Form # P 04 DISPLAY THIS CARI	O ON PRINCIPAL FRONTAGE OF WORK
Please Read Application And Notes, If Any, Attached	PERMIT ISSUED PERMIT ISSUED Permit Numble No600% 2006
This is to certify thatBRADCO REALTY CORP	1. Construction CITY OF PORTLAND
has permission to FOUNDATION ONLY for 2	00 sq. F Actal and Ing
AT _238 RIVERSIDE ST	. 316_B002001
of the provisions of the Statutes of the construction, maintenance and uthis department.	ine and of the finances of the City of Portland regulating of buildings and surveys, and of the application on file in fication of inspect on must be n and when permit on procure re this and ing or and there as ed or a survey solution of the application on file in IB NOTWORD FIEQUIBED.
OTHER REQUIRED APPROVALS	
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PENA	LTY FOR REMOVING THIS CARD

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					PERMIT IS	SUED	
City of Portland, Maine 389 Congress Street 04101	• Building or Use Tel: (207) 874-8703	Permit Applicatio	n ^{Perr}	nit No: 06-0075	Issue Date:	CBL: 316 B(02001
Location of Construction:	Owner Name:	, 1 u. (207) 074 071		Address	UAN 1 8	7/1/1C	
238 RIVERSIDE ST	BRADCORE	ALTY CORP	PO B	OX 67		inone.	
Business Name:	Contractor Name P.M. Construc	tion	Contra 19 Inc	ctorAddr C	ITY OF POI	TIAN P one 2072827	697
Lessee/Buyer's Name	Phone:		Permit	Туре:		· ·	Zone:
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Past Use:	Proposed Use:		Permit	Fee:	Cost of Work:	CEO District:	
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Proposed Project Description:			Signatu	re:	s	Signature:	junzi 1
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			Action:	Appro	oved 🗌 Appro	ved w/Conditions	Denied
			Signatu	ire.			
Permit Taken By: ldobson	Date Applied For: 01/18/2006			Zonin	g Approval		
1 This permit application do	es not preclude the	Special Zone or Revie	ews	Zon	ing Appeal	Historic Pre	servation
Applicant(s) from meeting Federal Rules.	applicable State and	Shoreland		📃 Varian	ce	Not in Distri	ict or Landmarl
2. Building permits do not in septic or electrical work.	clude plumbing,	Wetland		Miscel	laneous	Does Not Re	equire Review
3. Building permits are void within six (6) months of th	if work is not started e date of issuance.	Flood Zone		Condit	ional Use	Requires Re	view
False information may inv permit and stop all work	alidate a building	Subdivision		Interpr	etation	Approved	
		Site Plan		Approv	ved	Approved w	/Conditions
				Denied	l	Denied	
		Date: PARVIC		late:		late:	

CERTIFICATION

I hereby certify that I am the owner of record of the named property, or that the proposed work is authorized by the owner of record and that I have been authorized by the owner to make this application as his authorized agent and I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in the application is issued, I certify that the code official's authorized representative shall have the authority to enter all areas covered by such permit at any reasonable hour to enforce the provision of the code(s) applicable to such permit.

SIGNATURE OF APPLICANT	ADDRESS	DATE	PHONE
RESPONSIBLE PERSON IN CHARGE OF WORK, TITLE		DATE	PHONE





Report on Subsurface and Foundation Investigation

Proposed Building Riverside Street Portland, Maine

for

BRADCO Supply Corp. 13 Production Way Avenel, NJ 07001

April 20, 2005



sebagotechnics.com

One Chabot Street P.O. **Box** 1339 Westbrook, Maine 04098-1339 Ph. 207-856-0277 Fax 856-2206

April 20, 2005 04435

Mr. Howard Roberts BRADCO Supply Corp. 13 Production Way Avenel, NJ 07001

<u>Report on Subsurface and Foundation Investigation</u> Proposed Building, Riverside Street, Portland, Maine

Dear Mr. Roberts:

This report presents the results of our evaluation of the subsurface conditions and foundation requirements for the proposed building at your Riverside Street facility in Portland, Maine.

In summary, it is our opinion that the building and storage sheds may be supported on spread and continuous footings bearing on naturally deposited, inorganic soil, or on compacted structural fill placed after removal of unsuitable soil. In addition, slabs-on-grade may be used for the ground floor slabs. Specific recommendations regarding foundation design and construction considerations are presented below.

Introduction

The building will be located in the rear of your facility at 238 Riverside Street in Portland. The building area is presently open, and ground surface elevations vary from approximately El. 73.0 to El. 71.0. We understand that the building will be a pre-engineered metal building with an approximate 22 foot high roof. The ground floor will be at approximately El. 74.0 with truck docks. We understand that the building will be used for storage of wood and other building materials. In addition, storage sheds consisting of metal structures with one side open and concrete floor slabs will be constructed along the north side of the site.

e Explorations

During the period April 7 and 8, 2005, Maine Test Borings, Inc. (MTB) drilled five borings, B1 to B5, at locations shown on Sheet 1, Site and Subsurface Exploration Plan. MTB drilled the borings to depths below ground surface varying from 50.0 feet to 70.0 feet. Sebago Technics monitored the borings and prepared the logs included in Appendix **A**. Table I summarizes the results of borings.

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Borings B1 to B4 were drilled using 2.5-inch inside diameter hollow stem augers to a depth of 32 feet with pushed drill rods to 50 feet below ground surface. Boring B5 was drilled using 2.5-inch inside diameter hollow stem augers to a **depth** of **32** feet with pushed **drill** rods to 70 feet below ground surface. Samples were generally recovered at 5-foot intervals above 32 feet. Standard Penetration Resistance (N) was measured at each sample interval in accordance with **ASTM** Test Designation D1586. The undrained shear strength of the clay was measured by field vane shear tests at various depths in the borings.

Sebago Technics, Inc. determined the locations of borings by taping from existing site features.

The boring logs and related information depict subsurface conditions and water levels only at their specific locations at the time of excavation. Soil conditions at other locations may differ from conditions at these locations. Also, the passage of time may result in a change in groundwater conditions at exploration locations.

Subsurface Conditions

The borings encountered three principal soil units at the site: fill, sand and clay. Encountered thickness and generalized descriptions of these units are presented below in order of increasing depth below ground surface. Due to the complexity of the deposition process, strata thickness will vary.

Fill – Fill consists of loose to medium dense, gray to brown, silty SAND (SM); to well-graded SAND (SW). Encountered thickness varied from 2.5 feet to **4.0** feet.

Sand – The sand consists of loose, brown well-graded SAND (SW). Boring B5 encountered **3.3** feet of sand.

Clay – Clay consists of stiff to soft, gray brown to gray lean **CLAY** (**CL**) with sand lenses and partings. Borings penetrated **up** to *64.2* feet into the clay.

Water **was** observed in the borings at depths below ground surface varying from 2.0 feet to 13.2 feet. Observations of water were made over a relatively short period of time and may not reflect the stabilized groundwater level. In addition, water levels at the site will vary with season, precipitation, temperature and construction activity in the area. Therefore, water levels during and following construction will vary from those measured in the borings.

Recommendations for Foundation Design

Recommended Foundation Type and Design Criteria

The existing fill is not considered suitable for support of the building or floor slab. All fill should be removed from within the foundation limits. In our opinion, the building may be supported on spread and continuous footings bearing on undisturbed, naturally-deposited sand and clay or on compacted structural fill placed after removal of unsuitable soil. The floor slab may be supported on the existing fill following proofrolling, as **described** below, and removal of any unsuitable materials or soft and yielding soils.

For uniformity, footings may be proportioned for an allowable bearing stress in pounds per square foot (psf) equal to 1,000 multiplied by the least lateral dimension of the footing in feet, **up** to 3,000 **psf.** All footings **should be a** minimum of 2.0 feet wide.

Exterior footings should be founded at least **4.5** feet below the lowest adjacent ground surface exposed to freezing. Interior footings should be founded a minimum of 1.5 feet below the ground floor slab.

Compacted structural fill supporting footings should extend laterally from the footings to at least the limits defined by 1 horizontal to 1 vertical lines sloped outward and downward from points located at least 2 feet horizontally beyond the bottom edges of the footings.

In order to consider foundations bearing above the clay stratum, we estimated the settlement of the clay resulting from the increased stress from the raise-in-grade and building loads. We estimated the stress history of the clay stratum by correlating the undrained shear strength with that from other projects in the area. We estimate that the total settlement of the building will be on the order of 1.7 inches, with differential settlement on the order of 1.0 inch in 50 feet. We estimate that approximately **10** to 15 percent of this settlement will occur during the construction period and the remainder will be long-term settlement occurring over 15 to 30 years. We anticipate that settlement of this magnitude is acceptable. However, the structural engineer should determine final acceptability of settlement.

We recommend that the storage sheds be supported on continuous footings bearing on the undisturbed, naturally deposited sand or clay or on compacted structural fill placed after removal of unsuitable soil. Footings should be proportioned for an allowable bearing stress in pounds per square foot (psf) equal to 1,000 multiplied by the least lateral dimension of the footing in feet up to 3,000 psf. All footings should be a minimum of 1.5 feet wide.

Ground Floor Slabs

We recommend that the lowest level floor slab for the building be designed as **an** earth-supported slab-on-grade bearing on a minimum **6** inches of compacted structural fill. All fill containing debris should be removed from within the building limits prior to placing fill. All fill placed below the floor slab for raises-in-grade should consist of compacted structural fill. Normal dampproofing and vapor barriers should be provided below the slab. The existing fill should be proofrolled with a minimum of two passes using fully-loaded ten-wheel dump trucks or approved similar equipment. Any soft or unsuitable areas identified should be excavated and replaced with compacted structural fill.

Because the concrete floor slabs for the storage sheds will be subjected to freezing temperatures, we recommend that the slabs be designed as earth-supported slabs-on-grade bearing on 2 inches of rigid Styrofoam insulation and **6** inches of compacted structural fill. The insulation should be placed on the excavated subgrade and will minimize the potential for freezing of the subgrade below the open sheds.

Seismic Design Considerations

We recommend that the building be designed in accordance with the seismic requirements of the latest edition of the International Building Code, the site classification is Class E; the site response coefficient F_a is 2.1 for a short period spectral response acceleration S_a of 0.37g; the site response coefficient F_v is 3.5 for the I-second period spectral response acceleration S_1 of 0.10g. The subgrade soils are not considered liquefaction susceptible.

Lateral Foundation Loads

We recommend that lateral loads be resisted by bottom friction on footings and that a coefficient of friction equal to 0.35 be used for footings. If this does not provide sufficient lateral resistance, we will consider the problem in more detail to take into account other factors.

Lateral Soil Pressure

We recommend that the foundation walls at the loading docks which are restrained at the top and backfilled to create an unbalanced soil load be designed to resist a lateral earth pressure calculated on the basis of an equivalent fluid unit weight of 55 pounds per cubic foot. This fluid unit weight assumes an at-rest earth pressure coefficient of 0.45 and a free draining backfill. The portion of the foundation wall at the loading docks will be subject to surcharge due to the loads from people, materials and equipment. The wall should be designed for a uniform lateral pressure acting over the full height of wall, calculated on the basis of 0.5 times the surcharge stress (floor load), in addition to the lateral soil pressure recommended above.

Backfill Materials

Structural fill used below foundations and floor slabs and for backfill adjacent to walls should consist of sandy gravel to gravelly sand. It should be free of organic material, loam, trash, snow, ice, frozen soil and other objectionable material, and should conform to the following gradation:

Sieve Size	Percent Finer by Weight
6 in.	100
No. 4	30 to 90
No. 40	10 to 50
No. 200	0 to 8

Compacted structural fill should be placed in layers not exceeding eight inches in loose measure and compacted by self-propelled vibratory equipment at the approximate optimum moisture content to a dry density of at least 95 percent of the maximum dry density, as determined in accordance with **ASTM** Test Designation D1557. In confined areas, the maximum particle size should be reduced to 3 inches and the loose layer thickness should be reduced to 6 inches, and compaction performed by hand-guided vibratory equipment.

Compacted structural fill on the outside of the foundation walls should extend laterally a minimum of 2 feet from the wall. Backfill beyond this limit may consist of common fill. The **top 12** inches of fill on the exterior of the building should consist of low permeability material or bituminous concrete pavement to minimize water infiltration next to the building. Grading should provide for runoff away from the building.

Common fill may consist of inorganic mineral soil that can be placed in layers and compacted. Common fill should be placed and spread in layers not exceeding 12 inches in thickness and compacted with a minimum of two systematic passes of the equipment placing the fill.

Construction Considerations

General

The primary purpose of this section of the report is to comment on items related to excavation, earthwork, and related geotechnical aspects of proposed construction. It is written primarily for the engineer having responsibility for preparation of plans and specifications. Since it identifies potential construction problems related to foundations and earthwork, it will also aid personnel who monitor the construction activity. Contractors for this project must evaluate the construction problems on the basis of their own knowledge and experience in the Portland, Maine area, and on the basis of similar projects in other localities, taking into account their proposed construction methods, procedures, equipment and personnel.

Excavation, Lateral Support and Control of Water

We anticipate that foundation excavation can be accomplished with sloped open excavation through the overburden soils provided safe side slopes can be maintained. Some sloughing and raveling should be anticipated in temporary slopes. Temporary excavations should be made in accordance with all **OSHA** and other applicable regulatory agency requirements.

We anticipate that groundwater may be encountered at proposed subgrade level or bearing level of footings. If encountered, open pumping from sumps can likely control groundwater. In general, the contractor should control groundwater and water from runoff and other sources by methods which prevent disturbance of bearing surfaces or adjacent soils and allow construction in-the-dry.

Subgrade Preparation

The subgrade soil is susceptible to disturbance from construction traffic. Equipment and personnel should not be permitted to travel across exposed footing bearing surfaces or exposed slab subgrades. Any subgrade areas that are disturbed should be recompacted or excavated, and replaced with compacted structural fill prior to placing concrete. Subgrades should be protected against freezing temperatures if exposed during construction. Final excavation to subgrade should be performed using equipment with smooth-edge buckets.

Construction Monitoring

The foundation recommendations contained herein are based on the known and predictable behavior of a properly engineered and constructed foundation. Monitoring of the foundation construction is required to enable the geotechnical engineer to keep in contact with procedures and techniques used in construction. Therefore, we recommend that a person qualified by training and experience be present to provide monitoring at the site during preparation of foundation bearing surfaces and placement of compacted structural fill.

Limitations of Recommendations

This report has been prepared for specific application to the subject project in accordance with generally accepted geotechnical engineering practices. In the event that any changes in the nature, design or location of the building are planned, the conclusions and recommendations contained in this report should not be considered valid, unless the changes are reviewed and the conclusions of this report modified or verified in writing.

The recommendations presented herein are based in part on the data obtained from the referenced test borings. The nature and extent of variations between the explorations may not become evident until construction. If variations then appear evident, it will be necessary to re-evaluate the recommendations of this report.

We request that we be provided the opportunity for a general review of final design and specifications in order to determine that our earthwork and foundation recommendations have been interpreted and implemented in the design and specifications as they were intended.

It has been a pleasure to work with you on this project. Please do not hesitate to contact us if **you** have any questions or need additional information.

Sincerely,

SEBAGO TECHNICS, INC.

Kenneth L. Recker, P.E. Geotechnical Engineering Manager

KLR:klr/jc

Enclosures:

Table I	- Summary of Test Borings
Sheet 1	- Site and Subsurface Exploration Plan
Appendix A	- Logs of Test Borings



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TABLE I SUMMARY OF BORINGS

PROPOSED BUILDING 238 RIVERSIDE STREET PORTLAND, MAINE

Boring	Depth	Depth to	St	rata Thickness (I	Ft)
Number	(Ft)	Water (Ft)	Fill	Sand	Clay
B1	50.0	6.0	3.5		46.5*
B2	50.0	4.5	4.0		46.0*
B3	50.0	13.2	2.7		47.3*
B4	50.0	5.0	3.0		47.0*
B5	70.0	2.0	2.5	3.3	64.2*

NOTES:

- 1. -- INDICATES STRATUM NOT ENCOUNTERED WITHIN DEPTH OF BORING.
- 2. ***** INDICATES DEPTH OF PENETRATION INTO STRATUM.

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Appendix A

Logs of Borings

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TEST BORING REPORT

BORING NO **B2**

Page 1 of 2

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4/8/05	0829			15.0	4.5	-		Cuttings	Number of Samp	les					7S				
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Depth (ff.)	Sampler Blows per 6 in.	Sample No. & Recovery (in.)	Sample Depth (ff.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	% Coarse	% Fine	% Coarse	% Medium	% Fine	% Fines	Dilatancy	Plasticity	
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PROJEC LOCATIC CLIENT CONTRA	T DN ACTOR	PROPOSE 238 RIVER BRADCO MAINE TE	ED BUILDIN RSIDE STR SUPPLY C ST BORING	IG, WICKES EET, PORT ORP. GS, INC.	S LUMBER LAND, MAII	<u>NE</u>		· · · · · · · · · · · · · · · · · · ·	STI JOB NO. PROJECT MGR. FIELD REP. DATE STARTED		044 S. I K. 3 4/8	435 DOE STE	Page PHE1	1501	N
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Depth (ft.)	Sampler Blows per 6 in.	No. & Recovery (in.)	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle size*, structure, odor, moisture, optional descriptions, geologic interpretation)	% Coarse	% Fine	% Coarse	% Medium	% Fine	% Fines	Toughness	Plasticity	Strength
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CONTRA	ACTOR	MAINE	CO SUPPL	Y CORP.							FIELD REP.		<u>к.</u>	STEP	HEN	SON			
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Elevation			ft. Datum	NGV	/D 1920	Borin		0			DATE FINISHED		4/7	/2005					
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lammer F	Fall (in.)			30	<u>n er der</u> Ref 1973	S I ⊡ Sk	acx L úd ľ	Air Track		Roller Bit	Automatic	\Box	Nor	ie	HS	A/Sp	in/50	0.0 ft	t.
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epth (ft.)) Blows per in.	6 No. 6 Recov (in.)	& Samp ery Depth	ole V (ft.) Dia	Vell Igram	Stratum Change (ft.)	USCS Symbol	Visua (density/consistency, structure, odor,	al-Manu , color, C moistun	al Identification & D ROUP NAME & SYMBO , optional descriptions, g	escription L, maximum particle size eologic interpretation)	Coarse	Fine	Coarse	Fine	Fines	latancy	oughness	asticity
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- 30 - Water Level Data													u						
Date Time Depth in feet to: Sample ID Wetr Utagram Date Time (hr.) Bottom of Casino / Casino / Casino / Hole Bottom of Hole O Open End Rod IIII Riser Pipe Date Time (hr.) Bottom of Casino / Casino / Hole Water O Open End Rod IIII Riser Pipe		ear ft.	ft.) ft.)		SL	<u>um</u>)III)	ary		7	70.0)			_				
4/8/2005 1145 - 4.2 2.0 S Split Spoon Sample Image: Split Spoon Sample Split Spoon Sample G Geoprobe G Concrete BORING NO.	ear ft	es							 	B5	7S			_	-				
Field Tests Dilatancy: R - Rapid S - Slow N - None Plasticity: N - Nonplastic L - Low M - Medium H - High Toughness: L - Low M - Medium H - High Dry Strength: N - None L - Low M - Medium H - High V - Very H *NOTE: Maximum Particle Size is determined by direct observation within the limitations of samples size	iear f iear fi iles	<u> - н</u>	Hig ery	gh y H	-ligt	h		_					_						

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Depth (fl.)	Sampler Blows per 6 in.	Sample No. & Recovery {in.}	Sample Depth (ft.)	Well Diagram	Stratum Change (ft.)	USCS Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, maximum particle si structure, odor, moisture, optional descriptions, geologic interpretation)	G e*, Coace	Puis %	% Coarse	% Medium	% Fine	% Fines	Dilatancy	Toughness	Plasticity at
- 30 -	WOR	FV3	30.0-30.6		·····				T							
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