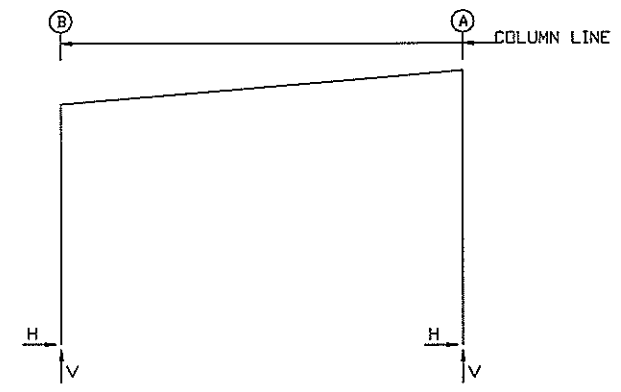


FRAME LINES: 2



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column Reactions (k)						Anc. Bolt Qty Dia	Base Plate (in)			Base EL. (in)	
		Load ID	Hmax	V	Load ID	Hmin	V		Width	Length	Thick		
2	B	3	4.8	17.1	5	-4.4	-1.4	4	0.750	6.000	11.00	0.375	0.0
		1	3.1	23.8	4	-1.4	-6.1						
2	A	6	3.7	0.0	2	-4.9	27.9	4	0.750	8.000	11.00	0.375	0.0
		1	-3.2	41.0	7	-1.2	-7.4						

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frm Line	Column Line	Dead		Collateral		Live		Snow		Snow Drift		Slide Snow	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
2	B	0.1	1.4	0.1	0.9	0.7	6.1	2.1	18.1	-0.1	-0.2	0.8	3.4
		-0.1	1.8	-0.1	1.2	-0.7	7.9	-2.1	23.3	0.1	7.5	-0.9	14.7
2	A	-1.5	-6.9	3.3	-1.8	-4.4	-2.2	1.0	2.8	1.3	-5.9	0.5	0.4
		-3.3	-4.8	1.5	-6.9	-0.4	0.9	3.8	-1.1	-2.3	-5.8	1.0	-1.8

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Snow Drift Vert	Slide Snow Vert	Wind Left1 Vert	Wind Right1 Vert	Wind Left2 Vert	Wind Right2 Vert	Wind Press Horiz	Wind Suct Horiz
1	A	0.9	0.4	3.0	8.7	2.8	5.4	-2.8	-2.0	-2.8	-2.0	-1.4	1.6

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column Reactions (k)						Anc. Bolt Qty Dia	Base Plate (in)			Base EL. (in)	
		Load ID	Hmax	V	Load ID	Hmin	V		Width	Length	Thick		
1	B	8	1.4	-2.3	9	-1.2	-1.9	4	0.750	8.000	8.000	0.375	0.0
		1	0.0	12.1	8	1.4	-2.3						
1	A	10	1.6	-2.3	9	-1.4	-2.0	4	0.750	8.000	8.000	0.375	0.0
		1	0.0	15.5	10	1.6	-2.3						
3	A	11	1.6	-2.3	9	-1.4	-1.9	4	0.750	8.000	8.000	0.375	0.0
		1	0.0	15.5	11	1.6	-2.3						
3	B	12	1.4	-2.3	9	-1.2	-1.8	4	0.750	8.000	8.000	0.375	0.0
		1	0.0	12.1	12	1.4	-2.3						

NOTES FOR REACTIONS

- The following Design Data is per Package Steel Systems, Inc.'s standard design practices and established procedures and recommendations of the following Organizations and/or Specifications.
- American Institute of Steel Construction (AISC 2005)  
 American Welding Society Structural Welding Code (AWS D1.1)  
 North American United States (NAUSD07)
- For maximum reactions tables, all loading conditions are examined and only the maximum/minimum horizontal or vertical reactions along with the corresponding horizontal or vertical for those load IDs are reported.
  - Positive reactions are shown in the sketch. Foundation loads are in the opposite directions.
  - Bracing reactions are in the plane of the brace with the horizontal pointing away from the braced bay. The vertical reaction can be downward or upward.
  - Reactions given are based on the design data below. Reactions are not furnished for loads not listed.
  - The endwall column reactions do NOT include wind and seismic reactions from endwall bracing. Reactions given in the bracing reactions table should be combined with the appropriate basic column reactions as necessary to determine the maximum reactions for foundation design.
  - The rigid frame maximum reactions include wind and seismic reactions from sidewall bracing. Reactions given in the bracing reactions table should not be combined with the appropriate basic column reactions as necessary to determine the maximum reactions for foundation design.
  - Foundation construction and design is not the responsibility of Package Steel Systems, Inc. The embedment of the anchor bolts in concrete is the responsibility of the foundation designer.
  - Suggested anchor rod diameter, quantity, minimum projection and placement are shown. All anchor rods are assumed to be ASTM F1544 Grade 36 or equal. Anchor rods (not by PSS) shall be set to a tolerance of +1/8" in both elevation and location.
  - Column base plates are designed not to exceed a bearing pressure of 1050 pounds per sq. inch (0.35F<sub>c</sub> where F<sub>c</sub> = 3000 psi) unless noted otherwise.
  - Basic design wind pressure is furnished. For components and cladding not specifically designed and/or furnished by PSS, the design pressures and suctions shall be increased based on tributary area and location. Confirmation of the design loads and adequacy to resist such loads shall be the responsibility of a licensed design professional by others.

Building Reactions are based on the following information:

Building Code/Edition:	IBC 09	Snow Loads:	
Building Size:		Ground Snow (Pg)	70.00 psf
Width (ft.)	25.00	Flat Roof Snow (Pf)	58.80 psf
Length (ft.)	45.00	Snow Exposure Factor (Ce)	0.70
Back Side Eave Height (ft.)	15.00	Snow Thermal Factor (Ct)	1.20
Front Side Eave Height (ft.)	17.08	Snow Importance Factor (Is)	1.00
Back Side Roof Slope	1:0.12	Sloped Roof Factor (Cs)	1.00
Front Side Roof Slope		Drift & Sliding Snow	98.20 psf
Roof Dead, Collateral, & Live Loads:		Seismic Loads:	
Dead Load	3.00 psf	Seismic Importance (Ie)	1.00
Collateral Load	3.00 psf	Seismic Design Category (A/B/C/D)	C
Live Load	20.00 psf	Site Class-Type	D
Live Load Reduction Taken	No	Seismic Response Coeff. (Sds)	0.259
Wind Loads:		Seismic Response Coeff. (Sd1)	0.126
Basic Wind Speed (3 Second Gust)	94.0 mph	Response Modification (Rf)	3.00
Wind Exposure	B	Response Modification (Rf)	3.00
Building Enclosure (O/C/P)	Partial	Design Base Shear (V) = Longit.	2.24 kips
Wind Importance Factor (Iw)	1.00	Design Base Shear (V) = Transv.	2.82 kips
Internal Pressure Coeff. (GCp1)	0.55	Analysis Procedure: Equivalent Lateral Force	
Acronyms:		Auxiliary Load(s):	None
AUXx = Auxiliary Load - Case x			
C = Closed			
CL = Collateral Load			
DL = Dead Load			
FxUNB_LL = Unbalanced Live Load for Frame IDx			
LL = Max. of (Live or Snow)			
LLR = Live Load Unbalanced			
LnWndL = Longitudinal Wind Load - Left			
LnWndR = Longitudinal Wind Load - Right			
mph = miles per hour			

Loading Conditions are as follows:

- Dead+Collateral+Snow+Slide\_Snow
- Dead+Collateral+0.75Snow+0.75Wind\_Left1+0.75Slide\_Snow+0.75Floor\_Live
- Dead+Collateral+0.75Snow+0.75Wind\_Right1+0.75Slide\_Snow+0.75Floor\_Live
- 0.6Dead+Wind\_Left1
- 0.6Dead+Wind\_Left2
- 0.6Dead+Wind\_Right2
- 0.6Dead+Wind\_Long1+LWIND1\_R2E
- 0.6Dead+Wind\_Left2+Wind\_Suction
- 0.6Dead+Wind\_Pressure+Wind\_Long1
- 0.6Dead+Wind\_Left1+Wind\_Suction
- 0.6Dead+Wind\_Right1+Wind\_Suction
- 0.6Dead+Wind\_Right2+Wind\_Suction

WIND BENT REACTIONS

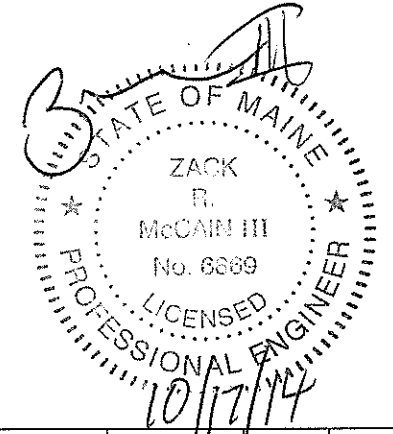
Wall Loc	Col Line	± Reactions (k)				Anc. Bolt Qty Dia	Base Plate (in)			
		Wind(k) Horiz	Seismic(k) Vert	Wind(k) Horiz	Seismic(k) Vert		Width	Length	Thick	
F_SW A	1	1.2	1.7	1.1	1.7	4	0.750	6.000	10.000	0.375
F_SW A	2	1.2	1.7	1.1	1.7	4	0.750	6.000	10.000	0.375

BUILDING BRACING REACTIONS

Wall Loc	Col Line	± Reactions (k)				Panel Shear (lb/Ft)	
		Wind Horiz	Seismic Vert	Wind Horiz	Seismic Vert		
L_EW 1					53	19	
F_SW A		Wind Bent In Wall					
R_EW 3					66	40	
B_SW B		Torsional Bracing Used					

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type	Proj (in)
O 16	Endwall	3/4"	A307	2.00
O 8	Frame	3/4"	A307	2.00
O 8	WindCol	3/4"	A307	2.00



REV.	DESCRIPTION:	DATE:	DRAFT	ENG.
3				
2				
1				
INITIAL DRAWING: RELEASED FOR CONSTRUCTION		CURRENT REVISION: 0		
PACKAGE STEEL SYSTEMS, INC.		Biskup Construction Inc.		
PROJECT	Alpine Realty	ANCHOR BOLT REACTIONS		
ID	1410-012	DESIGN: ZRM	DESIGN CHECK: ZRM	
PROJECT	380 Warren Ave.	DRAFT: RPG	DRAFT CHECK: RPG	
ADDRESS	Portland, ME 04103	DATE: 10/17/14	SCALE: NONE	SHEET: ABLT-2