

**GENERAL**

1. ALL WORK SHALL CONFORM TO THE REQUIREMENTS OF ALL APPLICABLE STATE AND LOCAL CODES, INCLUDING BUT NOT LIMITED TO:  
 2009 INTERNATIONAL BUILDING CODE  
 ANSI/ASCE 7-05  
 ACI 318-05 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE"  
 ACI 301 "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS"  
 AISC STEEL CONSTRUCTION MANUAL

ANY DISCREPANCIES BETWEEN THE ABOVE LISTED CODES AND THE CONSTRUCTION DOCUMENTS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER FOR CLARIFICATION BEFORE PROCEEDING WITH AFFECTED WORK.

2. ALL WORK SHALL BE PERFORMED BY PERSONS QUALIFIED IN THEIR TRADE AND LICENSED TO PRACTICE SUCH TRADE IN THE STATE IN WHICH THE PROJECT IS LOCATED.

3. THESE DRAWINGS SHALL BE USED IN CONJUNCTION WITH ANY ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS IN ADDITION TO SPECIFICATIONS AND ANY SHOP DRAWINGS PROVIDED BY SUBCONTRACTORS AND SUPPLIERS.

4. ALL DIMENSIONS, ELEVATIONS, AND CONDITIONS SHALL BE VERIFIED IN THE FIELD BY THE GENERAL CONTRACTOR (G.C.) AND ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER FOR CLARIFICATION BEFORE PROCEEDING WITH THE AFFECTED PART OF WORK.

5. UNLESS OTHERWISE NOTED, DETAILS, SECTIONS, AND NOTES SHOWN ON THESE DRAWINGS SHALL BE CONSIDERED TYPICAL FOR ALL SIMILAR DETAILS.

6. THESE DRAWINGS DO NOT SHOW SIZE, LOCATION, OR TYPE OF OPENINGS IN THE FOUNDATION SYSTEM FOR ELECTRICAL, PLUMBING, OR MECHANICAL EQUIPMENT. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR LOCATING THESE ITEMS.

7. ALL SHOP DRAWINGS PROVIDED BY OTHERS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW PRIOR TO THE FABRICATION OF MATERIAL OR THE PURCHASE OF NON-RETURNABLE STOCK. DIMENSIONAL REVIEW IS THE CONTRACTOR'S RESPONSIBILITY.

8. ANY AND ALL TEMPORARY BRACING OR SHORING WHICH IS NEEDED TO HOLD THE STRUCTURE IN A SAFE AND STABLE POSITION UNTIL THE BUILDING IS COMPLETE, IS SOLELY THE RESPONSIBILITY OF THE CONTRACTOR. CONSULT INDEPENDENT ENGINEER IF DESIGN ASSISTANCE OR REVIEW IS NEEDED.

9. THE BUILDING PERMIT APPLICANT (e.g. OWNER, CONTRACTOR) MUST PROVIDE SPECIAL INSPECTIONS PER THE REQUIREMENTS OF CHAPTER 17 OF THE 2009 INTERNATIONAL BUILDING CODE AND FURNISH INSPECTION REPORTS TO THE CODE OFFICIAL AND TO THE ENGINEER OF RECORD. THE TESTING/INSPECTION AGENCY(S) MUST BE APPROVED BY THE ENGINEER OF RECORD.

**DESIGN LOADS**

1. G.C. SHALL BE RESPONSIBLE FOR VERIFYING SUPPLIED BUILDING HAS BEEN DESIGNED BY MANUFACTURER IN ACCORDANCE WITH IBC 2009 TO CARRY ALL THE DEAD LOADS OF THE VARIOUS STRUCTURAL, ARCHITECTURAL, MECHANICAL, AND OTHER SYSTEMS AND THE FOLLOWING MINIMUM LIVE LOADS:

MEZZANINE	100 PSF	(+ CLIMBING WALL POINT LOADS)
BASIC GROUND SNOW LOAD	60 PSF	(UP TO EL. 500')
WIND SPEED AND EXPOSURE SEISMIC	100 MPH, Sds=0.32, Sdi=0.123	EXPOSURE "C"
SITE CLASS	D	

2. JSN ASSOCIATES, INC. HAS PROVIDED FOUNDATION DESIGN ONLY AND IS NOT RESPONSIBLE FOR THE METAL BUILDING DESIGN ITSELF. BUILDING DRAWINGS, STAMPED BY A MAINE REGISTERED ENGINEER, SHALL BE PROVIDED TO THE LOCAL BUILDING DEPARTMENT BY THE BUILDING MANUFACTURER.

**METAL BUILDING**

1. THIS FOUNDATION IS DESIGNED TO CARRY LOADS AND REACTIONS AS INDICATED ON A BUILDING DESIGN SUMMARY FOR EVOLUTION ROCK, PORTLAND, ME FOR A CECO BUILDING SYSTEMS PRE-MANUFACTURED METAL BUILDING.

G.C. SHALL BE RESPONSIBLE FOR VERIFYING SUPPLIED BUILDING HAS BEEN DESIGNED BY MANUFACTURER IN ACCORDANCE WITH IBC 2009 CODE TO CARRY ALL THE DEAD LOADS OF THE VARIOUS STRUCTURAL, ARCHITECTURAL, MECHANICAL, AND OTHER SYSTEMS AND THE FOLLOWING MINIMUM LIVE LOADS:

BASIC GROUND SNOW LOAD	60 PSF
WIND SPEED AND EXPOSURE SEISMIC	100 MPH, EXPOSURE "C", Ss = 0.32 S1 = 0.123

2. JSN ASSOCIATES, INC. HAS PROVIDED FOUNDATION DESIGN ONLY AND IS NOT RESPONSIBLE FOR THE METAL BUILDING DESIGN ITSELF. BUILDING DRAWINGS, STAMPED BY A MAINE REGISTERED ENGINEER, SHALL BE PROVIDED TO THE LOCAL BUILDING DEPARTMENT BY CECO.

3. METAL BUILDING MANUFACTURER SHALL BE A MEMBER OF MBMA AND AN AISC-CERTIFIED MANUFACTURER OF METAL BUILDING SYSTEMS AND COMPONENTS. METAL BUILDING MANUFACTURER SHALL HAVE A MINIMUM OF FIVE (5) CONSECUTIVE YEARS IN BUSINESS OF MANUFACTURING METAL BUILDINGS AND COMPONENTS WITHIN THE U.S.

4. LATERAL DRIFT OF METAL BUILDING SHALL BE LIMITED TO H/50. VERTICAL DEFLECTION OF PURLINS UNDER LIVE LOAD SHALL BE LIMITED TO L/240. HORIZONTAL DEFLECTION OF GIRTS UNDER LIVE LOAD SHALL BE LIMITED TO L/180 WHERE METAL SIDING IS USED (LIMIT TO L/240 FOR ALL OTHER SIDINGS).

5. METAL BUILDING ENGINEER SHALL MAINTAIN PROFESSIONAL LIABILITY INSURANCE WITH A MINIMUM ANNUAL AND AGGREGATE LIMIT OF \$1,000,000. G.C. SHALL VERIFY INSURANCE IS CARRIED.

6. MINIMUM ROOF SNOW LOAD RECOMMENDED BY THIS OFFICE IS 49 POUNDS PER SQUARE FOOT.

**CAST-IN-PLACE-CONCRETE**

1. ALL WORK SHALL CONFORM TO "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" (ACI 318-05) AND "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" (ACI 301)

2. INTERIOR SLABS ON GRADE TO BE OF THICKNESS SHOWN ON DRAWINGS WITH WELDED WIRE FABRIC REINFORCING.

3. PROVIDE 10-MIL POLYETHYLENE MOISTURE VAPOR RETARDER DIRECTLY BELOW ALL INTERIOR SLABS ON GRADE. OVERLAP SEAMS MINIMUM 6" AND TAPE AS REQUIRED TO MAINTAIN POSITION.

4. ALL FOOTINGS ARE TO REST ON UNDISTURBED SOIL OR CLEAN GRANULAR FILL COMPACTED IN LAYERS OF 12" OR LESS TO 95% COMPACTION.

5. MINIMUM CONCRETE PROTECTION FOR REINFORCING STEEL SHALL BE AS FOLLOWS:  
 CONCRETE CAST AGAINST EARTH: 3 INCHES  
 FORMED CONCRETE EXPOSED TO EARTH OR WEATHER:  
 1-1/2 INCHES FOR #5 BARS AND SMALLER  
 2 INCHES FOR #6 BARS AND GREATER

6. CALCIUM CHLORIDE IS PROHIBITED IN ANY CONCRETE MIX.

7. CONCRETE SHALL BE ADEQUATELY PROTECTED FROM HOT OR COLD WEATHER AS REQUIRED BY ACI PUBLICATIONS 305 AND 306, RESPECTIVELY.

8. ALL CONCRETE FOR WALLS, FOOTINGS, AND SLABS SHALL ATTAIN A MINIMUM ULTIMATE COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS (U.N.O.). CYLINDERS SHALL BE TAKEN AND TESTED IN ACCORDANCE WITH ACI RECOMMENDATIONS.

9. SLAB CONTROL JOINTS, WHERE SHOWN, SHALL BE SAW CUT AND SHALL BE CUT IMMEDIATELY AFTER FINISHING. JOINTS SHALL BE AT MINIMUM 1/4 OF THE THICKNESS OF THE SLAB.

10. WALL CONTROL JOINTS SHALL BE PLACED AS SHOWN ON DRAWINGS OR AT A MAXIMUM OF 40 FEET ON CENTER.

11. BACKFILL BOTH SIDES OF THE FOUNDATION WALL SIMULTANEOUSLY TO THE MAXIMUM HEIGHT POSSIBLE.

12. ALL CONCRETE SHALL BE CURED BY AN APPROVED METHOD AS PRESCRIBED BY ACI.

13. MAXIMUM WATER TO CEMENT RATIO SHALL BE 0.5 FOR 3000 PSI CONCRETE AND 0.45 FOR 4000 PSI CONCRETE MIXES WITH MID-RANGE WATER REDUCERS (MRWR) USED. W/C RATIO FOR 3000 PSI CONCRETE IN FOOTINGS MAY BE 0.53 WITHOUT THE USE OF MID-RANGE WATER REDUCERS. MINIMUM CEMENT QUANTITIES SHALL BE 517 LB./YD FOR 3000 PSI CONCRETE AND 611 LB./YD FOR 4000 PSI CONCRETE.

14. MAXIMUM CONCRETE SLUMP SHALL BE FOUR INCHES WITHOUT MRWR AND 6 INCHES WITH MRWR. MRWR MUST BE USED IN ALL CONCRETE EXCEPT FOOTINGS.

**REINFORCING STEEL**

1. ALL REINFORCING, EXCEPT AS NOTED, SHALL BE DEFORMED BARS CONFORMING TO ASTM A615 GRADE 60.

2. WELDED WIRE FABRIC REINFORCEMENT SHALL CONFORM TO THE REQUIREMENTS OF ASTM A185. USE FLAT SHEETS ONLY.

3. ALL REINFORCING SHALL BE DETAILED IN ACCORDANCE WITH THE LATEST ACI DETAILING MANUAL.

4. WHERE CONTINUOUS BARS ARE CALLED FOR, INDICATED, REQUIRED, THEY SHALL RUN CONTINUOUSLY AROUND CORNERS, LAPPED AT NECESSARY SPLICES, SPLICES STAGGERED AND HOOKED AT DISCONTINUOUS ENDS. LAP LENGTHS SHALL BE AS SHOWN OR NOTED ON THE DRAWINGS. IF LAP/SPLICE LENGTHS ARE NOT INDICATED, FOLLOW ACI STANDARDS.

**SLAB-ON-GRADE CONTROL JOINTS**

1. CONTROL JOINTS IN CONCRETE SLABS ARE GENERALLY SPACED IN A MANNER TO CONTROL CRACK LOCATIONS OCCURRING DUE TO CURING SHRINKAGE AND THERMAL MOVEMENT OF CONCRETE. WELDED WIRE FABRIC DOES NOT INHERIT CRACKING BUT HOLDS CONCRETE TIGHTLY TOGETHER AFTER CRACKING HAS OCCURRED. IN ORDER TO BETTER CONTROL RANDOM CRACKING OF CONCRETE THE FOLLOWING MEASURES ARE RECOMMENDED:

- A) SUPPLY A WELL COMPACTED AND CONSISTENT SUBGRADE.
  - B) LIMIT WATER VOLUME IN CONCRETE USING A STIFFER MIX.
  - C) SUPPLY ADEQUATE CURING MEASURES. WET CURE OR USE CURING SEALERS.
  - D) LIMIT JOINT SPACING TO 2 TIMES SLAB THICKNESS IN FEET.
2. SLAB CURLING IS ALSO A PROBLEM WHICH HAS BECOME MORE PREVALENT WITH MODERN CONCRETE MIXES WHICH HAVE HIGHER STRENGTHS. THE FOLLOWING MEASURES IN ADDITION TO THOSE STATED ABOVE ARE RECOMMENDED TO LIMIT CURLING OF CONCRETE SLABS-ON-GRADE:
- A) CURE THE SLAB PROPERLY.
  - B) USE HIGHER QUANTITY OF COARSE AGGREGATES IN THE MIX.
  - C) USE A LOWER AMOUNT OF CEMENT.

**SOIL BEARING**

1. ALL FOOTINGS SHALL BE CARRIED DOWN TO REST ON UNDISTURBED SOIL OR SHALL BEAR ON STRUCTURAL FILL COMPACTED IN 12" LAYERS TO 95% COMPACTION. THE UNDERLYING SOILS AND THE STRUCTURAL FILL SHALL HAVE A MINIMUM SAFE LOAD BEARING CAPACITY OF 3000 PSF.

2. REMOVE ALL EXISTING TOPSOIL, PAVEMENT, ORGANIC MATERIALS, OR OTHER SOIL THAT APPEAR TO BE UNSUITABLE PRIOR TO PREPARING THE FOOTING GRADE.

3. IF ANY ADVERSE SOIL CONDITIONS ARE ENCOUNTERED WHICH EXTEND BELOW FOOTING LEVEL, SUCH AS THOSE LISTED ABOVE, THE GENERAL CONTRACTOR SHALL CONTACT THE ENGINEER IMMEDIATELY FOR DETERMINATION OF HOW TO REMEDY THE CONDITION BEFORE CONTINUATION OF THE WORK.

4. NO FOOTINGS SHALL BE PLACED IN WATER OR ON FROZEN GROUND. ALL EXTERIOR CONSTRUCTION SHALL BE CARRIED DOWN TO A MINIMUM OF FOUR (4) FEET BELOW FINISHED, ADJACENT EXTERIOR GRADE.

5. REFER TO GEOTECHNICAL ENGINEER'S REPORT FOR ALL INFORMATION REGARDING SOILS, EXCAVATION, BACKFILL, SUBGRADE PREPARATION, ETC. SEE REPORT BY KMM GEOTECHNICAL CONSULTANTS, LLC. DATED NOVEMBER 4, 2013.

NOTE - ALL FILL AND ORGANICS MUST BE REMOVED FROM SITE AS NOTED IN GEOTECHNICAL REPORT. IT IS NOTED IN THE REPORT THAT THERE IS APPROXIMATELY 5' - 9' OF FILL REQUIRING REMOVAL.

**SCHEDULE OF SPECIAL INSPECTIONS**

PROJECT: EVOLUTION ROCK AND FITNESS  
 LOCATION: PORTLAND, MAINE

STRUCTURAL ENGINEER OF RECORD (SER): JEFFREY S. NAWROCKI, PE (FOR FOUNDATION ONLY)

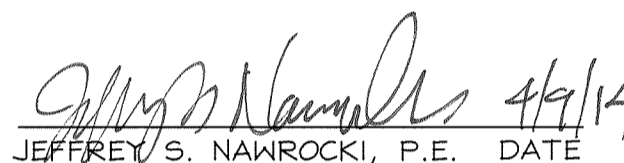
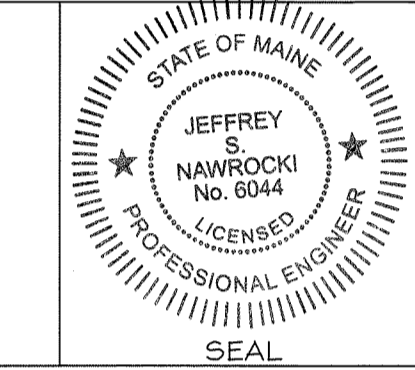
THIS STATEMENT OF SPECIAL INSPECTIONS IS SUBMITTED AS A CONDITION FOR PERMIT ISSUANCE IN ACCORDANCE WITH THE SPECIAL INSPECTION REQUIREMENTS OF THE 2009 INTERNATIONAL BUILDING CODE. IT INCLUDES A SCHEDULE OF SPECIAL INSPECTION SERVICES APPLICABLE TO THIS PROJECT AS WELL AS THE NAME OF SPECIAL INSPECTORS AND THE IDENTITY OF OTHER APPROVED AGENCIES INTENDED TO BE RETAINED FOR CONDUCTING THESE SERVICES.

THE SPECIAL INSPECTOR SHALL KEEP RECORDS OF ALL INSPECTIONS AND SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, STRUCTURAL ENGINEER AND ARCHITECT OF RECORD. DISCOVERED DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR.

A FINAL REPORT OF SPECIAL INSPECTIONS BY THE SPECIAL INSPECTOR(S) DOCUMENTING COMPLETION OF ALL REQUIRED SPECIAL INSPECTIONS AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED PRIOR TO ISSUANCE OF A CERTIFICATE OF USE AND OCCUPANCY.

THE SPECIAL INSPECTOR, WHO IS GENERALLY EMPLOYED BY THE PRIMARY TESTING AGENCY, MAY USE VARIOUS INSPECTORS WHO ARE FAMILIAR WITH EACH CATEGORY OF WORK. IF SPECIAL INSPECTIONS ARE ALSO PERFORMED BY AGENTS WHO ARE NOT EMPLOYED BY PRIMARY TESTING AGENCY, EACH OF THESE ADDITIONAL SPECIAL INSPECTORS SHALL ISSUE A FINAL REPORT FOR THEIR CATEGORY OF INSPECTION. ONLY AFTER THE FINAL REPORT(S) HAS(HAVE) BEEN ISSUED BY THE SPECIAL INSPECTOR(S) CAN THE ARCHITECT AND EOR ISSUE FINAL AFFIDAVITS FOR THE PROJECT COMPLETION.

JOB SITE SAFETY AND MEANS AND METHODS OF CONSTRUCTION ARE SOLELY THE RESPONSIBILITY OF THE CONTRACTOR.

PREPARED BY:  JEFFREY S. NAWROCKI, P.E. DATE 4/9/14	 SEAL
OWNER'S AUTHORIZATION:	BUILDING OFFICIAL'S AUTHORIZATION:
SIGNATURE _____ DATE _____	SIGNATURE _____ DATE _____

**SCHEDULE OF SPECIAL INSPECTION SERVICES**

THE FOLLOWING TABLES COMPRISE THE REQUIRED SCHEDULE OF SPECIAL INSPECTIONS FOR THIS PROJECT. THE CONSTRUCTION DIVISIONS WHICH REQUIRE SPECIAL INSPECTIONS FOR THIS PROJECT ARE AS FOLLOWS:

SOILS AND FOUNDATIONS  
 CAST-IN-PLACE CONCRETE  
 STRUCTURAL STEEL (MEZZ)

INSPECTION AGENTS	FIRM	ADDRESS
1. SPECIAL INSPECTOR*	TBD	
2. TESTING LABORATORY	TBD	
3. STRUCTURAL ENGINEER	JSN ASSOCIATES, INC.	ONE AUTUMN STREET PORTSMOUTH, NH 03801 (603) 433-8639

NOTE: THE INSPECTION AND TESTING AGENT SHALL BE ENGAGED BY THE OWNER OR THE OWNER'S AGENT, AND NOT BY THE CONTRACTOR OR SUBCONTRACTOR WHOSE WORK IS TO BE INSPECTED OR TESTED. ANY CONFLICT OF INTEREST MUST BE DISCLOSED TO THE BUILDING OFFICIAL, PRIOR TO COMMENCING WORK.

\* THE SPECIAL INSPECTOR IS GENERALLY AN EMPLOYEE OF THE TESTING AND GEOTECHNICAL COMPANY.  
 SEISMIC DESIGN CATEGORY: C

BASIC WIND SPEED: 100 MPH

WIND EXPOSURE CATEGORY: C

**QUALIFICATIONS OF INSPECTORS AND TESTING TECHNICIANS**

THE QUALIFICATIONS OF ALL PERSONNEL PERFORMING SPECIAL INSPECTION ACTIVITIES ARE SUBJECT TO THE APPROVAL OF THE BUILDING OFFICIAL. THE CREDENTIALS OF ALL INSPECTORS AND TESTING TECHNICIANS SHALL BE PROVIDED IF REQUESTED.

IT IS RECOMMENDED THAT THE PERSON ADMINISTERING THE SPECIAL INSPECTIONS PROGRAM BE A PROFESSIONAL ENGINEER EXPERIENCED IN THE DESIGN OF BUILDINGS.

**SOILS AND FOUNDATIONS**

ITEM	AGENT NO.	SCOPE
1. SHALLOW FOUNDATIONS	1	VERIFY THAT UNSUITABLE BEARING MATERIALS ARE REMOVED. VERIFY THE SOIL LOAD-BEARING CAPACITY COINCIDES WITH THAT IDENTIFIED IN THE CONSTRUCTION DOCUMENTS.
2. CONTROLLED STRUCTURAL FILL	1	INSPECT COMPACTED FILL OPERATIONS TO VERIFY THE FILL MATERIAL, LIFT HEIGHTS, AND LEVEL OF COMPACTION ARE IN CONFORMANCE WITH THE REQUIREMENTS OF CONSTRUCTION.
3. DEEP FOUNDATIONS	1	N/A
4. OTHER	1	N/A

**CAST-IN-PLACE CONCRETE**

ITEM	AGENT NO.	SCOPE
1. MIX DESIGN	3	REVIEW FOR COMPLIANCE WITH CONSTRUCTION DOCUMENTS.
2. MATERIAL CERTIFICATION	3	REVIEW FOR COMPLIANCE WITH CONSTRUCTION DOCUMENTS.
3. REINFORCEMENT INSTALLATION	1,3	(1) REVIEW THE INSTALLATION OF THE REINFORCING STEEL FOR COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS AND THE APPROVED SHOP DRAWINGS. REVIEW FOR 25% OF FOOTINGS, 50% OF FROST WALLS, 100% OF RETAINING WALLS, INTERIOR FOOTINGS, AND PIERS. (3) RANDOM REVIEW OF CONSTRUCTION PROCEDURE.
4. FORMWORK GEOMETRY	1	REVIEW GEOMETRY FOR COMPLIANCE WITH THE STRUCTURAL CONSTRUCTION DOCUMENTS. CONDUCT REVIEW WHEN REINFORCING STEEL INSTALLATION IS BEING REVIEWED. REVIEW ANCHOR BOLT PLACEMENT AND VERIFY USE OF TEMPLATES.
5. CONCRETE PLACEMENT	1	INSPECT THE PLACEMENT OF CONCRETE FOR CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS. TEST SLUMP AND TEMPERATURE OF EACH BATCH. TEST AIR CONTENT WHEN COMPRESSIVE STRENGTH TEST SPECIMENS ARE MOLDED.
6. EVALUATION OF CONCRETE STRENGTH	1	OBTAIN ONE SET OF (4) STANDARD CYLINDERS FOR EACH COMPRESSIVE STRENGTH TEST. TEST ONE SPECIMEN AT 7-DAYS, (2) AT 28-DAYS, AND RETAIN ONE IN RESERVE FOR LATER TESTING IF REQUIRED.  IN COLD WEATHER, TEST CYLINDERS SHALL BE FIELD CURED. ADDITIONAL CYLINDERS SHALL BE TAKEN AND LABORATORY CURED PER ACI REQUIREMENTS.  TESTING FREQUENCY: (1) COMPRESSIVE STRENGTH TEST SHOULD BE PERFORMED FOR EACH DAY'S POUR EXCEEDING 5 CU. YDS. AND (1) ADD'L SET FOR EACH 50 CU. YDS. MORE THAN THE FIRST 25 CU. YDS.
7. CURING AND PLACEMENT	1	VERIFY THE CONCRETE IS ADEQUATELY PROTECTED UNDER HOT AND COLD WEATHER CONDITIONS AS INDICATED IN THE CONCRETE SPECIFICATIONS. VERIFY THAT SLABS ARE CURED IN ACCORDANCE WITH ACI RECOMMENDED STANDARD PROCEDURES.

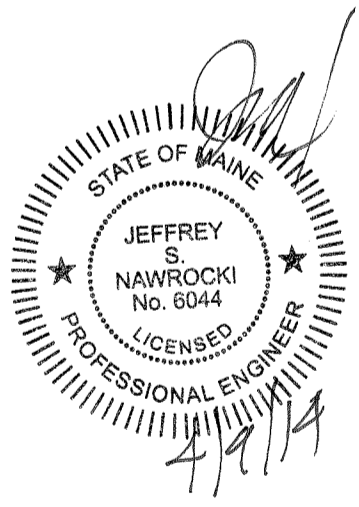
**STRUCTURAL STEEL (MEZZANINE)**

ITEM	AGENT NO.	SCOPE
1. FABRICATOR CERTIFICATION/QUALITY CONTROL PROCEDURES	1	VERIFY THAT THE FABRICATOR MAINTAINS DETAILED FABRICATION AND QUALITY CONTROL PROCEDURES WHICH CONFORM TO THE REQUIREMENTS OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION'S QUALITY CERTIFICATION PROGRAM. AISC CERTIFICATION SATISFIES THIS.
2. MATERIAL CERTIFICATION	1	REVIEW MILL CERTIFICATES FOR PLATES AND SHAPES. REVIEW BOLT MANUFACTURER'S CERTIFICATES OF COMPLIANCE FOR HIGH-STRENGTH BOLTS. REVIEW WELD MANUFACTURER'S CERTIFICATE OF COMPLIANCE FOR WELD FILLER MATERIAL.
3. BOLTING	1	INSPECT INSTALLATION OF HIGH-STRENGTH BOLTS FOR CONFORMANCE WITH THE *SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS" BY THE RESEARCH COUNCIL ON STRUCTURAL BOLTS, AND THE CONSTRUCTION DOCUMENTS.
4. WELDING	1	PERFORM VISUAL INSPECTION OF ALL WELDS IN ACCORDANCE WITH AWS D11. SUBMIT WELDER QUALIFICATION STATEMENTS. ADDITIONALLY, THE TESTING AGENCY (TO BE APPROVED BY JSN ASSOCIATES, INC.) MUST PERFORM A VISUAL INSPECTION OF ALL FIELD WELDS. MULTI PASS WELDS OR WELDS GREATER THAN 5/16" MUST BE SPOT TESTED AT A RATE OF ONE TEST PER MEMBER USING THE MAGNETIC PARTICLE METHOD. ONE HUNDRED PERCENT (100%) OF ALL FIELD AND SHOP FULL PENETRATION WELDS MUST BE TESTED USING THE ULTRASONIC METHOD.
5. SHEAR CONNECTORS	1	INSPECT SIZE, NUMBER, POSITIONING AND WELDING OF SHEAR CONNECTORS. INSPECT SUDS FOR FULL 360 DEGREE FLASH. RING TEST ALL SHEAR CONNECTORS WITH A 3 LB HAMMER. BEND TEST ALL QUESTIONABLE STUDS TO 15 DEGREES.
6. STRUCTURAL DETAILS	1, 3	(1) VERIFY THAT THE GENERAL GEOMETRY OF THE ERECTED STEEL FRAME CONFORMS TO THE CONSTRUCTION DOCUMENTS AND APPROVED SHOP DRAWINGS. (3) RANDOM REVIEW.
7. METAL DECK	1	INSPECT WELDING AND SIDE-LAP FASTENING OF METAL ROOF AND FLOOR DECK. VERIFY SIZE AND QUANTITY OF FASTENERS FOR CONFORMANCE WITH CONSTRUCTION DOCUMENTS.  FREQUENCY: 100% OF FASTENING PATTERNS. SPOT CHECK 10% OF ALL WORK FOR SIZE AND TYPE OF FASTENERS.



Evolution Rock & Fitness

65 Warren Ave.  
Portland, Maine



Date: 04/09/14

Scale: As Noted

By: JSN

Revisions

Notes

SN.0