**DOLLARS** 



Management of Construction Since 1993 63 JOHNSON ROAD SANBORNTON, NEW HAMPSHIRE 03269 (603) 286-4419

**器Citizens Bank®** 

54-153-114

11/27/2012

\*\*4,130.00

City of Portland

Four Thousand One Hundred Thirty and 00/100\*

City of Portland

Nilding Permit Fee

MFK ASSOCIATES, INC.

PAY TO THE

ORDER OF

**MEMO** 

City of Portland Date

11/27/2012 Bill

Type Reference

**Building Permit Fee** 

Original Amt. 4,130.00 Balance Due 4,130.00

11/27/2012 Discount Check Amount

10316 Payment

4,130.00

4,130.00

Citizens Bank Checkin Building Permit Fee

4,130.00

### General Building Permit Application

you or the property owner owes real estate or personal property taxes or user charges on any property within the City, payment arrangements must be made before permits of any kind are accepted.

MLD		***************************************	,	
Location/Address of Construction:				
4,798 s.F.	rea	Square Footage of Lot		
	Applicant * <u>n</u>	<u>aust</u> be owner, Lessee or Buyer	*	Telephone:
	Name Sull	WANTIRE OF MAINE, L	اعا	781-982-1550
263A A008 001	Address 41	ACCORD PARK DRIV	15	
	City, State &	Zip NORWELL, MA 0206	1	
Lessee/DBA (If Applicable)	Owner (if di	fferent from Applicant)	Cos	st Of
	Name BRI	XMOR SPEY, LLC	Wo	ork: \$ 411,000.00
Total Square Footage of Proposed Structure/Area  Y 798 S.F.  Tax Assessor's Chart, Block & Lot Chart# Block# Lot#  Abalian Footage of Lot N/A  Applicant *must be owner, Lessee or Buyer* Name Sullivan TIRE of Maine, LLC Address 41 Accord Park Drive  City, State & Zip Norwell, MA 02061				
,			Tot	ral Fee: \$ 4,130.00
		YORK, NY 10170		The second secon
Proposed Specific use:	<u> </u>			
		ves, please name		<del></del>
Project description:				<del>.</del>
ONE STORY AUTOMOTIVE	ie se <del>r</del> ui	ice center Renov	/AT)	hoh
Contractor's name: MFK ASSOC	LATES, 3	THC.		
Address: 63 JOHNSON ROAD	)			
City, State & Zip SANBORN TON,	NHO	3269 603-	28	66-4419 Telephone:
Who should we contact when the permit is re	ady: MICH	WEL KANIK 603	- a:	35-8623 Telephone:
Mailing address:				
Please submit all of the information	outlined o	n the applicable Checkli	st. I	Failure to
do so will result in the	e automatic	denial of your permit.		

In order to be sure the City fully understands the full scope of the project, the Planning and Development Department may request additional information prior to the issuance of a permit. For further information or to download copies of this form and other applications visit the Inspections Division on-line at <a href="https://www.portlandmaine.gov">www.portlandmaine.gov</a>, or stop by the Inspections Division office, room 315 City Hall or call 874-8703.

I hereby certify that I am the Owner of record of the named property, or that the owner of record authorizes the proposed work and that I have been authorized by the owner to make this application as his/her authorized agent. I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in this application is issued, I certify that the Code Official's authorized representative shall have the authority to enter all areas covered by this permit at any reasonable hour to enforce the provisions of the codes applicable to this permit.

		1 1	
Signature:	V/Wr/M	Date: 11/27/12	
	This is not a permit; you may not	commence ANV work until the normit is issue	



# Certificate of Design Application

From Designer:	John T Brennan, AIA, NCARB
Date:	October 23, 2012
Job Name:	One Story Automotive Service Center Renovation
Address of Construction:	1100 Brighton Ave, Portland, ME

#### 2009 International Building Code

Construction project was designed to the building code criteria listed below:

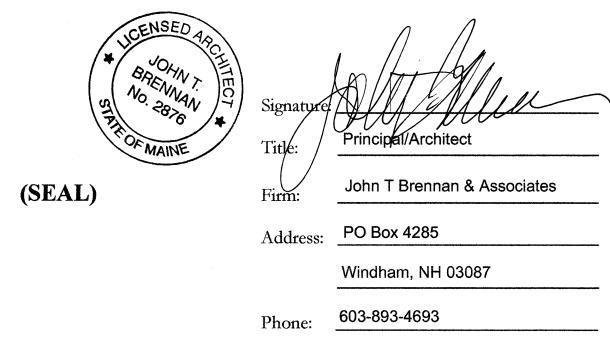
Building Code	e & Year IBC 2009	_ Use Group Classification (s)	S1	
Type of Cons	truction IIB	1 ()		
Will the Structs	ire have a Fire suppression	system in Accordance with Section	on 903.3.1 of the 2	2009 IRC No
	e mixed use? Yes	•		ed (section 302.3) Non-Separated
	rm System? Yes	_Geotechnical/Soils report requir		
Structural Des	sign Calculations		MP and the second secon	Live load reduction
Submitted for all structural members (106.1 – 106.11)				Roof live loads (1603.1.2, 1607.11)
	0		***************************************	Roof snow loads (1603.7.3, 1608)
	on Construction Docum outed floor live loads (7603.11, 1		60	Ground snow load, Pg (1608.2)
Floor Area			46.2	If $P_g > 10$ psf, flat-roof snow load $p_c$
Retail Lob	 by 100	PSF	1.0	If Pg > 10 psf, snow exposure factor, Ce
First Floor		0 PSF	1.0	If $Pg > 10$ psf, snow load importance factor, $I_0$
			1.1	Roof thermal factor, $G(1608.4)$
			_	Sloped roof snowload, <sub>Pr</sub> (1608.4)
Wind loads (1	603.1.4, 1609)		С	Seismic design category (1616.3)
***************************************	Design option utilized (1609.1.1	, 1609.6)		Basic seismic force resisting system (1617.6.2)
100	_Basic wind speed (1809.3)		6.5	Response modification coefficient, R1 and
1.0	_Building category and wind im	portance Factor, h		deflection amplification factor (1617.6.2)
B	table Wind exposure category (1609.	£ 1604.5, 1609.5)‴ 4)	~	Analysis procedure (1616.6, 1617.5)
0.18	_Internal pressure coefficient (ASC)		-	Design base shear (1617.4, 16175.5.1)
12.5	Component and cladding pressure	s (1609.1.1, 1609.6.2.2)	Flood loads (	1803.1.6, 1612)
13.5	_Main force wind pressures (7603.1	1, 1609.6.2.1)	_	Flood Hazard area (1612.3)
Earth design data (1603.1.5, 1614-1623)			Elevation of structure	
	Design option utilized (1614.1)		Other loads	Lievation of structure
1	_Seismic use group ("Category"	)	Other loads	
.37 & .16	_Spectral response coefficients,	SDs & SD1 (1615.1)	***	Concentrated loads (1607.4)
D	Site class (1615.1.5)		-	Partition loads (1607.5)
				Misc. loads (Table 1607.8, 1607.6.1, 1607.7, 1607.12, 1607.13, 1610, 1611, 2404



# Certificate of Design

Date:	November 19, 2012						
From:	John T. Brennan, AIA						
These plans and /	or specifications covering construction work on:						
One Story Automotive Service Building Renovation. Approx 4,798 gross sf.							
		<del></del>					

Have been designed and drawn up by the undersigned, a Maine registered Architect / Engineer according to the **2009 International Building Code** and local amendments.



For more information or to download this form and other permit applications visit the Inspections Division on our website at www.portlandmaine.gov



## Accessibility Building Code Certificate

Address of Project: 1100 Brig	John T Brennan, AIA, NCARB
Address of Project:	1100 Brighton Ave, Portland, ME
Nature of Project:	Renovation of One Story, 4,798 gsf. Automotive Service Building

The technical submissions covering the proposed construction work as described above have been designed in compliance with applicable referenced standards found in the Maine Human Rights Law and Federal Americans with Disability Act. Residential Buildings with 4 units or more must conform to the Federal Fair Housing Accessibility Standards. Please provide proof of compliance if applicable.

SEAL)

Signature

Principal/Architect

Firm:

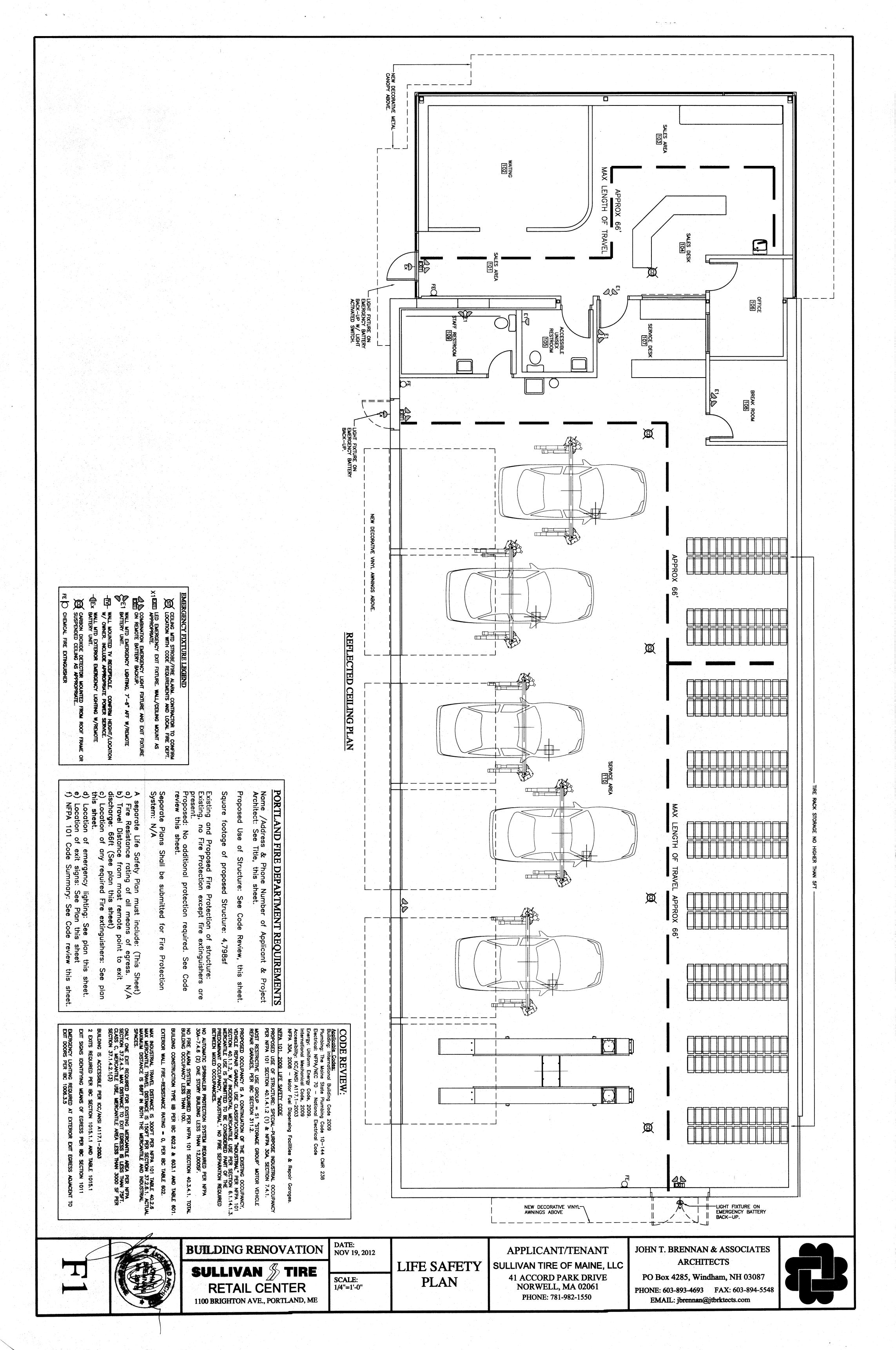
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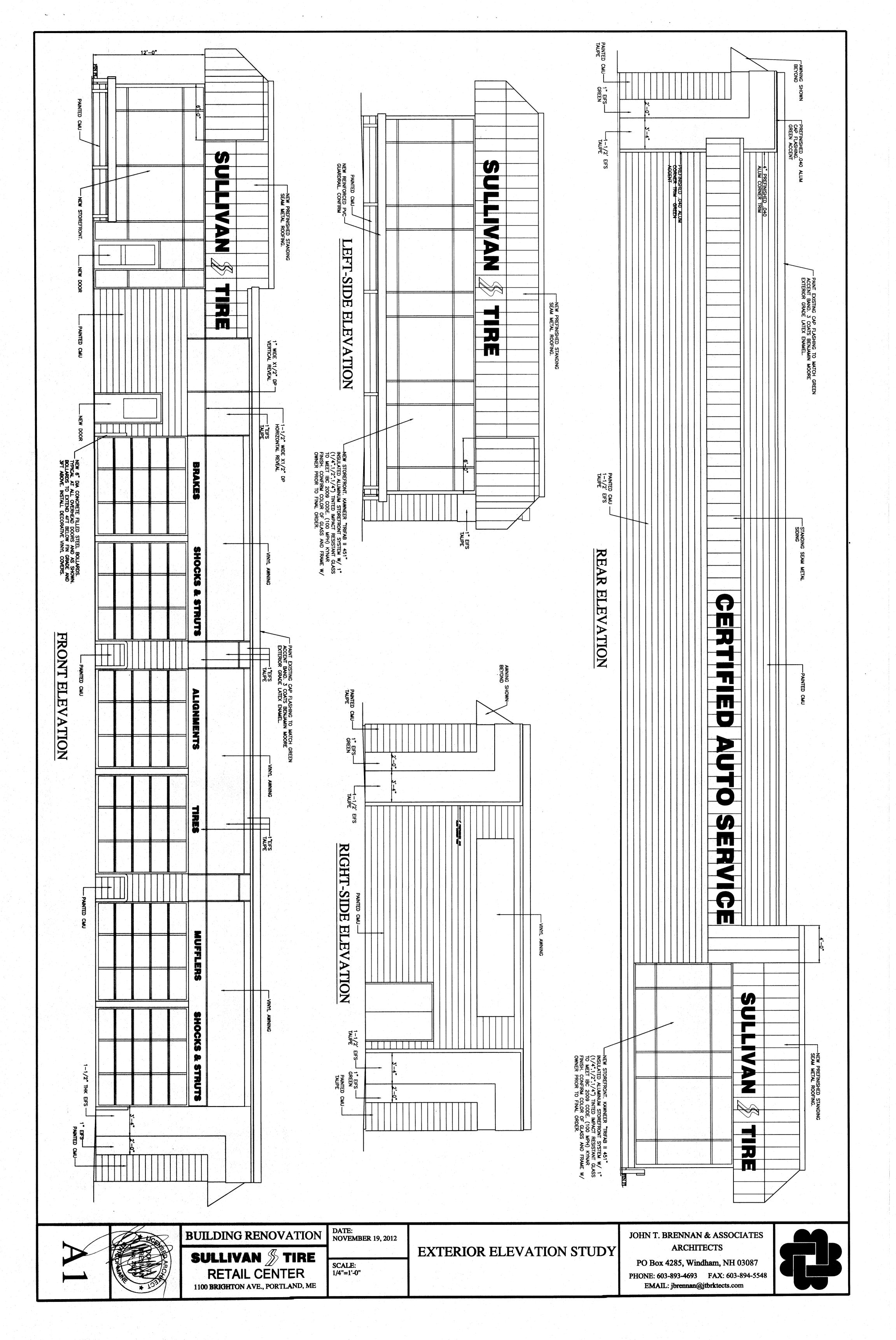
PO Box 4285, Windham, NH 03087

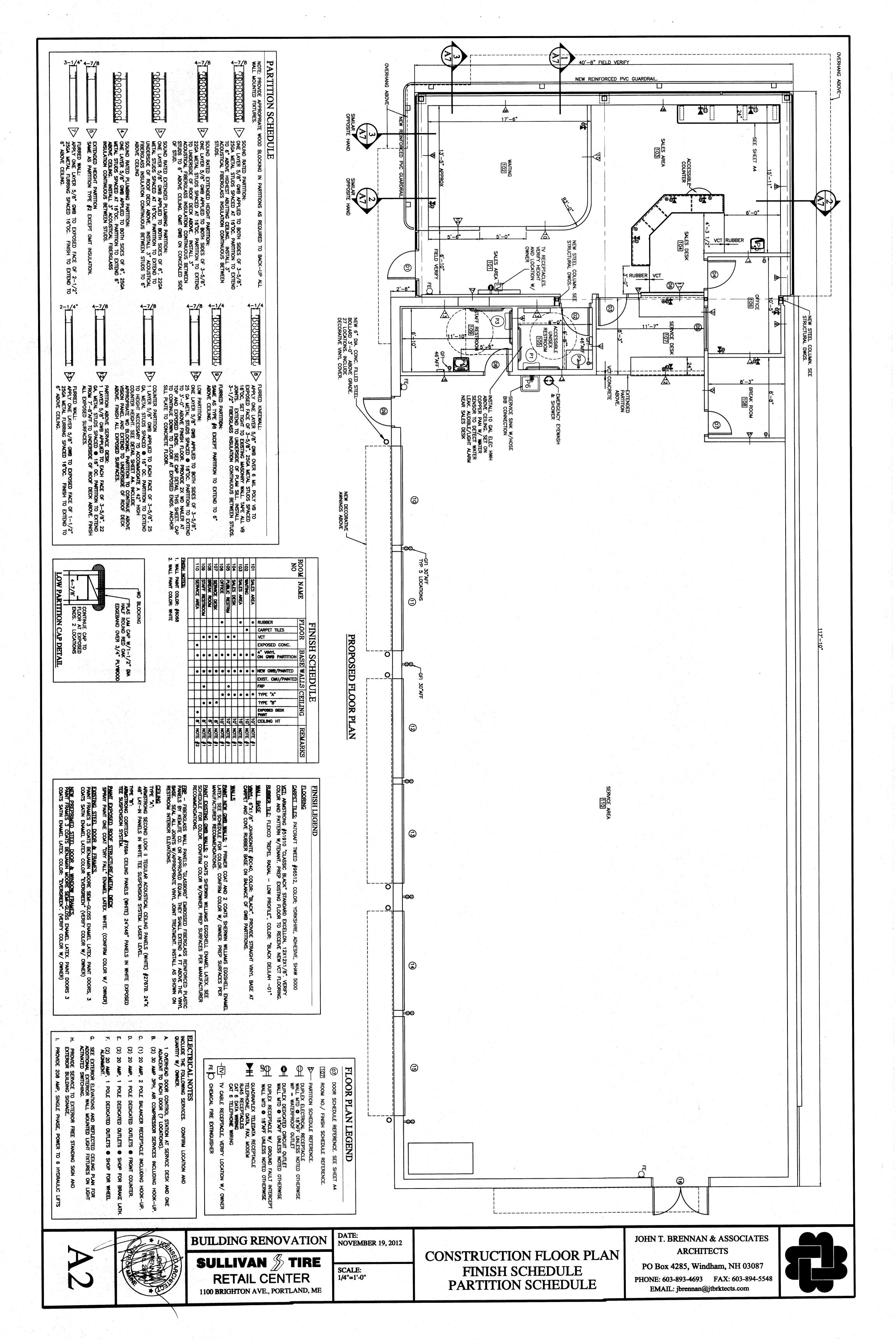
Phone:

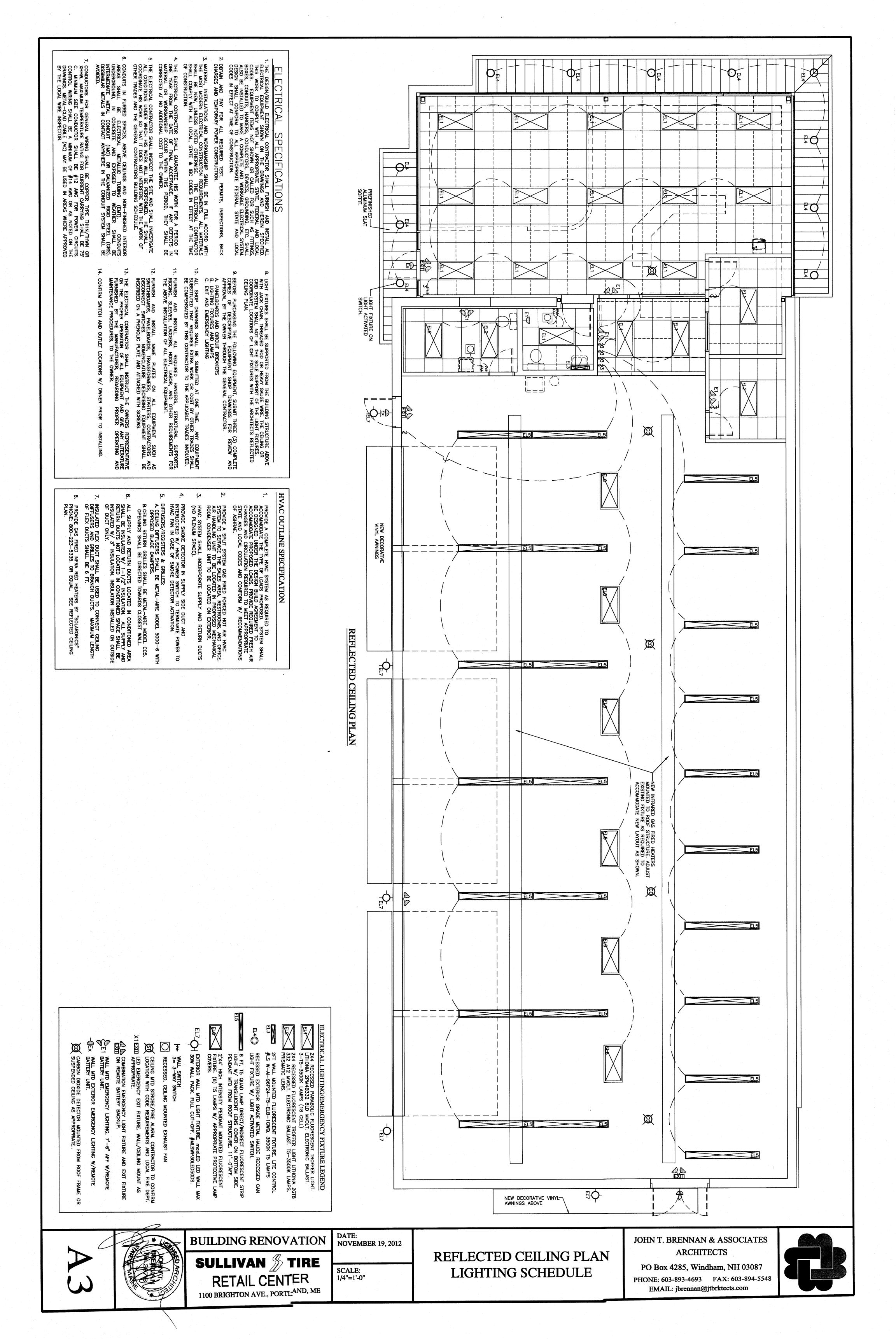
603-893-4693

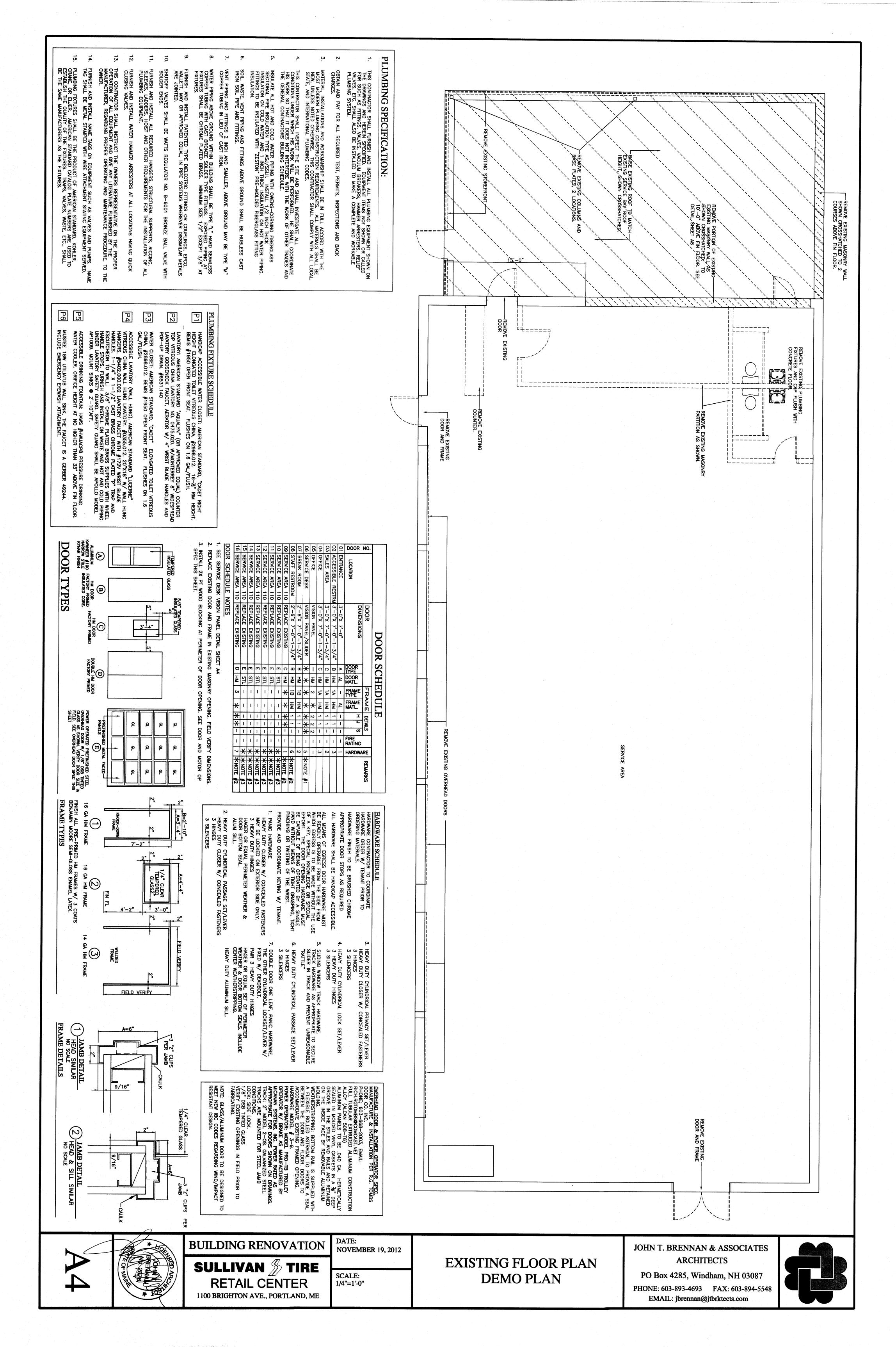
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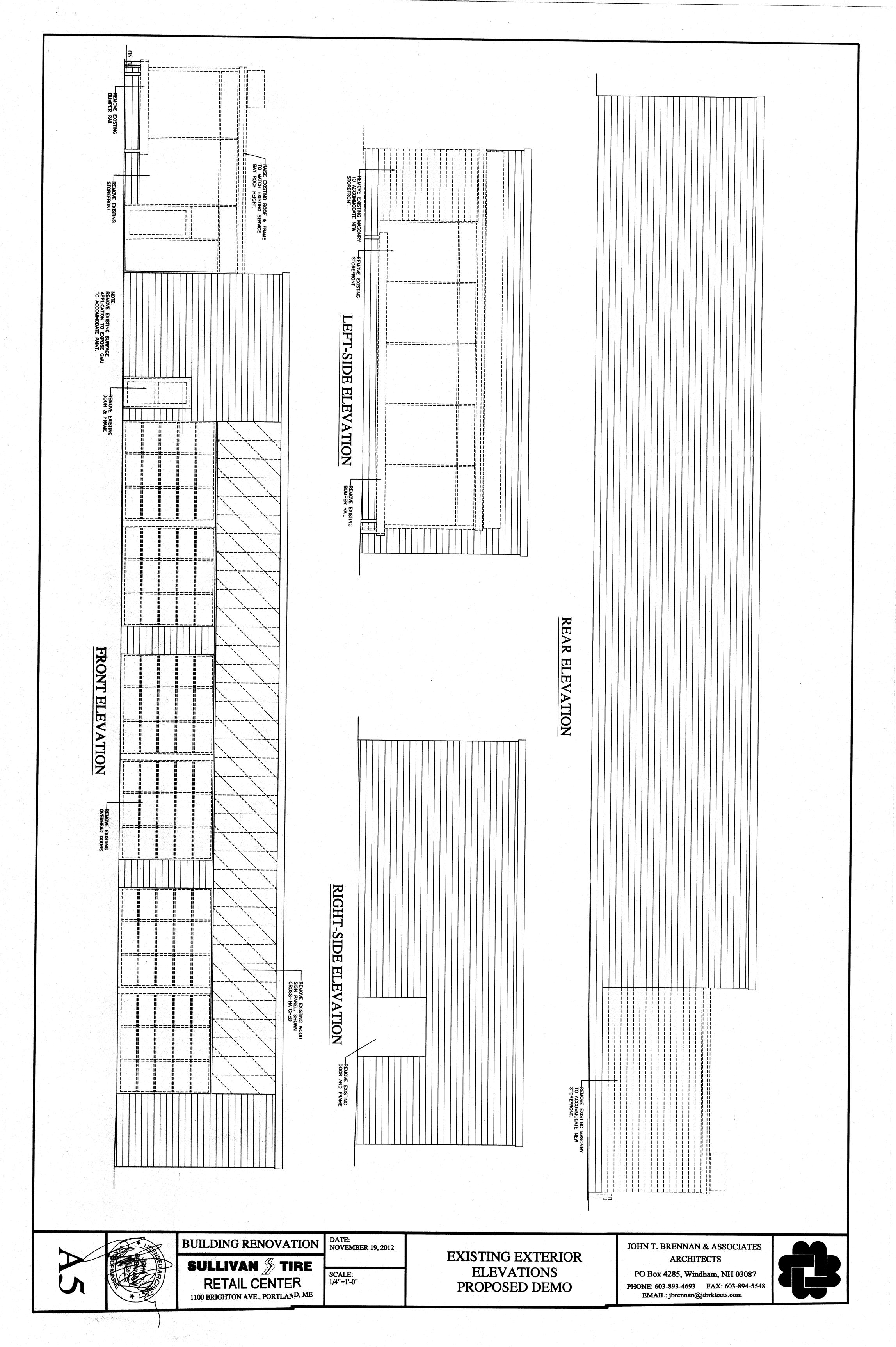


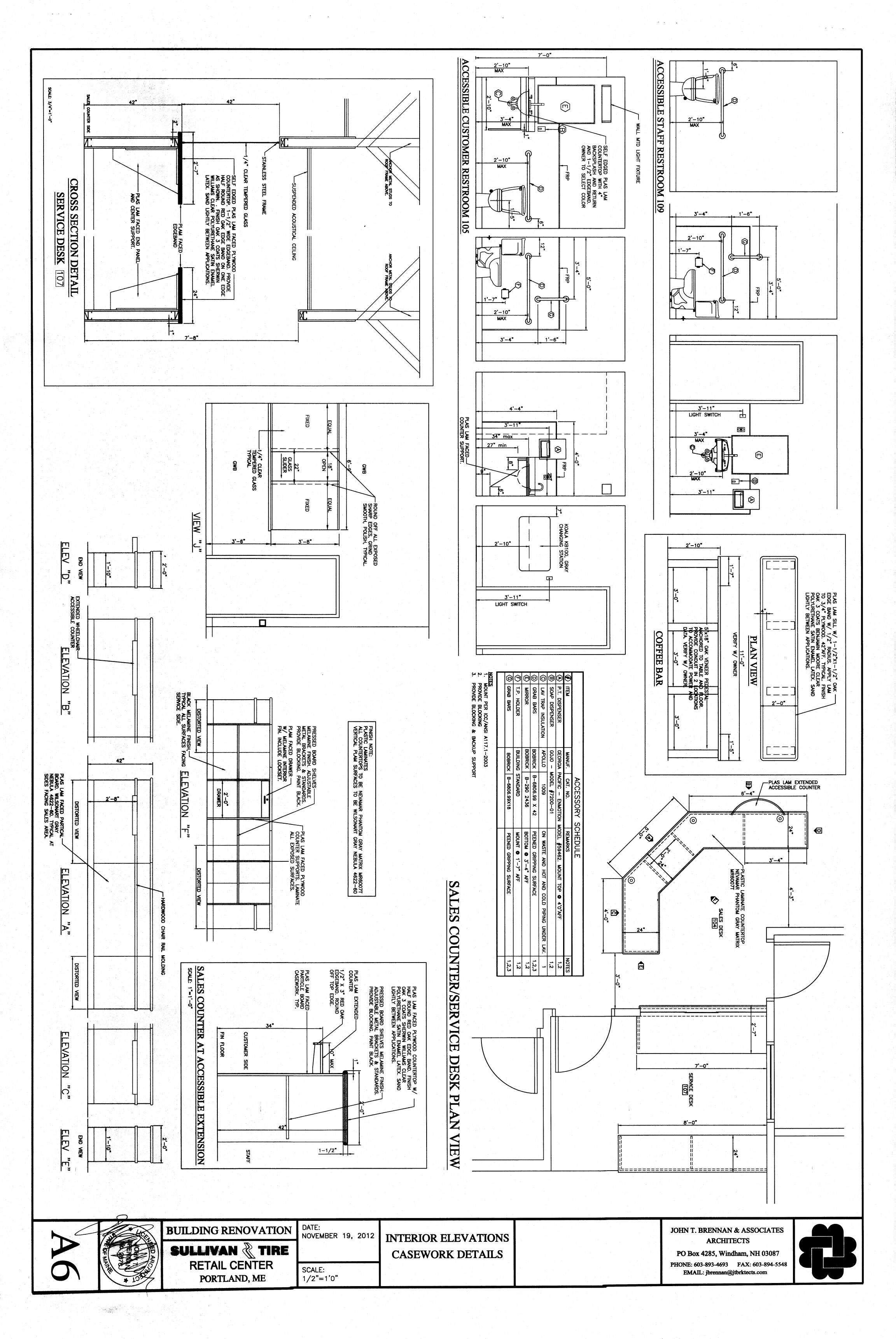


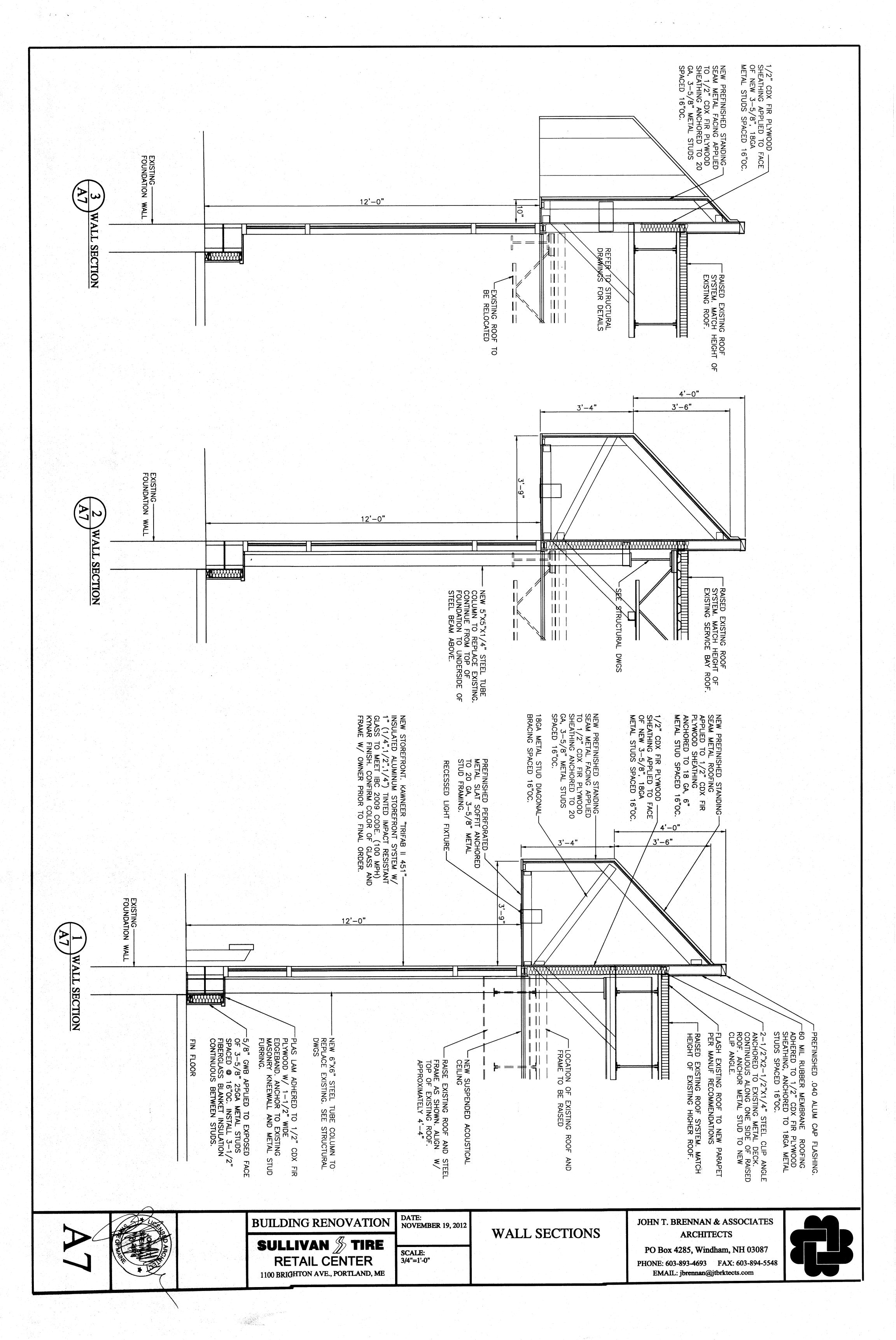


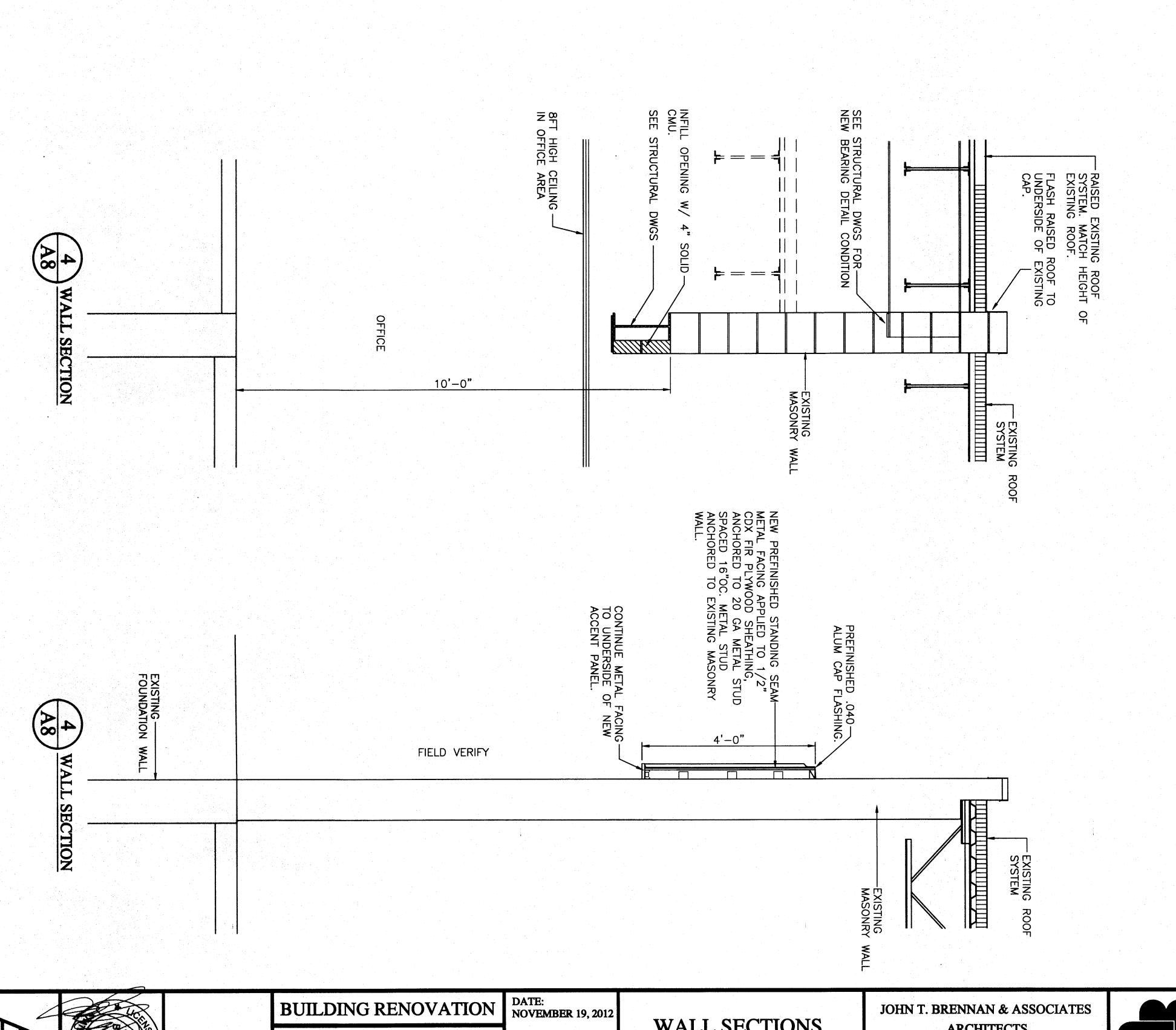


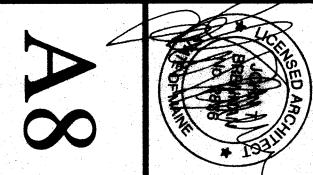












SULLIVAN > TIRE
RETAIL CENTER 1100 BRIGHTON AVE., PORTLAND, ME

SCALE: 3/4"=1'-0"

WALL SECTIONS **RAISED ROOF PLAN**  **ARCHITECTS** 

PO Box 4285, Windham, NH 03087 PHONE: 603-893-4693 FAX: 603-894-5548 EMAIL: jbrennan@jtbrktects.com



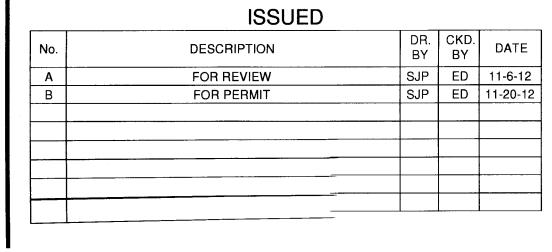
James 🔍

				PRINTED: Nov 20,	2012					
				DESIGN CRITERIA SCALE: NTS	SEISMIC CRITERIA:  SOIL SITE CLASSIFICATION = D DESIGN SPECITRAL RESPONSE ACCELERATION:  Sds = .37  Sd1 = .16  SEISMIC USE GROUP I SEISMIC DESIGN CATEGORY C RESPONSE MODIFICATION COEFFICIENTR = 6.5 OCCUPANCY IMPORTANCE FACTORI = 1.0 BASE SHEARV <sub>E</sub> = Cs * W = 0.06 * W  (W = SEISMIC WEIGHT)	SNOW EXPOSURE FACTOR, Ce = 1.0 SNOW LOAD IMPORTANCE FACTOR, 1 = 1.0 THERMAL FACTOR, Ct = 1.1 FLAT ROOF SNOW LOAD, Pf = 46.2 PSF + DRIFT WIND LOADS: BASIC WIND SPEED = 100 MPH IMPORTANCE FACTOR, Iw = 1.0 WIND EXPOSURE B MAIN WINDFORCE—RESISTING SYSTEM (INCLUDES WINDWARD + LEEWARD) = 13.5 PSF	GENERAL NOTES  SNOW LOADS: GROUND SNOW LOAD, Pg = 60 PSF GROUND SNOW LOAD, Pg = 60 PSF	EXISTING DIMENSIONS AND CONDITIONS ARE FOR REFERENCE ONLY. CONTRACTOR SHALL VERIFY ALL EXISTING CONSTRUCTION AND DIMENSIONS IN THE FIELD PRIOR TO CONSTRUCTION OR FABRICATION. ALL DISCREPANCIES SHALL BE REPORTED TO THE ENGINEER PRIOR TO COMMENCING WORK.  THE CONTRACTOR SHALL NOTIFY THE ENGINEER IF DEVIATIONS OR CHANGES ARE REQUIRED TO THE CONTRACT DOCUMENTS OR APPROVED SHOP DRAWNIGS DUE TO INTERFERENCES, FABRICATION ERRORS, OR OTHER CAUSES.  THE STRUCTURE IS SELF-SUPPORTING AND STABLE AFTER THE ENTIRE BUILDING IS COMPLETELY CONSTRUCTED. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR ERECTION PROCEDURES AND SEQUENCING DURING CONSTRUCTION AND ERECTION. TO PROVIDE AND ENSURE LOCAL AND OVERALL STABILITY OF THE BUILDING AND ITS COMPONENTS DURING CONSTRUCTION AND ERECTION. THE CONTRACTOR SHALL RETAIN A LICENSED STRUCTURAL ENGINEER TO DESIGN TEMPORARY BRACING/SHORING AND DETERMINE WHERE THE TEMPORARY BRACING/SHORING	NCRETE INSTITUTE BUILDING NCIETY OF TESTING AND MA SIGN SPECIFICATIONS FOR V ASSOCIATION, 2005.  INS FOR DIMENSIONS NOT SEES AND LOCATIONS OF WAL TOF A CONFLICT BETWEEN BE NOTIFIED PRIOR TO CO	OWING BUILDING CODES AND STANDARDS SHALL BE REFERENCED DURING CONSTRUCTION:  EDITION OF THE IBC INTERNATIONAL BUILDING CODE  AMERICAN SOCIETY OF CIVIL ENGINEERS, MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES  AMERICAN CONCRETE INSTITUTE SPECIFICATION FOR STRUCTURAL CONCRETE AMERICAN INSTITUTE OF STEEL CONSTRUCTION
			ROOF DECK NOTES SCALE: NTS	PUDDLE WELDS AND A 36/4 PATTERN, SPACING WELDS AT 6" o.c. AT EDGE OF DECK AND END LAPS, UNO. SIDE LAPS SHALL BE FASTENED TOGETHER WITH 10-#10 TEK SCREWS PER SPAN, U.N.O LAP ENDS OF DECK 2" MINIMUM.  TYPICAL ROOF DECK PENETRATION REINFORCEMENT SHALL CONSIST OF 14 GAGE PLATE WELDED TO THE DECK. EXTEND PLATE MINIMUM OF 6" BEYOND OPENING, ON EACH SIDE. MAXIMUM PENETRATION SIZE IS 1'-6" DIAMETER WITHOUT ADDITIONAL REINFORCEMENT.  CONTRACTOR SHALL SUBMIT SHOP DRAWINGS DEPICTING DECK SHEET LAYOUT, DIMENSIONS, PROPERTIES, FASTENING SCHEDULE, AND DETAILS FOR DECK SUPPORT AND EDGE CONDITIONS. THE CONTRACTOR SHALL PROVIDE ALL NECESSARY STOPS, EDGE PIECES, AND ACCESSORIES.	DECK* AND THE * GAGE SHOWN ON SEEL DECK SHALL OF DECKING SHALL OF MORE SPACE OR MORE SPACERE STEEL MEMBER ED TO STEEL WEMBER OF STEEL WEMP STEEL WEMBER OF STEEL WEMP STEEL WE	CMU WALL SUPPORTED FIXTURES, EQUIPMENT, ETC. TO ER'S RECOMMENDATIONS. DO NOT USE EXPANSION AS COMPANY NOTES	ROVIDE VERTICAL	B. VERTICAL REINFORCEMENT  1. ASTM A615, GRADE 60  2. UNLESS NOTED OTHERWISE, PROVIDE #5 BAR © 32" o.c.  3. PROVIDE ADDITIONAL VERTICAL REINFORCEMENT AT EACH SIDE OF CONTROL JOINTS, AT INTERSECTION OF EXTERIOR WALLS, AND AT EACH SIDE OF ALL MASONRY OPENINGS.  4. VERTICAL REINFORCEMENT SHALL BE CONTINUOUS FOR FULL HEIGHT OF WALL  C. GROUT  1. ASTM C476 WITH MINIMUM COMPRESSIVE STRENGTH = 2000 PSI  2. FULLY GROUT ALL CELLS WITH REINFORCEMENT AND ANCHORS  3. CONSOLIDATE AND RECONSOLIDATE GROUT ASSEMBLED WALLS WITH VIBRATOR  C. HORIZONTAL JOINT REINFORCEMENT	A. CONCRETE UNITS: ASTM C90 GRADE N, TYPE 1 WITH MINIMUM COMPRESSIVE STRENGTH = 1,900 PSI 1. UNITS: ASTM C90 GRADE N, TYPE 1 WITH MINIMUM COMPRESSIVE STRENGTH f'm = 1,500 PSI 2. MORTAR: ASTM C270, TYPE S 3. ASSEMBLY/PRISM MINIMUM COMPRESSIVE STRENGTH f'm = 1,500 PSI 4. ALL UNITS SHALL BE LAID IN RUNNING BOND, UNLESS NOTED OTHERWISE 5. PLACE MASONRY WHILE MORTAR IS SOFT AND PLASTIC 6. CONCRETE MASONRY UNITS (CMU) SHALL BE STANDARD UNITS WITH NOMINAL FACE DIMENSIONS LENGTH = 16 INCHES, HEIGHT = 8 INCHES, WIDTH AS INDICATED	MASONRY CONSTRUCTION AND MATERIALS SHALL CONFORM TO ALL REQUIREMENTS OF BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (ACI 530/ASCE5/TMS 402) AND SPECIFICATION FOR MASONRY STRUCTURES (ACI 530.1/ASCE6/TMS 602).
LIGHT GAGE FRAMING NOTES SCALBONES: NTS	PERMITTED FOR FRAMING MEMBERS. ANY SUBSTITUTIONS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER.  POR PROVIDE MINIMUM 1½" EMBEDMENT INTO CONCRETE HILTT X-DNI OR X-EDNI FOR STEEL, U.N.O. PROVIDE MINIMUM 1½" EMBEDMENT INTO CONCRETE. PROTECTIVE COATINGS ON SCREW FASTEMERS SHALL BE COMPATIBLE WITH LIGHT GAGE FRAMING.  TEMPORARY BRACING SHALL BE PROVIDED BY AND BE THE RESPONSIBILITY OF THE CONTRACTOR. PERMANENT BRACING AND BRIDGING SHALL BE IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS. END BLOCKING SHALL BE PROVIDED FOR STEEL JOISTS WHERE JOIST ENDS ARE NOT OTHERWISE RESTRAINED AGAINST ROTATION.  ALL TRACK BUTT JOINTS, ABUTTING PIECES OF TRACK SHALL BE SECURELY ANCHORED TO A COMMON STRUCTURAL ELEMENT, SUCH AS A STUD, OR THEY SHALL BE SECURELY ANCHORED TO A COMMON STRUCTURAL ELEMENT, SUCH AS A STUD, OR THEY SHALL BE BUTT WELDED TOGETHER. SPUCING OF OTHER MEMBERS IS NOT PERMITTED, U.N.O  JACK STUDS SHALL BE INSTALLED BELOW WINDOW SILLS, ABOVE WINDOW AND DOOR HEADS TO FURNISH SUPPORT AND SHALL BE SECURELY ATTACHED TO SUPPORTING MEMBERS. PROVIDE INSULATION EQUAL TO THAT SPECIFED IN ARCHITECTURAL DRAWINGS IN ALL DOUBLE OR MORE JAMB STUDS OR HEADERS WHICH WILL NOT BE ACCESSIBLE TO THE INSULATION CONTRACTOR.  ALL WELDING SHALL BE LOUCHED UP WITH A ZINC RICH PAINT.  STEEL TRUSSES SHALL BE LATERALLY SUPPORTED AT ALL PAINEL POINTS AND AS SHOWN ON PLANS. REQUIRED X-BRACING SHALL BE INSTALLED IMMEDIATELY AFTER ERECTION OF EACH ROOF TRUSS.	CAN IRON AND STEEL INSTITUTE (A.I.S.I.), LATEST EDITION OF JCTURAL MEMBERS"  CAN WELDING SOCIETY (A.W.S.) D.1.3, LATEST EDITION OF "STIL SHEET"  CAN SOCIETY FOR TESTING AND MATERIALS (A.S.T.M.)  CAN INSTITUTE OF STEEL CONSTRUCTION (A.I.S.C.)  STUD MANUFACTURERS ASSOCIATION (S.S.M.A.) ICBO ER—494  RMATION"  ANDLING (PROTECTION OF PRODUCTS): MATERIAL SHALL BE FAMILY ACCORDANCE AND FROM SHALL BE MANUFACTURED IN ACCORDANCE IN ACCORDANCE AND HEAVIER SHALL BE 50 KSI.  IZED MATERIAL:  L GALVANIZED STUDS AND JOISTS SHALL BE FORMED FROM SHALL AND STRUCTURAL PROPERTIES LISTED BY SSMA SHALL BE FORMED STRUCTURAL PROPERTIES LISTED BY SSMA SHALL BE STUDS AND STRUCTURAL PROPERTIES LISTED BY SSMA SHALL BE STUDS AND STRUCTURAL PROPERTIES LISTED BY SSMA SHALL BE STUDS.	QUALITY ASSURANCE: CONTRACTOR SHALL PROVIDE QUALITY CONTROL OVER ALL FABRICATION AND ERECTION ACTIVITIES.  STANDARDS: WORK SHALL MEET THE REQUIREMENTS OF THE FOLLOWING STANDARDS:	SHEAR CONNECTOR STUDS:  1. SHEAR CONNECTOR STUDS SHALL BE NELSON, OR EQUIVALENT, ¾" DIAMETER, U.N.O WELD STUDS PER STUD MANUFACTURER'S RECOMMENDATIONS THROUGH METAL DECKING. STUD LENGTH SHALL BE 1" BELOW TOP OF CONCRETE SLAB-ON-DECK.  2. SHEAR STUDS, WHERE REQUIRED, ARE INDICATED ON THE DRAWNGS AS [XX], WHERE XX IS THE NUMBER OF STUDS EQUALLY SPACED BETWEEN SUPPORTS ON A BEAM OR GIRDER.  STRUCTURAL STEEL NOTES  SCALE: NTS	STRUCTURAL STEEL SHALL RECEIVE THE FOLLOWING PROTECTIVE COATINGS:  1. DO NOT PAINT SURFACES TO RECEIVE METAL DECK AND/ OR SHEAR CONNECTORS FASTENED BY WELDING, CONTACT SURFACES OF HIGH STRENGTH BOLTED CONNECTIONS, FINISHED BEARING SURFACES, AND SURFACES TO BE WELDED IN THE FIELD. IF REQUIRED, PROTECT THESE SURFACES BY RUST-INHIBITING COATING THAT CAN BE REMOVED EASILY PRIOR TO ERECTION.  2. UNEXPOSED STRUCTURAL STEEL SHALL BE CLEANED IN ACCORDANCE WITH SSPC-SP3 AND PAINTED WITH PRIMER PAINT, TNEMEC 10-99, OR EQUIVALENT, U.N.O  3. EXPOSED STRUCTURAL STEEL TO RECEIVE ZINC-RICH EPOXY PAINT SHALL BE FIRST CLEANED IN ACCORDANCE WITH SSPC-SP6, COMMERCIAL BLAST CLEANING. USE TNEMEC ZIN-RICH EPOXY PAINT, OR EQUIVALENT. APPLY FINISH COAT PER ARCHITECT.  4. EXPOSED STRUCTURAL STEEL TO BE HOT-DIPPED GALVANIZED SHALL BE IN ACCORDANCE WITH ASTM A123.	2. HIGH STRENGTH BOLTS SHALL BE INSTALLED AND TIGHTENED FER AND STRUCTION ON STRUCTION OF STRENGTH BOLTS.  JOINTS USING ASTM A325 BOLTS.  3. ANCHOR BOLTS SHALL CONFORM TO ASTM F1554, GRADE 36, STANDARD HEX HEAD FURNISHED WITH HEAVY HEX NUTS AND LOCK WASHERS.  4. CONTRACTOR SHALL DESIGN CONNECTIONS NOT ALREADY DETAILED ON STRUCTURAL DRAWINGS. DESIGN SHALL BE STAMPED BY A LICENSED STRUCTURAL ENGINEER AND SUBMITTED PRIOR TO COMMENCING FABRICATION.  WELDED CONNECTIONS:  1. WELDING SHALL CONFORM TO AWS D1.1. USE LOW—HYDROGEN SMAW ELECTRODES WITH MINIMUM TENSILE STRENGTH OF 70 KSI.	TO AWS D1.1.  BOLTED CONNECTIONS:  1. FIELD CONNECTIONS SHALL UTILIZE MINIMUM ¾* DIAMETER A325 HIGH STRENGTH BOLTS, U.N.O  BOLTED CONNECTION SHALL BE SLIP CRITICAL (SC) AT ALL MOMENT FRAMES, BRACED FRAMES, AND AT ADDITIONAL LOCATIONS INDICATED IN THE DRAWINGS. SLIP CRITICAL CONNECTIONS SHALL UTILIZE LOAD INDICATOR WASHERS OR TENSION CONTROL BOLTS. BOLT HOLES SHALL BE STANDARD SIZE, U.N.O  BOLT CONTROL OF STANDARD SIZE, U.N.O  BOLT CONTROL OF STANDARD SIZE, U.N.O  BOLT CONTROL OF STANDARD SIZE, U.N.O	AND SHOP'S).  2. CERTIFIED MILL TEST REPORTS OF BOLTS, NUTS AND WASHERS (INCLUDING NAMES AND LOCATIONS OF MILLS AND SHOPS).  3. STRUCTURAL STEEL FABRICATION AND ERECTION DRAWINGS WHICH INCLUDE BOLTED CONNECTIONS (SHOP AND FIELD) AND WELDED CONNECTIONS (SHOP AND FIELD) DEPICTING AWS WELDING SYMBOLS.  4. METAL DECK SHOP DRAWINGS DEPICTING SHEAR STUD LAYOUT ON BEAMS AND GIRDERS.  OWNER SHALL RETAIN A QUALIFIED TESTING AGENCY TO PERFORM AND VERIFY THE FOLLOWING:  1. VISUAL INSPECTION OF ALL WELDS.  2. ULTRASONIC TESTING, IN ACCORDANCE WITH ASTM E-164, ON 100% OF ALL FIELD FULL PENETRATION WELDS.  3. PROVIDE RANDOM VERIFICATION VIA ULTRASONIC TESTING OF SHOP FULL PENETRATION WELDS.  4. FIELD BOLTED CONNECTIONS, INCLUDING VERIFICATION OF BOLT GRADES.  5. SHEAR STUD QUANTITY, PROPER INSTALLATION, SIZE, AND SPACING. SHEAR STUDS SHALL CONFORM	ANGLES, PLATES STRUCTURAL TUBING ASTM A500, GRADE B, Fy=46 KSI STEEL PIPE SHOP DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW PRIOR TO COMMENCING FABRICATION. SHOP DRAWINGS SUBMITTALS SHALL INCLUDE: 1. CERTIFIED MILL TEST REPORTS OF STRUCTURAL STEEL (INCLUDING NAMES AND LOCATIONS OF MILLS	WORK SHALL USTITUTE OF STANDARD PRACE MBERS SHALL
LEGEND		SLOPE DESIGNATION  ELEVATION MARK  ROOF PITCH  SPAN DIRECTION  SECTION MARK  SECTION No.  DWG. WHERE SHOWN	ABBREVIATIONS	IN INCH INFO INFORMATION  JT JOINT  K KIP (1 KIP = 1,000 LBS)  KSI KIPS PER SQUARE INCH	FLR FT FOOT FT FT FOOT FOOTING FY FV FV FOOTING FOOTIN	P 	DIM DIMENSION DISCONT DISCONTINUOUS DWG DRAWING  (E), EX, EXIST EXISTING EACH EACH EACH FACE EL, ELEV ELEVATION	J 0000000000	BM BEAM BOT BOTTOM BRG BEARING BRWN BETWEEN  C STRUCTURAL STEEL CHANNEL CANT CANTILEVER CIP CAST-IN-PLACE CONCRETE CJ CONTROL JOINT	,,, BOF B B A A A A
SCALE: NTS		UNDISTURBED EARTH  LEDGE  COMPACTED STRUCTURAL FILL  CONCRETE  GROUT  BRICK  CMU  CMU	SCALE: NTS	TEEL WID	T&B TOP AND BOTTOM TOC, T/CONC TOP OF CONCRETE T/FTG, TOF TOP OF FOOTING TEMP TOP OF SHELF T/SHAB TOP OF STEEL T/WALL TS TOP OF WALL STRUCTURAL TUBING TYP TOP TOP OF WALL STRUCTURAL TUBING	ST S	REINF REINFORCING STEEL REQ, REQD REQUIRED RD ROOF DRAIN SC SLIP CRITICAL SECT SECTION SHEATH SHEATHING SIMILAR	FAB	MAX MAXIMUM MECH MECH MECH MIR MIR MIN MIN MIN MISC MISCELLANEOUS NF NO NEAR FACE NO NEAR SIDE NO NEAR SIDE NOT TO SCALE	L ANGLE LL DOUBLE ANGLE LB POUND LF LINEAR FOOT LLH LONG LEG HORIZONTAL LLY LONG LEG VERTICAL
S000	SHEET TITLE:  STRUCTURAL NOTES  DESIGNED: SJP DRAWN: SJP DATE: 10-25-12 PROJECT NUMBER: 12-104	DESCRIPTION  DR. CKD. BY BY DATE  FOR REVIEW  FOR PERMIT  SJP ED 11-6-12	1	SULLIVAN TIF		PORTLAND	O, ME	PO BOX 4285 WINDHAM, NH 03087  ERIC DUBE No. 12630  COLUMBER No. 12630  OCHOSEN  AMDIGNATION  PO BOX 4285  WINDHAM, NH 03087	Portland, hone 207. Fax 207.1 ascobaye	CASCO BAY  424 Fore Street

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S100

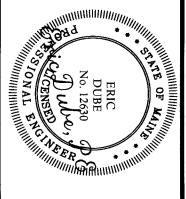
ROOF FRAMING PLAN



SULLIVAN TIRE
1100 BRIGHTON AVE.

THO BINGITION AVE

BUILDING RENOVATION



PORTLAND, ME

JOHN T. BRENNAN & ASSOCIATES
PO BOX 4285
WINDHAM, NH 03087

424 Fore Street
Portland, ME 04101
Phone 207.842.2800
Fax 207.842.2828
www.cascobayengineering.com



