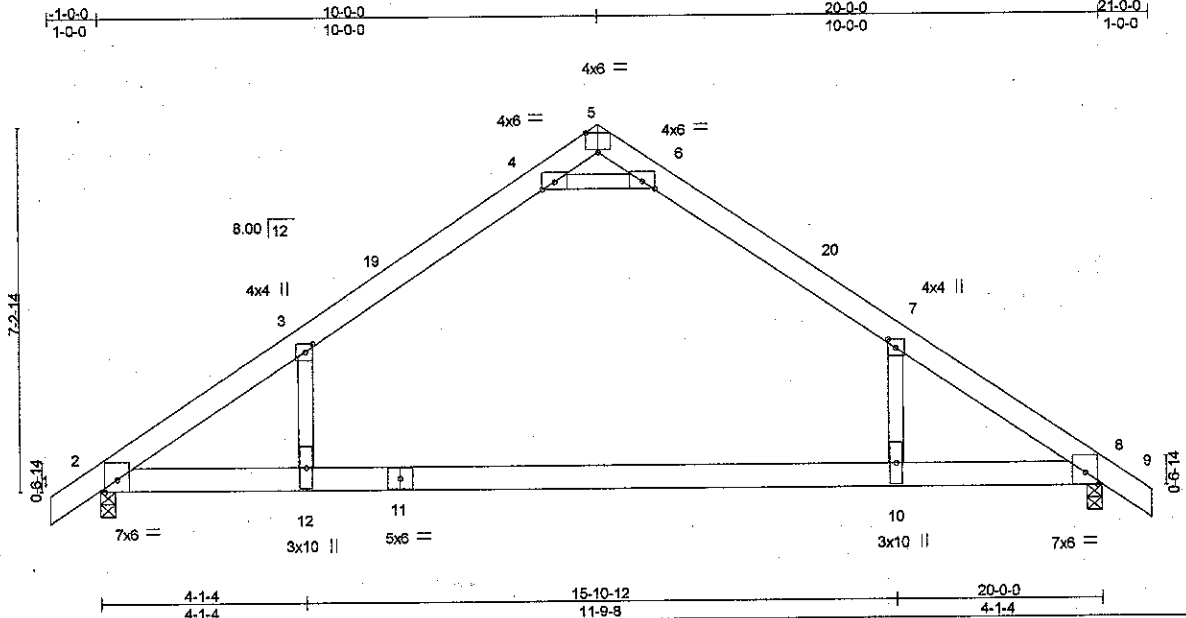


Job	Truss	Truss Type	Qty	Ply	PORTLAND	123130449
053917	TO1	ATTIC	10	1		

Mainly Trusses, Inc., Fairfield, ME

7.530 s Jul 11 2014 MiTek Industries, Inc. Mon Oct 27 10:13:29 2014 Page 1
ID:k1RAUbx3WC_8nRHUFFzjo8yKHBz_tsjUBWUwvr3947FoGja72wz_rZzE57r6E2T1EyPF7a



Scale = 1:44.2

Plate Offsets (X,Y)-- [3:0-2-0,0-1-12], [4:0-3-0,0-1-12], [5:0-3-0,Edge], [6:0-3-0,0-1-12], [7:0-2-0,0-1-12]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 46.2 (Ground Snow=60.0)	2-0-0 Plates Increase 1.15 Lumber Increase 1.15 Rep Stress Incr YES Code IRC2009/TP12007	TC 0.72 BC 0.88 WB 0.25 (Matrix-M)	in (loc) l/defl L/d Vert(LL) -0.55 10-12 >437 240 Vert(TL) -1.10 10-12 >218 180 Horz(TL) 0.02 8 n/a n/a Attic -0.36 10-12 394 360	MT20	197/144
TCDL 7.0				Weight: 92 lb	FT = 20%
BCLL 0.0					
BCDL 10.0					

LUMBER-

TOP CHORD 2x6 SPF 2100F 1.8E
BOT CHORD 2x6 SPF 1650F 1.5E
WEBS 2x4 SPF No.2

BRACING-

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 4-9-2 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS.

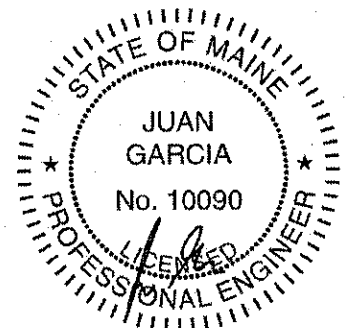
(lb/size) 2=1540/0-3-8, 8=1540/0-3-8
Max Horz 2=262(LC 7)
Max Uplift 2=-210(LC 8), 8=-210(LC 9)
Max Grav 2=1551(LC 2), 8=1551(LC 2)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-3582/419, 3-4=-1424/233, 4-5=-74/1391, 5-6=-75/1391, 6-7=-1424/233, 7-8=-3582/408
BOT CHORD 2-12=-607/4664, 10-12=-8/1339, 8-10=-501/4664
WEBS 4-6=-3011/323, 3-12=0/878, 7-10=0/878

NOTES- (13)

- 1) Wind: ASCE 7-05; 100mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-05; Pg=60.0 psf (ground snow); Ps=46.2 psf (roof snow); Category II; Exp C; Partially Exp.; Ct=1.1
- 3) Roof design snow load has been reduced to account for slope.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 16.0 psf or 1.00 times flat roof load of 46.2 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) Ceiling dead load (5.0 psf) on member(s). 3-4, 6-7, 4-6; Wall dead load (5.0psf) on member(s).3-12, 7-10
- 9) Bottom chord live load (30.0 psf) and additional bottom chord dead load (7.0 psf) applied only to room. 10-12
- 10) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 8. This connection is for uplift only and does not consider lateral forces.
- 11) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.
- 12) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.
- 13) 11'-6" X 5'-6" 40 PSF STORAGE 2X6 B.C.



October 27, 2014

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 rev. 1/25/2014 BEFORE USE

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D5B-89 and 8CSI Building Component Safety Information available from Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA 22314. If Southern Pine (SP) lumber is specified, the design values are those effective 06/01/2013 by ALSC.



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Chesterfield, MO 63017

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