

**City of Portland, Maine - Building or Use Permit Application**

389 Congress Street, 04101 Tel: (207) 874-8703, Fax: (207) 874-8716

Permit No: 05-1564	Issue Date: <b>PERMIT ISSUED</b> OCT 27	CBL: 314 A007001
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Location of Construction: 599 WARREN AVE	Owner Name: KIMCO REALTY LLC	Owner Address: 65 GRAY RD BOX 4	Phone:
Business Name:	Contractor Name: Thompson Building Services Inc	Contractor Address: 497 Northern Ave.	Phone: 2075826100
Lessee/Buyer's Name	Phone:	Permit Type: Foundation Only/Commercial	Zone:

Past Use: Vacant Land	Proposed Use: FOUNDATION ONLY - connected w/ permit #050990	Permit Fee:	Cost of Work: \$0.00	CEO District: 5
		FIRE DEPT: <input type="checkbox"/> Approved <input type="checkbox"/> Denied	INSPECTION: Use Group: <b>FOUNDATION ONLY</b>	

Proposed Project Description: FOUNDATION ONLY - connected w/ permit #050990	Signature:	Signature:
PEDESTRIAN ACTIVITIES DISTRICT (P.A.D.)		
Action: <input type="checkbox"/> Approved <input type="checkbox"/> Approved w/Conditions <input type="checkbox"/> Denied		
Signature:	Date:	

Permit Taken By: ldobson	Date Applied For: 10/26/2005	<b>Zoning Approval</b>		
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<p>1. This permit application does not preclude the Applicant(s) from meeting applicable State and Federal Rules.</p> <p>2. Building permits do not include plumbing, septic or electrical work.</p> <p>3. Building permits are void if work is not started within six (6) months of the date of issuance. False information may invalidate a building permit and stop all work..</p>	<p align="center"><b>Special Zone or Reviews</b></p> <input type="checkbox"/> Shoreland <input type="checkbox"/> Wetland <input type="checkbox"/> Flood Zone <input type="checkbox"/> Subdivision <input type="checkbox"/> Site Plan Maj <input type="checkbox"/> Minor <input type="checkbox"/> MM <input type="checkbox"/>	<p align="center"><b>Zoning Appeal</b></p> <input type="checkbox"/> Variance <input type="checkbox"/> Miscellaneous <input type="checkbox"/> Conditional Use <input type="checkbox"/> Interpretation <input type="checkbox"/> Approved <input type="checkbox"/> Denied	<p align="center"><b>Historic Preservation</b></p> <input type="checkbox"/> Not in District or Landmark <input type="checkbox"/> Does Not Require Review <input type="checkbox"/> Requires Review <input type="checkbox"/> Approved <input type="checkbox"/> Approved w/Conditions <input type="checkbox"/> Denied
	Date: <i>See</i>	Date:	Date:

*See 050990*

**CERTIFICATION**

I hereby certify that I am the owner of record of the named property, or that the proposed work is authorized by the owner of record and that I have been authorized by the owner to make this application as his authorized agent and I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in the application is issued, I certify that the code official's authorized representative shall have the authority to enter all areas covered by such permit at any reasonable hour to enforce the provision of the code(s) applicable to such permit.

SIGNATURE OF APPLICANT	ADDRESS	DATE	PHONE
RESPONSIBLE PERSON IN CHARGE OF WORK, TITLE		DATE	PHONE

DISPLAY THIS CARD ON PRINCIPAL FRONTAGE OF WORK

# CITY OF PORTLAND

DEPARTMENT OF BUILDING INSPECTION

## PERMIT

PERMIT ISSUED

Permit Number: 051564

OCT 27 2005

CITY OF PORTLAND

Please Read Application And Notes, If Any, Attached

This is to certify that KIMCO REALTY LLC / The son Building Services Inc

has permission to FOUNDATION ONLY - connected w/ mit #051564

AT 599 WARREN AVE

314 A007001

provided that the person or persons, firm or corporation accepting this permit shall comply with all of the provisions of the Statutes of the State and of the Ordinances of the City of Portland regulating the construction, maintenance and use of buildings and structures, and of the application on file in this department.

Apply to Public Works for street line and grade if nature of work requires such information.

Notification of inspection must be given and when permission is procured before this building or part thereof is started or closed-in. 24 HOUR NOTICE IS REQUIRED.

A certificate of occupancy must be procured by owner before this building or part thereof is occupied.

### OTHER REQUIRED APPROVALS

Fire Dept. \_\_\_\_\_

Health Dept. \_\_\_\_\_

Appeal Board \_\_\_\_\_

Other \_\_\_\_\_

Department Name

*[Signature]*  
Director - Building & Inspection Services

**PENALTY FOR REMOVING THIS CARD**

**City of Portland, Maine - Building or Use Permit**

389 Congress Street, 04101 Tel: (207) 874-8703, Fax: (207) 874-8716

<b>Permit No:</b> 05-1564	<b>Date Applied For:</b> 10/26/2005	<b>CBL:</b> 314 A007001
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<b>Location of Construction:</b> 599 WARREN AVE	<b>Owner Name:</b> KIMCO REALTY LLC	<b>Owner Address:</b> 65 GRAY RD BOX 4	<b>Phone:</b>
<b>Business Name:</b>	<b>Contractor Name:</b> Thompson Building Services Inc	<b>Contractor Address:</b> 497 Northern Ave. Farming Dale	<b>Phone:</b> (207) 582-6100
<b>Lessee/Buyer's Name</b>	<b>Phone:</b>	<b>Permit Type:</b> Foundation Only/Commercial	

<b>Proposed Use:</b> FOUNDATION ONLY - connected w/ permit #050990	<b>Proposed Project Description:</b> FOUNDATION ONLY - connected w/ permit #050990
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<b>Dept:</b> Building	<b>Status:</b> Approved	<b>Reviewer:</b> Mike Nugent	<b>Approval Date:</b> 10/26/2005
<b>Note:</b>	<b>Ok to Issue:</b> <input checked="" type="checkbox"/>		

<b>Dept:</b> Fire	<b>Status:</b> Approved	<b>Reviewer:</b> Lt. MacDougal	<b>Approval Date:</b> 06/16/2004
<b>Note:</b>	<b>Ok to Issue:</b> <input checked="" type="checkbox"/>		

<b>Dept:</b> DRC	<b>Status:</b> Approved with Conditions	<b>Reviewer:</b> Chris Earle/Steve Bush	<b>Approval Date:</b> 03/22/2005
<b>Note:</b>	<b>Ok to Issue:</b> <input checked="" type="checkbox"/>		
1) see planning conditions			

<b>Dept:</b> Planning	<b>Status:</b> Approved with Conditions	<b>Reviewer:</b> Kandi Talbot	<b>Approval Date:</b> 03/22/2005
<b>Note:</b>	<b>Ok to Issue:</b> <input checked="" type="checkbox"/>		
1) vi. No building permit shall be issued until the applicant receives the required wetlands permit from DEP. 2) vii. That the Traffic Engineer review and approve the plans based on his March 14, 2005 memo and that the applicant contribute \$30,000 prior to issuance of a building permit as stated in condition 1. 3) v. That the site plans be revised to reflect the crosswalks and caution sign, subject to the review and approval of the Traffic Engineer. 4) iv. That a revised site plan for the Wendy's property shall be submitted to staff for review and approval prior to issuance of a building permit. 5) ii. That the City Arborist review and approve the landscaping plan prior to issuance of a building permit. 6) i. That the applicant revise the plans based on the Traffic Engineer's memo dated March 14, 2005 and that the applicant contribute \$30,000 prior to issuance of a building permit to the improvements at the Riverside Street/Warren Avenue intersection. If the proposed MDOT/City improvements do not occur within 5 years from the contribution date, then the money shall be returned to the applicant. 7) iii. That a photometric plan be submitted for review and approval by staff, prior to issuance of a building permit. 8) viii. That the Traffic Engineer review the safety of the intersection regarding the fatal accident and any other records that the City may have on accidents at this location. Based on this analysis, the Traffic Engineer shall determine if the intersection is safe at this time.			

<b>Comments:</b> 10/26/2005-mjn: Fire and Zoning Signed off on permit # 050990
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# All Purpose Building Permit Application

If you or the property owner owes real estate or personal property taxes or user charges on any property within the City, payment arrangements must be made before permits of any kind are accepted.

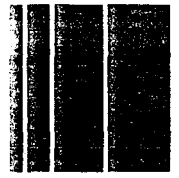
Location/Address of Construction: <u>599 WILSON AVE -</u>		
Total Square Footage of Proposed Structure <u>1870 sq ft</u>	Square Footage of Lot <u>58,465</u>	
Tax Assessor's Chart, Block & Lot Chart# <u>314</u> Block# <u>A</u> Lot# <u>7</u>	Owner: <u>KIMCO REALTY LLC</u>	Telephone: <u>797-7600</u>
Lessee/Buyer's Name (if Applicable)	Applicant name, address & telephone: <u>KIMCO REALTY LLC 65 GRAY RD UNIT 4 W. FALMOUTH, ME</u>	Cost Of Work: \$ <u>525,000</u> Fee: \$
Current use: <u>VACANT LOT</u>		
If the location is currently vacant, what was prior use: <u>VACANT</u>		
Approximately how long has it been vacant: <u>N/A</u>		
Proposed use: <u>NEW DUNKIN DONUTS</u>		
Project description:		
Contractor's name, address & telephone: <u>THOMPSON BUILDING SERVICES INC 497 NORTHERN AVE</u>		
Who should we contact when the permit is ready: <u>FARMING DALE, ME</u>		
Mailing address: <u>SAME - 582-6100</u>		
We will contact you by phone when the permit is ready. You must come in and pick up the permit and review the requirements before starting any work, with a Plan Reviewer. A stop work order will be issued and a \$100.00 fee if any work starts before the permit is picked up. PHONE: <del>582-8760</del> <u>215-6995</u>		

**IF THE REQUIRED INFORMATION IS NOT INCLUDED IN THE SUBMISSIONS THE PERMIT WILL BE AUTOMATICALLY DENIED AT THE DISCRETION OF THE BUILDING/PLANNING DEPARTMENT, WE MAY REQUIRE ADDITIONAL INFORMATION IN ORDER TO APPROVE THIS PERMIT.**

*I hereby certify that I am the Owner of record of the named property, or that the owner of record authorizes the proposed work and that I have been authorized by the owner to make this application as his/her authorized agent. I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in this application is issued, I certify that the Code Official's authorized representative shall have the authority to enter all areas covered by this permit at any reasonable hour to enforce the provisions of the codes applicable to this permit.*

Signature of applicant: <u>J. Bruce E. Thompson</u>	Date: <u>7-20-05</u>
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**This is NOT a permit, you may not commence ANY work until the permit is issued.  
If you are in a Historic District you may be subject to additional permitting and fees with the Planning Department on the 4<sup>th</sup> floor of City Hall.**



# **Report on Subsurface and Foundation Investigation**

**Proposed Dunkin Donuts  
Warren Avenue  
Portland, Maine**

for

Kimco Development Corp.  
Dunkin Donut Plaza  
65 Gray Road  
Falmouth, ME 04105

October 11, 2005

October 11, 2005  
03461

Mr. Ed Wolak  
Kimco Development Corp.  
Dunkin Donut Plaza  
65 Gray Road  
Falmouth, ME 04105

**Report on Subsurface and Foundation Investigation**  
**Proposed Dunkin Donuts, Warren Avenue, Portland, Maine**

Dear Mr. Wolak:

This report presents the results of our subsurface and foundation investigation for the proposed Dunkin Donuts Store on Warren Avenue in Portland, Maine.

In summary, it is our opinion that the store may be supported on footings bearing on the naturally deposited, inorganic soil, or on compacted structural fill placed after removal of unsuitable soil. In addition, a slab-on-grade may be used for the ground floor. Specific recommendations regarding subsurface conditions and foundation requirements are presented below.

**Introduction**

The site is located on Warren Avenue, adjacent to the east side of the Wendy's Restaurant site. The site is presently open and covered in tall grass and weeds. Ground surface elevations within the limits of the proposed building are on the order of El. 56 to El. 57. We understand that the store will be one story with ground floor at El. 57.1.

**Subsurface Explorations**

On October 10, 2005, C. H. Stevenson (CHS) of Wayne, Maine excavated four test pits, TP1 to TP4, at locations shown on Sheet 1, Subsurface Exploration Plan. CHS excavated the test pits to depths below ground surface varying from 6.0 feet to 7.0 feet. Sebago Technics, Inc. monitored the test pits and prepared the logs included in Appendix A. CHS backfilled the test pits with the excavated material.

Test pit locations were determined by Sebago Technics, Inc. by pacing from existing site features. Ground surface elevations at test pits were determined by linear interpolation between ground surface contours at the plotted locations.

The test pit logs and related information depict the subsurface conditions and water levels encountered at the locations and during the times indicated on the logs. Subsurface conditions at other locations may differ from those encountered in the test pits. The passage of time may result in a change in groundwater conditions at the explorations.

### **Subsurface Conditions**

The test pits encountered four principal soil units at the site: recent topsoil, fill, original topsoil and marine deposits. Encountered thickness and generalized descriptions are presented below in order of increasing depth below ground surface.

**Recent Topsoil** – Recent topsoil consists of gray brown SILT (ML) with roots. Encountered thickness varied from 0.4 foot to 0.5 foot.

**Fill** – Fill consists of gray brown mottled SILT (ML); to lean CLAY (CL) with various amounts of sand, gravel and cobbles and trace roots. Encountered thickness varied from 3.3 feet to 5.1 feet.

**Original Topsoil** – The original topsoil encountered below the fill consists of dark gray sandy SILT (ML) with occasional roots. Encountered thickness varied from 0.3 foot to 1.0 foot

**Marine Deposit** – The marine deposit consists of gray lean CLAY (CL). Undrained shear strength in the top of the deposit, as measured by Shear Vane tests, varied from 1,300 pounds per square foot (psf) to 2,500 psf. Test pits penetrated up to 2.3 feet into the marine deposit.

Water was observed seeping slowly into the test pits at depths below ground surface varying from 4.0 feet to 4.5 feet. However, observations of water were made over a relatively short period of time and may not reflect the stabilized groundwater level. In addition, water levels at the site will vary with season, precipitation, temperature and construction activity in the area. Therefore, water levels during and following construction will vary from those observed in the test pits.

### **Recommendations for Foundation Design**

#### **Recommended Foundation Type and Design Criteria**

The topsoil and existing fill are not considered suitable for support of the building. All topsoil and fill should be excavated from within the limits of foundations. We recommend that the building be supported on spread and continuous footings bearing on undisturbed, naturally deposited soil or on compacted structural fill placed after removal of unsuitable soil.

For uniformity, footings may be proportioned for an allowable bearing stress equal to 1,000 pounds per square foot (psf) multiplied by the least lateral dimension of the footing in feet, up to a maximum of 3,000 psf. All footings should be at least 1.5 feet wide.

Exterior footings should be founded at least 4.5 feet below the lowest adjacent ground surface exposed to freezing. Interior footings should be founded a minimum of 1.5 feet below the ground floor slab.

Compacted structural fill supporting footings should extend laterally from the footings to at least the limits defined by 1 horizontal to 1 vertical lines sloped outward and downward from points located at least 2 feet horizontally beyond the bottom edges of the footings.

Subsurface information in the vicinity indicates that the clay extends to depths of 40 feet or more. Correlations with shear strength indicate that the clay is overconsolidated, that is the previous stress is greater than the existing overburden stress. At the recommended bearing stress, we anticipate that settlement will be less than 1 inch. We anticipate that settlement of this magnitude is acceptable. However, final acceptability of settlement should be determined by the structural engineer.

#### Ground Floor Slab

We recommend that the lowest level floor slab be designed as an earth-supported slab-on-grade bearing on a minimum 6-inch thickness of compacted structural fill. All fill containing debris and wood and organics should be removed from within the building limits prior to placing structural fill. The existing fill should be proofrolled with fully-loaded, ten-wheel dump trucks, or equivalent. Any soft or unsuitable area disclosed should be excavated and replaced with compacted structural fill.

All fill placed below the floor slabs for raises-in-grade should consist of compacted structural fill. Normal dampproofing and vapor barriers should be provided below the slab.

#### Seismic Design Considerations

We recommend that the building be designed in accordance with the seismic requirements of the latest edition of the International Building Code; the site classification is Class E; the site response coefficient  $F_a$  is 2.1 for a short period spectral response acceleration  $S_s$  of 0.375g; the site response coefficient  $F_v$  is 3.5 for the one-second period spectral response acceleration  $S_1$  of 0.10g. The subgrade soils are not considered liquefaction susceptible.

#### Lateral Foundation Loads

We recommend that lateral loads be resisted by bottom friction on footings. We recommend that a coefficient of friction equal to 0.35 be used for footings bearing on soil. If this does not provide sufficient resistance, we will study the problem in more detail to take into account other factors.

#### Backfill Materials

Structural fill used below foundations and floor slabs and for backfill adjacent to walls should consist of sandy gravel to gravelly sand. It should be free of organic material, loam, trash, snow, ice, frozen soil and other objectionable material, and should conform to the following gradation:



<u>Sieve Size</u>	<u>Percent Finer by Weight</u>
6 inches	100
No. 4	30 to 90
No. 40	10 to 50
No. 200	0 to 8

Compacted structural fill should be placed in layers not exceeding eight inches in loose measure and compacted by self-propelled vibratory equipment at the approximate optimum moisture content to a dry density of at least 95 percent of the maximum dry density, as determined in accordance with ASTM Test Designation D1557. In confined areas, the maximum particle size should be reduced to 3 inches and the loose layer thickness should be reduced to 6 inches, and compaction performed by hand-guided vibratory equipment.

Compacted structural fill on the outside of the foundation walls should extend laterally a minimum of 2 feet from the wall. Backfill beyond this limit may consist of common fill. The surface of fill on the exterior of the building should consist of low permeability material or bituminous pavement to minimize water infiltration next to the building. Grading should provide for runoff away from the building.

Common fill may consist of inorganic mineral soil that can be placed in layers and compacted. Common fill should be placed and spread in layers not exceeding 12 inches in thickness and compacted with a minimum of two systematic passes of the equipment placing the fill.

### Construction Considerations

#### General

The primary purpose of this section of the report is to comment on items related to excavation, earthwork, and related geotechnical aspects of proposed construction. It is written primarily for the engineer having responsibility for preparation of plans and specifications. Since it identifies potential construction problems related to foundations and earthwork, it will also aid personnel who monitor the construction activity.

#### Excavation, Lateral Support and Control of Groundwater

We anticipate that foundation excavation can be accomplished with sloped open excavation through the overburden soils provided safe side slopes can be maintained. Some sloughing and raveling should be anticipated in temporary slopes. Temporary excavations should be made in accordance with all OSHA and other applicable regulatory agency requirements.

We anticipate that groundwater may be encountered at proposed subgrade level or bearing level of footings. If encountered, open pumping from sumps can likely control groundwater. In general, the contractor should control groundwater and water from runoff and other sources by methods which prevent disturbance of bearing surfaces or adjacent soils and allow construction in-the-dry.

Subgrade Preparation

The subgrade soil is susceptible to disturbance from construction traffic. Equipment and personnel should not be permitted to travel across exposed footing bearing surfaces or exposed slab subgrades. Any subgrade areas that are disturbed should be recompacted or excavated and replaced with compacted structural fill prior to placing of concrete. Subgrades should be protected against freezing temperatures if exposed during construction. Final excavation to subgrade should be performed using equipment with smooth-edge buckets.

Construction Monitoring

The foundation recommendations contained herein are based on the known and predictable behavior of a properly engineered and constructed foundation. Monitoring of the foundation construction is required to enable the geotechnical engineer to keep in contact with procedures and techniques used in construction. Therefore, we recommend that a person qualified by training and experience be present to provide monitoring at the site during excavation of bearing surfaces and placement of compacted structural fill.

Limitations of Recommendations

This report has been prepared for specific application to the subject project in accordance with generally accepted geotechnical engineering practices. In the event that any changes in the nature, design or location of the building are planned, the conclusions and recommendations contained in this report should not be considered valid, unless the changes are reviewed and the conclusions of this report modified or verified in writing.


The recommendations presented herein are based in part on the data obtained from the referenced test pits. The nature and extent of variations between the explorations may not become evident until construction. If variations then appear evident, it will be necessary to re-evaluate the recommendations of this report.

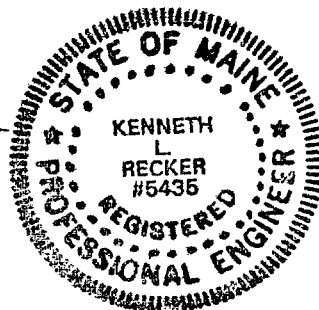
We request that we be provided the opportunity for a general review of final design and specifications in order to determine that our earthwork and foundation recommendations have been interpreted and implemented in the design and specifications as they were intended.

It has been a pleasure to work with you on this project. Please do not hesitate to contact us if you have any questions or need additional information.

Sincerely,

SEBAGO TECHNICS, INC.

  
Kenneth L. Recker, P.E.  
Geotechnical Engineering Manager



KLR:klr/jc

Enc.

- Sheet 1 - Subsurface Exploration Plan
- Appendix A - Logs of Test Pits

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# Appendix A

Logs of Test Pits

SEBAGO  
TECHNICS,  
INC.

# TEST PIT LOG

Test Pit No.

TP1

Page 1 of 1

<b>PROJECT</b>	DUNKIN DONUTS	<b>PROJECT NO.</b>	03461
<b>LOCATION</b>	WARREN AVENUE, PORTLAND, MAINE	<b>PROJECT MGR.</b>	J. PERRY
<b>CLIENT</b>	KIMCO DEVELOPMENT CORP.	<b>FIELD REP</b>	K. RECKER
<b>CONTRACTOR</b>	C.H. STEVENSON	<b>DATE</b>	10/10/2005
<b>EQUIPMENT</b>	DEERE 160C LC	<b>WEATHER</b>	Cloudy, 50s

Ground El. 56.0 ft Location See Plan Groundwater depths/entry rates (in/min):  
El. Datum \_\_\_\_\_ Not Encountered

Depth (ft)	Sample ID	Stratum Change Depth (ft)	USCS Group Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, % oversized, max particle size, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel		Sand			Field Test			
					% Coarse	% Fine	% Coarse	Medium	% Fine	% Fines	Dilatancy	Toughness	Plasticity
		0.4	ML	Gray brown SILT (ML), mps = 0.02 in., roots, damp -TOPSOIL-				5	95	S	N	N	
1			ML	Gray brown, mottled SILT (ML), mps = 0.02 in., rooys, ocassional gravel and cobble, damp				10	90	S	N	N	
2				-FILL-									
3													
4													
5		5.5											
6			CL	Gray lean CLAY (CL), damp -MARINE DEPOSIT-				10	90	N	M	M	
7													

Obstructions:

Remarks:  
Bottom of Exploration at 7.0 ft. below ground surface. No refusal

Standing water in completed pit: at depth _____ ft. measured after _____ hrs. elapsed	Boulders:			Test Pit Dimensions (ft): Pit Depth _____ 7.0 Pit Length X Width _____ 9.0 x 3.0
	Diameter (in.)	Number	Approx. vol. (cu. ft.)	
	12 to 24	_____	_____	
	over 24	_____	_____	



PROJECT: DUNKIN DONUTS PROJECT NO.: 03461  
 LOCATION: WARREN AVENUE, PORTLAND, MAINE PROJECT MGR.: J. PERRY  
 CLIENT: KIMCO DEVELOPMENT CORP. FIELD REP: K. RECKER  
 CONTRACTOR: C.H. STEVENSON DATE: 10/10/2005  
 EQUIPMENT: DEERE 160C LC WEATHER: Cloudy, 50s

Ground El. 57.0 ft Location See Plan Groundwater depths/entry rates (in/min):  
 El. Datum Slow seep at 4.5 ft.

Depth (ft)	Sample ID	Stratum Change Depth (ft)	USCS Group Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, % oversized, max particle size, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel		Sand			Field Test				
					% Coarse	% Fine	% Coarse	% Medium	% Fine	% Fines	Dilatancy	Toughness	Plasticity	Strength
		0.5	ML	Gray brown SILT (ML), mps = 0.02 in., roots, damp -TOPSOIL-					10	90	S	N	N	
1			CL	Gray brown lean CLAY (CL), roots and pockets of orange brown sand, damp					10	90	S	N	N	
2				-FILL-										
3														
4														
5		5.4												
			ML	Dark gray SILT (ML) with roots, damp					10	90	S	N	N	
		5.7		-ORIGINAL TOPSOIL-										
6			CL	Gray lean CLAY (CL), damp -MARINE DEPOSIT- At 6.0 ft., undrained shear strength by Shear Vane = 1,300 pounds per square foot					100		N	M	M	
				Bottom of Exploration at 6.0 ft. below ground surface No refusal										
7														

Obstructions: \_\_\_\_\_ Remarks: \_\_\_\_\_  
 \_\_\_\_\_  
 \_\_\_\_\_

Standing water in completed pit:  
 at depth 5.9 ft.  
 measured after 0.25 hrs. elapsed

Boulders:  
 Diameter (in.) Number = Approx. vol. (cu. ft.)  
 12 to 24 \_\_\_\_\_ = \_\_\_\_\_  
 over 24 \_\_\_\_\_ = \_\_\_\_\_

Test Pit Dimensions (ft):  
 Pit Depth 6.0  
 Pit Length X Width 8.0 x 3.0

SEEAG-O  
TECHNICS,  
INC.

# TEST PIT LOG

Test Pit No.  
**TP4**  
Page 1 of 1

<b>PROJECT</b>	DUNKIN DONUTS	<b>PROJECT NO.</b>	03461
<b>LOCATION</b>	WARREN AVENUE, PORTLAND, MAINE	<b>PROJECT MGR.</b>	J. PERRY
<b>CLIENT</b>	KIMCO DEVELOPMENT CORP.	<b>FIELD REP</b>	K. RECKER
<b>CONTRACTOR</b>	C.H. STEVENSON	<b>DATE</b>	10/10/2005
<b>EQUIPMENT</b>	DEERE 160C LC	<b>WEATHER</b>	Cloudy, 50s

Ground El. 57.0 ft Location See Plan  
El. Datum \_\_\_\_\_  
Groundwater depths/entry rates (in/min):  
Not Encountered

Depth (ft)	Sample ID	Stratum Change Depth (ft)	USCS Group Symbol	Visual-Manual Identification & Description (density/consistency, color, GROUP NAME & SYMBOL, % oversized, max particle size, structure, odor, moisture, optional descriptions, geologic interpretation)	Gravel		Sand			Field Test				
					% Coarse	% Fine	% Coarse	Medium	% Fine	% Fines	Dilatancy	Toughness	Plasticity	Strength
0.4			ML	Gray brown SILT (ML), mps = 0.02 in., roots, damp -TOPSOIL-					10	90	S	N	N	
			ML	Gray brown mottled SILT (ML), mps = 0.02 in., roots and pockets of sand, damp					10	90	S	N	N	
				-FILL-										
				Pocket of orange brown sand, wet										
3.8														
			ML	Dark gray sandy SILT (ML), mps = 0.02 in., roots, wet -ORIGINAL TOPSOIL-					30	70	S	N	N	
4.8														
			CL	Gray lean CLAY (CL), damp -MARINE DEPOSIT-					5	95	N	M	M	
				At 6.5 ft., undrained shear strength by Shear Vane = 2,500 pounds per square foot										
				Bottom of Exploration at 6.5 ft. below ground surface										
				No resusal										

Obstructions: \_\_\_\_\_  
Remarks: \_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_

Standing water in completed pit: at depth _____ ft. measured after _____ hrs. elapsed	Boulders:			Test Pit Dimensions (ft): Pit Depth 6.5 Pit Length X Width 8.0 x 3.0
	Diameter (in.)	Number	Approx. vol. (cu. ft.)	
	12 to 24	_____	= _____	
	over 24	_____	= _____	