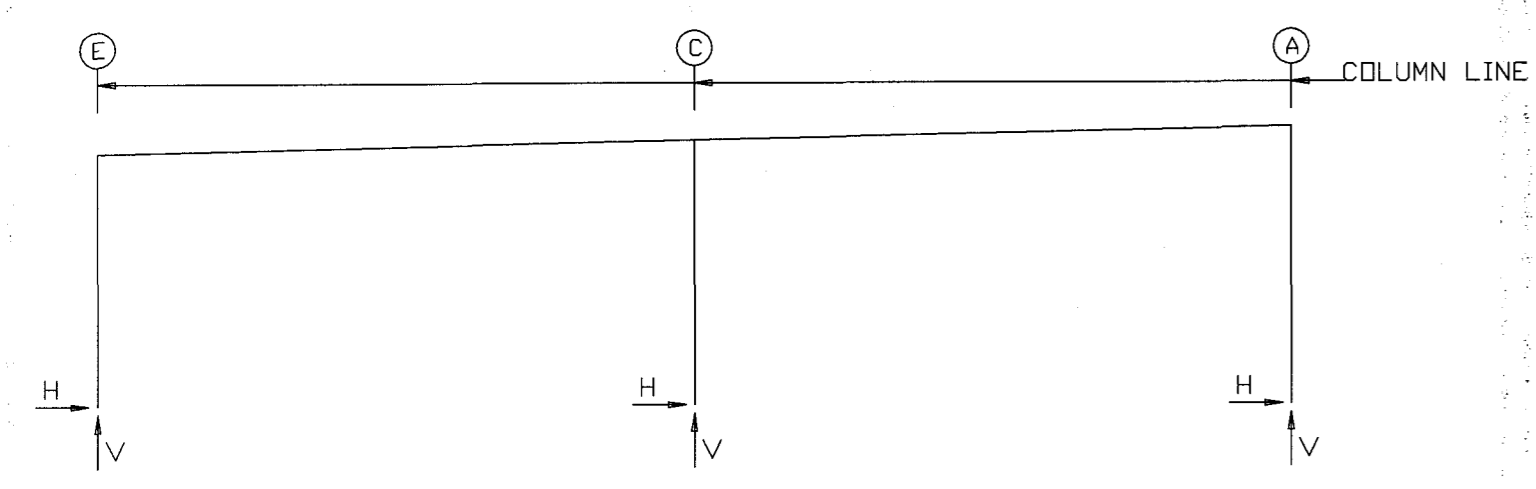
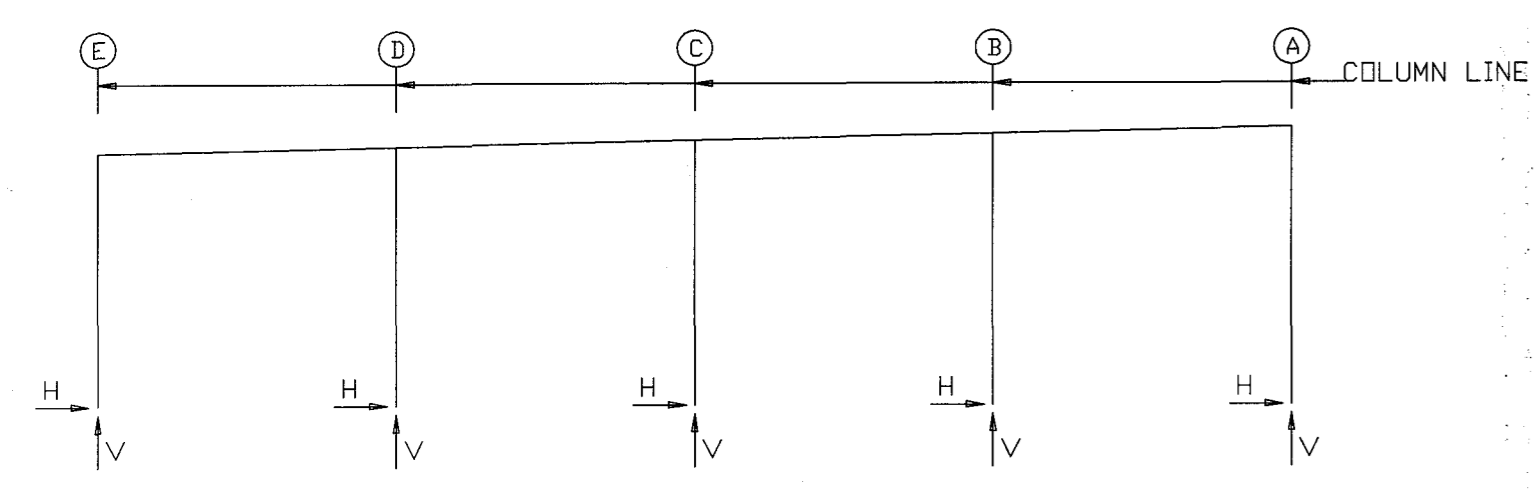


FRAME LINES: 2 3 4 5



FRAME LINES: 6



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column Reactions (k)				Anc. Bolt Qty Dia	Base Plate (In)			Base EL (In)			
		Load Hmax	V	Load Hmin	V		Width	Length	Thick				
2*	E	3	5.3	17.9	5	-3.7	-2.1	4	0.750	6.000	9.000	0.375	0.0
2*	A	12	2.7	0.6	10	-4.3	14.3	4	0.750	6.000	9.000	0.375	0.0
2*	C	6	0.0	-11.5	6	0.0	-11.5	4	0.750	8.000	8.500	0.500	-6.0

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column Reactions (k)				Anc. Bolt Qty Dia	Base Plate (In)			Base EL (In)			
		Load Hmax	V	Load Hmin	V		Width	Length	Thick				
6	E	9	2.4	4.3	11	-2.1	-1.7	4	0.750	6.000	9.000	0.375	0.0
6	A	12	1.8	-1.4	8	-2.1	4.2	4	0.750	6.000	9.000	0.375	0.0
6	D	6	0.0	-3.2	6	0.0	-3.2	4	0.750	6.000	8.000	0.375	0.0
6	C	4	0.0	-2.7	4	0.0	-2.7	4	0.750	6.000	8.000	0.375	0.0
6	B	4	0.0	-3.3	4	0.0	-3.3	4	0.750	6.000	8.000	0.375	0.0

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	---Dead---		---Collateral---		---Live---		---Snow---		---Wind_Left1---		---Wind_Right1---
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	
2*	E	0.5	3.3	0.4	2.3	1.6	9.3	-3.3	19.5	-3.3	-5.8	2.5
2*	A	-0.5	3.3	-0.4	2.3	-1.6	9.6	-3.3	20.9	-1.2	-6.1	2.6
2*	C	0.0	8.4	0.0	6.3	0.0	25.1	0.0	52.6	0.0	-16.3	0.0

2\* Frame Lines: 2 3 4 5

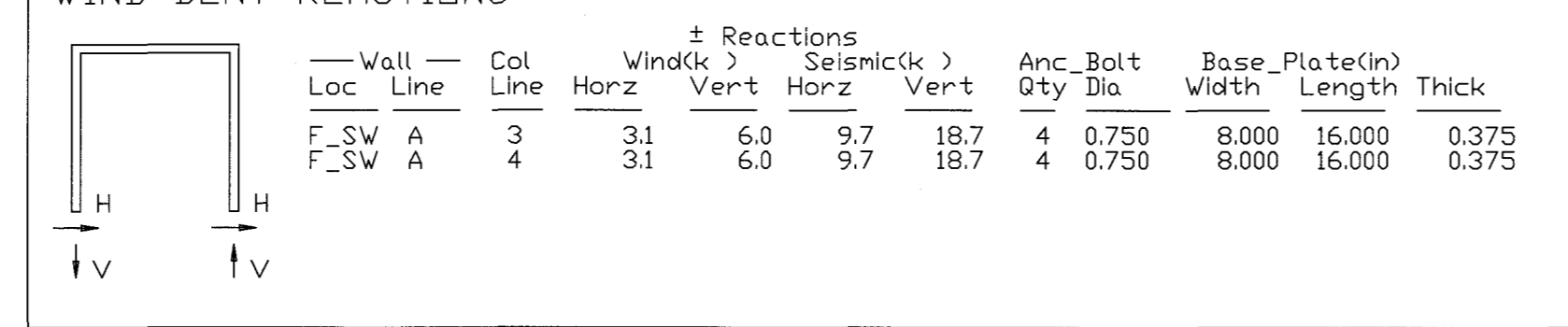
ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead		Collat		Live		Snow		Wind_Left1		Wind_Right1		Wind_Left2		Wind_Right2		Wind Press
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert			
1	E	0.9	0.6	2.3	4.9	0.0	-2.2	0.0	-2.2	0.0	-1.3	0.0	-2.2	0.0	-1.3	0.0	-1.3	-1.2

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column Reactions (k)				Anc. Bolt Qty Dia	Base Plate (In)			Base EL (In)			
		Load Hmax	V	Load Hmin	V		Width	Length	Thick				
1	E	20	1.4	-1.6	21	-1.2	-1.0	4	0.750	6.000	8.000	0.375	0.0

WIND BENT REACTIONS



BUILDING BRACING REACTIONS

Loc Line	Col Line	± Reactions (k)				Panel Shear (lb/ft)	Note
		Wind	Seis	Wind	Seis		
L_EW	1	C,B	2.2	1.8	5.4	4.5	(a)

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type	Proj (in)
0 20	Jamb	1/2"	A307	1.50
0 32	Endwall	3/4"	A307	2.00

NOTES FOR REACTIONS

- The following Design Data is per Package Steel Systems, Inc.'s standard design practices and established procedures and recommendations of the following Organizations and/or Specifications.
  - American Institute of Steel Construction (AISC 2005)
  - American Welding Society Structural Welding Code (AWS D1.1)
  - North American United States (NAUS07)
- For maximum reactions tables, all loading conditions are examined and only the maximum/minimum horizontal or vertical reactions along with the corresponding horizontal or vertical for those load IDs are reported.
- Positive reactions are shown in the sketch. Foundation loads are in the opposite directions.
- Bracing reactions are in the plane of the brace with the horizontal pointing away from the braced bay. The vertical reaction can be downward or upward.
- Reactions given are based on the design data below. Reactions are not furnished for loads not listed.
- The endwall column reactions do NOT include wind and seismic reactions from endwall bracing. Reactions given in the bracing reactions table should be combined with the appropriate basic column reactions as necessary to determine the maximum reactions for foundation design.
- The rigid frame maximum reactions include wind and seismic reactions from sidewall bracing. Reactions given in the bracing reactions table should not be combined with the appropriate basic column reactions as necessary to determine the maximum reactions for foundation design.
- Foundation construction and design is not the responsibility of Package Steel Systems, Inc. The embedment of the anchor bolts in concrete is the responsibility of the foundation designer.
- Suggested anchor rod diameter, quantity, minimum projection and placement are shown. All anchor rods are assumed to be ASTM F1554 Grade 36 or equal. Anchor rods (not by PSS) shall be set to a tolerance of +1/8" in both elevation and location.
- Column base plates are designed not to exceed a bearing pressure of 1050 pounds per sq. inch (0.35F<sub>c</sub> where F<sub>c</sub> = 3000 psi) unless noted otherwise.
- Basic design wind pressure is furnished. For components and cladding not specifically designed and/or furnished by PSS, the design pressures and suction shall be increased based on tributary area and location. Confirmation of the design loads and adequacy to resist such loads shall be the responsibility of a licensed design professional by others.

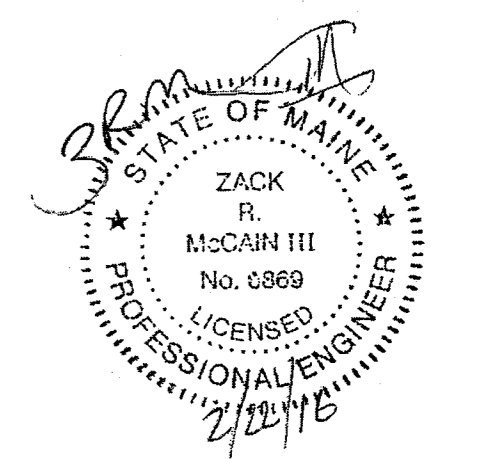
Building Reactions are based on the following information:

Building Code/Edition: IBC 09

Building Size:		Snow Loads:	
Width (Ft)	95	Ground Snow (Pg)	60.00 psf
Length (Ft)	115	Flat Roof Snow (Pf)	42.00 psf
Back Side Eave Height (Ft)	20	Snow Exposure Factor (Ce)	1.00
Front Side Eave Height (Ft)	21.98	Snow Thermal Factor (Ct)	1.00
Back Side Roof Slope	0.25:12	Snow Importance Factor (Is)	1.00
Front Side Roof Slope		Sloped Roof Factor (Cs)	1.00

Roof Dead, Collateral, & Live Loads:		Seismic Loads:	
Dead Load	5.00 psf	Seismic Importance (Ie)	1.00
Collateral Load	5.00 psf	Seismic Design Category (A/B/C/D)	C
Live Load	20.00 psf	Site Class-Type	E
Live Load Reduction Taken	No	Seismic Response Coeff. (Sds)	0.486
		Seismic Response Coeff. (Sd1)	0.182
		Response Modification (R)	3.00
		Response Modification (BF)	3.00

- Basic Wind Speed (3 Second Gust): 100 mph  
 Wind Exposure: B  
 Building Enclosure (D/C/P): Closed  
 Internal Pressure Factor (Iw): 1.00  
 Wind Importance Coeff. (GCp): 0.18
- Auxiliary Load - Case x  
 C = Closed  
 CL = Collateral Load  
 DL = Dead Load  
 FXUNB\_LL = Unbalanced Live Load for Frame IDx  
 LL = Max. of (Live or Snow)  
 LLR = Live Load Unbalanced  
 LnWndL = Longitudinal Wind Load - Left  
 LnWndR = Longitudinal Wind Load - Right  
 mph = miles per hour
- Design Base Shear (V) = Longit. 38.46 kips  
 Design Base Shear (V) = Transv. 39.87 kips  
 Analysis Procedure: Equivalent Lateral Force
- Auxiliary Load(s):  
 (4) 400# Unit Heaters Suspended off Purlins
- Loading Conditions are as follows:
- 1 Dead+Collateral+Snow
  - 2 Dead+Collateral+Snow+Slide Snow
  - 3 Dead+Collateral+0.75Snow+0.75Wind\_Right1
  - 4 0.6Dead+Wind\_Left1
  - 5 0.6Dead+Wind\_Left2
  - 6 0.6Dead+Wind\_Long1+LWIND1\_L2E
  - 7 0.6Dead+Wind\_Long1+LWIND1\_R2E
  - 8 1.07Dead+1.07Collateral+0.7Seismic\_Left
  - 9 1.07Dead+1.07Collateral+0.7Seismic\_Right
  - 10 1.05Dead+1.05Collateral+0.75Live+0.52Seismic\_Left
  - 11 0.53Dead+0.7Seismic\_Left
  - 12 0.53Dead+0.7Seismic\_Right
  - 13 0.53Dead+0.7Seismic\_Long
  - 14 Dead+Collateral+Snow/2+FIPAT\_SL\_1
  - 15 Dead+Collateral+Snow/2+FIPAT\_SL\_2
  - 16 Dead+Collateral+Snow/2+FIPAT\_SL\_3
  - 17 Dead+Collateral+Snow/2+FIPAT\_SL\_4
  - 18 Dead+Collateral+Snow/2+FIPAT\_SL\_5
  - 19 Dead+Collateral+Snow/2+FIPAT\_SL\_1
  - 20 0.6Dead+Wind\_Left2+Wind\_Suction
  - 21 0.6Dead+Wind\_Pressure+Wind\_Long1
  - 22 0.6Dead+Wind\_Right2+Wind\_Suction



REV.	DESCRIPTION	DATE	DRAFT	ENG.
3				
2				
1	BUILDING HAS BEEN REDESIGNED DUE TO SEISMIC SOIL SITE CLASS CHANGE.	12/21/15	TMZ	ZRM

