Site Name: Portland ME, ME Program Last Updated: 5/13/2014
Site Number: 10047 American Tower Corporation

Engineering Number: OAA702064_C3_07
Engineer: Brendan M Smith
Date: 06/21/17

Design Base Loads (Factored) per TIA-222-G

| Foundation Mapped: | N | | | |
|---|-------|---------|--|----------------------|
| Moment (M _u): | 0.0 | k-ft | Concrete Compressive Strength (f'c): | 3000 psi |
| Shear/Leg (V _u): | 0.6 | k | Vertical Steel Rebar Size #: | 6 |
| Compression/Leg (P _u): | 355.8 | k | Vertical Steel Rebar Area: | 0.44 in ² |
| Uplift/Leg (T _u): | 0.0 | k | # of Vertical Steel Rebars: | 8 |
| Tower Type (GT / SST): | GT | | Vertical Steel Rebar Yield Strength (F _y): | 60 ksi |
| Diameter of Prismatic Portion of Pier (d): | 4.0 | ft | Tie / Stirrup Size #: | 4 |
| Depth to Base of Foundation: | 4.0 | ft | Tie / Stirrup Area: | 0.20 in ² |
| Pier Height Above Ground (h): | 0.50 | ft | Tie / Stirrup Spacing: | 18.0 in |
| Length / Width of Pad (w): | 11.0 | ft | Tie / Stirrup Steel Yield Strength (F _y): | 60 ksi |
| Thickness of Pad (t): | 1.5 | ft | Rebar Cage Diameter: | 40.0 in |
| Depth Below Ground Surface to Water Table (w): | 99.0 | ft | Bending/Tension Reduction Factor (ϕ_B) : | 0.90 |
| Unit Weight of Concrete: | 150.0 | pcf | Shear Reduction Factor (ϕ_V): | 0.75 |
| Unit Weight of Water: | 62.4 | pcf | Compression Reduction Factor (ϕ_V): | 0.65 |
| Unit Weight of Soil Above Water Table: | 110.0 | pcf | Steel Elastic Modulus: | 29000 ksi |
| Unit Weight of Soil Below Water Table: | 55.0 | pcf | Pad Steel Rebar Size #: | 8 |
| Friction Angle of Uplift from Top of Pad: | 20 | Degrees | Pad Steel Rebar Area: | 0.79 in ² |
| Friction Angle of Uplift from Base of Pad: | 20 | Degrees | Pad Steel Rebar Yield Strength (F _y): | 60 ksi |
| Uplift Angle Started at Top or Base of Pad (T/B): | Т | | # of Rebar in Top of Pad: | 0 |
| Ultimate Skin Friction: | 220 | psf | # of Rebar in Base of Pad: | 11 |
| Ultimate Compressive Bearing Pressure: | 4500 | - | Pad Clear Cover: | 3 in |
| Capacity Increase (Due to Transient Loads): | 1.00 | | | |
| Bearing Strength Reduction Factor (ϕ_s): | 0.60 | | | |

Axial Capacities and Design Moment

2.5

2.0

Uplift Strength Reduction Factor (ϕ_s):

| Weight of S Ultimate Sk | oncrete (Bouyanc oil (Bouyancy Con in Friction Resista Failure Mode (Top | sidered): nce: | 32.9 35.6 14.5 Top | k | | n in | y- • | - |
|--|---|------------------------|-----------------------------|-----------------|----------|--|-------------|--------------|
| Nominal Co | lift Capacity per L mpressive Capacit | | 57.9 326.7 362.9 | k | | | | |
| T_u/ϕ_sT_n : P_u/ϕ_sP_n : | | | | Result: NG | • | | / | |
| Dept | th (ft) | Ultimate Lateral | Increment | γ_{Soil} | Cohesion | ф | | • |
| Тор | Bottom | Bearing Pressure (psf) | (psf/ft) | (pcf) | (psf) | (degree) | | |
| 0.0 | 2.0 | 0.0 | 110.0 | 110 | 0 | 0 | | |

110.0

110

400

0

0.75

Inflection Point (Below Ground Surface): 1.6 ft
Factored Design Moment At Inflection Point (M_u): 0.5 k-ft

1020.0