City of Portland,	Maina - Ruil	lding on Uso	Donnei	t Annliaatia	Pe	mit No:	ISSUE Date:	EV.	CBL:	
389 Congress Street	. 04101 Tel: (207) 874-870°	Permi 3 Fax: (t Applicatio (207) 874-87	n 16	02-0098 MA			317 B00)5001
Location of Construction:		Owner Name:	-	(207) 074 07		Address:	RI 4 200	4	Phone:	5001
375 Riverside St		Reynolds Mar	rianne M	r	1	Box 99			RGN 1.	, - 19
Business Name:		Contractor Name		DENNIS		ractor Address.	F PORTL	AND	1 W	2/-33
n/a		Patco Constru	سة			3 Main St Sar	ford		Phone 3:2 20732455	14 - 168
Lessee/Buyer's Name		Phone:	etion =	RUN		it Type:	iioiu —		20/32455	
n/a		n/a		-	l.	ditions - Com	mercial			Zone:
Past Use:		Proposed Use:		MERCIE	\			Inv		D-Y
Commercial / Motorc	vole Dealershin	1 -	Motoro	role	Perm	it Fee:	Cost of Work		O District:	
	yele Dealership	Dealership; 50			FIDE	\$779.00 E DEPT:	\$108,000		1	<u> </u>
		Addition. Site			FIRE	DEFI:	Approved	INSPECT:		T 7
						-	Denied	Use Group	DO 11.	Type:
							i		2/4/9	ኣ //
Proposed Project Descript	tion:	L.,			4		!	. ,	\times	11/
Build 5000 sq. ft. Stor									73/10	
Duna 5000 sq. it. Stor	rage Addition				Signa		<u> </u>	Signature:	JAN Y	$\underline{\sim}$
					PEDE	ESTRIAN ACTI	VITIES DISTE	CICT (P:A	.15 .)	•
					Actio	n: Approv	ed Appro	oved w/Co	nditions 📋 🗎	Denied
					Signa	iture:		D	ite:	
Permit Taken By:	Date An	plied For:	1		O Gilla		4		ite.	
gg	I	/2002				Zoning	Approval			
1. This permit applie	cation does not	preclude the	Spec	cial Zone or Revi	ews	Zoniı	ng Appeal		Historic Prese	rvation
Applicant(s) from			I □ Sh	oreland NA		Variance			Not in District	
Federal Rules.				ereame 10 /11	١	Variance	•	"	Not in District	oi Landina
2. Building permits	do not include n	lumbing	$ _{\mathbf{w}_{\epsilon}}$	etland		Miscella	neous		Does Not Requ	nire Review
septic or electrica	do not morado p il work.	rumonig,	_		0 %		ncous		Does Not Requ	ine Review
3. Building permits		is not started	Flo	od Zone p AN	عبارا	Condition	mal Use		Requires Revie	ew.
within six (6) mor				2000	ナ					
False information		a building	☐ Sul	bdivision		Interpret	ation		Approved	
permit and stop al	ll work]							
			Site	e Plan		☐ Approve	d		Approved w/C	onditions
			1	2001-00	D					
			Maj ┌	Minor MM		☐ Denied			Denied (
			الأه	with cm	YYVX	} , —				> >>
			Date:		امام	Date:		Date:	-	—)
				-0) 101	510	<u>Loane.</u>		Date.		/
			C	ERTIFICATI	ON					
hereby certify that I as	m the owner of i	record of the na	med pro	perty, or that the	ie pron	osed work is	authorized h	v the ow	ner of record	and that
have been authorized	by the owner to	make this appli	cation as	s his authorize	i agent	t and I agree t	o conform to	all annli	cable laws o	f this
urisaiction. In addition	n, it a permit for	work described	d in the a	application is in	ssued. 1	I certify that t	he code offic	ial's auth	orized repres	sentative
hall have the authority uch permit.	to enter all area	is covered by su	ich perm	it at any reason	nable h	our to enforce	e the provisi	on of the	code(s) appl	icable to
uen permit.										
SIGNATURE OF APPLICA	NT			ADDRES	S		DATE		PHON	 Е
RESPONSIBLE PERSON II	N CHARGE OF W	ORK TITTE	.					···		
INDON	CLIMICE OF WI	OKK, IIILE					DATE		PHON	E

					PERMIT		
City of Portland, Maine 389 Congress Street, 04101	•			1 00.010	Is ue Date:	CBL: 317 B00	05001
Location of Construction:	Owner Name:	•		Owner Address:		Phone:	
375 Riverside St	Reynolds Mar	rianne M	1	Po Box 99	CITY OF F	PORTLAND	
Business Name:	Contractor Nam	e:		Contractor Addres	ss:	Phone	
	Patco Constru	ction		1293 Main St S	anford	20732455	574
Lessee/Buyer's Name	Phone:			Permit Type:			Zone:
				Commercial			
Past Use:	Proposed Use:	-		Permit Fee:	Cost of Work:	CEO District:	1
Parking lot for Bull Moose Ha	arley Storage Build	ing FO	UNDATION		\$0.0	0 1	
Davidson	ONLY			FIRE DEPT:	Approved INS	PECTION:	
					Denied	e Group:	Туре:
					sul sal	2/11/02	1
				06	Je ope	و ۱۱۱۱ ج	
Proposed Project Description:				2 che	Approved INS Use	\sim	/ <u> </u>
50' X 100' storage building FC	OUNDATION ONLY			Signature:	Sig	nature: U	lux 1
				PEDESTRIAN AC	TIVITIES DISTRIC	T (P.A.D.)	
				Action: App	roved Approve	d w/Conditions	Denied
				Signature:		Date:	
Permit Taken By:	Date Applied For:			Zonii	ng Approval		
mjn	02/11/2002	C				Tri-ta-da Dana	
1. This permit application d		Spe	ecial Zone or Revie	ws Zo	oning Appeal	Historic Pres	ervation
Applicant(s) from meeting Federal Rules.	g applicable State and		ioreland	☐ Varia	nce	Not in Distric	t or Landma
2. Building permits do not in septic or electrical work.	nclude plumbing,	w	Wetland		ellaneous	Does Not Rec	quire Review
3. Building permits are void within six (6) months of t		☐ Flood Zone			itional Use	Requires Rev	iew
False information may in permit and stop all work		☐ Sī	ıbdivision	Interp	pretation	Approved	
		☐ Si	te Plan	Appro	oved	Approved w/0	Conditions
		Мај	Minor MM	Denie	ed	☐ Denied	
		Date:		Date:		Date:	
			PART	5		<u> </u>	
			<i>O</i>				
		(CERTIFICATION	ON			
I hereby certify that I am the ov I have been authorized by the c jurisdiction. In addition, if a po	owner to make this applermit for work describe	ication and in the	as his authorized application is is	l agent and I agre sued, I certify the	ee to conform to all at the code official	Il applicable laws of the laws of the laws of the law in the law i	of this esentative
shall have the authority to enter such permit.	an areas covered by s	ucn peri	ini ai any reasor	avie nour to enfo	orce the provision	or the code(s) app	piicable to
SIGNATURE OF APPLICANT			ADDRESS	;	DATE	PHO	NE
RESPONSIBLE PERSON IN CHAR	GE OF WORK, TITLE	, ,,			DATE	PHO	NF

02 0098

All Purpose Building Permit Application

If you or the property owner owes real estate or personal property taxes or user charges on any property within the City, payment arrangements must be made before permits of any kind are accepted.

Location/Address of Construction:	375 F	Riverside	57.	
Total Square Footage of Proposed Structu	ıre	Square Footage o	of Lot	
5000 Sq. ++.		2.	87 ACT	E S
Tax Assessor's Chart, Block & Lot Chart# 317 Block# B Lot# 5	Owner: Big M	oose Harley Z)AvidsoN	Telephone:
Lessee/Buyer's Name (If Applicable) N/A	Applicant telephone Patcs Co	name, address & Dennis Waters/k onstruction In in St. Santon	douots Fe	ost Of ork: \$ <u>/08,00</u> 0, ⁻ e: \$ 780 , 0°
Current use: Motorcycle Deal	ership	324-5574		
If the location is currently vacant, what we		NA		_
Approximately how long has it been vacc	ant:	N/A		
Proposed use: 5000 59.	ff. s	N/A torage bu	ildina	
Project description:	, _	, 90	// /	
Contractor's name, address & telephone:	•	Patco Con		ien
Who should we contact when the permit	is ready:	DENNIS/RI	c ~	
Mailing address:		293 Main	51.	
		SANFORD, ME	0407	13
We will contact you by phone when the preview the requirements before starting a and a \$100.00 fee if any work starts before	oermit is read ny work, with	dy. You must come n a Plan Reviewer.	in and pick A stop work	up the permit and
IF THE DECLURED INCORNATION IS NOT THE		01101410010110 =::= 0		

IF THE REQUIRED INFORMATION IS NOT INCLUDED IN THE SUBMISSIONS THE PERMIT WILL BE AUTOMATICALLY DENIED AT THE DISCRETION OF THE BUILDING/PLANNING DEPARTMENT, WE MAY REQUIRE ADDITIONAL INFORMATION IN ORDER TO APROVE THIS PERMIT

I hereby certify that I am the Owner of record of the named property, or that the owner of record authorizes the proposed work and that I have been authorized by the owner to make this application as his/her authorized agent. I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in this application is issued, I certify that the Code Official's authorized representative shall have the authority to enter all areas covered by this permit at any reasonable hour to enforce the provisions of the codes applicable to this permit.

•		<u> </u>				7
	Signature of applicant:	Dermi M. h	-/P97	Lo Date:	1/29/0	52
					· •	

This is NOT a permit, you may not commence ANY work until the permit is issued.

If you are in a Historic District you may be subject to additional permitting and fees with the Planning Department on the 4th floor of City Hall 1999

EGELV



October 31, 2001 01430

Marge Schmuckal, Zoning Administrator City of Portland 389 Congress Street Portland, ME 04101

Big Moose Harley-Davidson Dealership, Riverside Street - Minor Site Plan

B-4

Dear Marge:

Please find attached 10 copies of the minor site plan for the Big Moose Harley-Davidson motorcycle dealership's proposed 5,000 square foot building. The building is proposed to provide a storage facility for the motorcycle shop to store Harley-Davidson motorcycles seasonally on behalf of their customers. Big Moose feels that many Harley-Davidson owners need a facility to safely store their motorcycles and provide expert winter storage maintenance.

The 5,000 square foot building will be metal sided with a masonry front matching the existing store front in color and texture. The building will be located 25' from the existing structure in a parallel formation. The current site is paved, so no new impervious areas will be generated and therefore will not necessitate stormwater calculations. Because the southerly side of the existing parking lot adjacent to the proposed structure will be landscaped, there is a decrease in pavement and roof surface by 1,250 square feet.

Since the facility is just for storage, no new water or toilet facilities are needed. Three existing light poles in the parking lot where the building is to be located will be discontinued. No new lighting other than standard 100W lighting at the door entrances is anticipated on the structure. The existing electrical feed once serving the lights will provide the electrical service to the new building.

Although the existing parking is well over the requirement for parking spaces (since the site was once a Subaru dealership), five new spaces are shown directly attached with the storage use. Drainage will continue to sheet flow between the buildings and into an existing ditch between Handyman Rental and Big Moose. New paving will be laid upon completion of the structure to accommodate the new grades. The site will require an 8' high pre-load of borrow material to prepare the soil for the proposed foundation. Big Moose is anxious to start the pre-load as soon as possible so that they can begin construction in the spring of 2002. Silt fence will surround the pre-load and winter hay mulch will be spread during the winter months. The area is currently not utilized and the pre-load will not interfere with loading or accessing the rear of the existing building.

We look forward to working with the planning staff and hope to obtain approvals as soon as possible. Please feel free to contact us if you have any questions.

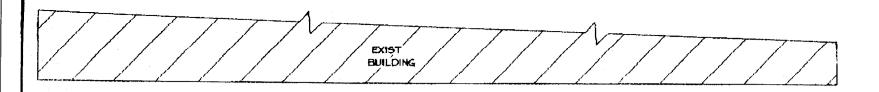
Sincerely,

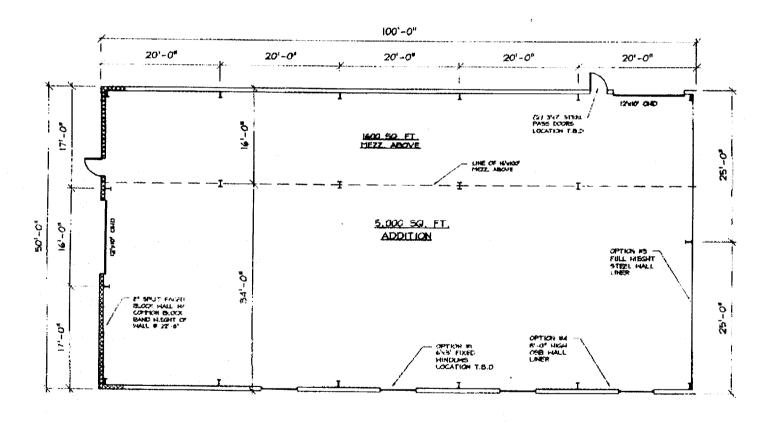
SEBAGO TECHNICS, INC.

James R. Seymour Project Engineer

JRS:jc Enc.

cc: Dennis Waters, Patco





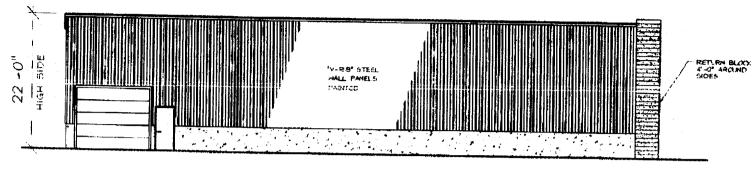
BIG MOOSE HARLEY DAVIDSON

FLOOR PLAN

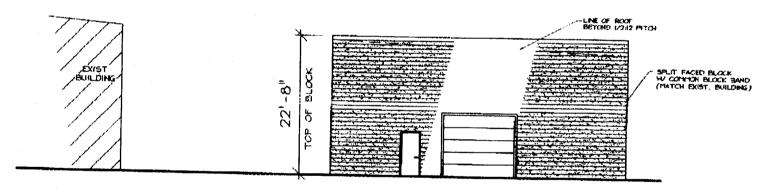
DATE: 8/1/01 NUMBER: 01067

J.B.S

293 MAIN STREET SANFORD, ME 04073 TEL. (207)324-5574 FAX: (207)324-1643 www.patco-construction.com

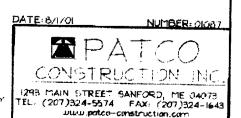


SIDEWALL ELEVATION



ENDMALL ELEVATION

BIG MOOSE HARLEY DAVIDSON ELEVATIONS SCALE 1/10"=1"-0"



DRWN, BY

SCHEDULE A

Certain lots or parcels of land with the buildings and improvements situated thereon, located on the southeasterly side of Riverside Street in Portland, County of Cumberland and State of Maine, and being more particularly bounded and described as follows:

PARCELIONE: Parcel Three as shown on a Plan entitled "Plot Plan for Turner Barker Associates," made by C.R. Störer, Inc., and recorded in the Cumberland County Registry of Deeds in Plan Book 36, Page 22, except the strip of land twenty-five (25) feet in width at the northeasterly end of said Parcel, said strip being shown on said Plan. Being the same premises which were conveyed to Kenneth L. Cianchette by Deed of Gardiner A. Ball et al dated January 14, 1975 and recorded in said Registry in Book 3642, Page 218, except for that portion of said premises which was conveyed by Kenneth L. Cianchette to Talma, Inc. by Deed dated February 19, 1976 and recorded in said Registry in Book 3808, Page 344.

PARCEL: TWO: Beginning at the most westerly corner of Lot #445 on Plan of Riverton frome Sites dated July, 1924 and recorded in said Registry in Plan Book 16, Page 11; thence by PARCEL ONE above north 27° 10' 05": east a distance of ninety (90) feet to an iron; thence south 60° 53' 55" east a distance of one hundred (100) feet to an iron; thence south 27° 10' 05" west a distance of ninety (90) feet to an iron; thence by PARCEL ONE above north 60° 53' 55" west a distance of one hundred (100) feet to the point of beginning. Courses are magnetic, 1975. Being the same premises which were conveyed to Kenneth L. Cianchette by Deed from Harry E. Waning and Jane W. Waning dated February 25, 1976 and recorded in said Registry in Book 1811, Page 29.

Reing the same premises conveyed to Marianna M. Reynolds by deed from Kenneth L. Cianchette recorded at the Cumberland County Registry of Deeds in Book 4499 Page 48.

Ken 1	Zuch Lan	The De	ed to	325	
Riveri	De St.	esomet.	It in	registers of	
witt	the Cun	Merline 9	a. 1	with of	. **
derl	Book	8705	Page	03/	
		•		·	

2001-0298

			Zoning Copy Appl	cation I. D. Number
				2/2001
	Harley Davidson	<u>.,</u>	— Appl	ication Date
Applicant		= a.10a	EDOS	Ca # Duilding/storons for hikon
	ie St., Portland, M	£ 04103		Sq.ft. Building/storage for bikes oct Name/Description
• •	lailing Address hnlcs/Jlm Seymou	1 °	375 - 375 Riverside St, Portland, Ma	•
Consultant/A			Address of Proposed Site	
	h: (207) 797-6061	Agent Fax: (207) 856-2206	317 B005001	
	Agent Daytime Tele	 	Assessor's Reference: Chart-Block-Lot	
• •	velopment (check a	•	Building Addition Change Of Use Re	sidential 🗌 Office 🕡 Retail
		<u> </u>	Other (specify	
Manufac	cturing U vvarei	house/Distribution Parking Lot	United (Specify	
5000 s.f			6.04	B4
Proposed Bu	ilding square Feet o	r#of Units Ach	eage of Site	Zoning
Check Revie	w Required:			
Site Plan		Subdivision	PAD Review	14-403 Streets Review
(major/mir	nor)	# of lots		
Flood Haz	zard	Shoreland	☐ HistoricPreservation	DEP Local Certification
Zoning Co		Zoning Variance		☐ Other
Fees Paid:	Site Plan	\$400.00 Subdivision	Engineer Review \$300.00	Date: 01/25/2002
7	A 04	atura.	Reviewer Marge Schmuckal	
∠oning / ☐ Approve	Approval St	Approved w/Conditions		
Approval D			/05/2003 Extension to	Additional Sheets Attached
Condition	Compliance	signature Colo	02/05/2002 date	
Performance	e Guarantee	Required*	Not Required	
* No building	nemit may be issu	ed until a performance guarantee has b	neen suhmitted as indicated helow	
_	•	•		4070000
Performa	nce Guarantee Acc		\$14,120.00	10/29/2002
		date	amount	expiration date
nspection	n Fee Paid			
		date	amount	
Building F	Permit Issued			
		date		
Performar	nce Guarantee Red	uced		
		date	remaining balance	signature
Temporar	ry Certificate of Occ	upancy	Conditions (See Attached)	
	•	date		expiration date
Final Insp	ection			
		date	signature	
Certificate	Of Occupancy			
		date		
Performar	nce Guarantee Rele			
		date	signature	
□ Defect Gu	Jarantee Submitted	2		
= 5561 50		submitted date	amount	expiration date

2001-0298 Application I. D. Number ADDENDUM

	ADDENDOM
Big Moose Harley Davidson	11/02/2001
Applicant	Application Date
375 Riverside St., Portland, ME 04103	5000 Sq.ft. Building/storage for blkes
Applicant's Mailing Address	Project Name/Description
Sebago Technics/Jim Seymour	375 - 375 Riverside St, Portland, Maine
Consultant/Agent	Address of Proposed Site
Applicant Ph: (207) 797-8061 Agent Fax: 2078562206	317 B005001
Applicant or Agent Daytime Telephone, Fax	Assessor's Reference: Chart-Block-Lot

Approval Conditions of Planning

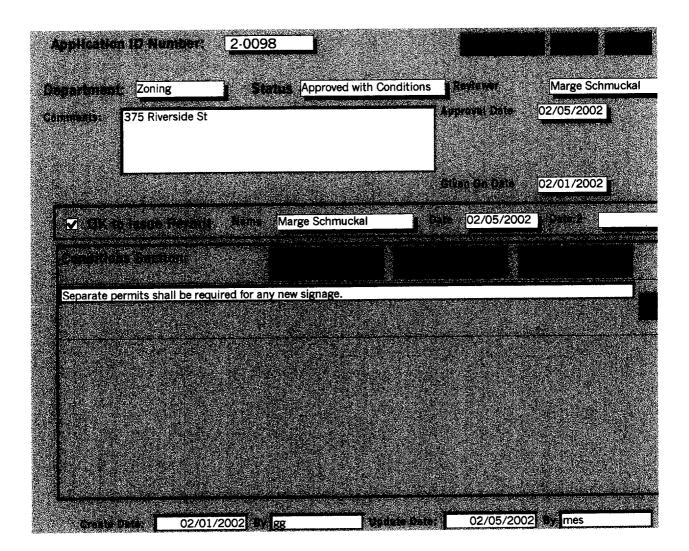
1 that the applicant provide evidence of financial capability to undertake and complete the development, which will include a letter from a responsible financial institution stating that it has reviewed the planned development and would seriously consider financing it.

Approval Conditions of DRC

The existing wheel stops along the existing building shall be removed prior to occupancy. This should be done to discourage parking between the buildings, which would interfere with traffic circulation around the new building.

Approval Conditions of Zoning

Separate permits shall be required for any new signage.



7	Seignem & Sebagotechnic 3. Com
J .	Applicant: BIG Moose Holay DAVIDAVE: 11/6/01
	Address: 375 Riverside St C-B-L: 317-B-005
	CHECK-LIST AGAINST ZONING ORDINANCE
	Date-EXIST
3	Zone Location - B-4
	Interior or corner lot -
	Proposed UserWork- 5000# Bldgfor Dinta Storage of motorcylces Servage Disposal - (f AGESSORY to The retail Sales)
	Lot Street Frontage - 60 m ~
	Front Yard - 20 mi - 50+
	Rear Yard - 20' m - 99.5'8h
	Side Yard - 10 Man _ 40'8 km consulstory are 121 high
	Projections - N
	Width of Lot - 60' - 100'+
	Height - 65 mpx - 23' Show = 95 x 240 = 22,800
	Lot Area - 10,000 fm - 121,839 fm ASSESS, 25 × 3/0 = 7,750 ft Lot Coverage Impervious Surface - 80 6 MAX - 97471,2 mAX 30,550 ft Lot Coverage Impervious Surface - 80 6 MAX - 97471,2 mAX
	Lot Coverage Impervious Surface - 80 6 MAX - 97471,7 MAY Just Along Area par Family - 1) A 367,84 ofen \$C. Side & Feet
	min open
	Off-street Parking - Stores (101,000)
	Loading Bays - Show
	Site Plan - Mino (# 7001-0298)
	Shoreland Zoning/Stream Protection - NA
	Flood Plains - Parel 6 - Zac X 6140
	FLOOR ATTER RATIO - ABBO 65 Tey - TOTAL Floor Arch 1960 1960 FLOOR ATTER RATIO - ABBO 65 Tey - TOTAL Floor Arch 121,839 - 162

2001-0298

		Planning Copy	Application I. D. Number
Big Moose Harley Davidson			11/02/2001
Applicant		-	Application Date
375 Riverside St., Portland, ME 04103			5000 Sq.ft. Building/storage for bikes
Applicant's Mailing Address		_	Project Name/Description
Sebago Technics/Jim Seymour		375 - 375 Riverside St, Portla	
Consultant/Agent		Address of Proposed Site	
	Fax: (207) 856-2206	317 B005001	
Applicant or Agent Daytime Telephone, Fa		Assessor's Reference: Chart-B	Block-Lot
Proposed Development (check all that appl	ly): 📝 New Building 🗌	Building Addition	☐ Residential ☐ Office 📝 Retail
Manufacturing Warehouse/Distril	bution Parking Lot	☐ Other (specify) storage facility
5000 s.f	- -	-	B4
Proposed Building square Feet or # of Units	s Acres	age of Site	Zoning
Check Review Required:			
	Cubdidalan	- DAG Daviena	
✓ Site Plan (major/minor)	Subdivision # of lots	☐ PAD Review	14-403 Streets Review

Flood Hazard	Shoreland	HistoricPreservation	DEP Local Certification
Zoning Conditional	Zoning Variance		☐ Other
Use (ZBA/PB)			
Fees Paid: Site Plan \$400.00	Subdivision	Engineer Review \$300.	.00 Date 01/25/2002
Planning Approval Status:		Reviewer Kandi Talbot	
	A		
☐ Approved	Approved w/Conditions See Attached	☐ Denied	
	See Allached		
Approval Date 11/21/2001 A	Approval Expiration 11/21	1/2002 Extension to	
✓ OK to Issue Building Permit	Kandi Talbot	04/04/0000	Attached
ON to issue building Ferrint	signature	01/31/2002 date	
	oignataro		
Performance Guarantee	Required*	☐ Not Required	
* No building permit may be issued until a pe	erformance quarantee has	been submitted as indicated below	
✓ Performance Guarantee Accepted	01/24/2002	\$14,120.00	10/29/2002
Inspection Eco Deid	date	amount	expiration date
Inspection Fee Paid			***************************************
Duilding Downik Issue	date	amount	
Building Permit Issue	data		
Dorformana Currentes Bad	date		
Performance Guarantee Reduced	data		
Tomorrow On William 1 CO	date	remaining balance	signature
Temporary Certificate of Occupancy	4-4-	Conditions (See Attached)	
Circl Insure attack	date		expiration date
Final Inspection	data		
Contidents of Orange	date	signature	
Certificate Of Occupancy	dete		
Performance Cuerantee Beleased	date		
Performance Guarantee Released	plants.		·
Defect Guerontee Outitt	date	signature	
Defect Guarantee Submitted	عاداد الممالالمطاري		
Defect Guarantee Released	submitted date	amount	expiration date
Scient Guarantee Released	data		
	date	signature	

2001-0298 Application I. D. Number

Big Moose Harley Davidson	11/02/2001
Applicant	Application Date
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Applicant's Mailing Address	Project Name/Description
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Consultant/Agent	Address of Proposed Site
Applicant Ph: (207) 797-6061 Agent Fax: 2078562206	317 B005001
Applicant or Agent Daytime Telephone, Fax	Assessor's Reference: Chart-Block-Lot

Approval Conditions of Planning

that the applicant provide evidence of financial capability to undertake and complete the development, which will include a letter from a responsible financial institution stating that it has reviewed the planned development and would seriously consider financing it.

Approval Conditions of DRC

1 The existing wheel stops along the existing building shall be removed prior to occupancy. This should be done to discourage parking between the buildings, which would interfere with traffic circulation around the new building.

2001-0298

		DRC Copy	Application I. D. Number
Big Moose Harley Davidson			11/02/2001
Applicant		_	Application Date
375 Riverside St., Portland, ME 0410	03		5000 Sq.ft. Building/storage for bikes
Applicant's Mailing Address		-	Project Name/Description
Sebago Technics/Jim Seymour		375 - 375 Riverside St, Port	
Consultant/Agent		Address of Proposed Site	
Applicant Ph: (207) 797-6061 Applicant or Agent Daytime Telephone	gent Fax: (207) 856-2206	317 B005001	
·		Assessor's Reference: Chart-	Block-Lot
Proposed Development (check all that		Building Addition	Residential Office 😿 Retail
Manufacturing Warehouse/D	istribution Parking Lot	Other	(specify) storage facility
5000 s.f			B4
Proposed Building square Feet or # of	Units Acre	age of Site	Zoning
Check Review Required:			
Site Plan	Subdivision	☐ PAD Review	☐ 14-403 Streets Review
(major/minor)	# of lots		
Flood Hazard	Shoreland	☐ HistoricPreservation	DED Lacel Continue
لسنا		HistoricPreservation	DEP Local Certification
Zoning Conditional Use (ZBA/PB)	Zoning Variance		☐ Other
OSE (ZDAT B)			
Fees Paid: Site Plan \$400	.00 Subdivision	Engineer Review \$300	0.00 Date 01/25/2002
DPC Approval Status		Reviewer Jay Reynolds	
DRC Approval Status: ☐ Approved	Ammuored auto anditi		
Approved	Approved w/Conditions See Attached	☐ Denied	
	oce Allachieu		
Approval Date11/21/2001	Approval Expiration 11/21	1/2002 Extension to	Additional Sheets
Condition Compliance	Jay Reynolds	11/21/2001	Attached
	signature	date	
Performance Guarantee		Not Required	
* No building permit may be issued until	a performance guarantee has	been submitted as indicated below	
Performance Guarantee Accepted	01/24/2002	\$14,120.00	10/29/2002
<u> </u>	date	amount	expiration date
Inspection Fee Paid		arricant	ospiialion uate
<u></u>	date	amount	
Building Permit Issue		will be a second	
	date		
Performance Guarantee Reduced			
_	date	remaining balance	signature
Temporary Certificate of Occupancy		Conditions (See Attached)	0.3.1440
	date		expiration date
Final Inspection			onplication date
-	date	signature	
Certificate Of Occupancy			
_ · · ·	date		
Performance Guarantee Released			
	date	signature	
Defect Guarantee Submitted			
	submitted date	amount	expiration date
Defect Guarantee Released			z.p., audit dato
	date	signature	

signature

CITY OF PORTLAND, MAINE **DEVELOPMENT REVIEW APPLICATION**

PLANNING DEPARTMENT PROCESSING FORM **ADDENDUM**

Application I. D. Number 11/02/2001 **Application Date** 5000 Sq.ft. Building/storage for bikes Project Name/Description

2001-0298

375 Riverside St., Portland, ME 04103 Applicant's Mailing Address Sebago Technics/Jim Seymour Consultant/Agent

375 - 375 Riverside St, Portland, Maine

Address of Proposed Site

317 B005001

Assessor's Reference: Chart-Block-Lot

Applicant Ph: (207) 797-6061 Agent Fax: 2078562206 Applicant or Agent Daytime Telephone, Fax

Big Moose Harley Davidson

Applicant

Approval Conditions of Planning

1 that the applicant provide evidence of financial capability to undertake and complete the development, which will include a letter from a responsible financial institution stating that it has reviewed the planned development and would seriously consider financing it.

Approval Conditions of DRC

The existing wheel stops along the existing building shall be removed prior to occupancy. This should be done to discourage parking between the buildings, which would interfere with traffic circulation around the new building.

D. Itment of Planning and Urban Develops. It SUBDIVISION/SITE DEVELOPMENT

COST ESTIMATE OF IMPROVEMENTS TO BE COVERED BY PERFORMANCE GUARANTEE

				Da	ite: <u>12/4</u>	1/01
Name of Project: B16	MOOSE H	arley-Di	AVIDSON -	BUILDING AD	MOTIO	,
Address/Location: 375	RIVERS	DE ST.	PORTLAND	ME		
Developer CONTRACTIC. PA	TO CON	TRUCTION	U INC.	·	·	
Form of Performance Guarantee:						
Type of Development: Subdivis	ion	Site I	Plan (Major/Mino	MINOR		
TO BE FILLED OUT BY THE						
		PUBLIC			PRIVATE	
<u>Ytem</u>	Quantity	Unit Cost	<u>Subtotal</u>	Quantity	Unit Cost	Subtotal
1. STREET/SIDEWALK Road / PANEMENT Granic Curbing Sidewalks Esplanades Monuments				2201	\$46/T	8800
Street Lighting Street Opening Repairs Other 2. EARTH WORK		ENTS				
Cut Fill		N SEE				
3. SANITARY SEWER Manholes Piping Connections Main Line Piping House Sewer Service Piping Pump Stations Other		LICE INTERCE	<u> </u>			
4. WATER MAINS		N/A				
5. STORM DRAINAGE Manholes Catchbasins Piping Detention Basin Stormwater Quality Units Other		1 000				

12/	/04/2001 16:00	SEBAGO TECHNIC	CS				03
5.	SITE LIGHTING	- 12			450 EA.	1900 -	
7.	EROSION CONTROL Silt Fence Check Dams Ripe Inlet/Outlet Protection Level Lip Spreader Slope Stabilization Geotextile Hay Bale Barriers Catch Basin Inlet Protection		200 EP		# 4 LP	1520	
8.	RECREATION AND OPEN SPACE AMENITI	res $\frac{\partial}{\partial z}$	7	_			
9.	LANDSCAPING (Attach breakdown of plan materials, quantities, and to costs)		 	B Austr Pine (u-7	ian <u> </u>	<u>₹ 2000.</u> 00	
10.	MISCELLANEOUS						
	TOTAL:			#14	120		
	GRAND TOTAL:	-		#14	120. =		
IN	SPECTION FEE (to be fi		private	Т	OTAL]
	-	PUBLIC	PRIVATE	• ·	<u> </u>		
A:	2.0% of totals:						
	<u>or</u>						
В:	Alternative Assessment:			-			
	Assessed by:	(name)	(name)				



Site Review Pre-Application Multi-Family/Attached Single Family Dwellings/Two-Family Dwelling or Commercial Structures and Additions Thereto

In the interest of processing your application in the quickest possible manner, please complete the Information below for Site Plan
Review

NOTE**If you or the property owner owes real estate or personal property taxes or user charges on ANY PROPERTY within the City, payment arrangements must be made before permits of any kind are accepted.

BIG MOOSE HARLEY DAVIDSON	10 31 01					
Applicant 375 RIVERSIDE ST. PORTLAND	Application Date MOTORCYCLE - STORAGE BUILDING					
Applicant's Mailing Address SEBAGO TECHNICS INC	Project Name Description 375 RIVERSIDE ST.					
Consultant/Agent C/O JIM SEYMOUR 856-0277 TEL 856-7206 EAX	Address Of Proposed Size MAP (317) BLOCK(B) LOT(5)					
Applicant/Agent Daytime telephone and FAX	Assessor's Reference, Chart#, Block. Lot#					
Proposed Development (Check all that apply)New Building	Building Addition Change of Use Residential Office Retail					
Manufacturing Warehouse/Distribution Oth	er(Specify)					
5000 SF	2.8± AC = B-A					
Proposed Building Square Footage and /or # of Units	Acreage of Site Zoning					
You must Include the following with you application: 1) A Copy of Your Deed or Purchase and Sale Agreement 2) 7 sets of Site Plan packages containing the information found in the attached sample plans and checklist. Checklist. (Section 14-522 of the Zoning Ordinance outlines the process, copies are available for review at the counter, photocopies are \$ 0.25 per page)						
that I have been authorized by the owner to make this appli- this jurisdiction. In addition, if an approval for the proposed	property, or that the proposed work is authorized by the owner of record and cation as his/her authorized agent. I agree to conform to all applicable laws of I project or use described in this application is issued, I certify that the Code to enter all areas covered by this approval at any reasonable hour to enforce					
Signature of applicant: ames Person	nou Date: 10/3/01					
Site Review Fee	e: Major \$500.00 Minor 400.00					
	uilding Permit application and associated fees will be required					
priq	or to construction.					



VP BUILDINGS, INC.

273 Water Street
Evansville, WI 53536

Facsimile Cover Sheet

To:	Bill Rudman
Company:	Patco Construction
Fax:	207-342-1643
From:	David Cockrum
Company:	VP Buildings, Inc.
Phone:	(608) 882-5001 Ext. 209
Fax:	(608) 882-2364
Date:	2/12/02
Pages including this cover page:	3
Subject:	WI0115377-01CE1

MESSAGE:

The building for Big Moose Harley-Davidson, which was originally designed for 1996 B.O.C.A. has been found adaquete to meet or exceed 1999 B.O.C.A. loading.



LETTER OF CERTIFICATION

Date: 02/12/2002 Time: 2:02 PM

Page: 1 of 2

Contact: Bill

Name: PATCO Construction Inc

City, State: Sanford, Maine 04073

Address: 1293 Main St.

Country: United States

Project: Big Moose Harle /-Davidson Reference: Big Moose Harley.vpc

Jobsite: 375 Riverside

City, State: Portland, Mane 04103

County, Country: Cumberland, United States

This is to certify that the above referenced VP BUILDINGS project has been designed for the applicable portions of the following Building Code and in accordance with the order documents which have stipulated the following applied environmental loads and conditions:

Overall Building Description

Shape	Overall Width	Overall Length	Floor Area	Wall Arca	Roof Area (sq. fl.)	Max. Eave Height	Min. Eave			
Big Moose Harley	50/0/0	100/0/0	5000	6287	5004	22/0/0	Height 2 19/11/0	Pitch 0.500:12	Pitch	Height

Loads and Codes - Shape: Big Moose Harley

Building Use: Standard Occupancy Structures

City: Portland Building Code: BOCA - 1999 - National Building Code

County: Cumberland

Snow Load

State:

Maine

Country: United States Rainfall: 4.00 in per hour

Built Up: Cold Form:

89AISC **96AIS1**

Allow. Overstress:

Frm: 1.03, Sec: 1.03, Brc: 1.03

Dead and Collateral Loads

Collateral Gravity:1.00 psf Collateral Uplift: 0.00 psf

Roof Covering + Second. Dead Load: 2.44 psf Frame Weight (assumed for seismic):2.50 psf

Snow Exposure Category (Factor): 2 (1.00)

Thermal Category (Factor): Heated (1.00)

Ground Snow Load: 70.00 psf

Snow Importance: 1.000

Design Snow (Sloped): 49.00 psf

Ground / Roof Conversion: 0.70

% Snow Used in Seismic: 20.00

Seismic Snow Load: 9.80 psf

Live Load

Live Load: 20.00 psf Reducible Li. for Below Eave Canopy:N/A

Wind Load

Wind Speed: 90.00 mph

Primaries Wind Exposure (Factor): B (0.427)

Parts Wind Exposure (Factor): C (0.881)

Wind Enclosure: Enclosed Wind Importance Factor: 1.094 Distance to Coast: 6.0 Miles

Base Elevation: 0/0/0

Primary Zone Strip Width: N/A

Parts / Portions Zone Strip Width: 5/0/0

Basic Wind Pressure: 9.68, (Parts) 19.99 psf

Moment-Resisting Frame System Ordinary Steel Frames (R=4.5 Cd= 4.0) Building Frame System Concentrically Braced Frames (R=5.0 Cd= 4.5)

Analysis Procedure 1610.4 used

Scismic Load

Scismic Hazard / Use Group: Group 1

Scismic Importance: 1,000

Seismic Performance / Design Category: C

Framing Seismic Period: 0.5828 B acing Scismic Period: 0.3330

A t: 0.1000, Av: 0.1000

Figme Seismic Factor (Cs): 0.0556

B ace Seismic Factor (Cs): 0.0500

Per Article 2.9 in the Builder Agreement, VP Buildings assumes that the Builder has called the local Building Official or Project Engineer to obtain all code and loading information for this specific building site.

The steel design is in accordance with VP BUILDINGS standard design practices, which have been established based upon pertinent procedures and recommendations of the following organizations:

American Institute of Steel Construction (AISC)

American Iron and Steel Institute (AISI)

American Welding Society (AWS) [D1.1]

American Society for Testing and Materials (ASTM)

Metal Building Manufacturers Association (MBMA)

AISC Category MB Manufacturer Certification.

This certification DOES NOT apply to the design of the foundation or other on-site structures or components not supplied by VP BUILDINGS, nor does it apply to unauthorized modification is based upon the premise that all components furnished by VP BUILDINGS will be erected or constructed in

Strict compliance with perturbil documents furthered by VP BUILDINGS.

CARL W.

WALKER

3200 Players Child Circle, Memphis TN 38133-1843

GISTER SONAL ENGLISH SCISTERY

P.E. Prepared by:

VPC File: WI0115377-010E1.vpc



LETTER OF CERTIFICATION

Date: 02/12/2002 Time: 2:02 PM

Page: 2 of 2

The Structural Design and/or Manufacture of this VP BUILDINGS building will be or has been at one of the following VP Buildings locations:

Rainesville, ALVP Alabama Plant	[Manufacture Only]
Memphis, TNVP Headquarters	Design Onlyl
Pine Bluff, AR VP Arkansas Service Center	Design and Manufacturel
Turlock, CA VP California Service Center	Design and Manufacture
St. Joseph, MO VP Missouri Service Center	Design and Manufacture
Kernersville, NCVP North Carolina Service Center.	Design and Manufacture
VanWert, OHVP Ohlo Service Center	Design and Manufacture
Evansville, WIVP Wisconsin Service Center	[Design and Manufacture]

Additional Structural Material may be fabricated and provided for use in a VP Buildings building by one of the following fabricators:

BAR JOISTS-

SMI, Inc. Hopc, AR

Hancock Salem, VA

Canam

Washington, MO

Vulcraft Grapeland, TX Vulcraft Norfolk, NE Vulcraft Florence, SC Vulcraft Brigham City, UT

ISP El Paso, TX

Socar Florence, SC

Quincy Quincy, FL

(This information is presented in compliance with VP Building's AISC Certification responsibilities.)

2/8/02 Pre-con w/ Contractors and Jay Raynolds, Dev-New Coodmitn, Question on electristy To proposed belowing, Al.

389 Congress St. Portland, ME 04101 Phone: (207)874-8700 Fax: (207)874-8716

facsimile transmital

То:	Dennis Water	From:	Mike Nugent	
Fax:	(207)324-1643	Date:	February 11, 200	02
Phone:	(207)324-5574	Pages:	2 5	
Re:	Bull Mosse Plans (317 B005)	CC:		
□ Urgei	nt ☐ For Review ☐ Please Comm	nent	☑ Please Reply	☐ Please Recycle
Notes	• • • • • • • • • • • • • • • • • • •	w of the	• • plans and ne	• eed the following
	the issuance of either the foundation			
	ppears that the Mezzanine is not accou		•	
	dings. Does this additional load affect			
office of the	nponents as well as their footings/fou		, spacing or des	ight of these
* 53 * * * * * * * * * * * * * * * * * *	Certification docs do not reflect com		rith the 1000 DO	
19 9		phance v	Mar the 1999 BO	CA code but the
	ne 6 inch thickness of unreinforced co			
	de compliant? I find no foundation wal	I thickne	sses of less that	7.5 inches in
	tion 1812 of BOCA			
The Fo	ollowing are preliminary question	ns to c	ommence rev	iew of the
structu	ire:			
	he " Mezzanine" exceeds the 1/3 floo	r area ra	tion and must be	treated as a
S	eparate floor.			

	3) There are no Stair details.
	4) A second means of egress if required for this "mezzanine"
	5) There are hand drawn modifications for overhead door relocations. Were these
	done by the design professional?
	6) Attached is Section 1705. Special Inspections. Please have your design
•	professional file the required statement of special inspection.
	7) The Construction Cost of \$108,000, or 21.60 per sq. ft. is less than 1/3 of the
	R.S. Means estimates. Please justify this number.

There are not wall or guard details.



CITY OF PORTLAND MAINE

389 Congress St., Rm 315 Portland, ME 04101 Tel. – 207-874-8704 Fax - 207-874-8716

TO:

Inspector of Buildings City of Portland, Maine

Planning & Urban Development	
Division of Housing & Community Services	
FROM DESIGNER: JOHN W. EINSIEDWE, R.A.	
DATE: JANUARY 28, 2002	
Job Name: BIG MOOSE HAZLEY- COLD STORAGE BLG. Address of Construction: 375 RIVERSIDE ST. PORTLAND	•
THE BOCA NATIONAL BUILDING CODE/1999 FourteenthEDITION Construction project was designed according to the building code criteria listed below:	
Building Code and Year Boca '99 ARCHTECT Use Group Classification(s) 50-55-5000 5F	12T MEZZAWINE
IN MAT MERRAMAN INKK MUMITS	7
Seismic Zone Group Class CI 2000	
Roof Snow Load Per Sq. Ft. 49+1 COLLARGE Dead Load Per Sq. Ft. PER VALLO PRODI	
Basic Wind Speed (mph) 90 mph Effective Velocity Pressure Per Sq. Ft. 2018	STRUGURE
basic wind opera (inproj	•
Floor Live Load Per Sq. Ft. 12.5 psf	•
~	PER VARLO PROBO
Structure has full sprinkler system? YesNo Alarm System? YesNo Sprinkler & Alarm systems must be installed according to BOCA and NFPA Standards with approval from the	•
Structure has full sprinkler system? YesNo Alarm System? YesNo Sprinkler & Alarm systems must be installed according to BOCA and NFPA Standards with approval from the Portland Fire Department.	•
Structure has full sprinkler system? Yes No Alarm System? Yes No Sprinkler & Alarm systems must be installed according to BOCA and NFPA Standards with approval from the Portland Fire Department. Is structure being considered unlimited area building: YesNo If mixed use, what subsection of 313 is being considered List Occupant loading for each room or space, designed into this Project.	•
Structure has full sprinkler system? YesNo Alarm System? YesNo Sprinkler & Alarm systems must be installed according to BOCA and NFPA Standards with approval from the Portland Fire Department. Is structure being considered unlimited area building: YesNo If mixed use, what subsection of 313 is being considered List Occupant loading for each room or space, designed into this Project.	•



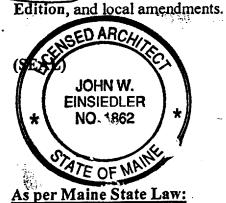


CITY OF PORTLAND **BUILDING CODE CERTIFICATE** 389 Congress St., Rm 315 Portland, ME 04101

TO:	Inspector of Buildings City of Portland, Maine Department of Planning & Urban Development Division of Housing & Community Service						
FROM:	JOHN W.	EINSIEOLOR	12.1				
RE:	Certificate of Design						
DATE:	JANUARY	28,2002					
These plans	and/or specifications cov	ering construction work on:					

FREVIEWED Have been designed and drawn up by the undersigned, a Maine registered architect/ongineer according to the BOCA National Building Code/1999 Fourteenth

BILL MODSE HARLEY - COLD STORAGE BLG.



Signature_

ARCHITEU Title

W. ENSIEDUSE, R.A.

Address

148 SEA ROGO KENNEBUNK, ME 04043

\$50,000.00 or more in new construction, repair, expansion, addition, or modification for Building or Structures, shall be prepared by a registered design Professional.

PSH 6/20/2k



City of Portland, Maine 389 Congress St., Rm 315 Portlaind, ME 04101

ACCESSIBILITY CERTIFICATE

TO:	Inspector of Buildings City of Portland, Maine
	Department of Planning & Urban Development
	Division of Housing & Community Services
EDOM:	JOHN W. EINSKEDUR
FROM:	•
RE:	Certificate of Design, HANDICAP ACCESSIBILITY
DATE:	JANUARY 28, 2002
These plans a	and/or specifications covering construction work on:
These plans	Co. a Smarry Bla
Big_	MOOSE HARLEY- COLD STORMER BLG,
374	RIVERSION ST. PORTLAND
	_
Have been de	esigned and drawn up by the undersigned, a Maine registered
engineer/arch	nitect according to State Regulations as adopted by the State of Maine on
Handicapped	Accessibility.
	Signature
(SEAL)	
•	Title ARCHITECT
GED	Firm JOHN W. EINSIEDLER, R.A.
SHIP	Firm JOHN W. EINSNEDLER, R.A.
	Address 148 SEA Borno KENNED UNG ME 1862 *
11	EDLER KEUNED UNG ME
/ */ NO	1862 (*) 04043



PATCO

CONSTRUCTION, INC.

February 11, 2002

Mike Nugent
Building Inspection Dept.
City of Portland
389 Congress St.
Portland, ME 04101

Re: Big Moose Harley, Riverside St. (317 B005)

Dear Mike:

In response to your fax dated February 11, 2002, we submit the following:

<u>Needs</u>

- The mezzanine is part of the pre-engineered building that is to be engineered, manufactured and delivered by V.P. Buildings, Inc. V.P. will provide support beams, columns, bar joists and any necessary "beef-up" to the structure to support the mezzanine load.
 - We will provide a V.P. drawing showing mezzanine framing.
- 2. We will provide certification from V.P. to show compliance with Boca 1999 code.
- Details "D" and "A" on the foundation plan does show a portion that is to be 6" thick and unreinforced. The portion that is to be 6" thick is only a +/- 12" long "notch" to allow the sidewall columns to be set on the foundation pier. This portion of the wall is completely above grade inside and outside. Typical wall section "I-I" show six rows of horizontal rebar. These runs will be continuous through the entire wall.

Questions

- 1. We will discuss the "mezzanine" exceeding the 1/3 floor area with the building owner and our architect. One possible solution is to eliminate one bay of the mezzanine to reduce the size.
 - We will submit revised plan(s).





- No interior walls were planned for the inside of the storage building.
 We will submit detail for guardrail at the exposed edge of the mezzanine.
- 3. We will submit stair detail.
- 4. See #1.
- 5. Any hand drawn modifications (including OHD relocations) were done by design professionals at V.P. Buildings, Inc. This is their common practice for some details and late changes.
- 6. We will file the required statement of special inspection.
- 7. The building will be used for cold storage. Our price is for a building shell (foundation, slab, metal building, overhead doors and pass doors) and does not include a heat, interior finish, etc., at this time. A price of \$20.00 per sq. ft. (or less) for a pre-engineered metal building shell in this area is the current competitive rate.

We will forward the above listed information as soon as possible. I hope that this response is sufficient to release the foundation permit. Please call with any questions.

Sincerely,

Dennis M. Waters Vice President

DMW/cmp



PATCO

February 28, 2002

Mike Nugent Building Inspection Dept. City of Portland 389 Congress St. Portland, ME 04101

Re: Big Moose Harley, Riverside St. (317 B005)

Dear Mike:

In response to your fax dated February 11, 2002, we submit the following:

Needs

1. The mezzanine is part of the pre-engineered building that is to be engineered, manufactured and delivered by V.P. Buildings, Inc. V.P. will provide support beams, columns, bar joists and any necessary "beef-up" to the structure to support the mezzanine load.

Attached please find:

- Structural design data, stamped 12/18/01 (mezzanine loading highlighted).
- V. P. cover sheet (mezzanine loading specs)
- V. P. Sheet SP1 (mezzanine detail sheet)
- 2. Attached please find V. P. Buildings letter of certification (3 pages), recompliance with BOCA 99.
- 3. Details "D" and "A" on the foundation plan does show a portion that is to be 6" thick and unreinforced. The portion that is to be 6" thick is only a +/- 12" long "notch" to allow the sidewall columns to be set on the foundation pier. This portion of the wall is completely above grade inside and outside. Typical wall section "I-I" show six rows of horizontal rebar. These runs will be continuous through the entire wall.





Questions

- 1. We have reduced the proposal size of the mezzanine by one (1) "bay". The mezzanine is now 1,640 sq. ft., which is less than 1/3 of the 5,000 sq. ft. first floor. See sheet A-1 attached.
- 2. No interior walls were planned for the inside of the storage building.See sheet A-1 for guard rail detail.
- 3. See sheet A-1 for stair detail.
- 4. Second means of egress not required.
- 5. Any hand drawn modifications (including OHD relocations) were done by design professionals at V.P. Buildings, Inc. This is their common practice for some details and late changes.
- 6. Statement of special inspection (Sect. 1205) to be submitted seperatly.
- 7. The building will be used for cold storage. Our price is for a building shell (foundation, slab, metal building, overhead doors and pass doors) and does not include a heat, interior finish, etc., at this time. A price of \$20.00 per sq. ft. (or less) for a pre-engineered metal building shell in this area is the current competitive rate.

I hope that this response is sufficient to release the building permit. Please call with any questions.

Sincerely,

Dennis M. Waters Vice President

DMW/cmp

From:

Marge Schmuckal

To:

Kandi Talbot

Subject:

Big Moose Harley- 375 Riverside St

Kandi,

I have reviewed the submitted site plan for Big Moose Harley. It is located within the B-4 zone not the I-M zone.

All setbacks are being met. The height requirements are being met. Parking requirements are being met. The maximum 80% impervious surface requirement is being met. I did some quick figures just of the partial lot plan that we have, and that portion (without adding in open frontage areas) are more than meeting the impervious surface requirements. Also the F.A.R. (floor area ratio) is more than being met.

Zoning is being met on this proposal. - Marge

CC:

Internet@jseymour@sebagotechnics.com; Sarah Hopk...

DESIGN LOADS AND REACTIONS

Date: 12/21/2001 **Time:** 2:36 PM

Page: 1 of 27

VP Buildings, Inc.

3200 Players Club Circle Memphis, TN 38125-8843

STRUCTURAL DESIGN DATA

Project: Big Moose Harley-Davidson Name: WI0115377-010E1 Reference: Big Moose Harley.vpc Jobsite: 375 Riverside

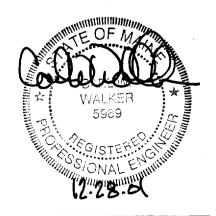
City, State: Portland, Maine 04103 County: Cumberland Country: United States

TABLE OF CONTENTS

Building Loading - Expanded Report	2
Reactions - Expanded Report	
Reactions - Expanded Report	

Designed by David Cockrum, EIT

MEZZANINE LOAding INcluded (SEE pages 6,7,8)





DESIGN LOADS AND REACTIONS

Date: 12/21/2001 Time: 2:36 PM

Page: 2 of 27

Building Loading - Expanded Report

Shape: Big Moose Harley

Loads and Codes - Shape: Big Moose Harley

County: Cumberland City: Portland Building Code: BOCA - 1996 - National Building Code

Building Use: Standard Occupancy Structures Allow, Overstress:Frm: 1.03, Sec: 1.03, Brc: 1.03 State: Maine

Built Up: 89AISC Cold Form: 89AISI

Rainfall: 4.00 in per hour

Country: United States

Dead and Collateral Loads

Collateral Gravity:1.00 psf

Collateral Uplift: 0.00 psf

Frame Weight (assumed for seismic):2.50 psf

Applied to Side Type Mag Shape

2.437 psf Entire Frm D D 1.130 psf Entire Pur

Description Covering Weight - 24 SSR + Secondary Weight 1.31 - Roof: A Covering Weight - 24 SSR - Roof: A

Live Load

Live Load: 20.00 psf Reducible

LL for Below Eave Canopy:N/A

Wind Load

Wind Speed: 90.00 mph Wind Enclosure: Enclosed

Height Used: 20/11/8 (Type: Mean) Base Elevation: 0/0/0

Primary Zone Strip Width: N/A Velocity Pressure: (qz) 22.68 psf

Primaries

Primaries Wind Exposure (Factor): B (0.427)

Basic Wind Pressure: 9.68 psf

Basic Wind Pressure: 19.99 psf

Snow Load

Ground Snow Load: 70.00 psf Design Snow (Sloped): 49.00 psf

Snow Importance: 1.000

Ground / Roof Conversion: 1.00

Snow Exposure Category (Factor): 3 (0.70)

Slope Used: 2.386 (0.500:12)

Seismic Load

Seismic Hazard / Use Group: Group 1 Seismic Performance / Design Category: C

Aa: 0.1000, Av: 0.1000 Seismic Importance: 1.000

Frame Seismic Factor (Cs): 0.0556

Brace Seismic Factor (Cs): 0.0500

Moment-Resisting Frame System Ordinary Steel Frames (R=4.5 Cd= 4.0) Building Frame System Concentrically Braced Frames (R=5.0 Cd= 4.5)

Analysis Procedure 1610.4 used

Gust Factor: 1.5789 Wind Importance Factor: 1.046 Least Horiz. Dimension: 50/0/0 Distance to Coast: 6.0 Miles Parts / Portions Zone Strip Width: 5/0/0 $qz = 0.00256 * (1.05 * 90.00)^2 * (1.00)$ Parts and Portions Parts Wind Exposure (Factor): C (0.881)

Rain Surcharge: 0.00 Slope Reduction: 1.00

Seismic Snow Load: 9.80 psf

Soil Factor 2.00

Framing Seismic Period: 0.5828 Bracing Seismic Period: 0.3330

Side	Туре	Mag	Units	Shape	Applied to	Description
1	E	0.161	psf	Spec	Frm	Seismic: Covering Weight - 26 Vee Rib + Secondary Weight 1.96 - Wall: 1
1	Ē	0.161	psf	Spec	Frm	Seismic: Covering Weight - 26 Vee Rib + Secondary Weight 1.96 - Wall: 1
1	Ē	0.161	psf	Spec	Frm	Seismic: Covering Weight - 26 Vee Rib + Secondary Weight 1.96 - Wall: 1
1	E	0.161	psf	Spec	Frm	Seismic: Covering Weight - 26 Vee Rib + Secondary Weight 1.96 - Wall: 1
1	E	0.145	psf	Spec	Brc	Seismic: Covering Weight - 26 Vee Rib + Secondary Weight 1.96 - Wall: 1
1	Ē	0.145	psf	Spec	Brc	Seismic: Covering Weight - 26 Vee Rib + Secondary Weight 1.96 - Wall: 1
ī	Ē	0.145	psf	Spec	Brc	Seismic: Covering Weight - 26 Vee Rib + Secondary Weight 1.96 - Wall: 1
1	E	0.145	psf	Spec	Brc	Seismic: Covering Weight - 26 Vee Rib + Secondary Weight 1.96 - Wall: 1
1	E	3.056	psf	Rect	Frm	Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 1
1	Ē	3.056	psf	Rect	Frm	Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 1
1	E	3.056	psf	Rect	Frm	Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 1
1	E	3.056	psf	Rect	Frm	Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 1
1	E	2.750	psf	Rect	Brc	Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 1
1	Е	2.750	psf	Rect	Brc	Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 1
1	E	2.750	psf	Rect	Brc	Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 1
1	E	2.750	psf	Rect	Brc	Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 1
2	·E	0.102	psf	Rect	Frm	Seismic: Covering Weight - 26 Vee Rib + Secondary Weight 0.89 - Wall: 2



DESIGN LOADS AND REACTIONS

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```
Seismic: Covering Weight - 26 Vee Rib + Secondary Weight 0.89 - Wall: 2
     Ē
             0.102
                     psf
                               Rect
                                       Frm
2
                                                         Seismic: Covering Weight - 26 Vee Rib + Secondary Weight 0.89 - Wall: 2
                                       Frm
2
     E
             0.102
                     psf
                              Rect
                                                         Seismic: Covering Weight - 26 Vee Rib + Secondary Weight 0.89 - Wall: 2
2
             0.102
                     psf
                               Rect
                                       Frm
     E
                                                         Seismic: Covering Weight - 26 Vee Rib + Secondary Weight 0.89 - Wall: 2
     E
             0.092
                     psf
                               Rect
                                       Brc
                                                         Seismic: Covering Weight - 26 Vee Rib + Secondary Weight 0.89 - Wall: 2
2
                     psf
     E
             0.092
                               Rect
                                       Brc
                                                         Seismic: Covering Weight - 26 Vee Rib + Secondary Weight 0.89 - Wall: 2
2
     E
             0.092
                     psf
                               Rect
                                       Brc
                                                         Seismic: Covering Weight - 26 Vee Rib + Secondary Weight 0.89 - Wall: 2
2
                     psf
                               Rect
                                       Brc
     Е
             0.092
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 2
                                       Frm
     E
             3.056
                     psf
                               Rect
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 2
2
                     psf
     E
             3.056
                               Rect
                                       Frm
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 2
                                       Frm
     Ε
             3.056
                     psf
                               Rect
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 2
2
     Ε
                     psf
                               Rect
                                       Frm
             3.056
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 2
2
     E
             2.750
                     psf
                               Rect
                                       Brc
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 2
2
     E
             2.750
                     psf
                               Rect
                                       Brc
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 2
2
     Ε
             2.750
                     psf
                               Rect
                                       Brc
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 2
2
     E
                     psf
                               Rect
                                       Brc
             2.750
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 2
                                       Frm
2
     E
             3.056
                     psf
                               Rect
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 2
2
     E
                     psf
                               Rect
                                       Frm
             3.056
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 2
                                       Frm
2
     E
             3.056
                     psf
                               Rect
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 2
                     psf
2
     E
             3.056
                               Rect
                                       Frm
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 2
                                       Brc
2
     E
             2.750
                               Rect
                     psf
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 2
2
                               Rect
                                       Brc
      E
             2.750
                     psf
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 2
2
                                       Brc
      Ε
                               Rect
             2.750
                     psf
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 2
2
             2.750
                               Rect
                                       Brc
      E
                     psf
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry + Secondary Weight 0.00 - Wall: 3
                     psf
                              Entire
                                       Fm
3
      E
             3.056
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry + Secondary Weight 0.00 - Wall: 3
      Ε
             2.750
                              Entire
                                       Rrc
                     psf
                                                         Seismic: Covering Weight - 26 Vee Rib + Secondary Weight 1.10 - Wall: 4
      E
             0.114
                     psf
                               Rect
                                       Frm
                                                         Seismic: Covering Weight - 26 Vee Rib + Secondary Weight 1.10 - Wall: 4
                                       Frm
                               Rect
      E
             0.114
                     psf
                                                         Seismic: Covering Weight - 26 Vee Rib + Secondary Weight 1.10 - Wall: 4
      E
                               Rect
                                       Frm
4
             0.114
                     psf
                                                         Seismic: Covering Weight - 26 Vee Rib + Secondary Weight 1.10 - Wall: 4
      E
                                       Frm
                     psf
                               Rect
             0.114
                                                         Seismic: Covering Weight - 26 Vee Rib + Secondary Weight 1.10 - Wall: 4
             0.102
                               Rect
                                       Brc
      E
                     psf
                                                         Seismic: Covering Weight - 26 Vee Rib + Secondary Weight 1.10 - Wall: 4
      E
             0.102
                     psf
                               Rect
                                       Brc
                                                         Seismic: Covering Weight - 26 Vee Rib + Secondary Weight 1.10 - Wall: 4
      E
             0.102
                               Rect
                                       Brc
                     psf
                                                         Seismic: Covering Weight - 26 Vee Rib + Secondary Weight 1.10 - Wall: 4
                     psf
      E
             0.102
                               Rect
                                       Brc
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 4
                                       Frm
      E
                               Rect
             3.056
                     psf
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 4
      E
             3.056
                               Rect
                                       Frm
                     psf
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 4
                                       Fπn
                               Rect
      Ε
             3.056
                     psf
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 4
      E
             3.056
                               Rect
                                       Frm
                     psf
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 4
      E
             2.750
                               Rect
                                       Вгс
                     psf
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 4
      E
             2.750
                               Rect
                                       Brc
                     psf
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 4
                     psf
      E
             2.750
                               Rect
                                       Brc
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 4
                     psf
                                       Втс
      E
             2.750
                               Rect
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 4
      Е
             3.056
                     psf
                               Rect
                                       Frm
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 4
      E
                                       Frm
             3.056
                     psf
                               Rect
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 4
      E
             3.056
                               Rect
                                       Fm
                     psf
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 4
      Ε
             3.056
                     psf
                               Rect
                                       Frm
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 4
      E
             2.750
                     psf
                               Rect
                                       Brc
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 4
                     psf
      E
             2.750
                               Rect
                                       Brc
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 4
                                       Brc
             2.750
                               Rect
      E
                     psf
                                                         Seismic: Covering Weight - 55.00 Not by VP - Masonry - Wall: 4
                               Rect
                                       Brc
      E
             2.750
                     psf
                                                         Seismic: Covering Weight - 24 SSR + Secondary Weight 1.31 + 9.800 Snow + Seismic
                                       Frm
      E
                              Entire
A
             0.874
                     psf
                                                         (Includes 1.000 Collateral 2.500 Frame Weight) - Roof: A
                                                         Seismic: Covering Weight - 24 SSR + Secondary Weight 1.31 + 9.800 Snow + Seismic
      E
             0.787
                     psf
                              Entire
                                       Вгс
A
                                                         (Includes 1.000 Collateral 2.500 Frame Weight) - Roof: A
```

Deflection Conditions

Frames are vertically supporting:Metal Roof Purlins and Panels Frames are laterally supporting:Metal Wall Girts and Panels Purlins are supporting:Metal Roof Panels Girts are supporting:Metal Wall Panels

Per Article 2.9 in the Builder Agreement, VP Buildings assumes that the Builder has called the local Building Official or Project Engineer to obtain all code and loading information for this specific building site.

D Material Dead Weight C Collateral Load

CG Collateral Load for Gravity Cases CU Collateral Load for Wind Cases

L Live Load Alternate Span Live Load, Shifted Left PL2 Partial Live, Full, 2 Spans



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WARRED TRANSPORT			
S	Snow Load	US1*	Unbalanced Snow Load 1, Shifted Right
USI	Unbalanced Snow Load 1, Shifted Left	US2	Unbalanced Snow Load 2, Shifted Right
	Unbalanced Snow Load 2, Shifted Left	SD	Snow Drift Load
*US2		PH1	Partial Load, Half, 1 Span
PF1	Partial Load, Full, 1 Span	PH2	Partial Load, Half, 2 Spans
PF2	Partial Load, Full, 2 Spans	W1>	Wind Load, Case 1, Right
W	Wind Load	W2>	Wind Load, Case 2, Right
<w1< td=""><td>Wind Load, Case 1, Left</td><td>W2> W3></td><td>Wind Load, Case 3, Right</td></w1<>	Wind Load, Case 1, Left	W2> W3>	Wind Load, Case 3, Right
<w2< td=""><td>Wind Load, Case 2, Left</td><td></td><td>Wind Load, Case 4, Right</td></w2<>	Wind Load, Case 2, Left		Wind Load, Case 4, Right
<w3< td=""><td>Wind Load, Case 3, Left</td><td>W4></td><td>Wind Load, Case 5, Right</td></w3<>	Wind Load, Case 3, Left	W4>	Wind Load, Case 5, Right
<w4< td=""><td>Wind Load, Case 4, Left</td><td>W5></td><td></td></w4<>	Wind Load, Case 4, Left	W5>	
<w5< td=""><td>Wind Load, Case 5, Left</td><td>W6></td><td>Wind Load, Case 6, Right</td></w5<>	Wind Load, Case 5, Left	W6>	Wind Load, Case 6, Right
<w6< td=""><td>Wind Load, Case 6, Left</td><td>WP</td><td>Wind Load, Parallel to Ridge</td></w6<>	Wind Load, Case 6, Left	WP	Wind Load, Parallel to Ridge
WPR	Wind Load, Ridge, Right	WPL	Wind Load, Ridge, Left
WB1>	Wind Brace Reaction, Case 1, Right	<wb1< td=""><td>Wind Brace Reaction, Case 1, Left</td></wb1<>	Wind Brace Reaction, Case 1, Left
WB2>	Wind Brace Reaction, Case 2, Right	<wb2< td=""><td>Wind Brace Reaction, Case 2, Left</td></wb2<>	Wind Brace Reaction, Case 2, Left
WB3>	Wind Brace Reaction, Case 3, Right	<wb3< td=""><td>Wind Brace Reaction, Case 3, Left</td></wb3<>	Wind Brace Reaction, Case 3, Left
WB4>	Wind Brace Reaction, Case 4, Right	<wb4< td=""><td>Wind Brace Reaction, Case 4, Left</td></wb4<>	Wind Brace Reaction, Case 4, Left
WB5>	Wind Brace Reaction, Case 5, Right	<wb5< td=""><td>Wind Brace Reaction, Case 5, Left</td></wb5<>	Wind Brace Reaction, Case 5, Left
WB6>	Wind Brace Reaction, Case 6, Right	<wb6< td=""><td>Wind Brace Reaction, Case 6, Left</td></wb6<>	Wind Brace Reaction, Case 6, Left
	Seismic Load	E>	Seismic Load, Right
E	Seismic Load, Left	EG	Vertical Seismic Effect
<e< td=""><td></td><td>EG-</td><td>Vertical Seismic Effect, Subtractive</td></e<>		EG-	Vertical Seismic Effect, Subtractive
EG+	Vertical Seismic Effect, Additive	<eb< td=""><td>Seismic Brace Reaction, Left</td></eb<>	Seismic Brace Reaction, Left
EB>	Seismic Brace Reaction, Right	FL*	Alternate Span Floor Live Load, Shifted Right
FL	Floor Live Load	FD	Floor Dead Load
FL	Alternate Span Floor Live Load, Shifted Left	AL>	Auxiliary Live Load, Right, Right
AL	Auxiliary Live Load		Auxiliary Live Load, Left, Right
AL>	Auxiliary Live Load, Right, Left	<al< td=""><td>Aux Live, Right</td></al*<>	Aux Live, Right
<*AL	Auxiliary Live Load, Left, Left	AL*	Aux Live, Right Auxiliary Live Load, Right, Right, Aisle 1
AL	Aux Live, Left	AL>(1)	Auxiliary Live Load, Right, Right, Aisle 1
AL>(1)	Auxiliary Live Load, Right, Left, Aisle 1	<al(1)< td=""><td></td></al*(1)<>	
<*AL(1)	Auxiliary Live Load, Left, Left, Aisle 1	AL*(1)	Aux Live, Right, Aisle 1
AL(1)	Aux Live, Left, Aisle 1	AL>(2)	Auxiliary Live Load, Right, Right, Aisle 2
AL>(2)	Auxiliary Live Load, Right, Left, Aisle 2	<al(2)< td=""><td>Auxiliary Live Load, Left, Right, Aisle 2</td></al*(2)<>	Auxiliary Live Load, Left, Right, Aisle 2
<*AL(2)	Auxiliary Live Load, Left, Left, Aisle 2	AL*(2)	Aux Live, Right, Aisle 2
AL(2)	Aux Live, Left, Aisle 2	AL>(3)	Auxiliary Live Load, Right, Right, Aisle 3
AL>(3)	Auxiliary Live Load, Right, Left, Aisle 3	<al(3)< td=""><td>Auxiliary Live Load, Left, Right, Aisle 3</td></al*(3)<>	Auxiliary Live Load, Left, Right, Aisle 3
<*AL(3)	Auxiliary Live Load, Left, Left, Aisle 3	AL*(3)	Aux Live, Right, Aisle 3
AL(3)	Aux Live, Left, Aisle 3	AL>(4)	Auxiliary Live Load, Right, Right, Aisle 4
AL>(4)	Auxiliary Live Load, Right, Left, Aisle 4	<al(4)< td=""><td>Auxiliary Live Load, Left, Right, Aisle 4</td></al*(4)<>	Auxiliary Live Load, Left, Right, Aisle 4
<*AL(4)	Auxiliary Live Load, Left, Left, Aisle 4	AL*(4)	Aux Live, Right, Aisle 4
AL(4)	Aux Live, Left, Aisle 4	AL>(5)	Auxiliary Live Load, Right, Right, Aisle 5
* * .	Auxiliary Live Load, Right, Left, Aisle 5	<al*(5)< td=""><td>Auxiliary Live Load, Left, Right, Aisle 5</td></al*(5)<>	Auxiliary Live Load, Left, Right, Aisle 5
AL>(5)	Auxiliary Live Load, Left, Left, Aisle 5	AL(5)	Aux Live, Right, Aisle 5
<*AL(5)	Aux Live, Left, Aisle 5	ALB	Aux Live Bracing Reaction
*AL(5)	Aux Live, Leit, Aisie 3 Aux Live Bracing Reaction, Right	<alb< td=""><td>Aux Live Bracing Reaction, Left</td></alb<>	Aux Live Bracing Reaction, Left
ALB>	Wind, Aux Live Bracing Reaction, Right	<walb< td=""><td>Wind, Aux Live Bracing Reaction, Left</td></walb<>	Wind, Aux Live Bracing Reaction, Left
WALB>	Wind, Aux Live Dischig Reaction, Aight	<alb(1)< td=""><td>Aux Live Bracing Reaction, Left, Aisle 1</td></alb(1)<>	Aux Live Bracing Reaction, Left, Aisle 1
ALB>(1)	Aux Live Bracing Reaction, Right, Aisle 1	<walb(1)< td=""><td>Wind, Aux Live Bracing Reaction, Left, Aisle 1</td></walb(1)<>	Wind, Aux Live Bracing Reaction, Left, Aisle 1
WALB>(1)	Wind, Aux Live Bracing Reaction, Right, Aisle 1	<alb(2)< td=""><td>Aux Live Bracing Reaction, Left, Aisle 2</td></alb(2)<>	Aux Live Bracing Reaction, Left, Aisle 2
ALB>(2)	Aux Live Bracing Reaction, Right, Aisle 2	<walb(2)< td=""><td>Wind, Aux Live Bracing Reaction, Left, Aisle 2</td></walb(2)<>	Wind, Aux Live Bracing Reaction, Left, Aisle 2
WALB>(2)	Wind, Aux Live Bracing Reaction, Right, Aisle 2		Aux Live Bracing Reaction, Left, Aisle 3
ALB>(3)	Aux Live Bracing Reaction, Right, Aisle 3	<alb(3)< td=""><td>Wind, Aux Live Bracing Reaction, Left, Aisle 3</td></alb(3)<>	Wind, Aux Live Bracing Reaction, Left, Aisle 3
WALB>(3)	Wind, Aux Live Bracing Reaction, Right, Aisle 3	<walb(3)< td=""><td>Aux Live Bracing Reaction, Left, Aisle 4</td></walb(3)<>	Aux Live Bracing Reaction, Left, Aisle 4
ALB>(4)	Aux Live Bracing Reaction, Right, Aisle 4	<alb(4)< td=""><td>Wind, Aux Live Bracing Reaction, Left, Aisle 4</td></alb(4)<>	Wind, Aux Live Bracing Reaction, Left, Aisle 4
WALB>(4)	Wind, Aux Live Bracing Reaction, Right, Aisle 4	<walb(4)< td=""><td>Aux Live Bracing Reaction, Left, Aisle 5</td></walb(4)<>	Aux Live Bracing Reaction, Left, Aisle 5
ALB>(5)	Aux Live Bracing Reaction, Right, Aisle 5	<alb(5)< td=""><td>Wind Ann Live Draging Descript Left Aigle 5</td></alb(5)<>	Wind Ann Live Draging Descript Left Aigle 5
WALB>(5)	Wind, Aux Live Bracing Reaction, Right, Aisle 5	<walb(5)< td=""><td>Wind, Aux Live Bracing Reaction, Left, Aisle 5</td></walb(5)<>	Wind, Aux Live Bracing Reaction, Left, Aisle 5
WALB	Wind, Aux Live Bracing Reaction	AD	Auxiliary Dead Load
· U0	User Defined Load	U1	User Defined Load - 1
U2	User Defined Load - 2	U3	User Defined Load - 3
U4	User Defined Load - 4	U5	User Defined Load - 5
U6	User Defined Load - 6	U7	User Defined Load - 7
U8	User Defined Load - 8	U9	User Defined Load - 9
UB	User Brace Reaction	UB1	User Brace Reaction - 1
UB2	User Brace Reaction - 2	UB3	User Brace Reaction - 3
	User Brace Reaction - 4	UB5	User Brace Reaction - 5
UB4	User Brace Reaction - 6	UB7	User Brace Reaction - 7
UB6	User Brace Reaction - 8	UB9	User Brace Reaction - 9
UB8		T	Temperature Load
R	Rain Load	1	
V	Shear		



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User Applied Surface Loads (Local Coordinate System)

Side	Shape	Units	Type	Description	Mag	X-Loc	Y-Loc	Frm	Brc	Grt	Pur	Pnl	Supp.	Dir.	Loc.
1	PT	k		Wind Rectification	1.00	0/0/0	19/10/0	Ÿ	Ÿ	N	N	N	N	ĪN	OF
1 1	PT	k		Wind Rectification	1.00	0/0/0	19/10/0	Ÿ	Ÿ	N	N	N	N I	IN	OF
;	PT	1 7	W2>	Wind Rectification	1.00	0/0/0	19/10/0	Ŷ	Ŷ	N	N	N	N	IN	OF
		k		Wind Rectification	1.00	0/0/0	19/10/0	v	Ŷ	N	N	N	N	IN	OF
1	PT	k	–	,,,,,,,	1.00	50/0/0	21/11/0		Ŷ	N	N	N	N	ĪN	OF
1	PT	k	1	Wind Rectification				ν.	Ÿ	N	N	N	N	ĪN	OF
1	PT	k		Wind Rectification	1.00	50/0/0	21/11/0	I	Y	N	N	N	N N	IN	OF
1	PT	k	–	Wind Rectification	1.00	50/0/0	21/11/0	Y	•				IN I	ĪN	OF
1	PT	k	<w2< td=""><td>Wind Rectification</td><td>1.00</td><td>50/0/0</td><td>21/11/0</td><td>Y</td><td>Y</td><td>N</td><td>N</td><td>N</td><td>N</td><td></td><td></td></w2<>	Wind Rectification	1.00	50/0/0	21/11/0	Y	Y	N	N	N	N		
3	PT	k	W1>	Wind Rectification	1.00	0/0/0	21/11/0	Y	Y	N	N	N	N	IN	OF
3	PT	k	<w1< td=""><td>Wind Rectification</td><td>1.00</td><td>0/0/0</td><td>21/11/0</td><td>Y</td><td>Y</td><td>N</td><td>N</td><td>N</td><td>N</td><td>IN</td><td>OF</td></w1<>	Wind Rectification	1.00	0/0/0	21/11/0	Y	Y	N	N	N	N	IN	OF
3	PT	k	W2>	Wind Rectification	1.00	0/0/0	21/11/0	Y	. Y	Ν	N	N	N	IN	OF
3	PT	k	<w2< td=""><td>Wind Rectification</td><td>1.00</td><td>0/0/0</td><td>21/11/0</td><td>Y</td><td>Y</td><td>N</td><td>N</td><td>N</td><td> N </td><td>IN</td><td>OF</td></w2<>	Wind Rectification	1.00	0/0/0	21/11/0	Y	Y	N	N	N	N	IN	OF
3	PT	k	<w2< td=""><td>Wind Rectification</td><td>1.00</td><td>50/0/0</td><td>19/10/0</td><td>Y</td><td>Y</td><td>N</td><td>N</td><td>N</td><td> N </td><td>IN</td><td>OF</td></w2<>	Wind Rectification	1.00	50/0/0	19/10/0	Y	Y	N	N	N	N	IN	OF
3	PT	·k		Wind Rectification	1.00	50/0/0	19/10/0	Y	Y	N	N	N	N	IN	OF
3	PT	k	<w1< td=""><td>Wind Rectification</td><td>1.00</td><td>50/0/0</td><td>19/10/0</td><td>Y</td><td>Y</td><td>N</td><td>N</td><td>N</td><td>N</td><td>IN</td><td>OF</td></w1<>	Wind Rectification	1.00	50/0/0	19/10/0	Y	Y	N	N	N	N	IN	OF
3	PT	l k		Wind Rectification	1.00	50/0/0	19/10/0	Y	Y	N	N	N	N	IN	OF
A	SP	psf		Parapet drift	0.00	100/0/0	0/0/0	Y	N	N	Y	Y	N	IN	OF
Â	SP	psf	SD	Parapet drift	15.00	100/0/0	50/0/0	Y	N	N	Y	Y	N	IN	OF
\ \frac{1}{4}	SP	nef	SD	Paranet drift	0.00	97/6/0	50/0/0	Y	N	N	Y	Y	N	IN	OF

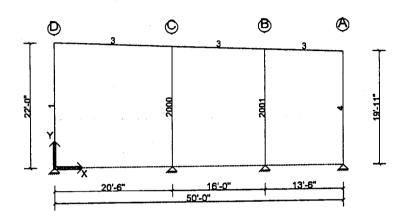


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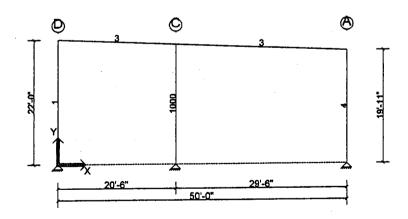
User Defined Frame Point Loads for Cros	s Section: 1
	December

Side	Units	Type	Description	Magl	Locl	Offset	H or V	Supp.	Dir.	Coef.	Loc.
3100	Units	- 71		-14.00	11/0/0	NA	NA	N	DOWN	1.000	CL
1	K	CG	mmm	3.00	6/0/0	NA		l n	l IN	1.000	WA
1	k	W1>	Му	3.00	6/0/0	NA			IN	1.000	WA
1	k	<w1< th=""><th>[My</th><th>1 1</th><th></th><th>NA NA</th><th>NA.</th><th>1 -</th><th>IN</th><th>1.000</th><th>WA</th></w1<>	[My	1 1		NA NA	NA.	1 -	IN	1.000	WA
1	k	W2>	My	3.00			1	1 '	IN	1.000	WA
1	k	<w2< th=""><th>My</th><th>3.00</th><th></th><th>NA</th><th>ı</th><th>1</th><th></th><th>1.000</th><th>IF</th></w2<>	My	3.00		NA	ı	1		1.000	IF
2000	l k l	CG	mmm	-14.00	11/0/0	NA NA	NA	N	DOWN	1.000	



User Defined Frame Point Loads for Cross Section: 2

User Def	ined Fra	ame Point 1	Loads for Cross Section: 2				*****	_	D:-	Case	Too	
Side	Units	Type	Description	Magl	Locl	Offset	H or V	Supp.	Dir.	Coef.	Loc.	
Side	Units	1300	2000.17	30.00	11/0/0	NA	NA	N	DOWN	1.000	CL	
1	- L	CG	mmm	-28.00	11/0/4	1477	אונ ו	14				
	h - 1				11/0/0	NA	NA.	N N	DOWN	1.000	CL	
1000	l Ir I	CG	me22	-28.00	11/0/4	1477	11/7		201111	1 2.200		



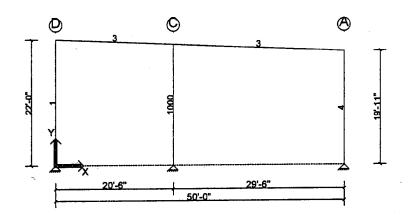


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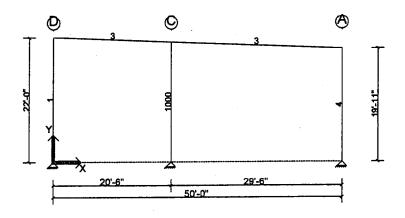
User Defined Frame Point Loads for Cross Section: 3

Side	Units	Type	Description	Magl	Loci	Offset	H or V	Supp.	Dir.	Coef.	Loc.
1	k	CG	mmm	-28.00	11/0/0	NA	NA	1 1	DOWN	1.000	CL
1000	k	CG	mmm	-28.00	11/0/0	NA	NA	N	DOWN	1.000	CL



User Defined Frame Point Loads for Cross Section: 4

OSCL DCI	ineu Fra	ine Point 1	JURUS IUI CI USS SECTION. 4								
Side	Units	Type	Description	Mag1	Loci	Offset	H or V	Supp.	Dir.	Coet.	Loc.
			L	-28.00	11/0/0	NA	NA	N	DOWN	1.000	CL
1 1	K	CG	mmm				1	37			
1000	k	CG	mezz	-28.00	11/0/0	NA	l NA	N	DOWN	1.000	CL





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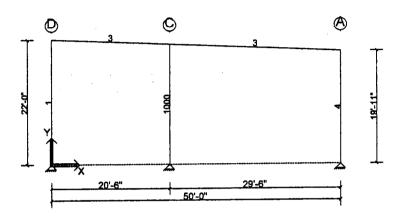
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User Defined Frame Point Loads for Cross Section: 5

ſ	Side	Units	Type	Description	Mag1	Loc1	Offset	H or V	Supp.	Dir.	Coef.	Loc.
ł	1	k	ĆĠ	mmm	-28.00		NA	NA		DOWN	1.000	1
	1000	k	CG	mezz	-28.00	11/0/0	NA	NA	N	DOWN	1.000	CL

ser Defined Frame Line Loads for Cross Section: 5

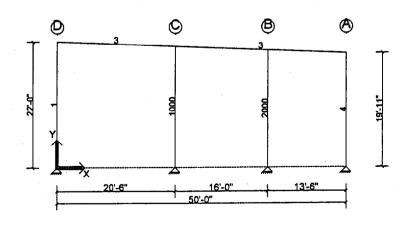
OSCI IVCI	IIICA I.I.	tine Dine D	oads for Cross Section C								T T
Side	Units	Type	Description	Magi	Locl	Mag2	Loc2	Supp.	Dir.	Coet.	Loc.
3	plf	SD	Parapet drift->Resolved From Plane	0.00	20/0/0	-0.18	50/0/0	N	DOWN	1.000	OF



User Defined Frame Point Loads for Cross Section: 6

1	Side	Units	Type	Description	Magl	Loci	Offset	H or V	Supp.	Dir.	Coef.	Loc.
	1	k	ĆĠ	mmm	-14.00	11/0/0		NA	N	DOWN	1.000	
	1000	k	CG	mmm	-14.00	11/0/0	NA	NA	N	DOWN	1.000	IF

User Dei	uned Fra	ame Line L	Loads for Cross Section: 0									,
Side	Units	Type	Description	Mag1	Loc1	Mag2	Loc2	Supp.	Dir.	Coef.	Loc.	
- 3	nlf	ŠĎ	Paranet drift->Resolved From Plane	0.00	0/0/0	-18.91	50/0/0	N	DOWN	1.000	OF	1





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Reactions - Expanded Report

Shape: Big Moose Harley Builder Contact: Bill Rudman Name: PATCO Construction Inc Address: 1293 Main St.

City, State Zip: Sanford, Maine 04073

Country: United States

Deflection Conditions

Loads and Codes - Shape: Big Moose Harley Building Code: BOCA - 1996 - National Building Code Building Use: Standard Occupancy Structures

Live Load Live Load: 20.00 psf Reducible Collateral Gravity: 1.00 psf Collateral Uplift: 0.00 psf

Purlins are supporting: Metal Roof Panels Girts are supporting:Metal Wall Panels

Wind Load Wind Speed: 90.00 mph Wind Exposure: B Wind Enclosure: Enclosed Distance to Coast: 6.0 Miles

Base Elevation: 0/0/0 Frames are vertically supporting:Metal Roof Purlins and Panels Frames are laterally supporting: Metal Wall Girts and Panels

Project: Big Moose Harley-Davidson Reference: Big Moose Harley.vpc

Jobsite: 375 Riverside

City, State Zip: Portland, Maine 04103 County, Country: Cumberland, United States

Built Up: Cold Form: 89AISC 89AISI

Snow Load Ground Snow Load: 70.00 psf Snow Exposure Category: 3

Rainfall: 4.00 in per hour Allow. Overstress:

Frm: 1.03, Sec: 1.03, Brc: 1.03

Seismic Load

Seismic Hazard / Use Group: Group 1

Aa: 0.1000, Av: 0.1000

% Snow Used in Seismic: 20.00

Per Article 2.9 in the Builder Agreement, VP Buildings assumes that the Builder has called the local Building Official or Project Engineer to obtain all code and loading information for this specific building site.

Load Type Desc	criptions		
D	Material Dead Weight	C	Collateral Load
CG	Collateral Load for Gravity Cases	CU	Collateral Load for Wind Cases
Ĺ	Live Load	ASL^	Alternate Span Live Load, Shifted Right
^ASL	Alternate Span Live Load, Shifted Left	S .	Snow Load
US1*	Unbalanced Snow Load 1, Shifted Right	*US1	Unbalanced Snow Load 1, Shifted Left
US2*	Unbalanced Snow Load 2, Shifted Right	*US2	Unbalanced Snow Load 2, Shifted Left
SD	Snow Drift Load	W	Wind Load
W1>	Wind Load, Case 1, Right	<w1< td=""><td>Wind Load, Case 1, Left</td></w1<>	Wind Load, Case 1, Left
W2>	Wind Load, Case 2, Right	<w2< td=""><td>Wind Load, Case 2, Left</td></w2<>	Wind Load, Case 2, Left
W3>	Wind Load, Case 3, Right	<w3< td=""><td>Wind Load, Case 3, Left</td></w3<>	Wind Load, Case 3, Left
W4>	Wind Load, Case 4, Right	<w4< td=""><td>Wind Load, Case 4, Left</td></w4<>	Wind Load, Case 4, Left
W5>	Wind Load, Case 5, Right	<w5< td=""><td>Wind Load, Case 5, Left</td></w5<>	Wind Load, Case 5, Left
W6>	Wind Load, Case 6, Right	<w6< td=""><td>Wind Load, Case 6, Left</td></w6<>	Wind Load, Case 6, Left
WP	Wind Load, Parallel to Ridge	WPR	Wind Load, Ridge, Right
WPL	Wind Load, Ridge, Left	E	Seismic Load
E>	Seismic Load, Right	< <u>E</u>	Seismic Load, Left
EG	Vertical Seismic Effect	EG+	Vertical Seismic Effect, Additive
EG-	Vertical Seismic Effect, Subtractive	FL	Floor Live Load
FL*	Alternate Span Floor Live Load, Shifted Right	*FL	Alternate Span Floor Live Load, Shifted Left
FD	Floor Dead Load	AL	Auxiliary Live Load
AL*>	Auxiliary Live Load, Right, Right	*AL>	Auxiliary Live Load, Right, Left
<al*< td=""><td>Auxiliary Live Load, Left, Right</td><td><*AL</td><td>Auxiliary Live Load, Left, Left</td></al*<>	Auxiliary Live Load, Left, Right	<*AL	Auxiliary Live Load, Left, Left
AL*	Aux Live, Right	*AL	Aux Live, Left
AL*>(1)	Auxiliary Live Load, Right, Right, Aisle 1	*AL>(1)	Auxiliary Live Load, Right, Left, Aisle 1
<al*(1)< td=""><td>Auxiliary Live Load, Left, Right, Aisle 1</td><td><*AL(1)</td><td>Auxiliary Live Load, Left, Left, Aisle 1</td></al*(1)<>	Auxiliary Live Load, Left, Right, Aisle 1	<*AL(1)	Auxiliary Live Load, Left, Left, Aisle 1
AL*(1)	Aux Live, Right, Aisle 1	*AL(1)	Aux Live, Left, Aisle 1
AL*>(2)	Auxiliary Live Load, Right, Right, Aisle 2	*AL>(2)	Auxiliary Live Load, Right, Left, Aisle 2
<al*(2)< td=""><td>Auxiliary Live Load, Left, Right, Aisle 2</td><td><*AL(2)</td><td>Auxiliary Live Load, Left, Left, Aisle 2</td></al*(2)<>	Auxiliary Live Load, Left, Right, Aisle 2	<*AL(2)	Auxiliary Live Load, Left, Left, Aisle 2
AL*(2)	Aux Live, Right, Aisle 2	*AL(2)	Aux Live, Left, Aisle 2
AL*>(3)	Auxiliary Live Load, Right, Right, Aisle 3	*AL>(3)	Auxiliary Live Load, Right, Left, Aisle 3
<al*(3)< td=""><td>Auxiliary Live Load, Left, Right, Aisle 3</td><td><*AL(3)</td><td>Auxiliary Live Load, Left, Left, Aisle 3</td></al*(3)<>	Auxiliary Live Load, Left, Right, Aisle 3	<*AL(3)	Auxiliary Live Load, Left, Left, Aisle 3
AL*(3)	Aux Live, Right, Aisle 3	*AL(3)	Aux Live, Left, Aisle 3
AL*>(4)	Auxiliary Live Load, Right, Right, Aisle 4	*AL>(4)	Auxiliary Live Load, Right, Left, Aisle 4
<al*(4)< td=""><td>Auxiliary Live Load, Left, Right, Aisle 4</td><td><*AL(4)</td><td>Auxiliary Live Load, Left, Left, Aisle 4</td></al*(4)<>	Auxiliary Live Load, Left, Right, Aisle 4	<*AL(4)	Auxiliary Live Load, Left, Left, Aisle 4
**************************************	travition? Tite thous, that, tribud tribud.	(-)	•



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AL*(4) AL*>(5) <al*(5) AL*(5) AD U1 U3 U5 U7 U9</al*(5) 	Aux Live, Right, Aisle 4 Auxiliary Live Load, Right, Right, Aisle 5 Auxiliary Live Load, Left, Right, Aisle 5 Aux Live, Right, Aisle 5 Auxiliary Dead Load User Defined Load - 1 User Defined Load - 3 User Defined Load - 5 User Defined Load - 7 User Defined Load - 9 Temperature Load	*AL(4) *AL>(5) <*AL(5) *AL(5) U0 U2 U4 U6 U8 R	Aux Live, Left, Aisle 4 Auxiliary Live Load, Right, Left, Aisle 5 Auxiliary Live Load, Left, Left, Aisle 5 Aux Live, Left, Aisle 5 User Defined Load User Defined Load - 2 User Defined Load - 4 User Defined Load - 6 User Defined Load - 8 Rain Load
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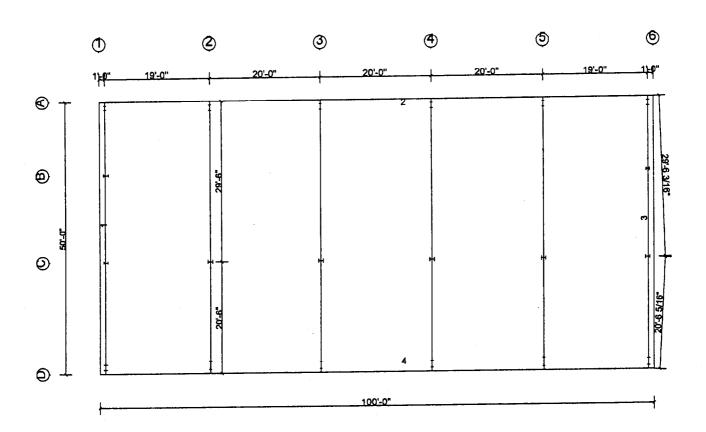


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Overall Building Description								D . C	N.C. Dasf	Deals
Shape	Overall	Overall	Floor Area	Wall Area	Roof Area	Max. Eave	Min. Eave	Max. Kooi		Peak
Shape	Width	Length	(sq. ft.)	(sq. ft.)	(sq. ft.)	Height	Height 2	Pitch	Pitch	Height
Big Moose Harley	50/0/0	100/0/0	5000	6287	5004	22/0/0	19/11/0	0.500:12		

Overall Sha	pe Descript	ion							- N': 1 0	Dist to Didge	Peak Height
Roof 1	Roof 2	From Grid	To Grid	Width	Length	Eave Ht.	Eave Ht. 2	Pitch	Pitch 2	Dist. to Ridge	reak neight
A		1-A	1-D	50/0/0	100/0/0	19/11/0	22/0/0	0.500:12			





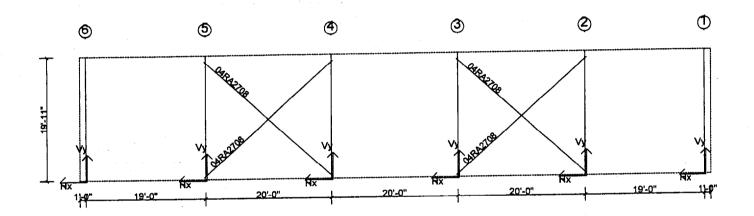
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Wall: 2 (Grid:A)

esign Load Comb			A!! a add	Description
No. Do Not Use	Origin	Factor	Application	Description
1	System	1.333 1.) E>	
2	System	1.333 1.4) <e< td=""><td><<u>E</u></td></e<>	< <u>E</u>
<u> </u>	System	1.333 1.4) W1>	W1>
4	System	1.333]1.		<w1< td=""></w1<>
5	System	1.333 1.		W2>
6	System	1.333 1.		<w2< td=""></w2<>

Wall: 2 (Grid:A)



sing Denction from Wall . 2

or acing	X-Loc	20	/0/0	40	/0/0	60	/0/0	80	/0/0	
	Grid1 - Grid2		\-5	. A	-4	A	1-3	A-2		
Ld	Load Combinations	Hx	Vy	Hx	Vy	Hx	Vy	Hx	Vy	
Cs		(k)	(k)	(k)	(k)	(k)	(k)	(k)	(k)	
1	E>	 ``	1.34	-1.34	-1.34	•	1.42	-1.43	-1.42	-
5	<e< td=""><td>1.44</td><td>-1.44</td><td></td><td>1.44</td><td>1.33</td><td>-1.32</td><td></td><td>1.32</td><td>-</td></e<>	1.44	-1.44		1.44	1.33	-1.32		1.32	-
2	W1>	1	3.10	-3.11	-3.10	-	3.26	-3.27	-3.26	
<u>. </u>	<w1< td=""><td>3.27</td><td>-3.26</td><td></td><td>3.26</td><td>3.11</td><td>-3.10</td><td>-</td><td>3.10</td><td>•</td></w1<>	3.27	-3.26		3.26	3.11	-3.10	-	3.10	•
7	W2>	3.27	3.07	-3.08	-3.07	_	3.29	-3.30	-3.29	•
2	<w2< td=""><td>3.30</td><td>-3.29</td><td></td><td>3.29</td><td>3.08</td><td>-3.07</td><td> -</td><td>3.07</td><td>-</td></w2<>	3.30	-3.29		3.29	3.08	-3.07	-	3.07	-

Note: Reactions shown in the Bracing Reactions table are not included in the Frame Reactions table. Bracing reactions are produced from loads applied parallel to the ridge and Frame reactions from loads applied perpendicular to the ridge of the building. Combine per Code as needed.

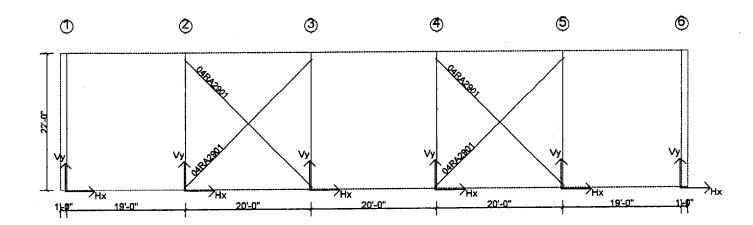
VPC Version:3.0



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Wall: 4 (Grid:D)



Bracing Reaction from Wall: 4

	X-Loc	20	/0/0	40	70/0	60	/0/0	80	0/0/0	
	Grid1 - Grid2	T.)-2	I)-3	I I)-4		D-5	
Ld	Load Combinations	Hx	Vy	Hx	Vy	Hx	Vy	Нх	Vy	
Cs		(k)	(k)	(k)	(k)	(k)	(k)	(k)	(k)	
1	E>	-1.39	-1.53	•	1.53	-1.34	-1.47	-	1.47	-
2	<e< td=""><td> </td><td>1.44</td><td>1.31</td><td>-1.44</td><td></td><td>1.56</td><td>1.42</td><td>-1.56</td><td>-</td></e<>		1.44	1.31	-1.44		1.56	1.42	-1.56	-
3	W1>	-3.27	-3.60		3.60	-3.14	-3.45	-	3.45	-
4	<w1< td=""><td></td><td>3.45</td><td>3.14</td><td>-3.45</td><td>- </td><td>3.60</td><td>3.27</td><td>-3.60</td><td>-</td></w1<>		3.45	3.14	-3.45	-	3.60	3.27	-3.60	-
5	W2>	-3.33	-3.66	- 1	3.66	-3.09	-3.40	-	3.40	-
6	<w2< td=""><td>- </td><td>3.40</td><td>3.09</td><td>-3.40</td><td> - </td><td>3.66</td><td>3.33</td><td>-3.66</td><td>•</td></w2<>	-	3.40	3.09	-3.40	-	3.66	3.33	-3.66	•

Note: Reactions shown in the Bracing Reactions table are not included in the Frame Reactions table. Bracing reactions are produced from loads applied parallel to the ridge and Frame reactions from loads applied perpendicular to the ridge of the building. Combine per Code as needed.



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Wall: 4, Frame at: 1/0/0

Design Load Combinations - Framing

	Do Not Use	Origin	Factor	Application	Description
1	201.00000	System		1.0 D + 1.0 CG + 1.0 L	D + CG + L
2		System		1.0 D + 1.0 CG + 1.0 S	D + CG + S
3	1	System		1.0 D + 1.0 CG + 1.0 SD + 1.0 S	D + CG + SD + S
4	1	System		1.0 D + 1.0 W1>	D + W1>
7	1 1	System		1.0 D + 1.0 <w1< td=""><td>D + <w1< td=""></w1<></td></w1<>	D + <w1< td=""></w1<>
6	1 1	System	1 2.22	1.0 D + 1.0 W2>	D + W2>
7	l i	System		1.0 D + 1.0 < W2	D + <w2< td=""></w2<>
8]	System		1.0 D + 1.0 WP	D + WP
9		System	1	1.0 D + 1.0 CG + 0.750 L + 0.750 W1>	D + CG + L + W1 >
10		System	1.000	1.0 D + 1.0 CG + 0.750 L + 0.750 <w1< td=""><td>D + CG + L + < W1</td></w1<>	D + CG + L + < W1
11	1	System		1.0 D + 1.0 CG + 0.750 L + 0.750 W2>	D + CG + L + W2 >
12		System		1.0 D + 1.0 CG + 0.750 L + 0.750 <w2< td=""><td>D + CG + L + < W2</td></w2<>	D + CG + L + < W2
13		System		1.0 D + 1.0 CG + 0.750 L + 0.750 WP	D + CG + L + WP
14		System		1.0 D + 1.0 CG + 0.750 SD + 0.750 S + 0.750 W1>	D + CG + SD + S + W1>
15		System		1.0 D + 1.0 CG + 0.750 SD + 0.750 S + 0.750 <w1< td=""><td>D + CG + SD + S + < W1</td></w1<>	D + CG + SD + S + < W1
16		System		1.0 D + 1.0 CG + 0.750 SD + 0.750 S + 0.750 W2>	D + CG + SD + S + W2
17		System		1.0 D + 1.0 CG + 0.750 SD + 0.750 S + 0.750 <w2< td=""><td>D + CG + SD + S + < W2</td></w2<>	D + CG + SD + S + < W2
18		System		1.0 D + 1.0 CG + 0.750 SD + 0.750 S + 0.750 WP	D + CG + SD + S + WP
19		System	1.333	1.0 D + 1.0 CG + 1.0 L + 1.0 E> + 1.0 EG+	D + CG + L + E> + EG+
20		System	1.333	1.0 D + 1.0 CG + 1.0 L + 1.0 <e +="" 1.0="" eg+<="" td=""><td>D + CG + L + <e +="" eg+<="" td=""></e></td></e>	D + CG + L + <e +="" eg+<="" td=""></e>
21		System	1.333	1.0 D + 1.0 CG + 0.200 SD + 0.200 S + 1.0 E> + 1.0 EG+	D+CG+SD+S+E>+EG+
21	1 1	System		10D+10CG+0200SD+0200S+10 <f+10fg+< td=""><td>D+CG+SD+S+<e+eg+< td=""></e+eg+<></td></f+10fg+<>	D+CG+SD+S+ <e+eg+< td=""></e+eg+<>

0533

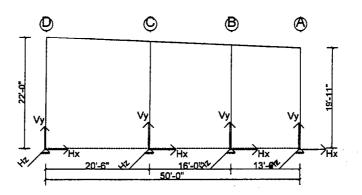


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Wall: 4, Frame at: 1/0/0 Frame ID:Post & Beam

Frame Type:Post & Beam



Values shown are resisting forces of the foundation.

Frm

Frm

Frm

Frm

-0.49

0.49

0.02

0.73

-0.02

-0.73

Reactions - Load Type at Frame Cross Section: 1 Type **Exterior Column** Exterior Column Interior Column Interior Column 36/6/0 50/0/0 X-Loc 0/0/0 20/6/0 Grid1 - Grid2 1-D 1-C 1-B 1-A Base Plate W x L (in.) 9 x 13 8 x 10 8 x 10 8 x 12 0.375 Base Plate Thickness (in.) 0.375 0.375 0.375 Anchor Bolt Qty/Diam. (in.) 4 - 0.750 4 - 0.750 4 - 0.7504 - 0.750100'-0" 100'-0" 100'-0" 100'-0" Column Base Elev. Load Type Hx Hz Hz Hx Desc. Hx Hx D Frm 0.74 1.03 0.67 0.36 CG 14.20 0.17 0.06 Frm 14.10 0.26 L Frm 1.50 3.82 3.10 1.25 Frm 3.11 S 4.65 10.69 7.33 SD Frm W1> -1.09 -1.20 2.97 -2.86 2.32 -1.98 -1.00 -0.87 Frm <W1 0.97 1.09 -1.28 -2.75 -2.88 -2.16 -1.96 -0.79Frm W2> -1.62 -0.77-1.80 -1.25-0.52-0.54Frm <W2 -0.85 -1.82 -1.23-0.46Frm 0.56 1.46 WP Frm 1.44 -1.18 -2.87 -1.97 -1.30 -0.88

-0.13

0.01

0.13

0.76

-0.76

F>

EG+

<E

EG-

-0.12

0.12

0.02

-0.02

0.18

-0.18

0.05

-0.05



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Frame Reactions - Load Cases at Frame Cross Section: 1

	X-Loc	0/	0/0		20/6/0			36/6/0			/0/0	
-	Grid1 - Grid2	1	-D		1-C			1-B			-A	
Ld	Description	Hx	Vy	Hx	Hz	Vy	Hx	Hz	Vy	Hx	Vy	
Cs	(application factor not shown)	(k)	(k)	(k)	(k)	(k)	(k)	(k)	(k)	(k)	(k)	
	D+CG+L	•	16.33	-	0.26	19.05	-	-	3.94	•	1.67	-
2	D + CG + S	-	19.49	-	0.26	25.93	-	-	8.17	•	3.54	•
3	D + CG + SD + S	-	19.49	-	0.26	25.93	-	-	8.17	•	3.54	-
4	D+W1>	-1.09	-0.46	-	2.97	-1.83	•	2.32	-1.30	-1.00	-0.51	•
5	D+ <w1< td=""><td>1.09</td><td>-0.54</td><td>-</td><td>-2.75</td><td>-1.85</td><td>-</td><td>-2.16</td><td>-1.29</td><td>0.97</td><td>-0.43</td><td>-</td></w1<>	1.09	-0.54	-	-2.75	-1.85	-	-2.16	-1.29	0.97	-0.43	-
6	D + W2>	-1.62	-0.03	-	-	-0.77	-	-	-0.57	-0.52	-0.18	-
7	D+ <w2< td=""><td>0.56</td><td>-0.11</td><td>-</td><td>-</td><td>-0.79</td><td>-</td><td> -</td><td>-0.56</td><td>1.46</td><td>-0.10</td><td>-</td></w2<>	0.56	-0.11	-	-	-0.79	-	-	-0.56	1.46	-0.10	-
8	D+WP	1.44	-0.44	•	-	-1.85	-	-	-1.30	-1.30	-0.52	-
9	D + CG + L + W1 >	-0.82	15.06	-	2.49	15.95	-	1.74	1.68	-0.75	0.71	-
10	D + CG + L + < W1	0.82	15.00	-	-1.80	15.94	-	-1.62	1.69	0.73	0.77	•
11	D+CG+L+W2>	-1.22	15.39	-	0.26	16.74	-	1 -	2.23	-0.39	0.95	•
12	D + CG + L + < W2	0.42	15.32	-	0.26	16.73	-	-	2.24	1.09	1.01	-
13	D+CG+L+WP	1.08	15.08	-	0.26	15.94	•	-	1.68	-0.98	0.70	•
14	D + CG + SD + S + W1 >	-0.82	17.42		2.49	21.11	-	1.74	4.85	-0.75	2.11	-
15	D + CG + SD + S + < W1	0.82	17.36		-1.80	21.10	-	-1.62	4.87	0.73	2.16	•
16	D + CG + SD + S + W2	-1.22	17.75	-	0.26	21.90	-		5.40	-0.39	2.35	•
17	D + CG + SD + S + < W2	0.42	17.69		0.26	21.89	-	-	5.41	1.09	2.41	-
18	D + CG + SD + S + WP	1.08	17.44	-	0.26	21.10	-	-	4.86	-0.98	2.10	-
19	D + CG + L + E> + EG+	-0.49	17.08	١.	0.14	19.81	•	0.18	3.98	-0.12	1.69	-
20	D+CG+L+ <e+eg+< td=""><td>0.49</td><td>17.04</td><td>-</td><td>0.41</td><td>19.81</td><td>-</td><td>-0.18</td><td>3.98</td><td>0.12</td><td>1.69</td><td>. •</td></e+eg+<>	0.49	17.04	-	0.41	19.81	-	-0.18	3.98	0.12	1.69	. •
21	D+CG+SD+S+E>+EG+	-0.49	16.52		0.14	18.14	-	0.18	2.35	-0.12	1.06	-
22	D+CG+SD+S+ <e+eg+< td=""><td>0.49</td><td>16.48</td><td>.</td><td>0.41</td><td>18.14</td><td>-</td><td>-0.18</td><td>2.35</td><td>0.12</td><td>1.06</td><td>•</td></e+eg+<>	0.49	16.48	.	0.41	18.14	-	-0.18	2.35	0.12	1.06	•

Maximum Reactions Summary - Framing

X-Loc	Grid	Hrz left (-Hx) (k)	Load Case	Hrz Right (Hx) (k)	Load Case	Hrz In (-Hz) (k)	Load Case	Hrz Out (Hz) (k)	Load Case	Uplift (-Vy) (k)	Load Case	Vrt Down (Vy) (k)	Load Case	Mom cw (-Mzz) (in-k)	Load Case	Mom ccw (Mzz) (in-k)	Load Case
0/0/0	1-D	1.62	6	1.44	8	- ()	-	-	- 1	0.54	5	19.49	2	-	•	-	-
20/6/0	i-C		-	•	-	2.75	5	2.97	4	1.85	5	25.93	2	-	-	-	-
36/6/0	1-B	١.	-		-	2.16	5	2.32	4	1.30	4	8.17	2	•	-	•	-
50/0/0	1-A	1.30	8	1.46	7 1				-	0.52	8	3.54	2	<u> </u>	-		لبتيا

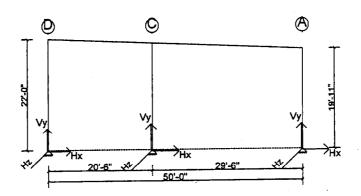


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Wall: 4, Frame at: 20/0/0 Frame ID:CB1 20/6

Frame Type:Continuous Beam



Values shown are resisting forces of the foundation.

Reactions - Load Type at Frame Cross Section: 2

Reactions - Load Type at	Frame Cit			,				O liver	T		
Type		Exterio	r Column	Inte	rior Col			r Column			
X-Loc		0/	0/0		20/6/0			/0/0			
Grid1 - Grid2		2	-D		2-C			-A			
Base Plate W x L		8 :	k 20		9 x 11			x 13			
Base Plate Thicknes	` '	0.	375		0.500			375			
Anchor Bolt Qty/Dia		4 - 1	0.750		4 - 0.750	0		0.750			
Column Base El		10	0'-0"		100'-0"		10	0'-0"			
Load Type	Desc.	Hx	Vy	Hx	Hz	Vy	Hx	Vy			
D	Frm	0.08	0.78	-	•	2.16	-0.08	1.10	•	•	
CG	Frm	0.03	28.18	-	-	28.52	-0.03	0.27	•	•	
l T	Frm	0.37	2.54	-	-	8.85	-0.37	4.21	•	· ·	
l s̃	Frm	1.15	7.78	 -	-	27.09	-1.15	12.90	-	-	
SD	Frm	-	-	-	-	-	•	-	• `	•	
W1>	Frm	-3.71	-3.55	-	-	-7.95	-4.54	-1.31		-	
<w1< td=""><td>Frm</td><td>3.06</td><td>-0.74</td><td>-</td><td>-</td><td>-6.61</td><td>4.94</td><td>-5.46</td><td>-</td><td>•</td><td></td></w1<>	Frm	3.06	-0.74	-	-	-6.61	4.94	-5.46	-	•	
W2>	Frm	-4.47	-2.64	-	-	-5.61	-3.93	0.16	•	•	
<w2< td=""><td>Frm</td><td>2.30</td><td>0.17</td><td>-</td><td>-</td><td>-4.27</td><td>5.55</td><td>-3.99</td><td>•</td><td>•</td><td></td></w2<>	Frm	2.30	0.17	-	-	-4.27	5.55	-3.99	•	•	
WP	Frm	2.06	-2.47	-	-	-6.34	-1.66	-4 .00	•	-	
E>	Frm	-1.46	-0.44	-	-0.67	-0.48	-1.50	0.96	· •	•	
EG+	Frm	-	1.45	-	-	1.56	•	0.08	/ -	•	
<e< td=""><td>Frm</td><td>1.46</td><td>0.44</td><td>-</td><td>0.67</td><td>0.48</td><td>1.50</td><td>-0.96</td><td>•</td><td>•</td><td></td></e<>	Frm	1.46	0.44	-	0.67	0.48	1.50	-0.96	•	•	
EG-	Frm		-1.45	-		-1.56	•	-0.08	•	<u> </u>	



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Frame Reactions - Load Cases at Frame Cross Section: 2

	X-Loc		0/0		20/6/0			/0/0		
	Grid1 - Grid2	2	-D		2-C			-A		
Ld	Description	Hx	Vy	Hx	Hz	Vy	Hx	Vy		
Cs	(application factor not shown)	(k)	(k)	(k)	(k)	(k)	(k)	(k)		
1	D+CG+L	0.48	31.50	•	•	39.53	-0.48	5.59	•	-
2	D + CG + S	1.25	36.74	٠.	·-	57.77	-1.25	14.28	•	-
3	D + CG + SD + S	1.25	36.74	-	-	57.77	-1.25	14.28	•	•
4	D + W1>	-3.63	-2.77	-		-5.79	-4 .61	-0.21	•	•
5	D+ <w1< td=""><td>3.13</td><td>0.04</td><td> -</td><td> -</td><td>-4.45</td><td>4.87</td><td>-4.36</td><td>-</td><td>•</td></w1<>	3.13	0.04	-	-	-4.45	4.87	-4.36	-	•
6	D + W2>	-4.39	-1.86	- ا	-	-3.45	-4 .00	1.27	•	•
7	D+ <w2< td=""><td>2.37</td><td>0.95</td><td>-</td><td>-</td><td>-2.11</td><td>5.48</td><td>-2.88</td><td>•</td><td>-</td></w2<>	2.37	0.95	-	-	-2.11	5.48	-2.88	•	-
8	D+WP	2.14	-1.69	-	-	-4.18	-1.73	-2.89	•	•
ا و ا	D + CG + L + W1 >	-2.40	28.20	•	-	31.35	-3.79	3.55	•	-
10	D + CG + L + < W1	2.68	30.30	-		32.36	3.32	0.44	•	-
11	D+CG+L+W2>	-2.97	28.88	-	-	33.11	-3.33	4.66	•	-
12	D+CG+L+ <w2< td=""><td>2.11</td><td>30.99</td><td>-</td><td>-</td><td>34.11</td><td>3.78</td><td>1.55</td><td>•</td><td>•</td></w2<>	2.11	30.99	-	-	34.11	3.78	1.55	•	•
13	D+CG+L+WP	1.93	29.01	•	-	32.56	-1.63	1.54	•	•
14	D + CG + SD + S + W1>	-1.82	32.13	- ·	-	45.04	-4.37	10.07	. •	-
15	D + CG + SD + S + < W1	3.26	34.24	-	-	46.04	2.74	6.96	•	-
16	D+CG+SD+S+W2>	-2.39	32.82			46.79	-3.91	11.18	•	•
17	D+CG+SD+S+ <w2< td=""><td>2.69</td><td>34.92</td><td>-</td><td>-</td><td>47.80</td><td>3.20</td><td>8.07</td><td>•</td><td>-</td></w2<>	2.69	34.92	-	-	47.80	3.20	8.07	•	-
18	D + CG + SD + S + WP	2.51	32.94	-	•	46.24	-2.21	8.06	•	•
19	D+CG+L+E>+EG+	-0.98	32.50	-	-0.67	40.61	-1.98	6.63	•	•
20	D+CG+L+ <e+eg+< td=""><td>1.94</td><td>33.39</td><td></td><td>0.67</td><td>41.57</td><td>1.02</td><td>4.71</td><td>•</td><td>•</td></e+eg+<>	1.94	33.39		0.67	41.57	1.02	4.71	•	•
21	D+CG+SD+S+E>+EG+	-1.12	31.51	-	-0.67	37.18	-1.83	5.00	•	•
22	D+CG+SD+S+ <e+eg+< td=""><td>1.79</td><td>32.40</td><td>-</td><td>0.67</td><td>38.14</td><td>1.16</td><td>3.08</td><td>•</td><td>-</td></e+eg+<>	1.79	32.40	-	0.67	38.14	1.16	3.08	•	-

Maximum Reactions Summary - Framing

MINTERLA	III WCACH	ons Sainm	ary -r	LETHINE							1- 1	77.4	T	37	T and	Mom ccw	Load
X-Loc	Grid	Hrz left	Load	Hrz Right	Load	Hrz In	Load	Hrz Out	Load	Uplift	Load	Vrt Down	Lozo	Mom cw			
A-Ecc	0.10		Case		Case		Case	(Hz)	Case	(-Vv)	Case	(Vy)	Case	(-Mzz)	Case	(Mzz)	Case
1 1		(-Hx)	Casc	. , ,	Casc	` '	Case	` '		,	1		1	(in-k)		(in-k)	
1 1		(k)		(k)		(k)	1 1	(k)	1 1	(k)		(k)		(III-K)		(111-14)	ļ
0/0/0	2-D	4.39	- 2	3.26	15		1	-	-	2.77	4	36.74	2	-	-	-	-
	2-10	4.39	ן ט ן	3.20	1 1		1		امما		1 4	57.77	2	_		_	1 - 1
20/6/0	2-C	-	l - I	<u> </u>		0.67	19	0.67	20	5.79	4	31.11			-	-	1 - E
		1		F 40			1 1	_	1 . 1	4.36	5	14.28	2		- 1	-	-
50/0/0	2-A	4.61	4	5.48	/	-			ا ئا	7.50				<u> </u>	L		

Bracing

X-Loc	Grid	Description
0/0/0	2-D	Diagonal bracing at base is attached to column. Reactions are NOT included with frame reactions. See wall elevation for reactions.
50/0/0	2-A	Diagonal bracing at base is attached to column. Reactions are NOT included with frame reactions. See wall elevation for reactions.

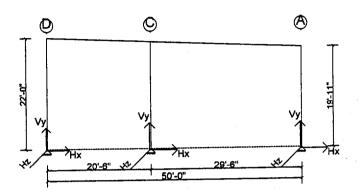


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Wall: 4, Frame at: 40/0/0 Frame ID:CB1 20/6

Frame Type:Continuous Beam



Values shown are resisting forces of the foundation.

Reactions - Load Type at Frame Cross Section: 3

FIXING CIO	33 Occuon.	-		·····			- C-luma		
			Inte		umn				
	0/	0/0							
	3	-D		3-C					
in.)	8 2	c 16		9 x 11					
• •	0.:	375		0.500					
• •	4 - (0.750		4 - 0.750)			1 - 1	*
	100	0'-0"		100'-0"	1	100			
	Hx	Vy	Hx	Hz	Vy	Hx			
	0.07	0.77	-	-	2.21	-0.07	1.12	•	•
			-	-	28.55	-0.03	0.28	•	•
	b			-	9.19	-0.35	4.28	•	-
-			-	۱.	28.15	-1.08	13.10	•	-
		-		-	- 1	-]	-	•	•
	-3 52	-3.12		-	-9.00	-4.93	-1.02	•	. •
			-	-	-6.15	5.29	-5.86	•	•
				-	-6.61	-4.31	0.50	•	-
	3 1			-	-3.76	5.91	-4.34	•	•
	1 -				-6.47	-1.68	-4.12	•	•
			-	-0.67	-0.79	-1.63	1.09	•	•
	••••			-	1.57	•	0.08	•	•
	1.37			0.67	0.79	1.63	-1.09	•	•
	/		-	-	-1.57	-	-0.08	•	•
֡	in.) s (in.) n. (in.) by. Desc. Frm Frm Frm Frm Frm Frm Frm Frm Frm Fr	Exterio 0/ 3 (in.) 8 7 8 7 8 7 8 7 8 7 9 9 9 9 9 9 9 9 9 9	S (in.) 0.375 4 - 0.750 100'-0"	Exterior Column 0/0/0 3-D 8 x 16 0.375 n. (in.) 4 - 0.750 v. 100'-0" Desc. Hx Vy Hx Frm 0.07 0.77 Frm 0.03 28.17 Frm 0.35 2.53 Frm 1.08 7.75 Frm Frm -3.52 -3.12 Frm 2.92 -1.13 Frm 2.92 -1.13 Frm 2.92 -1.13 Frm 2.14 -0.19 Frm 2.10 -2.55 Frm -1.37 -0.27 Frm - 1.45 Frm 1.37 0.27	Exterior Column O/O/O 3-D 3-C	Exterior Column	Exterior Column O/O/O 3-D 3-C 3 3-C 3-	Exterior Column 20/6/0 3-D 3-C 3-A 8 x 16 9 x 11 8 x 13 0.375 0.500 0.375 4 - 0.750 100'-0" 100'-0	Exterior Column



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Frame Reactions - Load Cases at Frame Cross Section: 3

Γ	X-Loc	0,	/0/0		20/6/0		50)/0/0		
	Grid1 - Grid2	3	-D		3-C			-A		
Ld	Description	Hx	Vy	Hx	Hz	Vy	Hx	Vy		
Cs	(application factor not shown)	(k)	(k)	(k)	(k)	(k)	(k)	(k)		
T	D+CG+L	0.45	31.48	-	-	39.95	-0.45	5.67	•	-
2 [D + CG + S	1.18	36.70	-	-	58.91	-1.18	14.50	-	-
3	D + CG + SD + S	1.18	36.70	-	-	58.91	-1.18	14.50	•	-
4	D + W1>	-3.45	-2.35	-	l -	-6.79	-5.01	0.10	•	-
5	D + < W1	2.99	-0.36		-	-3.94	5.22	-4.74	•	•
6	D + W2>	-4.22	-1.41		-	-4.40	-4.39	1.62	•	-
7	D + <w2< td=""><td>2.21</td><td>0.58</td><td>١ -</td><td> -</td><td>-1.55</td><td>5.84</td><td>-3.22</td><td>•</td><td>-</td></w2<>	2.21	0.58	١ -	-	-1.55	5.84	-3.22	•	-
8	D + WP	2.17	-1.77	-	-	-4.26	-1.75	-3.00	-	•
9	D + CG + L + W1 >	-2.28	28.50	١.	-	30.90	-4.06	3.84	•	-
10	D + CG + L + < W1	2.55	30.00	-	-	33.04	3.61	0.21	•	•
11	D + CG + L + W2 >	-2.86	29.21	-	i -	32.69	-3.60	4.98	•	•
12	D + CG + L + < W2	1.97	30.70		-	34.83	4.07	1.35	•	•
13	D + CG + L + WP	1.94	28.94	•	} -	32.80	-1.62	1.51	•	-
14	D + CG + SD + S + W1 >	-1.73	32.42	-	-	45.12	-4 .61	10.46	-	-
15	D + CG + SD + S + < W1	3.10	33.91	-		47.26	3.06	6.83	•	-
16	D + CG + SD + S + W2>	-2.31	33.12	-	-	46.91	-4.14	11.60	-	-
17	D + CG + SD + S + < W2	2.51	34.61	-	-	49.05	3.53.	7.97	•	•
18	D + CG + SD + S + WP	2.48	32.85	-	•	47.01	-2.17	8.13	•	•
19	D+CG+L+E>+EG+	-0.92	32.66	-	-0.67	40.73	-2.08 6.85		•	•
20	D+CG+L+ <e+eg+< td=""><td>1.82</td><td>33.19</td><td>-</td><td>0.67</td><td>42.31</td><td colspan="2">1.17 4.66</td><td>•</td><td></td></e+eg+<>	1.82	33.19	-	0.67	42.31	1.17 4.66		•	
21	D+CG+SD+S+E>+EG+	-1.05 31.680.67 37.17 -1.94 5.19					•	· -		
22	D+CG+SD+S+ <e+eg+< td=""><td>1.68</td><td>32.21</td><td></td><td>0.67</td><td>38.75</td><td>1.31</td><td>3.00</td><td>•</td><td><u> </u></td></e+eg+<>	1.68	32.21		0.67	38.75	1.31	3.00	•	<u> </u>

Maximum Reactions Summary - Framing

MINTERING	m Keacu	ons Samm	aly-I	ct semina														
X-Loc	Grid	Hrz left	Load	Hrz Right	Load	Hrz In	Load	Hrz Out	Load	Uplift	Load	Vrt Down	Load	Mom cw	Load	Mom ccw	Load	1
		(-Hx)	Case	(Hx)	Case	(-Hz)	Case	(Hz)	Case	(-Vy)	Case	(Vy)	Case	(-Mzz)	Case	(Mzz)	Case	
		(k)		(k)		(k)		(k)		(k)		(k)		(in-k)		(in-k)	į	
0/0/0	3-D	4.22	6	3.10	15	-	- 1	-	-	2.35	4	36.70	2	•	-	-	-	
20/6/0	3-C		-	-	-	0.67	19	0.67	20	6.79	4	58.91	2	•	-	-	ı -	
50/0/0	3-A	5.01	4	5.84	.7	-	-	-	-	4.74	5	14.50	2		-	-		

Bracing

X-Loc	Grid	Description
0/0/0	3-D	Diagonal bracing at base is attached to column. Reactions are NOT included with frame reactions. See wall elevation for reactions.
50/0/0	3-A	Diagonal bracing at base is attached to column. Reactions are NOT included with frame reactions. See wall elevation for reactions.

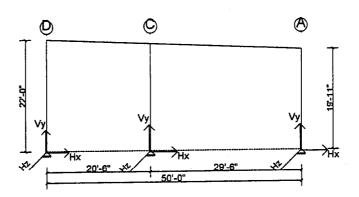


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Wall: 4, Frame at: 60/0/0 Frame ID:CB1 20/6

Frame Type:Continuous Beam



Values shown are resisting forces of the foundation.

Reactions - Load Type	at Frame Cro	ss Section:	4							
Туре		Exterio	r Column	Inte	rior Co			r Column		,
X-Loc		0/	/0/0	l	20/6/0			/0/0		
Grid1 - Grid	2	4	-D		4-C			-A		
Base Plate W x l	(in.)	8 :	x 16	1	9 x 11			x 13		
Base Plate Thickn		0.	375		0.500			375		
Anchor Bolt Qty/Di		4 - 0	0.750		4 - 0.75	0		0.750		
Column Base I		10	0'-0"		100'-0"			0'-0"		
Load Type	Desc.	Hx	Vy	Hx	Hz	Vy	Hx	Vy		
D	Frm	0.07	0.77	-		2.21	-0.07	1.12	•	•
ĆĠ	Frm	0.03	28.17	-	-	28.55	-0.03	0.28	• *	•
Ī.	Frm	0.35	2.53	-	-	9.19	-0.35	4.28	•	-
Š	Frm	1.08	7.75	-	-	28.15	-1.08	13.10	•	-
SD	Frm	-	-	-		-	•	-	•	•
W1>	Frm	-3.52	-3.12	-	-	-9.00	-4.93	-1.02	•	•
<w1< td=""><td>Frm</td><td>2.92</td><td>-1.13</td><td>-</td><td>-</td><td>-6.15</td><td>5.29</td><td>-5.86</td><td>•</td><td>•</td></w1<>	Frm	2.92	-1.13	-	-	-6.15	5.29	-5.86	•	•
W2>	Frm	-4.30	-2.18	-	-	-6.61	-4 .31	0.50	•	•
<w2< td=""><td>Frm</td><td>2.14</td><td>-0.19</td><td>-</td><td></td><td>-3.76</td><td>5.91</td><td>-4.34</td><td>•</td><td>•</td></w2<>	Frm	2.14	-0.19	-		-3.76	5.91	-4.34	•	•
WP	Frm	2.10	-2.55	-	-	-6.47	-1.68	-4.12	•	•
E>	Frm	-1.37	-0.27	-	-0.67	-0.79	-1.63	1.09	•	- .
EG+	Frm		1.45	-	-	1.57	•	0.08	-	•
<e< td=""><td>Frm</td><td>1.37</td><td>0.27</td><td>-</td><td>0.67</td><td>0.79</td><td>1.63</td><td>-1.09</td><td>. •</td><td>•</td></e<>	Frm	1.37	0.27	-	0.67	0.79	1.63	-1.09	. •	•
EG-	Frm	-	-1.45	١ -	-	-1.57	•	-0.08	•	•



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Tian	ie Keathons - Load Cases at Fra			,	551675			10.10	<u> </u>	,
Ĺ	X-Loc		/0/0		20/6/0)/0/0	L	<u> </u>
	Grid1 - Grid2	4	-D		4-C			-A		
Ld	Description	Hx	Vy	Hx	Hz	Vy	Hx	Vy		
Cs	(application factor not shown)	(k)	(k)	(k)	(k)	(k)	(k)	(k)		
1	D + CG + L	0.45	31.48	•	•	39.95	-0.45	5.67	•	•
2	D + CG + S	1.18	36.70	-	-	58.91	-1.18	14.50	•	-
3	D + CG + SD + S	1.18	36.70	-	-	58.91	-1.18	14.50	-	
4	D+W1>	-3.45	-2.35		-	-6.79	-5.01	0.10	•	•
5	D + <w1< td=""><td>2.99</td><td>-0.36</td><td>-</td><td>-</td><td>-3.94</td><td>5.22</td><td>-4.74</td><td>•</td><td>-</td></w1<>	2.99	-0.36	-	-	-3.94	5.22	-4.74	•	-
6	D + W2>	-4.22	-1.41		٠ -	-4.40	-4 .39	1.62	•	-
7	D + <w2< td=""><td>2.21</td><td>0.58</td><td>-</td><td>-</td><td>-1.55</td><td>5.84</td><td>-3.22</td><td>-</td><td>-</td></w2<>	2.21	0.58	-	-	-1.55	5.84	-3.22	-	-
8	D+WP	2.17	-1.77	-	-	-4.26	-1.75	-3.00	-	-
9	D + CG + L + W1>	-2.28	28.50	-	-	30.90	-4 .06	3.84	•	•
10	D + CG + L + < W1	2.55	30.00		-	33.04	3.61	0.21	•	-
11	D + CG + L + W2 >	-2.86	29.21	i -	(-	32.69	-3.60	4.98	•	-
12	D + CG + L + < W2	1.97	30.70	-	-	34.83	4.07	1.35	•	•
13	D + CG + L + WP	1.94	28.94	-	-	32.80	-1.62	1.51	-	-
14	D + CG + SD + S + W1 >	-1.73	32.42	-	-	45.12	-4.61	10.46	•	-
15	D + CG + SD + S + < W1	3.10	33.91	-	-	47.26	3.06	6.83	•	•
16	D + CG + SD + S + W2 >	-2.31	33.12	i -	-	46.91	-4.14	11.60	•	-
17	D + CG + SD + S + < W2	2.51	34.61	-	-	49.05	3.53	7.97	•	-
18	D + CG + SD + S + WP	2.48	32.85	-	-	47.01	-2.17	8.13	•	-
19	D + CG + L + E> + EG+	-0.92	32.66	-	-0.67	40.73	-2.08	6.85	•	•
20	D + CG + L + <e +="" eg+<="" td=""><td>1.82</td><td>33.19</td><td>-</td><td>0.67</td><td>42.31</td><td>1.17</td><td>4.66</td><td>-</td><td>•</td></e>	1.82	33.19	-	0.67	42.31	1.17	4.66	-	•
21	D+CG+SD+S+E>+EG+	-1.05	31.68		-0.67	37.17	-1.94	5.19	-	•
22	D+CG+SD+S+ <e+eg+< td=""><td>1.68</td><td>32.21</td><td>-</td><td>0.67</td><td>38.75</td><td>1.31</td><td>3.00</td><td>•</td><td>•</td></e+eg+<>	1.68	32.21	-	0.67	38.75	1.31	3.00	•	•

	VLUXIII U	m Keacu	ons Summ	IRLA - T	raming														
ſ	X-Loc	Grid	Hrz left	Load	Hrz Right	Load	Hrz In	Load	Hrz Out	Load	Uplift	Load	Vrt Down	Load	Mom cw	Load	Mom ccw	Load	
-1			(-Hx)	Case	(Hx)	Case	(-Hz)	Case	(Hz)	Case	(-Vy)	Case	(Vy)	Case	(-Mzz)	Case	(Mzz)	Case	
- 1			(k)] ~ ;	(k)))	(k)	1 1	(k))]	(k)		(k)		(in-k)		(in-k)		
ľ	0/0/0	4-D	4.22	6	3.10	15	-	- 1		-	2.35	4	36.70	2	-	-	-	-	
	20/6/0	4-C		-	-	i - 1	0.67	19	0.67	20	6.79	4	58.91	2	-	-	-	-	
- 1	50/0/0	4-A	5.01	4	5.84	7	_	1 - 1	_	-	4.74	5	14.50	2	-	-	1 - 1	1 - 1	

Bracing

X-Loc	Grid	Description	
0/0/0	4-D	Diagonal bracing at base is attached to column. Reactions are NOT included with frame reactions. See wall elevation for reactions.	
50/0/0	4-A	Diagonal bracing at base is attached to column. Reactions are NOT included with frame reactions. See wall elevation for reactions.	i

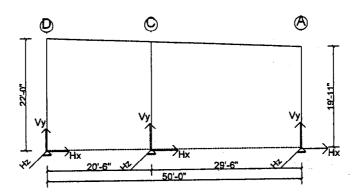


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Wall: 4, Frame at: 80/0/0 Frame ID:CB1 20/6

Frame Type:Continuous Beam



Values shown are resisting forces of the foundation.

Reactions - Load Type at Frame Cross Section: 5

Reactions - Load Type at	FIRME CIO							0.1		
Туре		Exterio	r Column	Inte	rior Col	umn		r Column		
X-Loc		0/	0/0		20/6/0			0/0/0	• •	
Grid1 - Grid2		5	-D		5-C			i-A		
Base Plate W x L	(in.)	8 7	k 20	1	9 x 11			x 13		
Base Plate Thicknes	s (in.)	0.	375	İ	0.500			375		
Anchor Bolt Qty/Dia		4 - 0	0.750		4 - 0.750			0.750		,
Column Base El		100	0'-0"		100'-0"		10	0'-0"		
Load Type	Desc.	Hx	Vy	Hx	Hz	Vy	Hx	Vy		
D	Frm	0.08	0.78	-	-	2.16	-0.08	1.10	•	•
CG	Frm	0.03	28.18	· -	-	28.52	-0.03	0.27	•	-
I.	Frm	0.37	2.54		-	8.85	-0.37	4.21		•
ŝ	Frm	1.15	7.78	-	-	27.09	-1.15	12.90	. •	•
SD	Frm	-	-	-	-	-	-	-	•	•
W1>	Frm	-3.71	-3.55	-	-	-7.95	-4.54	-1.31	•	•
<w1< td=""><td>Frm</td><td>3.06</td><td>-0.74</td><td>-</td><td>-</td><td>-6.61</td><td>4.94</td><td>-5.46</td><td>•</td><td>-</td></w1<>	Frm	3.06	-0.74	-	-	-6.61	4.94	-5.46	•	-
W2>	Frm	-4.47	-2.64	-		-5.61	-3.93	0.16	•	•
<w2< td=""><td>Frm</td><td>2.30</td><td>0.17</td><td> -</td><td>-</td><td>-4.27</td><td>5.55</td><td>-3.99</td><td>•</td><td>•</td></w2<>	Frm	2.30	0.17	-	-	-4.27	5.55	-3.99	•	•
WP	Frm	2.06	-2.47	-		-6.34	-1.66	-4.00		-
E>	Frm	-1.47	-0.45	-	-0.67	-0.48	-1.52	0.97	•	-
EG+	Frm	-	1.45	-	•	1.56	-	0.08	•	•
<e< td=""><td>Frm</td><td>1.47</td><td>0.45</td><td>-</td><td>0.67</td><td>0.48</td><td>1.52</td><td>-0.97</td><td>•</td><td>-</td></e<>	Frm	1.47	0.45	-	0.67	0.48	1.52	-0.97	•	-
EG-	Frm		-1.45	-	-	-1.56	-	-0.08	-	•



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Frame Reactions - Load Cases at Frame Cross Section: 5

	X-Loc	0/	/0/0	T	20/6/0		50)/0/0		
	Grid1 - Grid2	5	-D		5-C			5-A		
Ld	Description	Hx	Vy	Hx	Hz	Vy	Hx	Vy		
Cs	(application factor not shown)	(k)	(k)	(k)	(k)	(k)	(k)	(k)		_
1	D + CG + L	0.48	31.50	-	-	39.53	-0.48	5.59	•	•
2	D + CG + S	1.25	36.74	•	-	57.77	-1.25	14.28	-	-
3	D + CG + SD + S	1.25	36.74	-	-	57.77	-1.25	14.28	-	-
4	D + W1>	-3.63 -2.775.79 -4.61					-0.21	•	-	
5	D+ <w1< td=""><td colspan="6">3.13 0.04 4.45 4.87 4.36</td><td>-</td><td>-</td></w1<>	3.13 0.04 4.45 4.87 4.36						-	-	
6	D + W2>	-4.39	-1.86	1.863.45 -4.00 1.27					•	· -
7	D+ <w2< td=""><td>2.37</td><td>0.95</td><td>-</td><td>-</td><td>-2.11</td><td>5.48</td><td>-2.88</td><td>•</td><td></td></w2<>	2.37	0.95	-	-	-2.11	5.48	-2.88	•	
8	D + WP	2.14	-1.69	-	-	-4.18	-1.73	-2.89	•	
9	D + CG + L + W1 >	-2.40	28.20	-	-	31.35	- 3. 7 9	3.55	-	
10	D + CG + L + < W1	2.68	30.30	-	-	32.36	3.32	0.44	•	•
11	D + CG + L + W2 >	-2.97	28.88	-	- 1	33.11	-3.33	4.66	•	•
12	D + CG + L + < W2	2.11	30.99	-		34.11	3.78	1.55	•	-
13	D + CG + L + WP	1.93	29.01] -	32.56	-1.63	1.54	•	-
14	D + CG + SD + S + W1 >	-1.82	32.13	ļ <u>,-</u>	-	45.04	-4 .37	10.07	•	•
15	D + CG + SD + S + < W1	3.26	34.24	-	-	46.04	2.74	6.96	•	•
16	D + CG + SD + S + W2	-2.39	32.82	-		46.79	-3.91	11.18	•	-
17	D + CG + SD + S + < W2	2.69	34.92	-	-	47.80	3.20	8.07	•	•
18	D + CG + SD + S + WP	2.51	32.94	-	-	46.24	-2.21	8.06	-	•
19	D + CG + L + E> + EG+	-0.99	32.49	-	-0.67	40.61	-2.00	6.64	•	•
20	D+CG+L+ <e+eg+< td=""><td>1.95</td><td>33.39</td><td>-</td><td>0.67</td><td>41.57</td><td>1.03</td><td>4.70</td><td>•</td><td>-</td></e+eg+<>	1.95	33.39	-	0.67	41.57	1.03	4.70	•	-
21	D+CG+SD+S+E>+EG+	+ -1.13 31.510.67 37.18 -1.85 5.0				5.00	•	-		
22	D+CG+SD+S+ <e+eg+< td=""><td>1.81</td><td>32.41</td><td> -</td><td>0.67</td><td>38.14</td><td>1.18</td><td>3.07</td><td>•</td><td>•</td></e+eg+<>	1.81	32.41	-	0.67	38.14	1.18	3.07	•	•

MENTERIN	m Keacu	ons Summ	iary - i	rraming														
X-Loc	Grid	Hrz left	Load	Hrz Right	Load	Hrz In	Load	Hrz Out	Load	Uplift	Load	Vrt Down	Load	Mom cw	Load	Mom ccw	Load	
		(-Hx)	Case	(Hx)	Case	(-Hz)	Case	(Hz)	Case	(-Vy)	Case	(Vy)	Case	(-Mzz)	Case	(Mzz)	Case	
		(k)]	(k)		(k)		(k)		(k)		(k)		(in-k)		(in-k)		
0/0/0	5-D	4.39	6	3.26	15	-	-	-	-	2.77	4	36.74	2	•	-	-	- 1	
20/6/0	5-C		-	- 1	-	0.67	19	0.67	20	5.79	4	57.77	2	-	-	-	ı - I	
50/0/0	5-A	4.61	4	5.48	7	-	1 - 1	-	-	4.36	5	14.28	2		-	- 1	l - 1	

Bracing

X-Loc	Grid	Description
0/0/0	5-D	Diagonal bracing at base is attached to column. Reactions are NOT included with frame reactions. See wall elevation for reactions.
50/0/0	5-A	Diagonal bracing at base is attached to column. Reactions are NOT included with frame reactions. See wall elevation for reactions.



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Wall: 4, Frame at: 99/0/0
Design Load Combinations - Framing

No.	Do Not Use	Origin	Factor	Application	Description
1		System	1.000	1.0 D + 1.0 CG + 1.0 L	D+CG+L
2	[System	1.000	1.0 D + 1.0 CG + 1.0 S	D + CG + S
3		System	1.000	1.0 D + 1.0 CG + 1.0 SD + 1.0 S	D + CG + SD + S
4	[System	1.333	1.0 D + 1.0 W1>	D + W1>
5		System	1.333	1.0 D + 1.0 < W1	D + <w1< td=""></w1<>
6	i i	System	1.333	1.0 D + 1.0 W2>	D + W2>
7	ļ	System	1.333	1.0 D + 1.0 <w2< td=""><td>D + <w2< td=""></w2<></td></w2<>	D + <w2< td=""></w2<>
8	1	System	1.333	1.0 D + 1.0 WP	D + WP
9	, 1	System	1.000	1.0 D + 1.0 CG + 0.750 L + 0.750 W1>	D + CG + L + W1>
10	1 1	System	1.000	1.0 D + 1.0 CG + 0.750 L + 0.750 <w1< td=""><td>D + CG + L + < W1</td></w1<>	D + CG + L + < W1
11	Į.	System	1.000	1.0 D + 1.0 CG + 0.750 L + 0.750 W2>	D + CG + L + W2 >
12	1 1	System	1.000	1.0 D + 1.0 CG + 0.750 L + 0.750 <w2< td=""><td>D + CG + L + < W2</td></w2<>	D + CG + L + < W2
13		System		1.0 D + 1.0 CG + 0.750 L + 0.750 WP	D + CG + L + WP
14		System		1.0 D + 1.0 CG + 0.750 SD + 0.750 S + 0.750 W1>	D + CG + SD + S + W1>
15		System		1.0 D + 1.0 CG + 0.750 SD + 0.750 S + 0.750 <w1< td=""><td>D + CG + SD + S + < W1</td></w1<>	D + CG + SD + S + < W1
16	,	System	,	1.0 D + 1.0 CG + 0.750 SD + 0.750 S + 0.750 W2>	D + CG + SD + S + W2>
17		System		1.0 D + 1.0 CG + 0.750 SD + 0.750 S + 0.750 < W2	D + CG + SD + S + < W2
18]	System		1.0 D + 1.0 CG + 0.750 SD + 0.750 S + 0.750 WP	D + CG + SD + S + WP
19	[System		1.0 D + 1.0 CG + 1.0 L + 1.0 E> + 1.0 EG+	D + CG + L + E> + EG+
20		System		1.0 D + 1.0 CG + 1.0 L + 1.0 <e +="" 1.0="" eg+<="" td=""><td>D+CG+L+<e+eg+< td=""></e+eg+<></td></e>	D+CG+L+ <e+eg+< td=""></e+eg+<>
21	1 1	System		1.0 D + 1.0 CG + 0.200 SD + 0.200 S + 1.0 E> + 1.0 EG+	D+CG+SD+S+E>+EG+
22		System	1.333	1.0 D + 1.0 CG + 0.200 SD + 0.200 S + 1.0 <e +="" 1.0="" eg+<="" td=""><td>D+CG+SD+S+<e+eg+< td=""></e+eg+<></td></e>	D+CG+SD+S+ <e+eg+< td=""></e+eg+<>



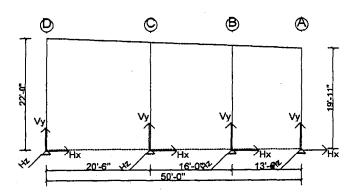
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Wall: 4, Frame at: 99/0/0

Frame ID:cb

Frame Type:Continuous Beam, End Posts



Values shown are resisting forces of the foundation. Reactions - Load Type at Frame Cross Section: 6

Type				Inte	erior Co	lumn	Inte	rior Col	lumn	Exterio	r Column	
X-Loc	0/0/0		20/6/0			36/6/0			50/0/0		j	
Grid1 - Grid2	1 6	5-D	i e	6-C		6-B			6-A			
Base Plate W x L	Base Plate W x L (in.)				8 x 11			8 x 9			x 13	
Base Plate Thicknes	Base Plate Thickness (in.)				0.375			0.375			375	
Anchor Bolt Qty/Dia	m. (in.)		0.750		4 - 0.75			4 - 0.750		1	0.750	
Column Base El	ev.		0'-0"	Ĺ	100'-0"			100'-0"			0'-0"	
Load Type	Desc.	Нх	Vy	Hx	Hz	Vy	Hx	Hz	Vy	Hx	Vy	
D	Frm	0.05	0.74	-	•	1.35	-	-	0.23	-0.05	0.86	•
CG	Frm	0.02	14.11	-	0.30	14.26	•	-	-	-0.02	0.16	•
L	Frm	0.22	1.56	-	-	4.47	-	-	-	-0.22	2.39	•
S	Frm	0.67	4.78	١ -	-	13.68	•	-	-	-0.67	7.33	. •
SD	Frm	-	•	-	-	0.24	-	-	-	-	0.22	-
WI>	Frm	-1.99	-2.10	-	-	-4.14	-	-	-	-2.68	-0.68	•
<w1< td=""><td>Frm</td><td>1.49</td><td>-0.65</td><td>-</td><td>-</td><td>-3.30</td><td>•</td><td>- </td><td>-</td><td>2.61</td><td>-2.96</td><td>•</td></w1<>	Frm	1.49	-0.65	-	-	-3.30	•	-	-	2.61	-2.96	•
W2>	Frm	-2.39	-1.56	-	-	-2.89	-	-	- '	-2.28	0.09	-
<w2< td=""><td>Frm</td><td>1.09</td><td>-0.12</td><td>-</td><td>-</td><td>-2.06</td><td>•</td><td> - </td><td>-</td><td>3.01</td><td>-2.19</td><td>•</td></w2<>	Frm	1.09	-0.12	-	-	-2.06	•	-	-	3.01	-2.19	•
WP	Frm	1.09	-1.45	-	- 1	-3.37	-	i - i	-	-1.09	-2.08	•
E>	Frm	-0.91	-0.32	-	0.21	-0.28	-	0.46	-	-1.09	0.62	-
EG+	Frm		0.73	-	0.01	0.78	•	-	-	-	0.04	•
< <u>E</u>	Frm	0.91	0.32	•	-0.21	0.28	•	-0.46	-	1.09	-0.62	•
EG-	Frm	•	-0.73	-	-0.01	-0.78	•	-	•	-	-0.04	•



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Frame Reactions - Load Cases at Frame Cross Section: 6

	X-Loc	0/		20/6/0			36/6/0			/0/0		
	Grid1 - Grid2	6	-D		6-C		6-B			6-A		
Ld	Description	Hx	Vy	Hx	Hz	Vy	Hx	Hz	Vy	Hx	Vy	
Cs	(application factor not shown)	(k)	(k)	(k)	(k)	(k)	(k)	(k)	(k)	(k)	(k)	
1	D+CG+L	0.29	16.41	-	0.30	20.08	-	•	0.23	-0.29	3.41	•
2	D + CG + S	0.74	19.62	ļ -	0.30	29.29	-		0.23	-0.73	8.35	•
3	D + CG + SD + S	0.74	19.62	-	0.30	29.54	-	-	0.23	-0.73	8.57	-
4	D + W1>	-1.94	-1.36	-	-	-2.78	-	-	0.23	-2.73	0.19	.
5	D+ <w1< td=""><td>1.54</td><td>0.08</td><td> -</td><td>-</td><td>-1.95</td><td>-</td><td> - </td><td>0.23</td><td>2.56</td><td>-2.09</td><td>-</td></w1<>	1.54	0.08	-	-	-1.95	-	-	0.23	2.56	-2.09	-
6	D + W2>	-2.34	-0.82	-		-1.54	-	-	0.23	-2.33	0.95	-
7	D + <w2< td=""><td>1.14</td><td>0.62</td><td>-</td><td> -</td><td>-0.70</td><td>-</td><td></td><td>0.23</td><td>2.96</td><td>-1.33</td><td>•</td></w2<>	1.14	0.62	-	-	-0.70	-		0.23	2.96	-1.33	•
8	D + WP	1.14	-0.72	-	-	-2.02	-	-	0.23	-1.14	-1.22	-
9	D + CG + L + W1 >	-1.26	14.44	-	0.30	15.86	-		0.23	-2.24	2.31	-
10	D + CG + L + < W1	1.35	15.53	-	0.30	16.49	•	-	0.23	1.72	0.60	-
11	D+CG+L+W2>	-1.56	14.85		0.30	16.80	-	-	0.23	-1.94	2.88	•
12	D+CG+L+ <w2< td=""><td>1.05</td><td>15.93</td><td>-</td><td>0.30</td><td>17.42</td><td>•</td><td>- 1</td><td>0.23</td><td>2.03</td><td>1.17</td><td>•</td></w2<>	1.05	15.93	-	0.30	17.42	•	- 1	0.23	2.03	1.17	•
13	D + CG + L + WP	1.05	14.92	-	0.30	16.44	-	-	0.23	-1.05	1.25	•
14	D + CG + SD + S + W1 >	-0.92	16.86	-	0.30	22.95	-	-	0.23	-2.58	6.18	-
15	D + CG + SD + S + < W1	1.69	17.94	-	0.30	23.58	-	-	0.23	1.39	4.47	•
16	D + CG + SD + S + W2 >	-1.22	17.26	-	0.30	23.89	-	- 1	0.23	-2.28	6.75	-
17	D + CG + SD + S + < W2	1.39	18.34		0.30	24.51	-	-	0.23	1.69	5.04	•
18	D + CG + SD + S + WP	1.38	17.34	-	0.30	23.53	•	-	0.23	-1.38	5.12	•
19	D+CG+L+E>+EG+	-0.63	16.81	-	0.52	20.59	-	0.46	0.23	-1.38	4.08	•
20	D + CG + L + <e +="" eg+<="" td=""><td>1.20</td><td>17.46</td><td>-</td><td>0.10</td><td>21.14</td><td>-</td><td>-0.46</td><td>0.23</td><td>0.81</td><td>2.84</td><td>•</td></e>	1.20	17.46	-	0.10	21.14	-	-0.46	0.23	0.81	2.84	•
21	D+CG+SD+S+E>+EG+	-0.71	16.21	-	0.52	18.90	•	0.46	0.23	-1.29	3.19	•
22	D+CG+SD+S+ <e+eg+< td=""><td>1.11</td><td>16.85</td><td></td><td>0.10</td><td>19.46</td><td>-</td><td>-0.46</td><td>0.23</td><td>0.89</td><td>1.96</td><td></td></e+eg+<>	1.11	16.85		0.10	19.46	-	-0.46	0.23	0.89	1.96	

Maximum Reactions Summary - Framing

Management Acoustic Comments - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -																	
X-Loc	Grid	Hrz left	Load	Hrz Right	Load	Hrz In	Load	Hrz Out	Load	Uplift	Load	Vrt Down	Load	Mom cw	Load	Mom ccw	Load
{	1	(-Hx)	Case	(Hx)	Case	(-Hz)	Case	(Hz)	Case	(-Vy)	Case	(Vy)	Case	(-Mzz)	Case	(Mzz)	Case
		(k)		(k)		`(k) ´		(k)		(k)		(k)		(in-k)		(in-k)	
0/0/0	6-D	2.34	6	1.69	15	-	T-	-	-	1.36	4	19.62	2	•	-	•	- 1
20/6/0	6-C		-		-	•	-	0.52	19	2.78	4	29.54	3	-	-		•
36/6/0	6-B		-	-	-	0.46	20	0.46	19 [•	1 - 1	0.23	1	-	-	-	[- [
50/0/0	6-A	2.73	4	2.96	7	-	-	-]	2.09	5	8.57	3	•	•	<u> </u>	اا

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the first periods. We see that the control of a stroy at all the control of the c

BUILDING PERMIT INSPECTION PROCEDURES Please call 874-8703 or 874-8693 to schedule your inspections as agreed upon

Permits expire in 6 months, if the project is not started or ceases for 6 months.

The Owner or their designee is required to notify the inspections office for the following inspections and provide adequate notice. Notice must be called in 48-72 hours in advance in order to schedule an inspection:

in order to schedule an inspection:	
By initializing at each inspection time, you are a inspection procedure and additional fees from a Work Order Release" will be incurred if the probelow. Pre-construction Meeting: Must be scheduled.	"Stop Work Order" and "Stop ocedure is not followed as stated
receipt of this permit. Jay Reynolds, Development also be contacted at this time, before any site work single family additions or alterations.	Review Coordinator at 874-8632 must
MA Footing/Building Location Inspection:	Prior to pouring concrete
A ↑ Re-Bar Schedule Inspection:	Prior to pouring concrete
<u>AA</u> Foundation Inspection:	Prior to placing ANY backfill
$\mathcal{M} \mathcal{A}$ Framing/Rough Plumbing/Electrical:	Prior to any insulating or drywalling
use. N	NOTE: There is a \$75.00 fee per etion at this point.
Certificate of Occupancy is not required for certain you if your project requires a Certificate of Occupa inspection	
\mathcal{M} If any of the inspections do not occur, the	
phase, REGARDLESS OF THE NOTICE OR C	CIRCUMSTANCES.
Er CERIFICATE OF OCCUPANICES MU	IST BE ISSUED AND PAID FOR,
BEFORE THE SPACE MAY BE OCCUPIED	
hh hand	3-4-02
Signature of applicant/designee/	Date
Signature of Inspections Official	3/9/0C Date
	30098

