Form # P 04

DISPLAY THIS CARD ON PRINCIPAL FRONTAGE OF WORK

CITY OF PORTLAND

BUILDING INSPECTION

Please Read Application And Notes, If Any, Attached

PERMIT ISSUED

JAN 18 2006

CITY OF PORTLAND

Permit Number: 060075

PERMIT

This is to certify that BRADCO REALTY CORP / M. Construction

has permission to FOUNDATION ONLY for 3[illegible]00 sq. F[illegible] Metal [illegible]ding

AT 238 RIVERSIDE ST [illegible] 316 B002001

provided that the person or persons, firm or [illegible]ion accepting this permit shall comply with all of the provisions of the Statutes of [illegible]ine and of the [illegible]dances of the City of Portland regulating the construction, maintenance and u[illegible]e of buildings and s[illegible]ctures, and of the application on file in this department.

Apply to Public Works for street line and grade if nature of work requires such information.

N[illegible]fication [illegible] inspe[illegible]n must [illegible]e g[illegible]n and w[illegible]en permi[illegible]on procu[illegible] b[illegible]re this [illegible]ding or [illegible]t there[illegible]s la[illegible]ed or [illegible]osed-in. 4 H[illegible]UR NOTICE IS REQUIRED.

A certificate of occupancy must be procured by owner before this building or part thereof is occupied.

OTHER REQUIRED APPROVALS

Fire Dept. _____

Health Dept. _____

Appeal Board _____

_____
Department Name

Inspection Services 1/18/06

PENALTY FOR REMOVING THIS CARD

City of Portland, Maine - Building or Use Permit Application

389 Congress Street, 04101 Tel: (207) 874-8703, Fax: (207) 874-8716

PERMIT ISSUED
JAN 18 2006
CITY OF PORTLAND

Permit No:	Issue Date:	CBL:
06-0075		316 B002001

Location of Construction:	Owner Name:	Owner Address:	Phone:
238 RIVERSIDE ST	BRADCO REALTY CORP	PO BOX 67	
Business Name:	**Contractor Name:**	**Contractor Address:**	**Phone**
	P.M. Construction	19 Industrial Park Road Saco	2072827697
Lessee/Buyer's Name	**Phone:**	**Permit Type:**	**Zone:**
		Foundation Only/Commercial	

Past Use:	Proposed Use:	Permit Fee:	Cost of Work:	CEO District:
Commercial / Vacant Storage Yard	Commercial/ FOUNDATION 051813		$0.00	5

FIRE DEPT: ☐ Approved ☐ Denied
Previously Approved
Signature:

INSPECTION:
Use Group: S1 Type: 25
FOUNDATION ONLY
1/18/06
Signature:

PEDESTRIAN ACTIVITIES DISTRICT (P.A.D.)
Action: ☐ Approved ☐ Approved w/Conditions ☐ Denied
Signature

Proposed Project Description:

Permit Taken By:	Date Applied For:
ldobson	01/18/2006

1. This permit application does not preclude the Applicant(s) from meeting applicable State and Federal Rules.
2. Building permits do not include plumbing, septic or electrical work.
3. Building permits are void if work is not started within six (6) months of the date of issuance. False information may invalidate a building permit and stop all work..

Zoning Approval

Special Zone or Reviews	Zoning Appeal	Historic Preservation
☐ Shoreland	☐ Variance	☐ Not in District or Landmark
☐ Wetland	☐ Miscellaneous	☐ Does Not Require Review
☐ Flood Zone	☐ Conditional Use	☐ Requires Review
☐ Subdivision	☐ Interpretation	☐ Approved
☐ Site Plan	☐ Approved	☐ Approved w/Conditions
Maj ☐ Minor ☐ MM ☐	☐ Denied	☐ Denied
Date: Previously	late:	late:

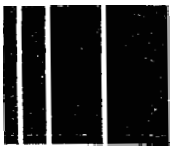
CERTIFICATION

I hereby certify that I am the owner of record of the named property, or that the proposed work is authorized by the owner of record and that I have been authorized by the owner to make this application as his authorized agent and I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in the application is issued, I certify that the code official's authorized representative shall have the authority to enter all areas covered by such permit at any reasonable hour to enforce the provision of the code(s) applicable to such permit.

SIGNATURE OF APPLICANT | ADDRESS | DATE | PHONE

RESPONSIBLE PERSON IN CHARGE OF WORK, TITLE | DATE | PHONE

Sebago Technics
Engineering Expertise You Can Build On



Report on Subsurface and Foundation Investigation

Proposed Building Riverside Street Portland, Maine

for

BRADCO Supply Corp.
13 Production Way
Avenel, NJ 07001

April 20, 2005

One Chabot Street, P.O. Box 1339, Westbrook, Maine 04098-1339 • Ph. 207-856-0277 Fax 856-2206

Sebago Technics
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sebagotechnics.com
One Chabot Street
P.O. Box 1339
Westbrook, Maine
04098-1339
Ph. 207-856-0277
Fax 856-2206

April 20, 2005
04435

Mr. Howard Roberts
BRADCO Supply Corp.
13 Production Way
Avenel, NJ 07001

Report on Subsurface and Foundation Investigation
Proposed Building, Riverside Street, Portland, Maine

Dear Mr. Roberts:

This report presents the results of our evaluation of the subsurface conditions and foundation requirements for the proposed building at your Riverside Street facility in Portland, Maine.

In summary, it is our opinion that the building and storage sheds may be supported on spread and continuous footings bearing on naturally deposited, inorganic soil, or on compacted structural fill placed after removal of unsuitable soil. In addition, slabs-on-grade may be used for the ground floor slabs. Specific recommendations regarding foundation design and construction considerations are presented below.

Introduction

The building will be located in the rear of your facility at 238 Riverside Street in Portland. The building area is presently open, and ground surface elevations vary from approximately El. 73.0 to El. 71.0. We understand that the building will be a pre-engineered metal building with an approximate 22 foot high roof. The ground floor will be at approximately El. 74.0 with truck docks. We understand that the building will be used for storage of wood and other building materials. In addition, storage sheds consisting of metal structures with one side open and concrete floor slabs will be constructed along the north side of the site.

e Explorations

During the period April 7 and 8, 2005, Maine Test Borings, Inc. (MTB) drilled five borings, B1 to B5, at locations shown on Sheet 1, Site and Subsurface Exploration Plan. MTB drilled the borings to depths below ground surface varying from 50.0 feet to 70.0 feet. Sebago Technics monitored the borings and prepared the logs included in Appendix **A.** Table I summarizes the results of borings.

Borings B1 to B4 were drilled using 2.5-inch inside diameter hollow stem augers to a depth of 32 feet with pushed drill rods to 50 feet below ground surface. Boring B5 was drilled using 2.5-inch inside diameter hollow stem augers to a **depth** of **32** feet with pushed **drill** rods to 70 feet below ground surface. Samples were generally recovered at 5-foot intervals above 32 feet. Standard Penetration Resistance (N) was measured at each sample interval in accordance with **ASTM** Test Designation D1586. The undrained shear strength of the clay was measured by field vane shear tests at various depths in the borings.

Sebago Technics, Inc. determined the locations of borings by taping from existing site features.

The boring logs and related information depict subsurface conditions and water levels only at their specific locations at the time of excavation. Soil conditions at other locations may differ from conditions at these locations. Also, the passage of time may result in a change in groundwater conditions at exploration locations.

Subsurface Conditions

The borings encountered three principal soil units at the site: fill, sand and clay. Encountered thickness and generalized descriptions of these units are presented below in order of increasing depth below ground surface. Due to the complexity of the deposition process, strata thickness will vary.

Fill – Fill consists of loose to medium dense, gray to brown, silty SAND (SM); to well-graded SAND **(SW).** Encountered thickness varied from 2.5 feet to **4.0** feet.

Sand – The sand consists of loose, brown well-graded SAND (SW). Boring B5 encountered **3.3** feet of sand.

Clay – Clay consists of stiff to soft, gray brown to gray lean **CLAY (CL)** with sand lenses and partings. Borings penetrated **up** to ***64.2*** feet into the clay.

Water **was** observed in the borings at depths below ground surface varying from 2.0 feet to 13.2 feet. Observations of water were made over a relatively short period of time and may not reflect the stabilized groundwater level. In addition, water levels at the site will vary with season, precipitation, temperature and construction activity in the area. Therefore, water levels during and following construction will vary from those measured in the borings.

Recommendations for Foundation Design

Recommended Foundation Type and Design Criteria

The existing fill is not considered suitable for support of the building or floor slab. **All** fill should be removed from within the foundation limits. In our opinion, the building may be supported on spread and continuous footings bearing on undisturbed, naturally-deposited sand and clay or on compacted structural fill placed after removal of unsuitable soil. The floor slab may be supported on the existing fill following proofrolling, as **described** below, and removal of any unsuitable materials or soft and yielding soils.

For uniformity, footings may be proportioned for an allowable bearing stress in pounds per square foot (psf) equal to 1,000 multiplied by the least lateral dimension of the footing in feet, up to 3,000 psf. All footings should be a minimum of 2.0 feet wide.

Exterior footings should be founded at least 4.5 feet below the lowest adjacent ground surface exposed to freezing. Interior footings should be founded a minimum of 1.5 feet below the ground floor slab.

Compacted structural fill supporting footings should extend laterally from the footings to at least the limits defined by 1 horizontal to 1 vertical lines sloped outward and downward from points located at least 2 feet horizontally beyond the bottom edges of the footings.

In order to consider foundations bearing above the clay stratum, we estimated the settlement of the clay resulting from the increased stress from the raise-in-grade and building loads. We estimated the stress history of the clay stratum by correlating the undrained shear strength with that from other projects in the area. We estimate that the total settlement of the building will be on the order of 1.7 inches, with differential settlement on the order of 1.0 inch in 50 feet. We estimate that approximately 10 to 15 percent of this settlement will occur during the construction period and the remainder will be long-term settlement occurring over 15 to 30 years. We anticipate that settlement of this magnitude is acceptable. However, the structural engineer should determine final acceptability of settlement.

We recommend that the storage sheds be supported on continuous footings bearing on the undisturbed, naturally deposited sand or clay or on compacted structural fill placed after removal of unsuitable soil. Footings should be proportioned for an allowable bearing stress in pounds per square foot (psf) equal to 1,000 multiplied by the least lateral dimension of the footing in feet up to 3,000 psf. All footings should be a minimum of 1.5 feet wide.

Ground Floor Slabs

We recommend that the lowest level floor slab for the building be designed as an earth-supported slab-on-grade bearing on a minimum 6 inches of compacted structural fill. All fill containing debris should be removed from within the building limits prior to placing fill. All fill placed below the floor slab for raises-in-grade should consist of compacted structural fill. Normal dampproofing and vapor barriers should be provided below the slab. The existing fill should be proofrolled with a minimum of two passes using fully-loaded ten-wheel dump trucks or approved similar equipment. Any soft or unsuitable areas identified should be excavated and replaced with compacted structural fill.

Because the concrete floor slabs for the storage sheds will be subjected to freezing temperatures, we recommend that the slabs be designed as earth-supported slabs-on-grade bearing on 2 inches of rigid Styrofoam insulation and 6 inches of compacted structural fill. The insulation should be placed on the excavated subgrade and will minimize the potential for freezing of the subgrade below the open sheds.

Seismic Design Considerations

We recommend that the building be designed in accordance with the seismic requirements of the latest edition of the International Building Code, the site classification is Class E; the site response coefficient F_a is 2.1 for a short period spectral response acceleration S_s of 0.37g; the site response coefficient F_v is 3.5 for the I-second period spectral response acceleration S_1 of 0.10g. The subgrade soils are not considered liquefaction susceptible.

Lateral Foundation Loads

We recommend that lateral loads be resisted by bottom friction on footings and that a coefficient of friction equal to 0.35 be used for footings. If this does not provide sufficient lateral resistance, we will consider the problem in more detail to take into account other factors.

Lateral Soil Pressure

We recommend that the foundation walls at the loading docks which are restrained at the top and backfilled to create an unbalanced soil load be designed to resist a lateral earth pressure calculated on the basis of an equivalent fluid unit weight of 55 pounds per cubic foot. This fluid unit weight assumes an at-rest earth pressure coefficient of 0.45 and a free draining backfill. The portion of the foundation wall at the loading docks will be subject to surcharge due to the loads from people, materials and equipment. The wall should be designed for a uniform lateral pressure acting over the full height of wall, calculated on the basis of 0.5 times the surcharge stress (floor load), in addition to the lateral soil pressure recommended above.

Backfill Materials

Structural fill used below foundations and floor slabs and for backfill adjacent to walls should consist of sandy gravel to gravelly sand. It should be free of organic material, loam, trash, snow, ice, frozen soil and other objectionable material, and should conform to the following gradation:

Sieve Size	Percent Finer by Weight
6 in.	100
No. 4	30 to 90
No. 40	10 to 50
No. 200	0 to 8

Compacted structural fill should be placed in layers not exceeding eight inches in loose measure and compacted by self-propelled vibratory equipment at the approximate optimum moisture content to a dry density of at least 95 percent of the maximum *dry* density, as determined in accordance with **ASTM** Test Designation D1557. In confined areas, the maximum particle size should be reduced to 3 inches and the loose layer thickness should be reduced to 6 inches, and compaction performed by hand-guided vibratory equipment.

Compacted structural fill on the outside of the foundation walls should extend laterally a minimum of 2 feet from the wall. Backfill beyond this limit may consist of common fill. The **top 12** inches of fill on the exterior of the building should consist of low permeability material or bituminous concrete pavement to minimize water infiltration next to the building. Grading should provide for runoff away from the building.

Common fill may consist of inorganic mineral soil that can be placed in layers and compacted. Common fill should be placed and spread in layers not exceeding 12 inches in thickness and compacted with a minimum of two systematic passes of the equipment placing the fill.

Construction Considerations

General

The primary purpose of this section of the report is to comment on items related to excavation, earthwork, and related geotechnical aspects of proposed construction. It is written primarily for the engineer having responsibility for preparation of plans and specifications. Since it identifies potential construction problems related to foundations and earthwork, it will also aid personnel who monitor the construction activity. Contractors for this project must evaluate the construction problems on the basis of their own knowledge and experience in the Portland, Maine area, and on the basis **of** similar projects in other localities, taking into account their proposed construction methods, procedures, equipment and personnel.

Excavation, Lateral Support and Control of Water

We anticipate that foundation excavation can be accomplished with sloped open excavation through the overburden soils provided safe side slopes can be maintained. Some sloughing and raveling should be anticipated in temporary slopes. Temporary excavations should be made in accordance with all **OSHA** and other applicable regulatory agency requirements.

We anticipate that groundwater may be encountered at proposed subgrade level or bearing level of footings. If encountered, open pumping from sumps can likely control groundwater. In general, the contractor should control groundwater and water from runoff and other sources by methods which prevent disturbance of bearing surfaces or adjacent soils and allow construction in-the-dry.

Subgrade Preparation

The subgrade soil is susceptible to disturbance from construction traffic. Equipment and personnel should not be permitted to travel across exposed footing bearing surfaces or exposed slab subgrades. Any subgrade areas that are disturbed should be recompacted or excavated, and replaced with compacted structural fill prior to placing concrete. Subgrades should be protected against freezing temperatures if exposed during construction. Final excavation to subgrade should be performed using equipment with smooth-edge buckets.

Construction Monitoring

The foundation recommendations contained herein are based on the known and predictable behavior of a properly engineered and constructed foundation. Monitoring of the foundation construction is required to enable the geotechnical engineer to keep in contact with procedures and techniques used in construction. Therefore, we recommend that a person qualified by training and experience be present to provide monitoring at the site during preparation of foundation bearing surfaces and placement of compacted structural fill.

Limitations of Recommendations

This report has been prepared for specific application to the subject project in accordance with generally accepted geotechnical engineering practices. In the event that any changes in the nature, design or location of the building are planned, the conclusions and recommendations contained in this report should not be considered valid, unless the changes are reviewed and the conclusions of this report modified or verified in writing.

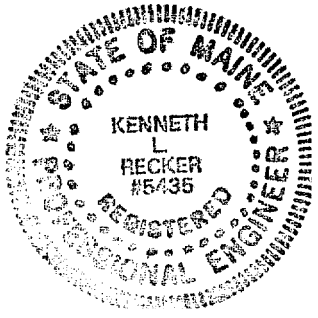
The recommendations presented herein are based in part on the data obtained from the referenced test borings. The nature and extent of variations between the explorations may not become evident until construction. If variations then appear evident, it will be necessary to re-evaluate the recommendations of this report.

We request that we be provided the opportunity for a general review of final design and specifications in order to determine that our earthwork and foundation recommendations have been interpreted and implemented in the design and specifications as they were intended.

It has been a pleasure to work with you on this project. Please do not hesitate to contact us if **you** have any questions or need additional information.

Sincerely,

SEBAGO TECHNICS, INC.



Kenneth L. Recker, P.E.
Geotechnical Engineering Manager

KLR:klr/jc

Enclosures:

Table I	- Summary of Test Borings
Sheet 1	- Site and Subsurface Exploration Plan
Appendix **A**	- Logs of Test Borings

04435

TABLE I
SUMMARY OF BORINGS

PROPOSED BUILDING
238 RIVERSIDE STREET
PORTLAND, MAINE

Boring Number	Depth (Ft)	Depth to Water (Ft)	Strata Thickness (Ft)		
			Fill	Sand	**Clay**
B1	*50.0*	6.0	3.5	--	46.5*
B2	50.0	4.5	4.0	--	46.0*
B3	50.0	13.2	2.7	--	47.3*
B4	*50.0*	5.0	3.0	--	47.0*
B5	70.0	2.0	2.5	3.3	64.2*

NOTES:

1. -- INDICATES STRATUM NOT ENCOUNTERED WITHIN DEPTH OF BORING.
2. * INDICATES DEPTH OF PENETRATION INTO STRATUM.

Appendix A

Logs of Borings

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO. B1

Page 1 of 2

PROJECT	PROPOSED BUILDING, WICKES LUMBER	STI JOB NO.	04435
LOCATION	238 RIVERSIDE STREET, PORTLAND, MAINE	PROJECT MGR.	S. DOE
CLIENT	BRADCO SUPPLY CORP.	FIELD REP.	K. STEPHENSON
CONTRACTOR	MAINE TEST BORINGS, INC.	DATE STARTED	4/7/2005
DRILLER	B. ENOS	DATE FINISHED	4/7/2005

Elevation ft. Datum NGVD 1929 Boring Location See Plan

Item	Casing	Sampler	Core Barrel
Type	HSA	SS	
Inside Diameter (in.)	2.5	1.375	
Hammer Weight (lb.)		140	
Hammer Fall (in.)		30	

Rig Make & Model: Mobile B47

☐ Truck ☐ Tripod ☐ Cat-Head
☐ ATV ☐ Geoprobe ☑ Winch
☑ Track ☐ Air Track ☐ Roller Bit
☐ Skid ☐ Trailer ☑ Cutting Head

Hammer Type: ☐ Safety ☑ Doughnut ☐ Automatic

Drilling Mud: ☐ Bentonite ☐ Polymer ☑ None

Casing Advance: Type Method Depth — HSA/Spin/50.0 ft.

Drilling Notes: 2 in. x 7 in. Field Vane

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity	Field Test Strength
0	3	S1	0.0			SW	Medium dense, brown well-graded SAND (SW), mps = 1.25 in., wet	5	5	30	40	15	5				
	8																
	15				1.6		-FILL-										
	18	12	2.0			SM	Medium dense, gray silty SAND (SM), mps = 0.2 in., wet			20	5	60	15				
							-FILL-										
					3.5												
5	8	S2	5.0			CL	Very stiff, gray-brown mottled lean CLAY (CL), occasional sand partings from 5.0 to 6.0 ft., mps = 0.02 in., damp					5	95	N	M	M	
	9																
	10																
	13	24	7.0														
							-MARINE DEPOSITS-										
10	1	S3	10.0		10.4	CL	Medium stiff, gray-brown mottled lean CLAY (CL), occasional sand partings, wet					5	95	N	M	M	
	2																
	WOH					CL	Soft, gray lean CLAY (CL), occasional sand partings, wet					5	95	N	M	M	
	WOH	24	12.0														
							-MARINE DEPOSITS-										
15	WOR	FV1	15.0-15.6				FV1 from 15.0 to 15.6 ft. = 1/1 ft. lb., Su = 40 psf										
	WOR	S4	15.0		16.0		Note: liquified clay and fine sand in split spoon from 15.0-16.0 ft.					50	50				
	WOR					CL	Soft, gray lean CLAY (CL), wet						100	N	M	M	
	WOR	24	17.0														
							-MARINE DEPOSITS-										
20	WOR	FV2	20.0-20.6				FV2 from 20.0 to 20.6 ft. = 12/7 ft. lb., Su = 440 psf										
	WOH	S5	20.0			CL	Soft, gray lean CLAY (CL), wet						100	N	M	M	
	WOH																
	WOH	24	22.0														
							-MARINE DEPOSITS-										
25	WOR	S6	25.0			CL	Soft, gray lean CLAY (CL), wet						100	N	M	M	
	WOR																
	WOR																
	WOR	24	27.0														
							-MARINE DEPOSITS-										
30																	

Water Level Data

Date	Time	Elapsed Time (hr.)	Depth in feet to: Bottom of Casing	Bottom of Hole	Water
4/7/05	1610		--	11.8	6.0

Sample ID

O Open End Rod
T Thin Wall Tube
U Undisturbed Sample
S Split Spoon Sample
G Geoprobe

Well Diagram

Riser Pipe
Screen
Filter Sand
Cuttings
Grout
Concrete
Bentonite Seal

Summary

Overburden (Linear ft.)	50.0
Rock Cored (Linear ft.)	--
Number of Samples	7S

BORING NO. B1

Field Tests — Dilatancy: R - Rapid S - Slow N - None
Toughness: L - Low M - Medium H - High
Plasticity: N - Nonplastic L - Low M - Medium H - High
Dry Strength: N - None L - Low M - Medium H - High V - Very High

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO. B1

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity	Field Test Strength
30	WOR	FV3	30.0 to 30.6				FV3 from 30.0 to 30.6 ft. = 12/7 ft. lbs., Su = 440 psf										
	WOR	S7	30.0			CL	Soft, gray lean CLAY (CL), wet						100	N	M	M	
	WOR						-MARINE DEPOSITS-										
	WOR	24	32.0				Begin rod probe at 32.0 ft.										
							Depth Blow Counts										
							32.0-33.0 ft. Hydraulic Push										
							33.0-34.0 ft. Hydraulic Push										
35							34.0-35.0 ft. Hydraulic Push										
							35.0-36.0 ft. Hydraulic Push										
							36.0-37.0 ft. Hydraulic Push										
							37.0-38.0 ft. Hydraulic Push										
							38.0-39.0 ft. Hydraulic Push										
40							39.0-40.0 Hydraulic Push										
							40.0-41.0 ft. Hydraulic Push										
							41.0-42.0 ft. Hydraulic Push										
							42.0-43.0 ft. Hydraulic Push										
							43.0-44.0 ft. Hydraulic Push										
45							44.0-45.0 Hydraulic Push										
							45.0-46.0 ft. Hydraulic Push										
							46.0-47.0 ft. Hydraulic Push										
							47.0-48.0 ft. Hydraulic Push										
							48.0-49.0 ft. Hydraulic Push										
50							49.0-50.0 ft. Hydraulic Push										
							Bottom of exploration at 50.0 ft. below ground surface										
							No refusal										
55																	
60																	
65																	
70																	

NOTES:

FILE NO. 04435 BORING NO. B1

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

EBAGO ECHNICS, IC.

TEST BORING REPORT

BORING NO **B2**

Page 1 of 2

DRILLER B. ENOS — DATE FINISHED 4/8/2005

Elevation ft. — Datum NGVD 1929 — Boring Location See Plan

	Casing	Sampler	Core Barrel
Item			
Type	HSA	SS	
Inside Diameter (in.)	2.5	1.375	
Hammer Weight (lb.)		140	
Hammer Fall (in.)		30	

Rig Make & Model: Mobile B47

☐ Truck ☐ Tripod ☐ Cat-Head ☐ Winch ☐ Roller Bit ☑ Cutting Head

☐ ATV ☐ Geoprobe

☑ Track ☐ Air Track

☐ Skid ☐ Trailer

Hammer Type: ☐ Safety ☑ Doughnut ☐ Automatic

Drilling Mud: ☐ Bentonite ☐ Polymer ☑ None

Casing Advance: Type Method Depth — HSA/Spin/50.0 ft.

Drilling Notes: 2 in. x 7 in. Field Vane

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity
0	2	S1	0.0			SW	Loose, brown well-graded SAND (SW), mps = 0.2 in., trace roots,			30	50	20				
	4				1.0		wet -FILL-									
	3					SM	Loose, gray-brown mottled silty SAND (SM), mps = 0.02 in., trace									
	5	14	2.0				roots, cinders, damp					85	15			
							-FILL-									
					3.0											
							Note: brown silty sand in auger cuttings from 3.0 to 4.0 ft.									
					4.0		-Probable Original TOPSOIL-									
5	4	S2	5.0			CL	Stiff, gray lean CLAY (CL), occasional sand partings, damp					5	95	N	M	M
	5															
	9															
	11	24	7.0													
							-MARINE DEPOSITS-									
10	3	S3	10.0			CL	Medium stiff, gray lean CLAY (CL), occasional sand partings,					5	95	N	M	M
	3						wet									
	2															
	2	24	12.0													
							-MARINE DEPOSITS-									
15	WOR	FV1	15.0-15.6				FV1 from 15.0									
	WOR	S4	15.0			CL	Soft, gray lean						100	N	M	M
	WOR															
	WOR	24	17.0													
20	WOR	S5	20.0			CL	S					10	90	N	M	M
	WOR						we									
	WOR															
	WOR	24	22.0													
25	WOR	S6	25.0			CL	Soft, gray lean CLAY (CL), 0.5 in. seam of gray-brown clay						100	N	M	M
	WOR						at 26.0 ft., wet									
	WOR															
	WOR	24	27.0													
30																

Water Level Data

Date	Time	Elapsed Time (hr.)	Depth in feet to: Bottom of Casing	Bottom of Hole	Water
4/8/05	0829		--	15.0	4.5

Sample ID

O Open End Rod
T Thin Wall Tube
G Geoprobe

Well Diagram

Riser Pipe
Screen
Filter Sand
Cuttings
Grout
Concrete
Bentonite Seal

Summary

	50.0
Rock Cored (Linear ft.)	--
Number of Samples	7S
BORING NO.	B2

Field Tests

Dilatancy: R - Rapid S - Slow N - None
Toughness: L - Low M - Medium H - High
Plasticity: N - Nonplastic L - Low M - Medium H - High
Dry Strength: N - None L - Low M - Medium H - High V - Very High

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO. B2

Page 2 of 2

Depth (ft.)	Sampler Blows per 6 In.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity	Field Test Strength
30																	
	WOR	FV2	30.0 to 30.6				FV2 from 30.0 to 30.6 ft. = 10/5 ft. lbs., Su = 370 psf										
	WOR	S7	30.0			CL	Soft, gray lean CLAY (CL), wet						100	N	M	M	
	WOR						-MARINE DEPOSITS-										
	WOR	24	32.0				Begin rod probe at 32.0 ft.										
							Depth Blow Counts										
							32.0-33.0 ft. Hydraulic Push										
							33.0-34.0 ft. Hydraulic Push										
35							34.0-35.0 ft. Hydraulic Push										
							35.0-36.0 ft. Hydraulic Push										
							36.0-37.0 ft. Hydraulic Push										
							37.0-38.0 ft. Hydraulic Push										
							38.0-39.0 ft. Hydraulic Push										
40							39.0-40.0 Hydraulic Push										
							40.0-41.0 ft. Hydraulic Push										
							41.0-42.0 ft. Hydraulic Push										
							42.0-43.0 ft. Hydraulic Push										
							43.0-44.0 ft. Hydraulic Push										
45							44.0-45.0 Hydraulic Push										
							45.0-46.0 ft. Hydraulic Push										
							46.0-47.0 ft. Hydraulic Push										
							47.0-48.0 ft. Hydraulic Push										
							48.0-49.0 ft. Hydraulic Push										
50							49.0-50.0 ft. Hydraulic Push										
							Bottom of exploration at 50.0 ft. below ground surface										
							No refusal										
55																	
60																	
65																	
70																	

NOTES:

FILE NO. 04435

BORING NO. B2

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING No. B3

Page 1 of 2

PROJECT	PROPOSED BUILDING, WICKES LUMBER	STI JOB NO.	04435
LOCATION	238 RIVERSIDE STREET, PORTLAND, MAINE	PROJECT MGR.	S. DOE
CLIENT	BRADCO SUPPLY CORP.	FIELD REP.	K. STEPHENSON
CONTRACTOR	MAINE TEST BORINGS, INC.	DATE STARTED	4/8/2005
DRILLER	B. ENOS	DATE FINISHED	4/8/2005

Elevation ft. | Datum NGVD 1929 | Boring Location See Plan

Item	Casing	Sampler	Core Barrel
Type	HSA	SS	
Inside Diameter (in.)	2.5	1.375	
Hammer Weight (lb.)		140	
Hammer Fall (in.)		30	

Rig Make & Model: Mobile B47

☐ Truck ☐ Tripod ☐ Cat-Head
☐ ATV ☐ Geoprobe ☑ Winch
☑ Track ☐ Air Track ☐ Roller Bit
☐ Skid ☐ Trailer ☑ Cutting Head

Hammer Type: ☐ Safety ☑ Doughnut ☐ Automatic

Drilling Mud: ☐ Bentonite ☐ Polymer ☑ None

Casing Advance: Type Method Depth — HSA/Spin/50.0 ft.

Drilling Notes: 2 in. x 7 in. Field Vane

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity	Field Test Strength
0	3	S1	0.0			SW	Medium dense, brown well-graded SAND (SW), mps = 0.25 in., wet		5	40	30	15	10				
	5						Note: probable gravel and/or cobbles										
	11																
	50/.1	3	1.6				-FILL-										
					2.7												
5	8	S2	5.0			CL	Stiff, gray-brown mottled lean CLAY (CL), occasional sand partings, damp					5	95	N	M	M	
	8																
	8				6.4												
	6	24	7.0			CL	Stiff, gray lean CLAY (CL), occasional sand partings, wet					5	95	N	M	M	
							-MARINE DEPOSITS-										
10	4	S3	10.0			CL	Medium stiff, gray lean CLAY (CL), occasional sand partings, wet					5	95	N	M	M	
	3																
	2																
	3	24	12.0														
							-MARINE DEPOSITS-										
15	WOR	FV1	15.0-15.6				FV1 from 15.0 to 15.6ft. = 1015ft. lb., Su = 370psf										
	WOR	S4	15.0			CL	Soft, gray lean CLAY (CL), wet						100	N	M	M	
	WOR																
	WOH	12	17.0														
							-MARINE DEPOSITS-										
20	WOR	S5	20.0			CL	Soft, gray lean CLAY (CL), wet						100	N	M	M	
	WOH																
	WOH																
	WOH	24	22.0														
							-MARINE DEPOSITS-										
25	WOR	S6	25.0			CL	Soft, gray lean CLAY (CL), wet						100	N	M	M	
	WOR																
	WOR																
	WOR	24	27.0														
							-MARINE DEPOSITS-										
30																	

Water Level Data

Date	Time	Elapsed Time (hr.)	Depth in feet to: Bottom of Casing	Bottom of Hole	Water
4/8/05	1400		--	16.0	13.2

Sample ID

O Open End Rod
T Thin Wall Tube
U Undisturbed Sample
S Split Spoon Sample
G Geoprobe

Well Diagram

Riser Pipe
Screen
Filter Sand
Cuttings
Grout
Concrete
Bentonite Seal

Summary

Overburden (Linear ft.)	50.0
Rock Cored (Linear ft.)	--
Number of Samples	7S

BORING NO. B3

Field Tests
Dilatancy: R - Rapid S - Slow N - None
Toughness: L - Low M - Medium H - High
Plasticity: N - Nonplastic L - Low M - Medium H - High
Dry Strength: N - None L - Low M - Medium H - High V - Very High

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO. B3

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (In.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity	Field Test Strength
30	WOR	S7	30.0				Attempted FV at 30.0 ft.- obstruction in augers- probable gravel										
	WOR					CL	Very soft, gray lean CLAY (CL), wet						100	N	M	M	
	WOR						-MARINE DEPOSITS-										
	WOR	24	32.0				Begin rod probe at 32.0 ft.										
							Depth Blow Counts										
							32.0-33.0 ft. Hydraulic Push										
							33.0-34.0 ft. Hydraulic Push										
35							34.0-35.0 ft. Hydraulic Push										
							35.0-36.0 ft. Hydraulic Push										
							36.0-37.0 ft. Hydraulic Push										
							37.0-38.0 ft. Hydraulic Push										
							38.0-39.0 ft. Hydraulic Push										
40							39.0-40.0 Hydraulic Push										
							40.0-41.0 ft. Hydraulic Push										
							41.0-42.0 ft. Hydraulic Push										
							42.0-43.0 ft. Hydraulic Push										
							43.0-44.0 ft. Hydraulic Push										
45							44.0-45.0 Hydraulic Push										
							45.0-46.0 ft. Hydraulic Push										
							46.0-47.0 ft. Hydraulic Push										
							47.0-48.0 ft. Hydraulic Push										
							48.0-49.0 ft. Hydraulic Push										
50							49.0-50.0 ft. Hydraulic Push										
							Bottom of exploration at 50.0 ft. below ground surface No refusal										
55																	
60																	
65																	
70																	

NOTES:

FILE NO. 04435

BORING NO. B3

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications basea on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO. **B4**

Page 1 of 2

PROJECT	PROPOSED BUILDING, WICKES LUMBER	STI JOB NO.	04435
LOCATION	238 RIVERSIDE STREET, PORTLAND, MAINE	PROJECT MGR.	S. DOE
CLIENT	BRADCO SUPPLY CORP.	FIELD REP.	K. STEPHENSON
CONTRACTOR	MAINE TEST BORINGS, INC.	DATE STARTED	4/7/2005
DRILLER	B. ENOS	DATE FINISHED	4/7/2005

Elevation ft. Datum NGVD 1929 — Boring Location See Plan

Item	Casing	Sampler	Core Barrel
Type	HSA	SS	
Inside Diameter (in.)	2.5	1.375	
Hammer Weight (lb.)		140	
Hammer Fall (in.)		30	

Rig Make & Model: Mobile B47

☐ Truck ☐ Tripod ☐ Cat-Head
☐ ATV ☐ Geoprobe ☑ Winch
☑ Track ☐ Air Track ☐ Roller Bit
☐ Skid ☐ Trailer ☑ Cutting Head

Hammer Type: ☐ Safety ☑ Doughnut ☐ Automatic

Drilling Mud: ☐ Bentonite ☐ Polymer ☑ None

Casing Advance: Type Method Depth — HSA/Spin/50.0 ft.

Drilling Notes: 2 in. x 7 in. Field Vane

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity	Field Test Strength
0	15	S1	0.0			SW	Medium dense, brown well-graded SAND with silt (SW), mps = 0.25 in., wet		5	30	30	25	10				
	14						Note: probably pushed gravel- brown sand with gravel in auger cuttings from 0.0 to 3.0 ft.										
	8						-FILL-										
	3	1	2.0		3.0												
5	6	S2	5.0			CL	Very stiff, gray-brown mottled lean CLAY (CL), frequent sand partings, damp					10	90	N	M	M	
	9																
	10																
	14	24	7.0				-MARINE DEPOSITS-										
10	5	S3	10.0			CL	Very stiff, gray-brown mottled lean CLAY (CL), frequent sand partings, wet					5	95	N	M	M	
	5				10.8												
	5					CL	Stiff, gray lean CLAY (CL), occasional sand partings, wet					10	90	N	M	M	
	4	24	12.0				-MARINE DEPOSITS-										
15	WOR	FV1	15.2-15.8				FV1 from 15.2 to 15.8 ft. = 20/5 ft. lb., Su = 740 psf										
	WOR	S4	15.0			CL	Medium stiff, gray lean CLAY (CL), wet						100	N	M	M	
	WOR																
	WOH	12	17.0				-MARINE DEPOSITS-										
20	WOR	S5	20.0			CL	Medium stiff, gray lean CLAY (CL), wet						100	N	M	M	
	WOR																
	WOR																
	WOR	16	22.0				-MARINE DEPOSITS-										
25	WOR	S6	25.0			CL	Medium stiff, gray lean CLAY (CL), wet						100	N	M	M	
	WOR																
	WOR																
	WOR	24	27.0				-MARINE DEPOSITS-										
30																	

Water Level Data

Date	Time	Elapsed Time (hr.)	Depth in feet to: Bottom of Casing	Bottom of Hole	Water
4/7/2005	1825		--	14.5	5.0

Sample ID

O Open End Rod
T Thin Wall Tube
U Undisturbed Sample
S Split Spoon Sample
G Geoprobe

Well Diagram

Riser Pipe
Screen
Filter Sand
Cuttings
Grout
Concrete
Bentonite Seal

Summary

Overburden (Linear ft.)	50.0
Rock Cored (Linear ft.)	--
Number of Samples	7S

BORING NO. B4

Field Tests
Dilatancy: R - Rapid S - Slow N - None
Toughness: L - Low M - Medium H - High
Plasticity: N - Nonplastic L - Low M - Medium H - High
Dry Strength: N - None L - Low M - Medium H - High V - Very High

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO, **B4**

Depth (ft.)	Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity	Field Test Strength
30	WOR	S7	30.0				FV2 from 30.0 to 30.6 ft. = 7/3 ft. lb., Su = 260 psf										
	WOR					CL	Soft, gray lean CLAY (CL), wet						100	N	M	M	
	WOR						-MARINE DEPOSITS-										
	WOR	24	32.0				Begin rod probe at 32.0 ft.										
							Depth — Blow Counts										
							32.0-33.0 ft. — Hydraulic Push										
							33.0-34.0 ft. — Hydraulic Push										
35							34.0-35.0 ft. — Hydraulic Push										
							35.0-36.0 ft. — Hydraulic Push										
							36.0-37.0 ft. — Hydraulic Push										
							37.0-38.0 ft. — Hydraulic Push										
							38.0-39.0 ft. — Hydraulic Push										
40							39.0-40.0 — Hydraulic Push										
							40.0-41.0 ft. — Hydraulic Push										
							41.0-42.0 ft. — Hydraulic Push										
							42.0-43.0 ft. — Hydraulic Push										
							43.0-44.0 ft. — Hydraulic Push										
45							44.0-45.0 — Hydraulic Push										
							45.0-46.0 ft. — Hydraulic Push										
							46.0-47.0 ft. — Hydraulic Push										
							47.0-48.0 ft. — Hydraulic Push										
							48.0-49.0 ft. — Hydraulic Push										
50							49.0-50.0 ft. — Hydraulic Push										
							Bottom of exploration at 50.0 ft. below ground surface										
							No refusal										
55																	
60																	
65																	
70																	

NOTES:

FILE NO. 04435 BORING NO. B4

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO. B5

Page 1 of 2

PROJECT	PROPOSED BUILDING, WICKES LUMBER	STI JOB NO.	04435
LOCATION	238 RIVERSIDE STREET, PORTLAND, MAINE	PROJECT MGR.	S. DOE
CLIENT	BRADCO SUPPLY CORP.	FIELD REP.	K. STEPHENSON
CONTRACTOR	MAINE TEST BORINGS, INC.	DATE STARTED	4/8/2005
DRILLER	B. ENOS	DATE FINISHED	4/8/2005

Elevation ft. | Datum NGVD 1929 | Boring Location See Plan

Item	Casing	Sampler	Core Barrel
Type	HSA	SS	
Inside Diameter (in.)	2.5	1.375	
Hammer Weight (lb.)		140	
Hammer Fall (in.)		30	

Rig Make & Model: Mobile B47

☐ Truck ☐ Tripod ☐ Cat-Head
☐ ATV ☐ Geoprobe ☑ Winch
☑ Track ☐ Air Track ☐ Roller Bit
☐ Skid ☐ Trailer ☑ Cutting Head

Hammer Type: ☐ Safety ☑ Doughnut ☐ Automatic

Drilling Mud: ☐ Bentonite ☐ Polymer ☑ None

Casing Advance: Type Method Depth — HSA/Spin/70.0 ft.

Drilling Notes: 2 in. x 7 in. Field Vane

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity	Field Test Strength
0	9	S1	0.0			SM	Medium dense, gray-brown silty SAND with gravel (SM), mps =	10	10	10	5	50	15				
	9				0.6		1.38 in., wet										
	10					SW	Medium dense, brown well-graded SAND (SW), mps = 1.0 in.,	5	5	40	30	15	5				
	9	14	2.0				damp										
					2.5		-FILL-										
							Note: brown sand in auger cuttings at 2.5 ft.										
5	6	S2	5.0			SW	Loose, brown well-graded SAND (SW), mps = 0.2 in., wet			50	40	10					
	4				5.8												
	4					CL	Medium stiff, gray lean CLAY (CL), occasional sand partings, wet					5	95	N	M	M	
	6	20	7.0														
							-MARINE DEPOSITS-										
10	4	FV1	10.0-10.6				FV1 from 10.0 to 10.6 ft. =35/15 ft. lb., Su = 1,300 psf										
	5	S3	10.0				Note: vane probable binding on sand										
	6					CL	Stiff, gray lean CLAY (CL), medium to fine sand lenses from 10.0					10	90	N	M	M	
	7	24	12.0				to 10.5 ft., occasional sand partings, wet										
							-MARINE DEPOSITS-										
15	WOH	FV2	15.0-15.6				FV2 from 15.0 to 15.6 ft. = 7/3 ft. lb., Su = 260 psf										
	WOH	S4	15.0			CL	Soft, gray lean CLAY (CL), occasional sand partings, wet					5	95	N	M	M	
	WOH																
	1	12	17.0														
							-MARINE DEPOSITS-										
20	WOR	S5	20.0			CL	Soft, gray lean CLAY (CL), occasional sand partings, wet					5	95	N	M	M	
	WOR																
	WOH																
	WOH	24	22.0														
							-MARINE DEPOSITS-										
25	WOR	S6	25.0			CL	Soft, gray lean CLAY (CL), wet						100	N	M	M	
	WOR																
	WOR																
	WOR	24	27.0														
							-MARINE DEPOSITS-										
30																	

Water Level Data

Date	Time	Elapsed Time (hr.)	Depth in feet to: Bottom of Casing	Bottom of Hole	Water
4/8/2005	1145		--	4.2	2.0

Sample ID

O Open End Rod
T Thin Wall Tube
U Undisturbed Sample
S Split Spoon Sample
G Geoprobe

Well Diagram

Riser Pipe
Screen
Filter Sand
Cuttings
Grout
Concrete
Bentonite Seal

Summary

Overburden (Linear ft.)	70.0
Rock Cored (Linear ft.)	--
Number of Samples	7S

BORING NO. B5

Field Tests — Dilatancy: R - Rapid S - Slow N - None
Toughness: L - Low M - Medium H - High
Plasticity: N - Nonplastic L - Low M - Medium H - High
Dry Strength: N - None L - Low M - Medium H - High V - Very High

*NOTE: Maximum Particle Size is determined by direct observation within the limitations of sampler size.

NOTE: Soil identifications based on visual-manual methods of the USCS system as practiced by Sebago Technics, Inc.

SEBAGO TECHNICS, INC.

TEST BORING REPORT

BORING NO. B5

Depth (ft.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel % Coarse	Gravel % Fine	Sand % Coarse	Sand % Medium	Sand % Fine	% Fines	Field Test Dilatancy	Field Test Toughness	Field Test Plasticity
30	WOR	FV3	30.0-30.6				FV3 from 30.0 to 30.6 ft. = 5/5 ft. lb., Su = 190 psf									
	WOR	S7	30.0			CL	Soft, gray lean CLAY (CL), wet						100	N	M	M
	WOR						-MARINE DEPOSITS-									
	WOR	24	32.0				Begin rod probe at 32.0 ft.									
							Depth Blow Counts									
							32.0-33.0 ft. Hydraulic Push									
							33.0-34.0 ft. Hydraulic Push									
35							34.0-35.0 ft. Hydraulic Push									
							35.0-36.0 ft. Hydraulic Push									
							36.0-37.0 ft. Hydraulic Push									
							37.0-38.0 ft. Hydraulic Push									
							38.0-39.0 ft. Hydraulic Push									
40							39.0-40.0 Hydraulic Push									
							40.0-41.0 ft. Hydraulic Push									
							41.0-42.0 ft. Hydraulic Push									
							42.0-43.0 ft. Hydraulic Push									
							43.0-44.0 ft. Hydraulic Push									
45							44.0-45.0 Hydraulic Push									
							45.0-46.0 ft. Hydraulic Push									
							46.0-47.0 ft. Hydraulic Push									
							47.0-48.0 ft. Hydraulic Push									
							48.0-49.0 ft. Hydraulic Push									
50							49.0-50.0 ft. Hydraulic Push									
							50.0-51.0 ft. Hydraulic Push									
							51.0-52.0 ft. Hydraulic Push									
							52.0-53.0 ft. Hydraulic Push									
							53.0-54.0 ft. Hydraulic Push									
55							54.0-55.0 ft. Hydraulic Push									
							55.0-56.0 ft. Hydraulic Push									
							56.0-57.0 ft. Hydraulic Push									
							57.0-58.0 ft. Hydraulic Push									
							58.0-59.0 ft. Hydraulic Push									
60							59.0-60.0 ft. Hydraulic Push									
							60.0-61.0 ft. Hydraulic Push									
							61.0-62.0 ft. Hydraulic Push									
							62.0-63.0 ft. Hydraulic Push									
							63.0-64.0 ft. Hydraulic Push									
65							64.0-65.0 ft. Hydraulic Push									
							65 0-66 0 ft Hydraulic Push									
							66 0-67 0 ft Hydraulic Push									
							67 0-68 0 ft Hydraulic Push									
							68 0-69 0 ft Hydraulic Push									
70							69.0-70.0 ft. Hydraulic Push									
							Bottom of exploration at 70.0 ft. below gr. surface-No refusal									

NOTES

JOB NO. 04435 BORING NO. B5

*NOTE Maximum Parbcle Sue is determined by direct observation within the limitations of sampler size.

NOTE. boll identifications basea an visual-manual methods of the USCS system as practiced by Sebago Techn... Inc.