THE FOLLOWING BUILDING CODES AND STANDARDS SHALL BE REFERENCED DURING CONSTRUCTION:

AMERICAN CONCRETE INSTITUTE SPECIFICATION FOR STRUCTURAL CONCRETE AMERICAN CONCRETE INSTITUTE SPECIFICATION FOR STRUCTURAL CONCRETE AMERICAN CONCRETE INSTITUTE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE ACI 318 AMERICAN SOCIETY OF TESTING AND MATERIALS

NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION BY NATIONAL FOREST PRODUCTS ASSOCIATION, 2001.

REFERENCE ARCHITECTURAL PLANS FOR DIMENSIONS NOT SHOWN. REFERENCE MECHANICAL, ELECTRICAL, AND ARCHITECTURAL PLANS FOR SIZES AND LOCATIONS OF WALL AND SLAB OPENINGS, DUCTS, PIPING, CURBS, AND EQUIPMENT PADS. IN THE EVENT OF A CONFLICT BETWEEN THE DRAWINGS, SPECIFICATIONS, OR NOTES ON THE DRAWINGS, THE ENGINEER SHALL BE NOTIFIED PRIOR TO CONSTRUCTION.

EXISTING DIMENSIONS AND CONDITIONS ARE FOR REFERENCE ONLY. CONTRACTOR SHALL VERIFY ALL EXISTING CONSTRUCTION AND DIMENSIONS IN THE FIELD PRIOR TO CONSTRUCTION OR FABRICATION. ALL DISCREPANCIES SHALL BE REPORTED TO THE ENGINEER PRIOR TO COMMENCING WORK.

THE CONTRACTOR SHALL NOTIFY THE ENGINEER IF DEVIATIONS OR CHANGES ARE REQUIRED TO THE CONTRACT DOCUMENTS OR APPROVED SHOP DRAWINGS DUE TO INTERFERENCES, FABRICATION ERRORS, OR OTHER CAUSES.

THE STRUCTURE IS SELF-SUPPORTING AND STABLE AFTER THE ENTIRE BUILDING IS COMPLETELY CONSTRUCTED. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR ERECTION PROCEDURES AND SEQUENCING DURING CONSTRUCTION AND ERECTION TO PROVIDE AND ENSURE LOCAL AND OVERALL STABILITY OF THE BUILDING AND ITS COMPONENTS DURING CONSTRUCTION AND ERECTION. THE CONTRACTOR SHALL RETAIN A LICENSED STRUCTURAL ENGINEER TO DESIGN TEMPORARY BRACING/SHORING AND DETERMINE WHERE THE TEMPORARY BRACING/SHORING

GENERAL NOTES

SCALE: NTS

FIRST FLOOR SLAB = 100 PSF MEZZANINE LIGHT STORAGE = 125 PSF GROUND SNOW LOAD, Pg = 50 PSF SNOW EXPOSURE FACTOR, Ce = 1.0SNOW LOAD IMPORTANCE FACTOR, I = 1.0

FLAT ROOF SNOW LOAD, Pf = 35 PSF + DRIFT MAIN WINDFORCE-RESISTING SYSTEM: BASIC WIND SPEED = 100 MPH

EXPOSURE B WIND LOADS (INCLUDES WINDWARD + LEEWARD) = 19 PSF

SOIL SITE CLASSIFICATION = DDESIGN SPECTRAL RESPONSE ACCELERATION: Sds = .324Sd1 = .123SEISMIC USE GROUP I SEISMIC DESIGN CATEGORY B RESPONSE MODIFICATION COEFFICIENT.....R = 2OCCUPANCY IMPORTANCE FACTOR...... = 1.0 BASE SHEAR.... $..V_E = Cs * W = 0.162 * W$ (W = SEISMIC WEIGHT)

DESIGN CRITERIA

SCALE: NTS

ALL CONCRETE WORK, INCLUDING MATERIAL SELECTION, ADMIXTURES, MIXING, AND PLACEMENT OF CONCRETE SHALL BE IN CONFORMANCE WITH APPLICABLE BUILDING CODES. IN ADDITION, REFERENCE THE FOLLOWING CONCRETE STANDARDS AND SPECIFICATIONS:

ACI 318 AMERICAN CONCRETE INSTITUTE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE ACI 301 AMERICAN CONCRETE INSTITUTE SPECIFICATIONS FOR STRUCTURAL CONCRETE

ACI 305 STANDARD SPECIFICATION FOR HOT WEATHER CONCRETING

ACI 306 STANDARD SPECIFICATION FOR COLD WEATHER CONCRETING ACI 308 STANDARD PRACTICE FOR CURING CONCRETE

REQUIRED CONCRETE PARAMETERS ARE AS FOLLOWS:

LOCATION	MAX W/C RATIO	f'c	AIR-ENTRAINMENT
INT. CONC./WALLS/SLABS	.52	3,000 PSI	2% ± 1½%
FOUNDATIONS, FOOTINGS, & FOUNDATION WALLS	.52	3,000 PSI	5–7%
INT. SLAB-ON-GRADE	.47	4,000 PSI	NONE
EXT. SLAB-ON-GRADE	.45	4,000 PSI	6% ± 1½%

WHERE: W/C = WATER TO CEMENT RATIO AND f'c = COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS

MAXIMUM AGGREGATE SIZE SHALL BE 34", IN CONFORMANCE WITH ASTM C33. USE PORTLAND CEMENT TYPE II. IN CONFORMANCE WITH ASTM 150. AIR ENTRAINING ADMIXTURES SHALL CONFORM TO ASTM C 260. ADMIXTURES SHALL CONFORM TO "SPECIFICATION FOR CHEMICAL ADMIXTURES FOR CONCRETE" ASTM C 494. FLY ASH USED AS ADMIXTURES SHALL CONFORM TO ASTM C 618.

MAXIMUM SLUMP AFTER THE ADDITION OF A WATER-REDUCING ADMIXTURE IS 8 INCHES.

CALCIUM CHLORIDE OR ADMIXTURES CONTAINING CALCIUM CHLORIDE IS NOT PERMITTED.

CONCRETE EXPOSED TO FREEZING AND THAWING, INCLUDING FOUNDATIONS, FOOTINGS, FOUNDATION WALLS, AND EXTERIOR WALKWAYS SHALL BE AIR ENTRAINED WITH AIR CONTENT BETWEEN 5% AND 6%. CONTRACTOR SHALL NOT PLACE CONCRETE ON FROZEN GROUND OR IN WATER. ADEQUATE EQUIPMENT SHALL BE PROVIDED FOR HEATING CONCRETE MATERIALS AND PROTECTING CONCRETE DURING NEAR-FREEZING OR FREEZING WEATHER. REFERENCE ACI 306, AS NOTED ABOVE, FOR RECOMMENDATIONS FOR COLD WEATHER CONCRETING.

CONTRACTOR SHALL SUBMIT PROPOSED CONCRETE MIX DESIGN AND LABORATORY TESTS OF FABRICATED CYLINDERS VERIFYING CONCRETE STRENGTH OR PERFORMANCE HISTORY OF MIX TO ENGINEER FOR ACCEPTANCE PRIOR TO PLACEMENT OF CONCRETE. CONCRETE USED ON SITE SHALL BE FIELD TESTED IN ACCORDANCE WITH AND IN THE PRESENCE OF AN APPROVED TESTING AGENCY. FIELD TESTING INFORMATION SHALL INDICATE SLUMP, AIR CONTENT, AND TEMPERATURE. COMPRESSION TEST 1 CYLINDER AT 7 DAYS AND 2 AT 28 DAYS. HOLD AN ADDITIONAL CYLINDER FOR A 56 DAY BREAK, IF NECESSARY. PROVIDE A SET OF 4 CYLINDERS FOR EACH PLACEMENT AND PER 50 CUBIC YARDS OF CONCRETE PLACED. THE OWNER SHALL PAY FOR ALL CONCRETE TESTING.

CONSTRUCTION JOINTS IN WALLS SHALL BE PERMITTED AS DETAILED ON THE STRUCTURAL DRAWINGS. SURFACES OF CONCRETE CONSTRUCTION JOINTS SHALL BE CLEANED AND LAITANCE REMOVED. IMMEDIATELY BEFORE NEW CONCRETE IS PLACED, CONSTRUCTION JOINTS SHALL BE WETTED AND STANDING WATER

WHERE ELECTRICAL CONDUIT/ RADIANT HEATING TUBES RUN IN THE SLAB, THEY SHALL BE LOCATED AT MID-DEPTH OF THE SLAB. ALUMINUM CONDUIT AND SLEEVES ARE NOT PERMITTED.

CONCRETE NOTES

ANCHOR BOLTS SHALL CONFORM TO ASTM A307. ANCHOR BOLTS SHALL HAVE HEAVY HEX NUTS AND LOCK WASHERS.

REMOVED. VERTICAL CONSTRUCTION JOINTS IN WALLS SHALL NOT EXCEED A SPACING OF 40 FEET.

USE DEFORMED BILLET-STEEL REINFORCING BARS, GRADE 60, IN CONFORMANCE WITH ASTM A615. REINFORCEMENT SHALL BE ACCURATELY PLACED AND SUPPORTED PRIOR TO CONCRETE PLACEMENT, AND SHALL BE SECURED AGAINST DISPLACEMENT.

THE CONTRACTOR SHALL SUBMIT REINFORCING SHOP DRAWINGS TO THE ENGINEER FOR REVIEW AND ACCEPTANCE PRIOR TO COMMENCING FABRICATION. REINFORCEMENT SHALL BE DETAILED IN ACCORDANCE WITH ACI "MANUAL OF STANDARD PRACTICE FOR DETAILING OF REINFORCED CONCRETE STRUCTURES". SHOP DRAWINGS SHALL SHOW REINFORCING STEEL PLACEMENT DETAILS AND SECTIONS.

MINIMUM CONCRETE COVER FOR REINFORCEMENT	
CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH	3 INCHES
CONCRETE EXPOSED TO EARTH OR WEATHER	2 INCHES
CONCRETE NOT EXPOSED TO EARTH OR WEATHER IN SLABS AND WALLS (FOR PRIMARY REINFORCEMENT, TIES, AND STIRRUPS)	1⅓ INCHES
CONCRETE NOT EXPOSED TO EARTH OF WEATHER IN COLUMNS AND BEAMS	1½ INCHES

CONTINUOUS REINFORCEMENT SHALL BE TENSION LAP SPLICED PER LAP SPLICE LENGTH TABLE, U.N.O.

LAP SPLICE LENGTH TABLE							
BAR SIZE	#3	#4	# 5	#6	# 7	#8	#9
MIN LAP SPLICE (INCHES)	18	24	30	36	48	64	81

REINFORCEMENT HOOKS SHALL CONFORM TO STANDARD HOOKS ACCORDING TO ACI 318. WELDING OF REINFORCEMENT IS NOT PERMITTED, U.N.O.

CONCRETE REINFORCING NOTES

SCALE: NTS

SUBGRADE PREPARATION AND DETERMINATION (INCLUDING ALLOWABLE BEARING PRESSURE, STRUCTURAL FILL GRADATION REQUIREMENTS, COMPACTION REQUIREMENTS AND POST—CONSTRUCTION SETTLEMENT ANALYSIS) BENEATH FOOTINGS AND SLABS-ON-GRADE AND BEHIND FOUNDATION WALLS SHALL BE PROVIDED BY A GEOTECHNICAL ENGINEER. ALL FILL USED TO SUPPORT FOUNDATIONS AND SLABS-ON-GRADE SHALL CONSIST OF A WELL-GRADED, GRANULAR MATERIAL PER THE RECOMMENDATIONS OF THE GEOTECHNICAL ENGINEER. STRUCTURAL SLABS SHALL BE CONSTRUCTED ON A MINIMUM 12" THICK LAYER OF STRUCTURAL FILL SOIL WITH PROPERTIES PER THE GEOTECHNICAL ENGINEER.

PRESUMED ALLOWABLE SOIL BEARING PRESSURE USED IN DESIGN = 2,000 PSF. BEARING CAPACITIES SHALL BE VERIFIED BY GEOTECHNICAL ENGINEER. MINIMUM FROST DEPTH COVER = 4'-0" FOR EXTERIOR FOOTINGS BELOW FINAL EXTERIOR GRADE. CONTRACTOR

FOUNDATIONS SHALL BEAR ON UNDISTURBED NATIVE SOIL, UNLESS NOTED OTHERWISE. BEARING ELEVATIONS SHALL BE LOWERED WHERE SUITABLE SOILS ARE NOT ENCOUNTERED. WHERE OVEREXCAVATION HAS OCCURRED, CONTRACTOR MAY PLACE LEAN CONCRETE ON TOP OF NATIVE SOIL. THE CONTRACTOR SHALL NOTIFY THE GEOTECHNICAL AND STRUCTURAL ENGINEER IF ANY UNSUITABLE SOILS ARE ENCOUNTERED PRIOR TO PLACING

FOUNDATION WALLS SHALL BE BACKFILLED SIMULTANEOUSLY ON BOTH SIDES OF THE WALL. FOUNDATION WALLS AND SLAB-ON-GRADES SHALL REACH THEIR FULL 28 DAY COMPRESSIVE STRENGTH PRIOR TO BACKFILLING. THE CONTRACTOR SHALL PROVIDE TEMPORARY SHORING/BRACING FOR WALLS WHEN BACKFILL IS PLACED PRIOR TO CONCRETE ACHIEVING ITS FULL 28 DAY STRENGTH. BACKFILL FOR FOUNDATION WALLS IS BASED ON DRAINED CONDITIONS. SEE ARCHITECTURAL, CIVIL, AND MECHANICAL DRAWINGS FOR FOUNDATION DRAINAGE SYSTEM.

PROTECT FOUNDATIONS FROM FROST AND KEEP BOTTOM OF TRENCH DRY DURING CONSTRUCTION. IF GROUNDWATER IS ENCOUNTERED NEAR OR ABOVE THE BASE OF THE FOOTINGS, EXCAVATIONS SHALL BE DEWATERED DURING CONSTRUCTION. SURFACE WATER SHALL BE DIVERTED AWAY FROM EXCAVATIONS.

CONTRACTOR SHALL BE RESPONSIBLE FOR THE SHORING AND BRACING OF EXISTING STRUCTURES DURING EXCAVATION, BACKFILLING, AND CONSTRUCTION. CONTRACTOR SHALL SLOPE EXCAVATIONS TO ACHIEVE SOIL

FOUNDATION NOTES

SHALL NOTIFY ENGINEER OF ANY DISCREPANCIES.

SCALE: NTS

REFERENCE THE FOLLOWING STANDARD SPECIFICATIONS FROM THE LATEST EDITION OF THE STEEL JOIST INSTITUTE:

SPECIFICATIONS FOR OPEN WEB STEEL JOISTS, K-SERIES SPECIFICATIONS FOR LONGSPAN STEEL JOISTS, LH-SERIES AND DEEP LONGSPAN STEEL JOISTS, DLH-SERIES SPECIFICATIONS FOR JOIST GIRDERS RECOMMENDED CODE OF STANDARD PRACTICE FOR STEEL JOISTS AND JOIST GIRDERS

STEEL JOIST MANUFACTURER SHALL BE A MEMBER OF THE STEEL JOIST INSTITUTE. STEEL USED FOR THE FABRICATION OF STEEL JOISTS, BRIDGING, AND MISCELLANEOUS SHALL CONFORM TO ASTM A36 OR ASTM A572, GRADE 50 WITH PROVISIONS FROM AISC TECHNICAL BULLETIN NO. 3. U.N.O., A SHOP COAT OF GREY PAINT SHALL COMPLY WITH THE APPLICABLE STEEL JOIST INSTITUTE SPECIFICATION OF LATEST ADOPTION.

STEEL JOISTS SHALL BE DESIGNED AND FABRICATED IN ACCORDANCE WITH THE PARAMETERS OUTLINED IN THE STRUCTURAL DRAWINGS INCLUDING SIZE, SPACING AND LOADING FOR THE STEEL JOISTS. IN ADDITION, STEEL ROOF JOISTS SHALL BE DESIGNED FOR A NET WIND UPLIFT OF XXX PSF. FASTEN ENDS OF ALL JOISTS TO SUPPORTING MEMBERS IN ACCORDANCE WITH THE JOIST MANUFACTURER'S RECOMMENDATIONS, UNO BY STRUCTURAL DRAWINGS.

CONTRACTOR SHALL SUBMIT STEEL JOIST SHOP DRAWINGS FOR REVIEW BY THE ENGINEER OF RECORD PRIOR TO CONSTRUCTION. SHOP DRAWINGS SHALL SHOW ALL JOIST AND BRIDGING SIZES, DIMENSIONS AND LAYOUT IN ADDITION TO SUPPORT DETAILS AND BRIDGING CONNECTIONS. SHOP DRAWINGS SHALL INDICATE DEAD, LIVE, AND SPECIAL LOADING CONDITIONS ALONG WITH CAMBER REQUIREMENTS ON ERECTION DRAWINGS. CONTRACTOR SHALL ALSO SUBMIT STAMPED CALCULATIONS FOR STEEL JOIST DESIGNS DEPICTING EACH APPLICABLE LOADING CONDITION AS INDICATED ON THE STRUCTURAL DRAWINGS.

STEEL JOISTS MANUFACTURER SHALL SUBMIT DESIGN DATA TO THE STEEL JOIST INSTITUTE, OR DESIGNATED AGENCY BY THE STEEL JOIST INSTITUTE, FOR VERIFICATION OF COMPLIANCE WITH THE STEEL JOIST INSTITUTE REGULATIONS AND SPECIFICATIONS. MANUFACTURER SHALL VERIFY THROUGH PERIODIC IN-PLANT INSPECTIONS, PER SPECIFICATIONS, THAT ALL MATERIALS MEET SPECIFICATION REQUIREMENTS.

FOR CONCENTRATED LOADS BETWEEN PANEL POINTS, INSTALL MINIMUM WEB MEMBER L2x2x1/4 FROM LOCATION OF CONCENTRATED LOAD TO PANEL POINT ON OPPOSITE CHORD. USE 3/6" FILLET WELD WITH 3" MINIMUM LENGTH.

STEEL JOIST NOTES

STEEL DECK NOTES

SCALE: NTS

SCALE: NTS

SCALE: NTS

ROOF DECK, ACCESSORIES, AND WORKMANSHIP SHALL CONFORM TO THE THE STEEL DECK INSTITUTE'S "SDI SPECIFICATIONS AND COMMENTARY FOR STEEL ROOF DECK" AND THE "CODE OF RECOMMENDED STANDARD PRACTICE". STEEL DECK SHALL BE OF DEPTH AND GAGE SHOWN ON STRUCTURAL DRAWINGS. STEEL DECK AND FLASHINGS SHALL CONFORM TO ASTM A653. THE STEEL DECK SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A924 WITH MINIMUM COATING OF G60, U.N.O.. ROOF DECKING SHALL CONFORM TO ASTM A653. MINIMUM YIELD STRENGTH OF DECK SHALL BE 33 KSI.

SPAN OVER FOUR SUPPORTS (CONTINUOUS OVER THREE OR MORE SPANS) WHERE FRAMING PERMITS. WHERE TWO UNITS ABUT, FASTEN EACH TO STEEL FRAMING. WHERE STEEL MEMBERS ARE PARALLEL TO FLUTES OF DECK, ADJUST DECK LAYOUT SO THAT DECK MAY BE WELDED TO STEEL. WELD DECK TO STEEL WITH SAME WELDING AS REQUIRED AT SIDE BOUNDARIES. ALL WELDING SHALL BE IN CONFORMANCE WITH AWS D1.3. TOUCH-UP ALL WELDS AND ABRASIONS IN THE FIELD.

FASTEN ROOF DECK TO SUPPORTS PER STRUCTURAL DRAWINGS. FOR 1½" DECK, MINIMUM FASTENING IS %" PUDDLE WELDS AND A 36/4 PATTERN, SPACING WELDS AT 6" o.c. AT EDGE OF DECK AND END LAPS, UNO. SIDE LAPS SHALL BE FASTENED TOGETHER WITH 10-#10 TEK SCREWS PER SPAN, U.N.O.. LAP ENDS OF DECK 2" MINIMUM.

TYPICAL ROOF DECK PENETRATION REINFORCEMENT SHALL CONSIST OF 14 GAGE PLATE WELDED TO THE DECK. EXTEND PLATE MINIMUM OF 6" BEYOND OPENING, ON EACH SIDE. MAXIMUM PENETRATION SIZE IS 1'-6" DIAMETER WITHOUT ADDITIONAL REINFORCEMENT.

CONTRACTOR SHALL SUBMIT SHOP DRAWINGS DEPICTING DECK SHEET LAYOUT, DIMENSIONS, PROPERTIES, FASTENING SCHEDULE, AND DETAILS FOR DECK SUPPORT AND EDGE CONDITIONS. THE CONTRACTOR SHALL PROVIDE ALL NECESSARY STOPS, EDGE PIECES, AND ACCESSORIES.

ALL STRUCTURAL STEEL WORK SHALL CONFORM TO:

AMERICAN INSTITUTE OF STEEL CONSTRUCTION, MANUAL OF STEEL CONSTRUCTION, NINTH EDITION

ALL STEEL, U.N.O. ASTM A572, GRADE 50 ANGLES, PLATES ASTM A36, Fy=36 KSI STRUCTURAL TUBING ASTM A500, GRADE B, Fy=46 KSI

SHOP DRAWINGS SUBMITTALS SHALL INCLUDE: 1. CERTIFIED MILL TEST REPORTS OF STRUCTURAL STEEL (INCLUDING NAMES AND LOCATIONS OF MILLS

2. CERTIFIED MILL TEST REPORTS OF BOLTS, NUTS AND WASHERS (INCLUDING NAMES AND LOCATIONS OF MILLS

FIELD) AND WELDED CONNECTIONS (SHOP AND FIELD) DEPICTING AWS WELDING SYMBOLS.

2. ULTRASONIC TESTING, IN ACCORDANCE WITH ASTM E-164, ON 100% OF ALL FIELD FULL PENETRATION WELDS.

4. FIELD BOLTED CONNECTIONS, INCLUDING VERIFICATION OF BOLT GRADES. 5. SHEAR STUD QUANTITY, PROPER INSTALLATION, SIZE, AND SPACING. SHEAR STUDS SHALL CONFORM TO AWS D1.1.

1. FIELD CONNECTIONS SHALL UTILIZE MINIMUM ¾" DIAMETER A325 HIGH STRENGTH BOLTS, U.N.O.. BOLTED CONNECTION SHALL BE SLIP CRITICAL (SC) AT ALL MOMENT FRAMES, BRACED FRAMES, AND AT ADDITIONAL LOCATIONS INDICATED IN THE DRAWINGS. SLIP CRITICAL CONNECTIONS SHALL UTILIZE LOAD

2. HIGH STRENGTH BOLTS SHALL BE INSTALLED AND TIGHTENED PER AISC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 BOLTS.

HEX NUTS AND LOCK WASHERS. 4. CONTRACTOR SHALL DESIGN CONNECTIONS NOT ALREADY DETAILED ON STRUCTURAL DRAWINGS. DESIGN

1. WELDING SHALL CONFORM TO AWS D1.1. USE LOW-HYDROGEN SMAW ELECTRODES WITH MINIMUM TENSILE

STRUCTURAL STEEL SHALL RECEIVE THE FOLLOWING PROTECTIVE COATINGS: 1. DO NOT PAINT SURFACES TO RECEIVE METAL DECK AND/ OR SHEAR CONNECTORS FASTENED BY WELDING, CONTACT SURFACES OF HIGH STRENGTH BOLTED CONNECTIONS, FINISHED BEARING SURFACES, AND SURFACES TO BE WELDED IN THE FIELD. IF REQUIRED, PROTECT THESE SURFACES BY RUST-INHIBITING

2. UNEXPOSED STRUCTURAL STEEL SHALL BE CLEANED IN ACCORDANCE WITH SSPC-SP3 AND PAINTED WITH

SHEAR CONNECTOR STUDS:

1. SHEAR CONNECTOR STUDS SHALL BE NELSON, OR EQUIVALENT, 3/4" DIAMETER, U.N.O.. WELD STUDS PER STUD MANUFACTURER'S RECOMMENDATIONS THROUGH METAL DECKING. STUD LENGTH SHALL BE 1" BELOW TOP OF CONCRETE SLAB-ON-DECK.

LONG LEG HORIZONTAL LONG LEG VERTICAL BOTTOM OF FOOTING BLDG BUILDING MAXIMUM MECH MECHANICAL MFR MANUFACTURER BEARING MINIMUM BTWN BETWEEN MISCELLANEOUS STRUCTURAL STEEL CHANNEL NEAR FACE **CANTILEVER** NUMBFR CAST-IN-PLACE CONCRETE NEAR SIDE CONTROL JOINT NOT TO SCALE CENTERLINE ON CENTER CONCRETE MASONRY UNIT OUTSIDE FACE CONSTRUCTION JOINT **OPNG** OPENING COLUMN OPP OPPOSITE CONC CONCRETE CONNECTION CONT PIER DESIGNATION CONTINUOUS CONTR CONTRACTOR PARTIAL PENETRATION WELD COMPLETE PENETRATION WELD PREFABRICATED CY CUBIC YARD POUNDS PER SQUARE FOOT PSI POUNDS PER SQUARE INCH DIAMETER REINFORCING STEEL DISCONT DISCONTINUOUS REQ, REQD REQUIRED DWG DRAWING ROOF DRAIN **FXISTING** SLIP CRITICAL SECTION EACH FACE SHEATH SHEATHING EL, ELEV ELEVATION SIMILAR SOG SLAB-ON-GRADE **EQUIPMENT** SPAC SPACING EACH SIDE SPECS SPECIFICATIONS EACH WAY STAINLESS STEEL EXPANSION STD STANDARD EXTERIOR STIFFENER FOOTING DESIGNATION STR STRAIGHT FDN FOUNDATION STRUCT STRUCTURAL FINISH FLOOR FLANGE FLOOR TOP AND BOTTOM FOOT TOC, T/CONC TOP OF CONCRETE FTG FIELD VERIFY T/FTG, TOF TOP OF FOOTING TEMPERATURE T/SHELF TOP OF SHELF GALV GALVANIZED T/SLAB TOP OF SLAB TOP OF STEEL HOR, HORIZ HORIZONTAL T/WALL TOP OF WALL HOLLOW STRUCTURAL SHAPE STRUCTURAL TUBING HEIGHT TYPICAL INSIDE FACE UNO INFORMATION VER, VERT VERIFY IN FIELD KIP (1 KIP = 1,000 LBS)

ANCHOR BOLT

ADDITIONAL

ARCHITECT

ADDL

UNLESS NOTED OTHERWISE STRUCTURAL STEEL WIDE FLANGE

DOUBLE ANGLE

LINEAR FOOT

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ENGINEERING

SHEET TITLE:

STRUCTURAL NOTES

DESIGNED:	TD
DRAWN:	TD
DATE:	9-29-09
CADD FILE:	9080-S1.dwg
PROJECT NUMBER:	9080

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WITHOUT WORK POINT WEIGHT WELDED WIRE FABRIC **ABBREVIATIONS** SCALE: NTS SCALE: NTS UNDISTURBED EARTH SLOPE DESIGNATION LEDGE ELEVATION MARK ROOF PITCH COMPACTED STRUCTURAL FILL MASONRY CONSTRUCTION AND MATERIALS SHALL CONFORM TO ALL REQUIREMENTS OF BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (ACI 530/ASCE5/TMS 402) AND SPECIFICATION FOR MASONRY STRUCTURES (ACI 530.1/ASCE6/TMS 602). SPAN DIRECTION CONCRETE 1. UNITS: ASTM C90 GRADE N. TYPE 1 WITH MINIMUM COMPRESSIVE STRENGTH = 1,900 PSI GROUT SECTION MARK 2. MORTAR: ASTM C270, TYPE S SECTION No. ——— 3. ASSEMBLY/PRISM MINIMUM COMPRESSIVE STRENGTH I'm = 1,500 PSI DWG. WHERE SHOWN -4. ALL UNITS SHALL BE LAID IN RUNNING BOND, UNLESS NOTED OTHERWISE BRICK 5. PLACE MASONRY WHILE MORTAR IS SOFT AND PLASTIC 6. CONCRETE MASONRY UNITS (CMU) SHALL BE STANDARD UNITS WITH NOMINAL FACE DIMENSIONS LENGTH = 16 INCHES, HEIGHT = 8 INCHES, WIDTH AS INDICATED 1. ASTM A615, GRADE 60 2. UNLESS NOTED OTHERWISE, PROVIDE #5 BAR @ 32" o.c. 3. PROVIDE ADDITIONAL VERTICAL REINFORCEMENT AT EACH SIDE OF CONTROL JOINTS, AT INTERSECTION OF EXTERIOR WALLS, AND AT EACH SIDE OF ALL MASONRY OPENINGS. 4. VERTICAL REINFORCEMENT SHALL BE CONTINUOUS FOR FULL HEIGHT OF WALL 1. ASTM C476 WITH MINIMUM COMPRESSIVE STRENGTH = 2000 PSI 2. FULLY GROUT ALL CELLS WITH REINFORCEMENT AND ANCHORS 3. CONSOLIDATE AND RECONSOLIDATE GROUT ASSEMBLED WALLS WITH VIBRATOR 1. ASTM 951, HOT DIPPED GALVANIZED OR STAINLESS STEEL

CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES

STRUCTURAL STEEL MEMBERS SHALL BE IN CONFORMANCE WITH THE FOLLOWING:

STEEL PIPE ASTM A53, TYPE E OR S, GRADE B, Fy=35 KSI

SHOP DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW PRIOR TO COMMENCING FABRICATION.

AND SHOPS). 3. STRUCTURAL STEEL FABRICATION AND ERECTION DRAWINGS WHICH INCLUDE BOLTED CONNECTIONS (SHOP AND

OWNER SHALL RETAIN A QUALIFIED TESTING AGENCY TO PERFORM AND VERIFY THE FOLLOWING:

4. METAL DECK SHOP DRAWINGS DEPICTING SHEAR STUD LAYOUT ON BEAMS AND GIRDERS.

1. VISUAL INSPECTION OF ALL WELDS. 3. PROVIDE RANDOM VERIFICATION VIA ULTRASONIC TESTING OF SHOP FULL PENETRATION WELDS.

BOLTED CONNECTIONS:

INDICATOR WASHERS OR TENSION CONTROL BOLTS. BOLT HOLES SHALL BE STANDARD SIZE, U.N.O..

3. ANCHOR BOLTS SHALL CONFORM TO ASTM A307, GRADE A, STANDARD HEX HEAD FURNISHED WITH HEAVY

SHALL BE STAMPED BY A LICENSED STRUCTURAL ENGINEER AND SUBMITTED PRIOR TO COMMENCING

COATING THAT CAN BE REMOVED EASILY PRIOR TO ERECTION

PRIMER PAINT, TNEMEC 10-99, OR EQUIVALENT, U.N.O.. 3. EXPOSED STRUCTURAL STEEL TO RECEIVE ZINC-RICH EPOXY PAINT SHALL BE FIRST CLEANED IN ACCORDANCE WITH SSPC-SP6, COMMERCIAL BLAST CLEANING. USE TNEMEC ZIN-RICH EPOXY PAINT, OR

EQUIVALENT. APPLY FINISH COAT PER ARCHITECT. 4. EXPOSED STRUCTURAL STEEL TO BE HOT-DIPPED GALVANIZED SHALL BE IN ACCORDANCE WITH ASTM A123.

2. SHEAR STUDS, WHERE REQUIRED, ARE INDICATED ON THE DRAWINGS AS [XX], WHERE XX IS THE NUMBER OF

STRUCTURAL STEEL NOTES

A. CONCRETE UNITS

B. VERTICAL REINFORCEMENT

C. GROUT

C. HORIZONTAL JOINT REINFORCEMENT

3. BEGIN HORIZONTAL REINFORCEMENT AT TOP OF FIRST COURSE AND LAP 6 INCHES, MIN. 4. MINIMUM COVER: 1/8 INCHES WHEN EXPOSED TO EARTH OR WEATHER, 1/2 INCH OTHERWISE

PROVIDE VERTICAL CONTROL JOINTS IN WALLS AT A MAXIMUM SPACING OF 24'-0" AND AT APPROXIMATELY ½ WALL HEIGHT FROM WALL INTERSECTIONS.

2. 2xW2.1 (8 GA) LADDER REINFORCEMENT SPACED AT 16" o.c. VERTICAL

SECURE ALL CMU WALL SUPPORTED FIXTURES, EQUIPMENT, ETC. TO CMU WALL PER STRUCTURAL DRAWINGS AND

MANUFACTURER'S RECOMMENDATIONS. DO NOT USE EXPANSION ANCHORS.

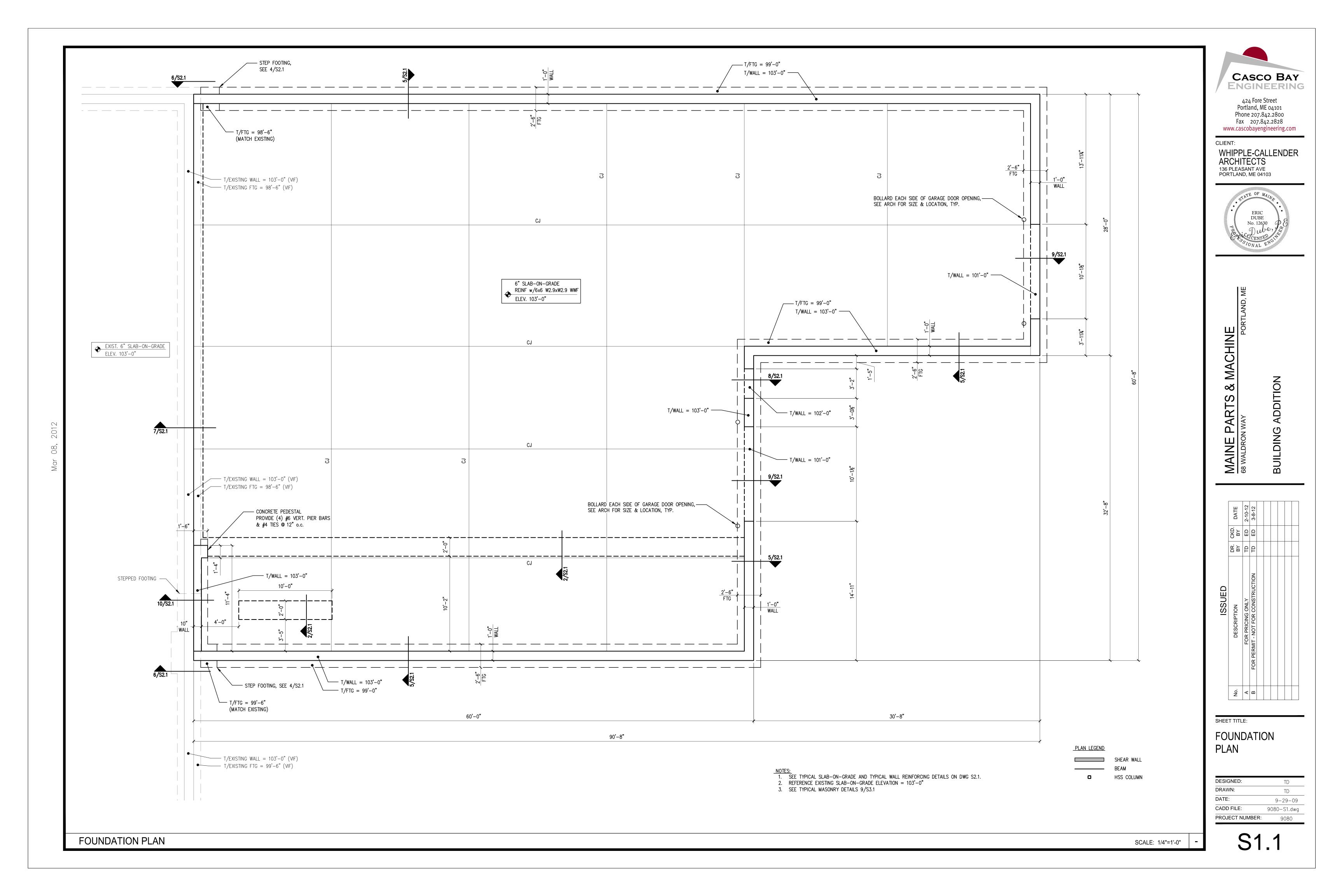
MASONRY NOTES

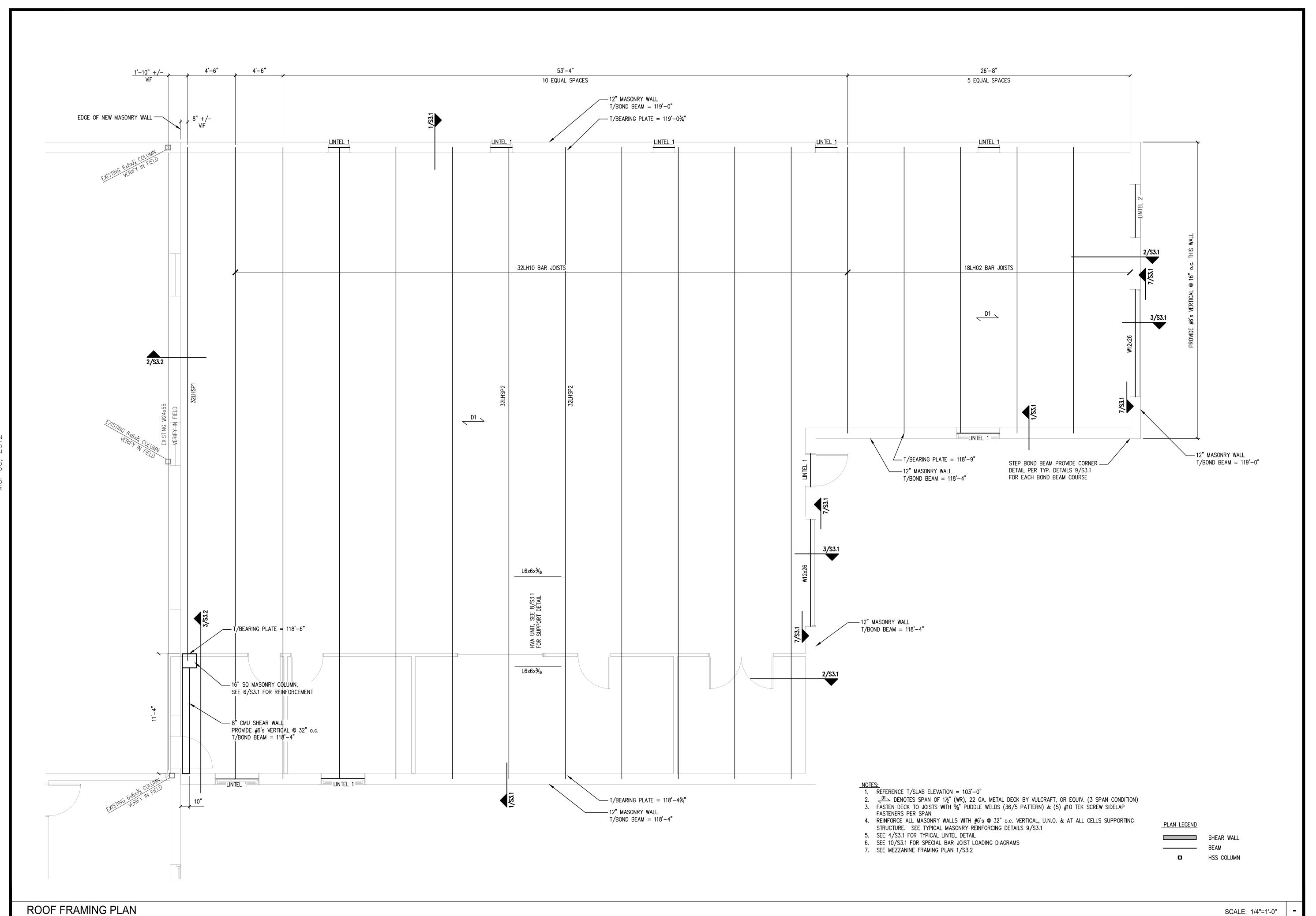
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LEGEND

KIPS PER SQUARE INCH

SCALE: NTS-



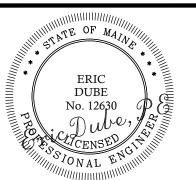


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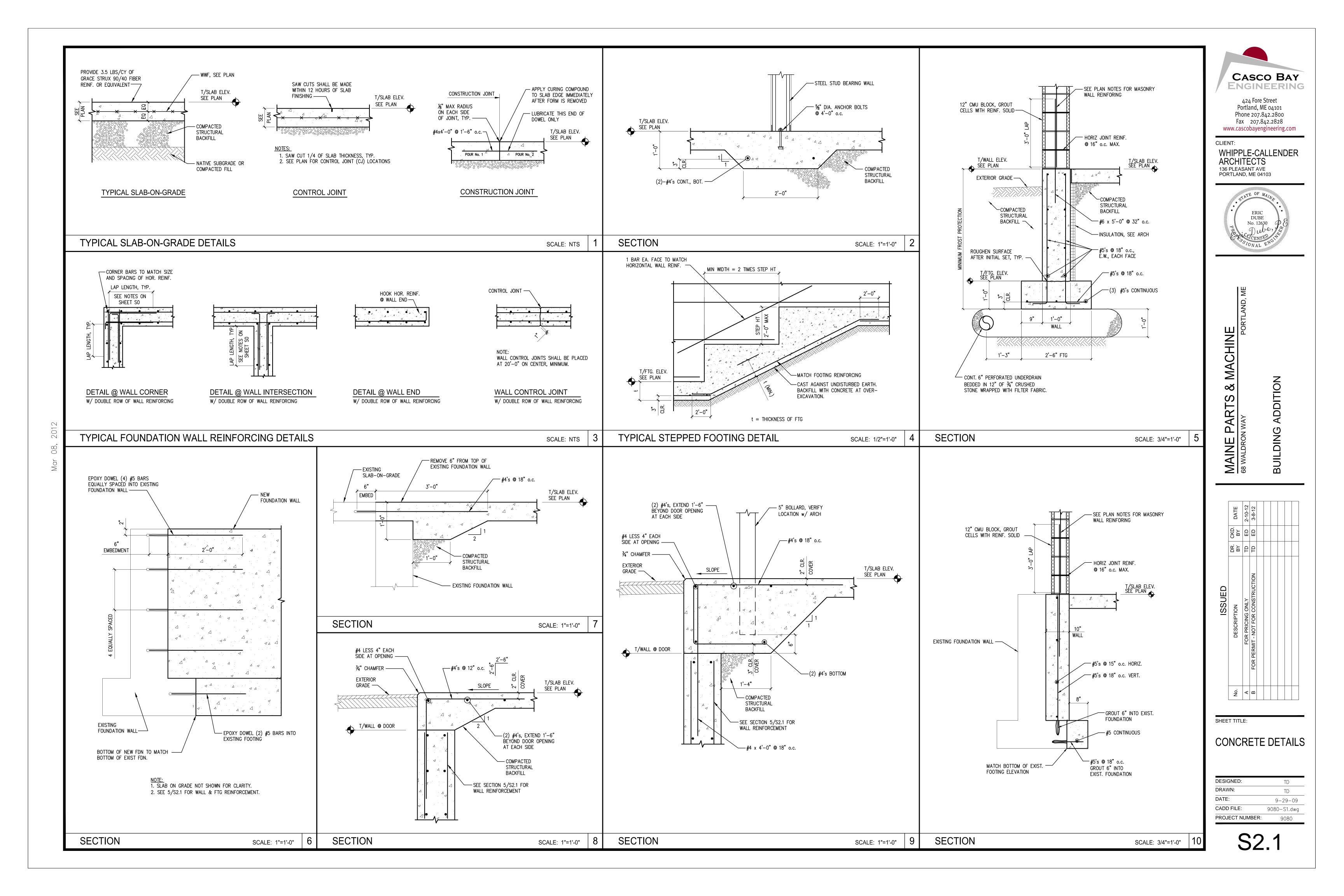
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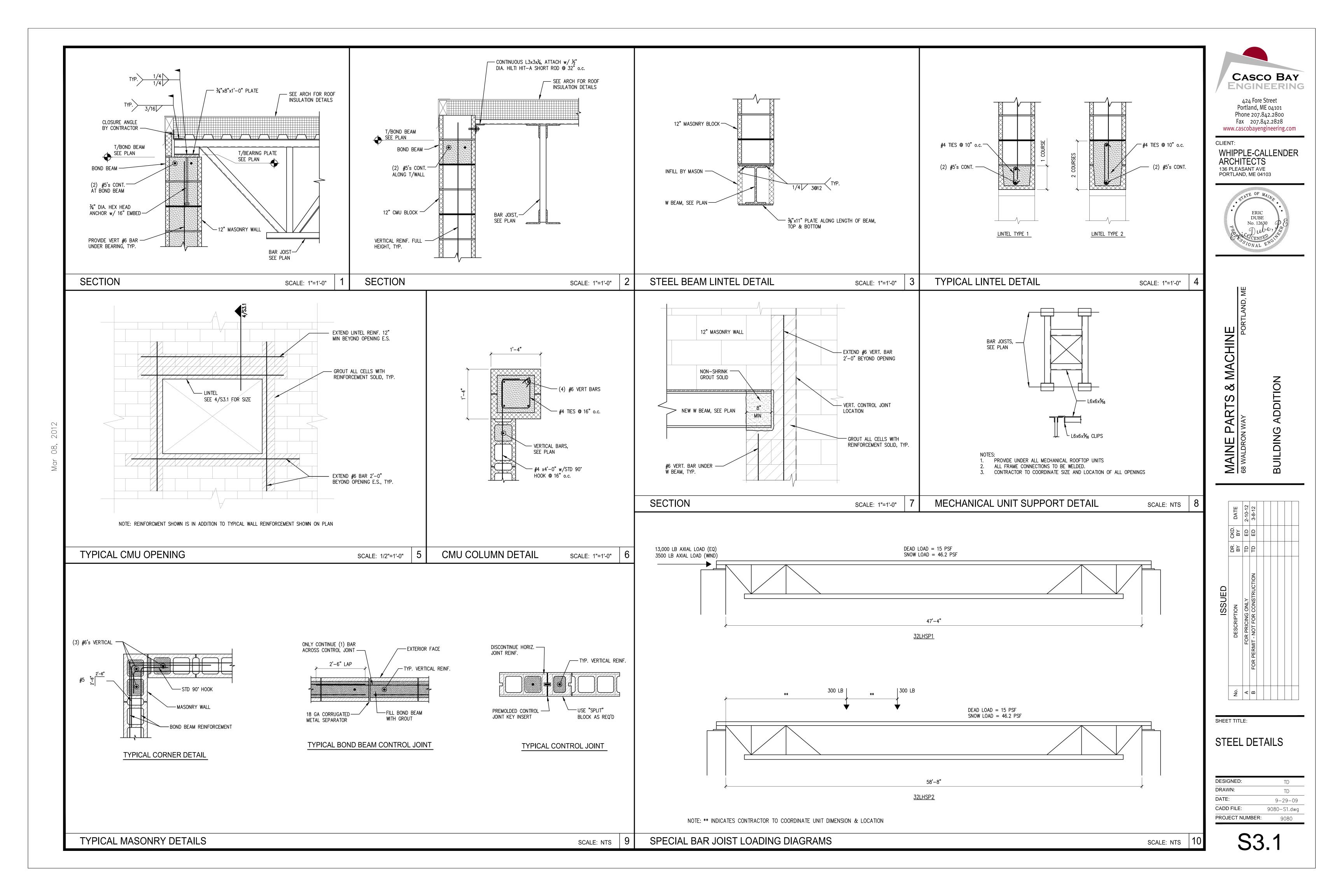
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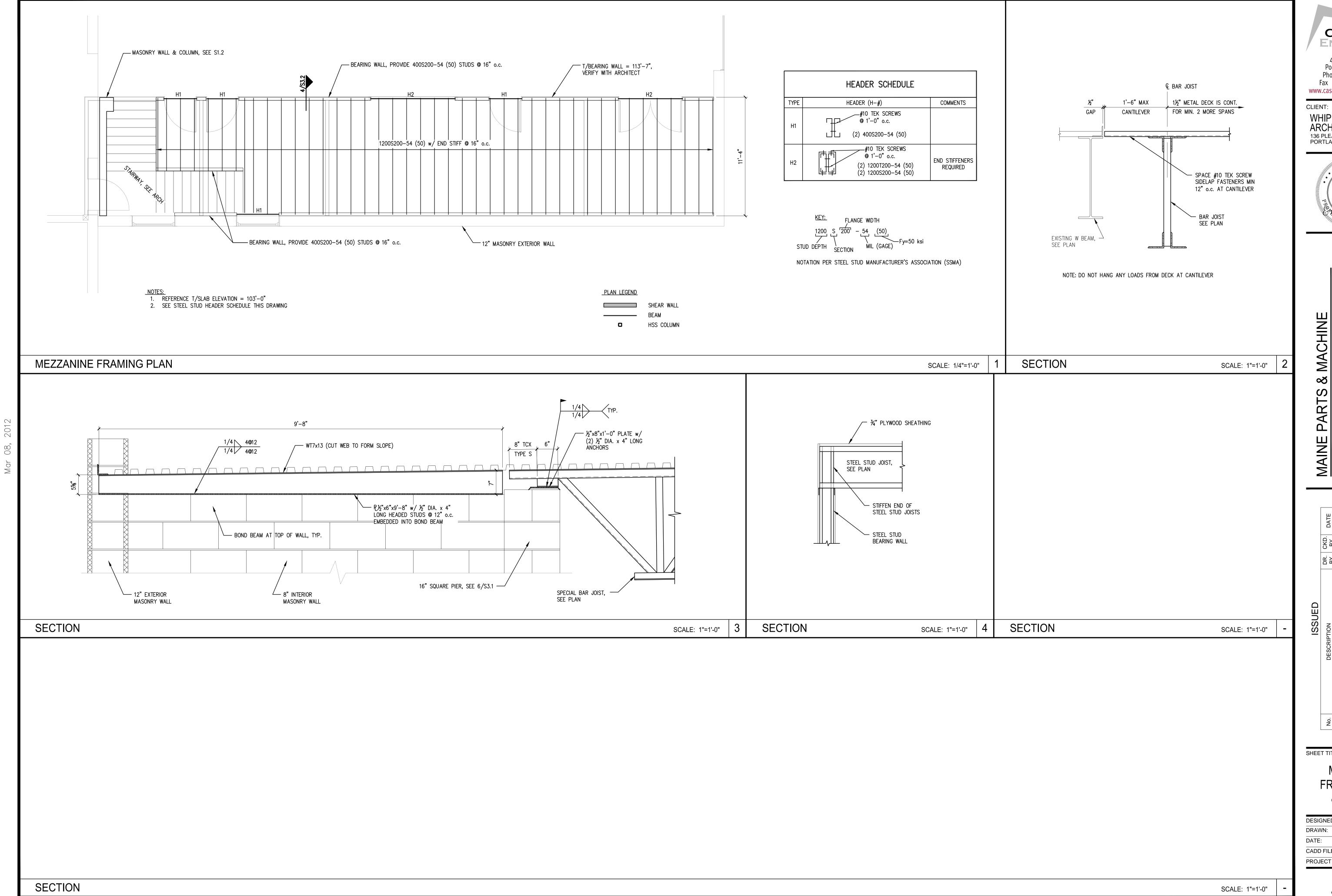
ROOF FRAMING PLAN

DESIGNED:	TD
DRAWN:	TD
DATE:	9-29-09
CADD FILE:	9080-S1.dwg
PROJECT NUMBER:	9080

S1.2





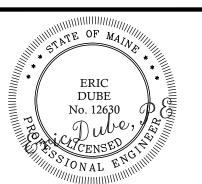


CASCO BAY ENGINEERING

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WHIPPLE-CALLENDER ARCHITECTS

136 PLEASANT AVE
PORTLAND, ME 04103



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SHEET TITLE:

MEZZANINE FRAMING PLAN & DETAILS

DESIGNED:	TD
DRAWN:	TD
DATE:	9-29-09
CADD FILE:	9080-S1.dwg
PROJECT NUMBER:	9080

S3.2