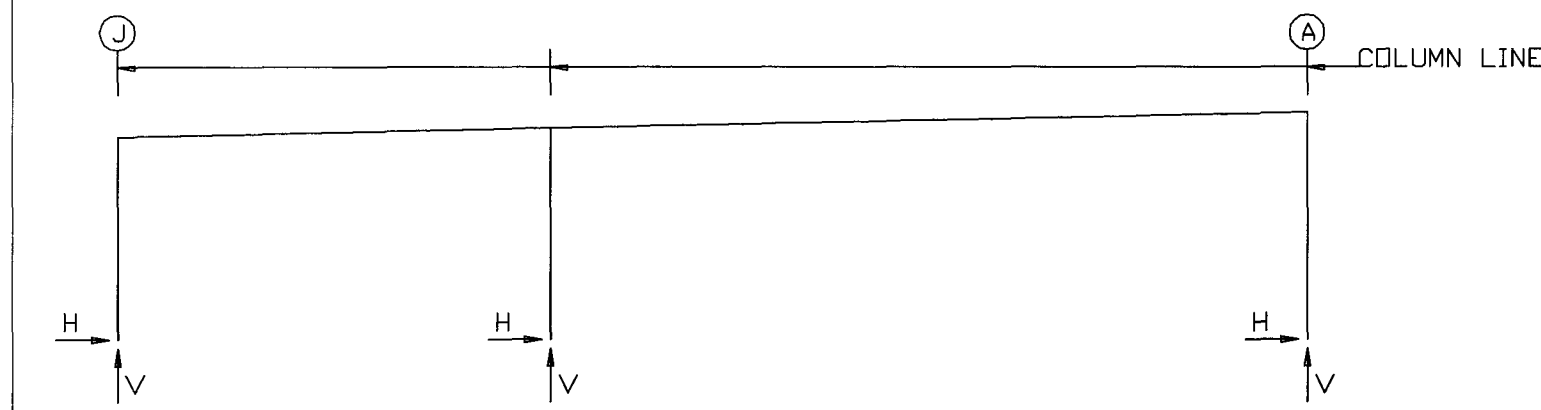


FRAME LINES: 2 3 4



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column Reactions (k)						Anc. Bolt Qty Dia	Base Plate (in)			Base EL. (in)	
		Load ID	Hmax	Vmax	Load ID	Hmin	Vmin		Width	Length	Thick		
2*	J	3	5.6	12.9	4	-3.1	-1.7	4	0.750	8.000	12.50	0.375	0.0
		8	4.2	20.7	6	0.0	-8.6						
2*	A	5	2.7	0.2	1	-5.6	4.1	4	0.750	8.000	13.00	0.375	0.0
		9	-4.7	41.6	7	0.3	-8.9						
2*	B	6	0.0	-14.4	6	0.0	-14.4	4	0.750	10.00	8.500	0.625	-6.0
		2	0.0	81.9									

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead		Collateral		Live		Snow		Wind_Left1		Wind_Right1	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
2*	J	0.5	2.1	0.3	1.1	2.3	7.1	4.7	14.9	-3.2	-4.8	1.6	-2.0
2*	A	-0.5	4.5	-0.3	2.4	-2.3	16.3	-4.7	34.2	-1.2	-9.2	3.0	-6.4
2*	B	0.0	8.3	0.0	4.9	0.0	32.7	0.0	68.7	0.0	-18.8	0.0	-12.4

WIND BENT REACTIONS

Wall Loc	Col Line	Reactions (k)				Anc. Bolt Qty Dia	Base Plate (in)			
		Wind Horz	Seismic Vert	Wind Vert	Seismic Horz		Width	Length	Thick	
F_SW	A	3	3.0	4.9	6.4	2	0.750	8.000	12.000	0.375
F_SW	A	4	3.0	4.9	6.4	2	0.750	8.000	12.000	0.375

BUILDING BRACING REACTIONS

Wall Loc	Col Line	Reactions (k)			Panel Shear (lb/ft)	
		Wind Horz	Seismic Vert	Wind Vert		
L_EW	1	F.D	2.0	1.7	3.6	2.9
F_SW	A	Wind Bent In Wall				0
R_EW	S	Bracing Not Used				0
B_SW	J	4.3	3.6	3.8	12.9	8.7

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type	Proj (in)
0 32	Jamb	1/2"	A307	1.50
0 32	Endwall	3/4"	A307	2.00
0 36	Frame	3/4"	A307	2.00
0 4	WindCol	3/4"	A307	2.00

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind_Left1 Horz	Wind_Right1 Horz	Wind_Left2 Horz	Wind_Right2 Horz	Wind Press Horz	EIPAT_SL_1-		EIPAT_SL_2-	
											Horz	Vert	Horz	Vert
1	J	0.7	0.4	2.4	5.0	0.0	-1.9	0.0	-1.1	0.0	-1.9	0.0	-1.1	0.0
1	H	1.5	1.0	6.4	13.4	0.0	-5.2	0.0	-3.0	0.0	-5.2	0.0	-3.0	0.0
1	F	1.3	0.8	5.7	11.9	1.3	-5.7	0.0	-0.9	1.5	-5.8	0.0	-0.9	1.7
1	D	1.3	0.8	5.6	11.9	0.0	-3.6	2.0	-4.3	0.0	-3.4	2.0	-4.3	1.7
1	B	1.5	1.0	6.4	13.4	0.0	-5.2	0.0	-3.0	0.0	-5.2	0.0	-3.0	1.8
1	A	0.7	0.4	2.4	5.0	0.0	-1.9	0.0	-1.1	0.0	-1.9	0.0	-1.1	0.0

Frm Line	Col Line	Wind Suct Horz	Wind_Long1 Horz	Wind_Long2 Horz	Seis_Left Horz	Seis_Right Horz	EIPAT_SL_3-		EIPAT_SL_4-		EIPAT_SL_5-		EIPAT_SL_6-		-LWIND1_L--		-LWIND1_R--		-LWIND2_L--		
							Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz
1	J	0.0	0.0	-1.4	0.0	-0.8	0.0	0.0	0.0	0.0	0.0	2.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
1	H	1.7	0.0	-3.7	0.0	-2.2	0.0	0.0	0.0	0.0	0.0	3.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
1	F	1.8	0.0	-3.0	0.0	-1.9	4.6	-3.8	0.0	3.9	0.0	-0.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.2
1	D	1.9	0.0	-3.6	0.0	-2.1	0.0	3.8	4.6	-3.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.6
1	B	1.9	0.0	-3.8	0.0	-2.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	3.8
1	A	0.0	0.0	-1.4	0.0	-0.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	2.7

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column Reactions (k)						Anc. Bolt Qty Dia	Base Plate (in)			Base EL. (in)	
		Load ID	Hmax	Vmax	Load ID	Hmin	Vmin		Width	Length	Thick		
1	J	10	0.0	-1.5	10	0.0	-1.5	4	0.750	8.000	8.000	0.375	0.0
1	H	10	1.7	-4.3	11	-1.6	-2.9	4	0.750	8.000	8.000	0.375	0.0
1	F	12	1.8	-5.1	11	-1.7	-2.2	4	0.750	8.000	8.000	0.375	0.0
1	D	13	1.9	-3.5	11	-1.7	-2.8	4	0.750	8.000	8.000	0.375	0.0
1	B	12	1.9	-4.3	11	-1.8	-2.9	4	0.750	8.000	8.000	0.375	0.0
1	A	10	0.0	-1.5	10	0.0	-1.5	4	0.750	8.000	8.000	0.375	0.0
5	I	13	1.6	-4.6	11	-1.5	-3.1	4	0.750	8.000	8.000	0.375	0.0
5	J	14	0.0	-1.3	14	0.0	-1.3	4	0.750	8.000	8.000	0.375	0.0

NOTES FOR REACTIONS

- The Following Design Data is per Package Steel Systems, Inc.'s standard design practices and established procedures and recommendations of the following Organizations and/or Specifications:
- American Institute of Steel Construction (AISC 2005)
 - American Welding Society Structural Welding Code (AWS D1.1)
 - North American United States (NAUS07)
- For maximum reactions tables, all loading conditions are examined and only the maximum/minimum horizontal or vertical reactions along with the corresponding horizontal or vertical for those load IDs are reported.
 - Positive reactions are shown in the sketch. Foundation loads are in the opposite directions.
 - Bracing reactions are in the plane of the brace with the horizontal pointing away from the braced bay. The vertical reaction can be downward or upward.
 - Reactions given are based on the design data below. Reactions are not furnished for loads not listed.
 - The endwall column reactions do NOT include wind and seismic reactions from endwall bracing. Reactions given in the bracing reactions table should be combined with the appropriate basic column reactions as necessary to determine the maximum reactions for foundation design.
 - The rigid frame maximum reactions include wind and seismic reactions from sidewall bracing. Reactions given in the bracing reactions table should not be combined with the appropriate basic column reactions as necessary to determine the maximum reactions for foundation design.
 - Foundation construction and design is not the responsibility of Package Steel Systems, Inc. The embedment of the anchor bolts in concrete is the responsibility of the foundation designer.
 - Suggested anchor rod diameter, quantity, minimum projection and placement are shown. All anchor rods are assumed to be ASTM F1554 Grade 36 or equal. Anchor rods (not by PSS) shall be set to a tolerance of +1/8" in both elevation and location.
 - Column base plates are designed not to exceed a bearing pressure of 1050 pounds per sq. inch (0.35F_c where F_c= 3000 psi) unless noted otherwise.
 - Basic design wind pressure is furnished for components and cladding not specifically designed and/or furnished by PSS, the design pressures and suction shall be increased based on tributary area and location. Confirmation of the design loads and adequacy to resist such loads shall be the responsibility of a licensed design professional by others.

Building Reactions are based on the following information:

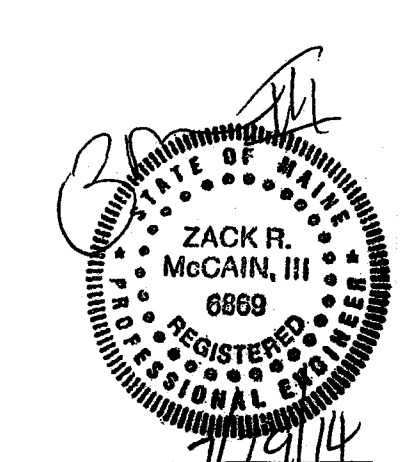
Building Code/Edition:	IBC 09	Snow Loads:	
Building Size:		Width (ft.)	110
		Length (ft.)	101
		Back Side Eave Height (ft.)	18.71
		Front Side Eave Height (ft.)	21
		Back Side Roof Slope	0.25:12
		Front Side Roof Slope	
Roof Dead, Collateral, & Live Loads:		Seismic Loads:	
Dead Load	3.00 psf	Seismic Importance (I _e)	1.00
Collateral Load	3.00 psf	Seismic Design Category (A/B/C/D)	C
Live Load	20.00 psf	Site Class-Type	E
Live Load Reduction Taken	No	Seismic Response Coeff. (S _{ds})	0.41
Wind Loads:		Seismic Response Coeff. (S _{d1})	0.18
Basic Wind Speed (3 Second Gust)	94 mph	Response Modification (R _f)	3.00
Wind Exposure	B	Response Modification (R _m)	3.00
Building Enclosure (D/C/P)	Closed	Design Base Shear (V) = Longit.	25.75 kips
Wind Importance Factor (I _w)	1.00	Design Base Shear (V) = Transv.	22.61 kips
Internal Pressure Coeff. (GCp)	0.18	Analysis Procedure: Equivalent Lateral Force	
		Auxiliary Load(s):	None

Acronyms:

AUXx = Auxiliary Load - Case x
 C= Closed
 CL= Collateral Load
 DL= Dead Load
 FxUNB_LL= Unbalanced Live Load for Frame IDx
 LL= Max. of (Live or Snow)
 SEIS= Seismic
 LLR= Live Load Unbalanced
 LnWindL= Longitudinal Wind Load - Left
 LnWindR= Longitudinal Wind Load - Right
 mph= miles per hour

D= Open
 BF= Braced Frame
 MF= Moment Frame
 P= Partially Enclosed
 psf= pounds per square foot
 WL= Wind Left - Case x
 WP= Wind Pressure
 WRx= Wind Right - Case x
 WS= Wind Suction

- Loading Conditions are as follows:
- Dead+Collateral+Snow
 - Dead+Collateral+Snow+Slide_Snow
 - Dead+Collateral+0.75Snow+0.75Wind_Right1+0.75Snow_Drift+0.75Floor_Live
 - 0.6Dead+Wind_Left2
 - 0.6Dead+Wind_Right2
 - 0.6Dead+Wind_Long1+LWIND1_L2E
 - 0.6Dead+Wind_Long1+LWIND1_R2E
 - Dead+Collateral+Snow/2+FIPAT_SL_1
 - Dead+Collateral+Snow/2+FIPAT_SL_2
 - 0.6Dead+Wind_Left1+Wind_Suction
 - 0.6Dead+Wind_Pressure+Wind_Long1
 - 0.6Dead+Wind_Left2+Wind_Suction
 - 0.6Dead+Wind_Right2+Wind_Suction
 - 0.6Dead+Wind_Right1+Wind_Suction



REV.	DESCRIPTION	DATE	DRAFT	ENG.
3				
Δ	REVISED SEISMIC CATEGORY FROM 'B' TO 'C'	8/5/14	JRB	ZRM
Δ	REVISED PROJECT ADDRESS AND SITE CLASS TYPE FROM 'D' TO 'E'	7/29/14	TMZ	ZRM
Δ	INITIAL DRAWING: RELEASED FOR CONSTRUCTION			CURRENT REVISION: 2
PACKAGE STEEL SYSTEMS, INC.		Biskup Construction Inc.		
PROJECT	Holmes #3	ANCHOR_BOLT REACTIONS & NOTES (SECTION A)		
ID	1407-012	DESIGN: ZRM	DESIGN CHECK: ZRM	
PROJECT	421 Warren Ave.	DRAFT: TMZ	DRAFT CHECK: TMZ	
ADDRESS	Portland, ME 04103	DATE: 7/23/14	SCALE: NONE	SHEET: ABLT-3