harder zone after penetrating a considerable depth through a weathered or disintegrated zone of the bedrock.



4 Ú U L L 5 Ľ L 

	CITY OF PORTLA	ND, MAINE
RESURGAN	Department of Build	ling Inspection
( C	ertificate of	Occupancy
STITATIS POR	LOCATION 159 Bever	ly St CBL 333 K01600101
CPW Development/CPW Developmen		Date of Issue 03/28/2002
This is to certify that the bu	uilding, premises, or part the	reof, at the above location, built – altered
- changed as to use under Building P	ng Ordinance and Building C e, as indicated below.	I final inspection, has been found to conform Code of the City, and is hereby approved for APPROVED OCCUPANCY
	EMISES	
Entire		Single Family Use Group R-3
		Type 5B
Limiting Conditions:		BOCA 1999
Temporary until June 15, 2002 due to DR Certificate required.	C requirements. Elevation	
		0011
This certificate supersedes		
certificate issued	/	
	1	VIA La TELILO
certificate issued	oute C	10 Juit 4/1/0
certificate issued	onke C	Inspector of Buildings

