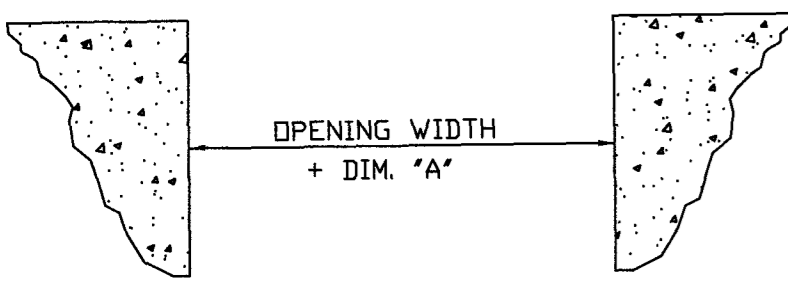


ELEVATION VIEW OF SUGGESTED DETAIL FOR OPENINGS WITH JAMBS TO FINISH FLOOR IN THE PRESENCE OF A RAISED FOUNDATION WALL.



- \*A' = 4 1/2" FOR PRE-ASSEMBLED PERSONNEL DOORS
- = 5 1/2" FOR KNOCK-DOWN PERSONNEL DOORS
- = 7 1/2" FOR OPENINGS WITH COLD-ROLLED JAMBS
- = 8" FOR OPENINGS WITH STRUCTURAL JAMBS

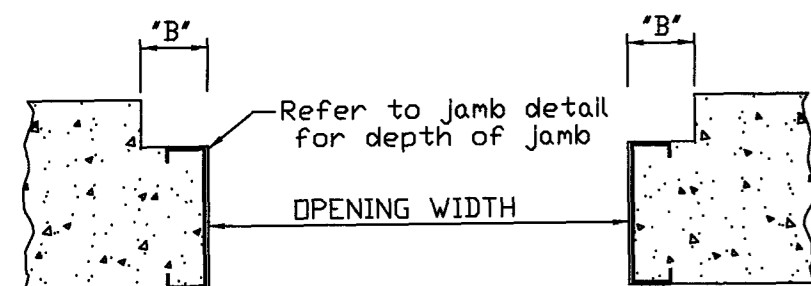
ROUGH OPENING SIZES ABOVE ARE FOR PASS DOORS AND FRAMED OPENINGS FURNISHED BY PII.

FOUNDATION ENGINEER TO VERIFY WITH BUILDER ALL DOOR TYPES AND ASSOCIATED ROUGH OPENING SIZES.

FOR DOORS NOT FURNISHED BY PII, CONSULT DOOR SUPPLIER FOR OPENING DETAILS.

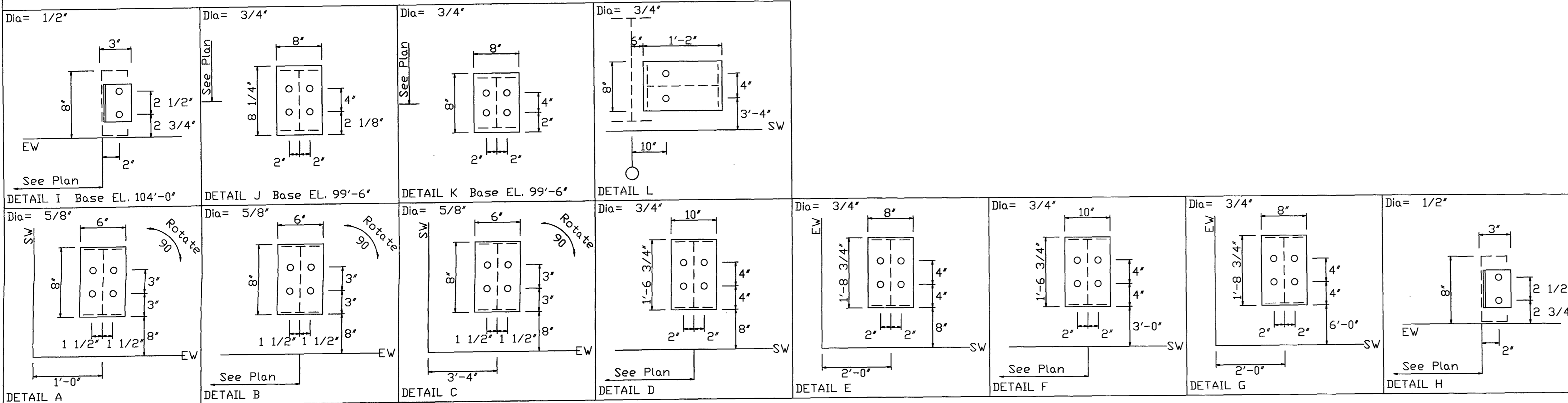
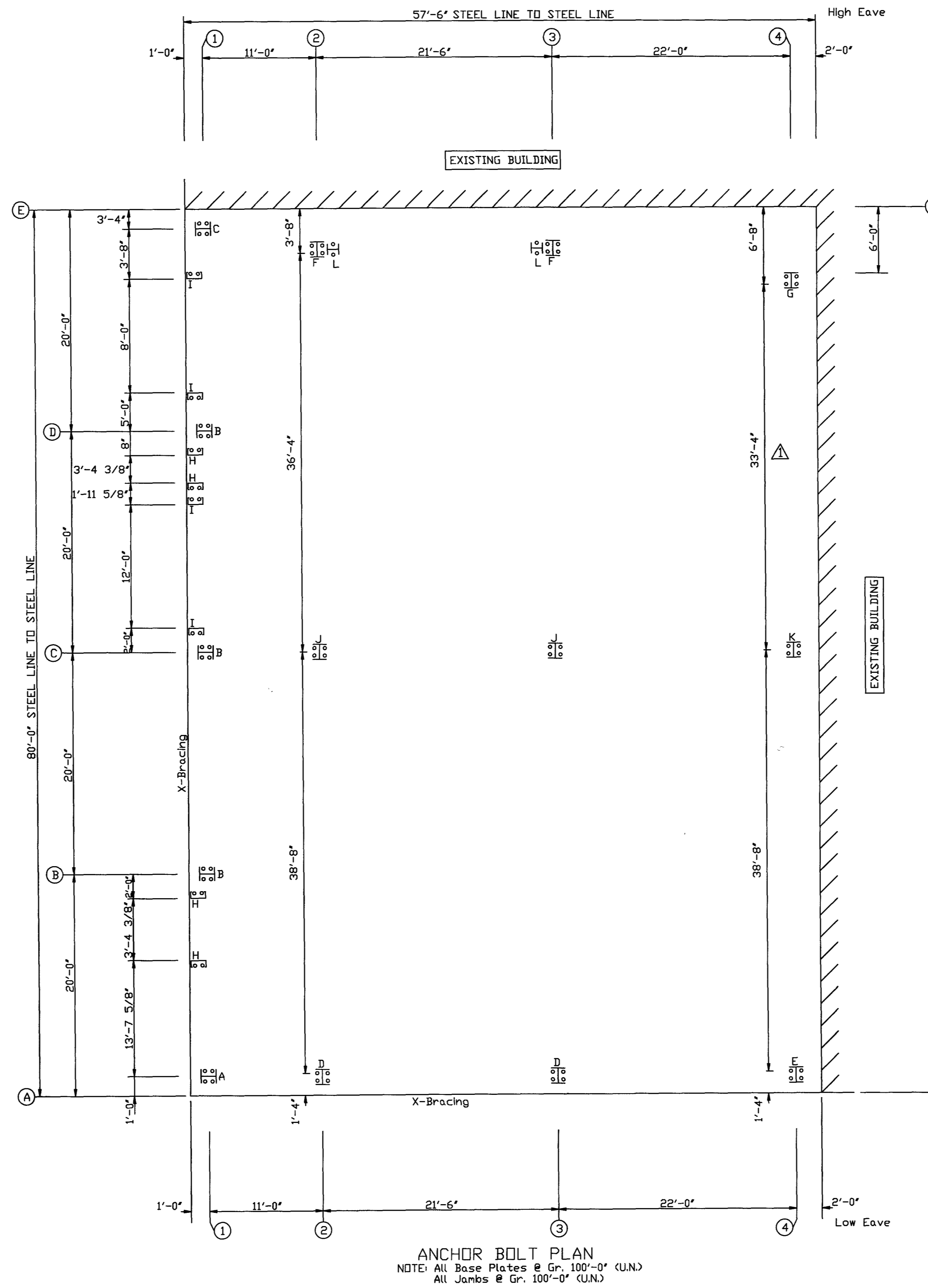
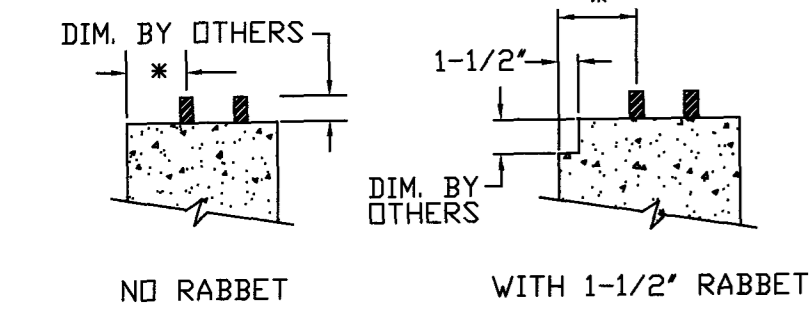
CLEARANCES SHOWN ARE MINIMUM REQUIRED FOR BASE CLIP. THERE MAY BE SMALL VOIDS BETWEEN JAMB FLANGES AND FOUNDATION WALL.

PLAN VIEW OF SUGGESTED DETAIL FOR FRAMED OPENINGS WITH JAMBS TO THE TOP OF A RAISED FOUNDATION WALL.



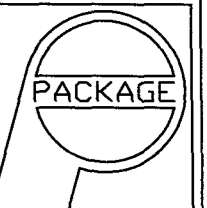
- \*B' = NOTCH FOR DOOR TRACK NOT BY PACKAGE INDUSTRIES, INC. SEE MANUFACTURER RECOMMENDATIONS

\* See Column Base Details

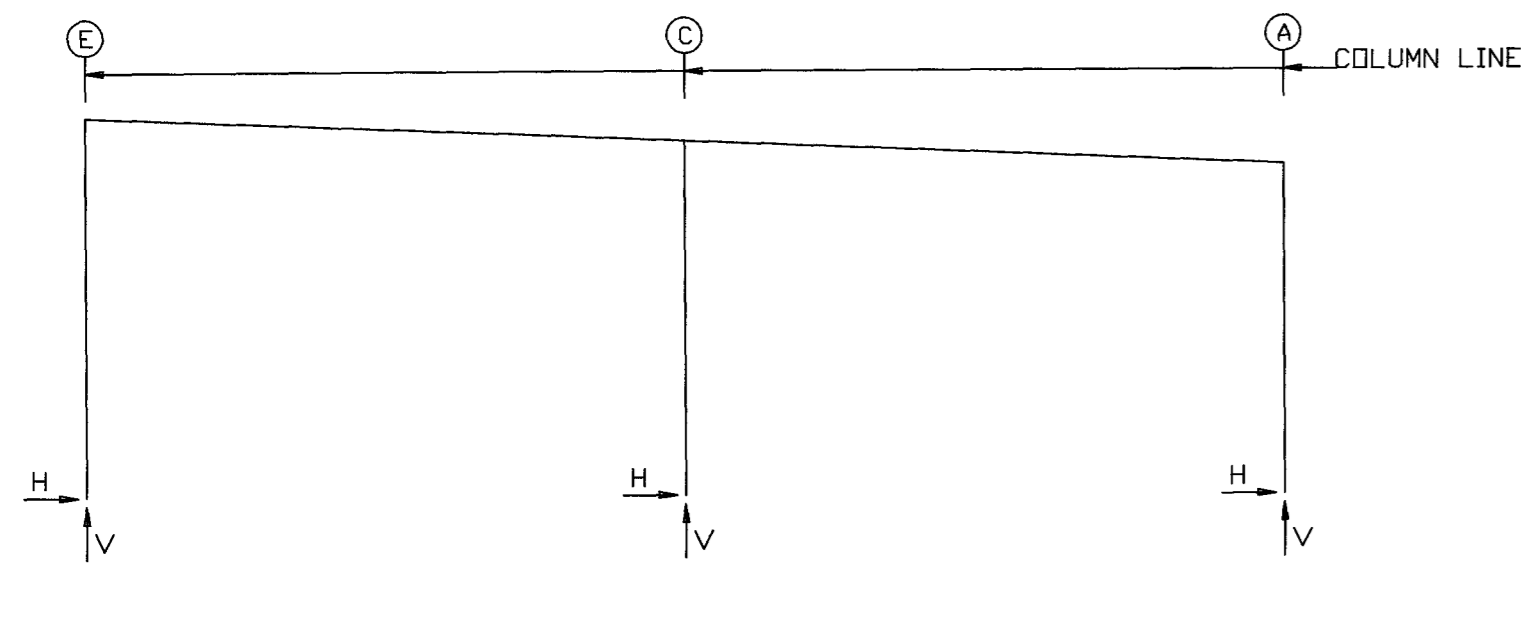


RELEASED FOR CONSTRUCTION

REV.	DESCRIPTION	DATE	DRAFT	ENG.
3				
2				
1	REVIS 36'-4" DIMENSION TO 33'-4" AT F.L. 4	4/28/11	TMZ	CWE
	INITIAL DRAWING RELEASED FOR CONSTRUCTION			CURRENT REVISION: 1
PACKAGE INDUSTRIES, INC.		Biskup Construction Inc.		
PROJECT	Hale Trailer	ANCHOR BOLT PLAN & DETAILS		
ID	11283	DESIGN: CWE	DESIGN CHECK: CWE	
PROJECT	20 Pinetree Industrial Park	DRAFT: RPG	DRAFT CHECK: RPG	
ADDRESS	Portland, ME 04102	DATE: 1/19/11	SCALE: NONE	SHEET: ABLT-1



FRAME LINES: 2 3 4



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column Reactions (k)						Base Plate (in)			Base EL.		
		Load Id	Hmax	V	Load Id	Hmin	Vmin	Anc No	Bolt Dia	Len		Thk	
2 * E	11	4	4.8	17.7	6	-3.3	0.1	4	0.750	10.00	18.75	0.375	0.0
		11	4.5	34.1	9	1.1	-5.4						
2 * A	7	7	3.3	-2.0	2	-4.8	15.1	4	0.750	10.00	18.75	0.375	0.0
		10	-2.8	20.7	8	-0.6	-8.8						
2 * C	8	8	0.0	-7.5	8	0.0	-7.5	4	0.750	8.000	8.250	0.500	-6.0
		11	0.0	52.7									

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column Reactions (k)						Base Plate (in)			Base EL.		
		Load Id	Hmax	V	Load Id	Hmin	Vmin	Anc No	Bolt Dia	Len		Thk	
4 E	11	11	3.1	20.8	6	-2.5	-1.8	4	0.750	8.000	20.75	0.375	0.0
		1	2.2	22.2	5	1.3	-4.5						
4 A	7	7	2.1	-1.7	3	-3.2	7.5	4	0.750	8.000	20.75	0.375	0.0
		12	-1.5	10.7	5	1.9	-2.7						
4 C	5	5	0.0	-7.5	5	0.0	-6.2	4	0.750	8.000	8.000	0.375	-6.0
		11	0.0	33.4									

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead		Collateral		Live		Snow		Wind L1		Wind R1	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
2 * E	11	0.3	3.2	0.4	2.5	1.5	10.2	3.2	2.4	-2.7	-4.3	2.2	-5.5
		-0.3	2.5	-0.4	1.9	-1.5	7.6	-3.2	15.9	-2.2	-1.7	3.0	-3.5
		0.0	4.5	0.0	4.4	0.0	17.4	0.0	36.6	0.0	-7.1	0.0	-9.6

NOTES FOR REACTIONS

The Following Design Data is per Package Industries, Inc.'s standard design practices and established procedures and recommendations of the following Organizations and/or Specifications.

- American Institute of Steel Construction (AISC 2005)
  - American Welding Society Structural Welding Code (AWS D1.1)
  - American Society for Testing and Materials (ASTM)
  - North American United States (NAUSUI)
  - Metal Building Manufacturers Association (MBMA)
- For maximum reactions tables, all loading conditions are examined and only the maximum/minimum horizontal or vertical reactions along with the corresponding horizontal or vertical for those load IDs are reported.
  - Positive reactions are shown in the sketch. Foundation loads are in the opposite directions.
  - Bracing reactions are in the plane of the brace with the horizontal pointing away from the brace bay. The vertical reaction can be downward or upward.
  - Reactions given are based on the design data below. Reactions are not furnished for loads not listed.
  - The endwall column reactions do NOT include wind and seismic reactions from endwall bracing. Reactions given in the bracing reactions table should be combined with the appropriate basic column reactions as necessary to determine the maximum reactions for Foundation design.
  - The rigid frame maximum reactions include wind and seismic reactions from sidewall bracing. Reactions given in the bracing reactions table should not be combined with the appropriate basic column reactions as necessary to determine the maximum reactions for Foundation design.
  - Foundation construction and design is not the responsibility of Package Industries, Inc. The embedment of the anchor bolts in concrete is the responsibility of the Foundation designer.
  - Suggested anchor rod diameter, quantity, minimum projection and placement are shown. All anchor rods are assumed to be ASTM F1544 Grade 36 or equal. Anchor rods (not by PII) shall be set to a tolerance of +1/8" in both elevation and location.
  - Column base plates are designed not to exceed a bearing pressure of 1050 pounds per sq. inch (0.35F<sub>c</sub> where F<sub>c</sub>= 3000 psi) unless noted otherwise.
  - Basic design wind pressure is furnished. For components and cladding not specifically designed and/or furnished by PII, the design pressures and suction shall be increased based on tributary area and location. Confirmation of the design loads and adequacy to resist such loads shall be the responsibility of a licensed design professional by others.

Building Reactions are based on the following information:

Building Code/Edition	IBC 09	Building Size:	Snow Loads:
Width (ft)	80	Ground Snow (Pg)	60,000 psf
Length (ft)	57.5	Flat Roof Snow (Pf)	42 psf
Back Side Eave Height (ft)	25.33	Snow Exposure Factor (Ce)	1.00
Front Side Eave Height (ft)	22	Snow Thermal Factor (Ct)	1.00
Back Side Roof Slope	0.5:12	Snow Importance Factor (Is)	1.0000
Front Side Roof Slope		Sloped Roof Factor (Cs)	1.0000

Acronyms:

- AUXx = Auxiliary Load - Case x
- C = Closed
- CL = Collateral Load
- DL = Dead Load
- FxUNB\_LL = Unbalanced Live Load for Frame IDx
- LL = Max. of (Live or Snow)
- LLR = Live Load Unbalanced
- LnWnL = Longitudinal Wind Load - Left
- LnWnR = Longitudinal Wind Load - Right
- mph = miles per hour
- DL = Dead Load
- DMF = Ordinary Moment Braced Frame
- DL = Dead Load
- psf = pounds per square foot
- SEIS = Seismic
- WL = Wind Left - Case x
- WR = Wind Right - Case x
- WS = Wind Suction

Loading Conditions are as follows:

- DL+CL+SL+Slide
- DL+CL+0.75SL+0.75WL1
- DL+CL+0.75SL+0.75WL1+0.75Slide
- DL+CL+0.75SL+0.75WR1+0.75Slide
- 0.60DL+WR1
- 0.60DL+WR2
- 0.60DL+LnWnL+LWIND1\_L2E
- 0.60DL+LnWnL+LWIND1\_R2E
- 0.60DL+LnWnL+LWIND2\_L3E
- 0.60DL+LnWnL+LWIND2\_R3E
- DL+CL+SL/2+FIPAT\_SL\_2
- DL+CL+FUNB\_SL\_2
- DL+CL+SL/2+FIPAT\_SL\_2
- DL+CL+SL/2+FIPAT\_SL\_1
- DL+CL+SL
- DL+CL+SL+Drift
- DL+CL+0.75SL+0.75WL1
- DL+CL+0.75SL+0.75WR1
- DL+CL+0.75SL+0.75WL2
- DL+CL+0.75SL+0.75WR2
- DL+CL+0.75SL+0.75WL1+0.75Drift
- DL+CL+0.75SL+0.75WR1+0.75Drift
- DL+CL+0.75SL+0.75WL2
- DL+CL+0.75SL+0.75WR2
- DL+CL+0.75SL+0.75WL2+0.75Drift
- DL+CL+0.75SL+0.75WR2+0.75Slide
- DL+CL+0.75SL+0.75WR2
- DL+CL+0.75SL+0.75WL1
- DL+CL+0.75SL+0.75WR1
- DL+CL+0.75SL+0.75WL2
- DL+CL+0.75SL+0.75WR2
- DL+CL+0.75SL+0.75WL1+0.75Drift
- DL+CL+0.75SL+0.75WR1+0.75Drift
- DL+CL+0.75SL+0.75WL2
- DL+CL+0.75SL+0.75WR2
- DL+CL+SL/2+FIPAT\_SL\_1
- DL+CL+SL/2+FIPAT\_SL\_1
- DL+CL+SL/2+FIPAT\_SL\_1

WIND BENT REACTIONS

Frm Line	Col Line	Reactions				Anc Bolt No	Base Plate (in)	
		Horz	Vert	Horz	Vert		Width	Length
1 E	3	2.6	6.3	2.2	5.3	2	0.750	8.00
		2.6	6.3	2.2	5.3	2	0.750	8.00

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead		Collat		Live		Snow		Rafter Wind L		Rafter Wind R		Brace Wind L		Brace Wind R		Wind P		Wind S	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
1 D	0.8	0.5	2.2	4.5	-1.0	-1.8	0.0	-1.0	0.0	-1.8	0.0	0.0	-1.7	1.9							
		0.6	2.6	5.4	-1.2	-2.2	0.0	-1.2	0.0	-2.2	0.0	-0.8	-1.8	1.9							

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column Reactions (k)						Base Plate (in)			Base EL.		
		Load Id	Hmax	V	Load Id	Hmin	Vmin	Anc No	Bolt Dia	Len		Thk	
1 E	13	13	0.0	-1.9	13	0.0	-1.3	4	0.625	6.000	8.000	0.375	0.0
		1	0.0	5.9									
1 D	13	13	1.9	-1.6	14	-1.7	-0.9	4	0.625	6.000	8.000	0.375	0.0
		1	0.0	7.0	13	1.9	-1.6						
1 C	15	15	1.9	-2.0	14	-1.8	-1.0	4	0.625	6.000	8.000	0.375	0.0
		1	0.0	7.0	15	1.9	-2.0						
1 B	13	13	1.8	-3.2	14	-1.6	-1.1	4	0.625	6.000	8.000	0.375	0.0
		1	0.0	7.8	13	1.8	-3.2						
1 A	13	13	0.0	-0.7	13	0.0	-0.7	4	0.625	6.000	8.000	0.375	0.0
		1	0.0	3.5									

BRACING REACTIONS, PANEL SHEAR

Frm Line	Col Line	Reactions (k)				Panel Shear (lb/Ft)
		Horz	Vert	Horz	Vert	
L_E_W 1	C_B	1.3	1.4	0.8	0.8	0.8
		4.9	4.6	3.0	2.8	2.8

ANCHOR BOLT SUMMARY

Qnt	Loc	Dia (in)	Type	Proj (in)
0 16	DJ	1/2"	A307	150
0 20	W	5/8"	A307	150
0 36	RF	3/4"	A307	200
0 4	WF	3/4"	A307	200

General Notes

**Design Responsibility:**  
Package Industries, Inc. (PII) is responsible only for the structural design of the Metal Building System it sells to the Builder. Neither PII nor PII's Engineer is the Design Professional or the Engineer of Record for the Construction Project. PII is not responsible for the design of any components or materials manufactured or supplied by others or their interaction and connection to the Metal Building System unless such design responsibility is specifically required by the Order Documents.

**Close Proximity Structures:**

PII is not responsible for loads (Seismic, Snow, etc.) imposed by, field modifications needed on, or structures in close proximity to this structure. It is the Builder's responsibility to verify that close proximity structures, together with their foundations, are capable of resisting all additional loads that may result from this structure.

**Bracing:**

Metal building brace rods and cables work in pairs to balance the forces caused by initial tensioning. Care must be taken when tightening brace rods or cables so as not to cause accidental damage or misalignment of building components. All rods/cables must be installed loose and then tightened sequentially and equally to maintain proper alignment of components. When properly tightened, rods and cables should not exhibit excessive sag. For long or large rod bracing it may be necessary to support the rod at mid-bay by suspending it from a purlin at the appropriate elevation.

A qualified professional engineer must design bracing for seismic or wind loading of suspended objects that are not part of the PII structure. The design must meet code requirements and safely deliver the lateral loads to one of the PII primary bracing systems. In addition, the bracing must be designed and erected in a manner that will not impose torsional or minor axis loads, or cause local failures in any PII structural components. No material may be cut, drilled, or otherwise removed from any part of this building without the written consent of PII. The engineer CANNOT rely on the roof deck to act as a diaphragm. PII accepts no responsibility for the design and installation of bracing for objects that are not furnished or specified by PII.

**Field Work:**

All local, state, and federal safety regulations are to be strictly followed. Temporary supports or bracing required for the building erection is the responsibility of the erector to determine, furnish and install. It is the responsibility of the Builder/Contractor to obtain appropriate approvals and necessary permits from city, county, state, or federal agencies, as required.

PII provides complete components to erect all projects with minimal modifications. However, minor fieldwork of structural, secondary, panel, and trim items may be necessary to ensure proper fit. Such work is considered a normal part of metal building erection. Back charges for minor fieldwork will not be honored.

Welds shall be made only by operators certified by the standard qualifications procedure of the American Welding Society for the type of weld required. All field welds to be done using E70XX electrodes and in accordance with the American Welding Society Structural Welding Code.

**A325 Bolt Tightening Requirements**

All high strength bolts are A325-N unless specifically noted otherwise. Structural bolts shall be tightened by the TURN-OF-THE-NUT method in accordance with the ninth-edition AISC "Specifications for Structural Joints using ASTM A325 or A490 Bolts" per section B.D1. A325 bolts may be installed without washers when tightened by the TURN-OF-THE-NUT method. All high strength bolts, except as noted otherwise, are subject to direct tension and may require inspection as defined by AISC/RCS "Specifications for Structural Joints using ASTM A325 or A490 Bolts and the applicable building code or standard. It is the responsibility of the erector to assure proper tightness.

PII accepts no responsibility for the consequences of any additions or alterations to this structure. Modifications to this structure must be performed under the supervision of a qualified licensed professional engineer who accepts responsibility for the adequacy and consequences of the additions or alterations.

The primary and secondary framing of this building may have been designed to support additional collateral loads (These loads may include sprinkler systems, mechanical equipment, ducts, ceilings, etc.). Care must be exercised however, to prevent local overstress of light gauge secondary members supporting concentrated loads.

**Masonry & Concrete:**

PII accepts no responsibility for the design of masonry walls, concrete walls, foundations, mezzanine slabs, and floor slabs. Also, the attachment to masonry or concrete is not designed or supplied by PII (Masonry anchor sizes, spacing, and quantity, unless specifically stated will be designed and supplied by others). The engineer responsible for the design of the masonry and concrete is also responsible for ensuring that the design (including wall base details) is compatible with the deflection criteria for this building. Eave purlins and rake channels are not designed to support lateral loads from masonry or other walls not by PII. Values given for bends and anchor bolt total lengths are suggested lengths only. It is the responsibility of the foundation engineer to determine these values since they are a function of concrete strength as well as other factors.

Base plates are designed assuming concrete has a minimum strength of 3000 psi at 28 days unless otherwise noted.

Janb foundations should be designed for a shear of 2 kips unless otherwise noted.

**Independent Mezzanines:**

Independent mezzanines must be designed by a qualified professional engineer to meet all code requirements. The engineer must also ensure that proper isolation from the PII building has been provided to avoid contact with PEMB structure due to differential movement. PII accepts no responsibility for the design of independent mezzanines.

**Panels:**

DI Canning is an inherent characteristic of cold rolled roof and wall panels. It is the result of several factors that include, but are not limited to, induced stresses in the base material, fabrication methods, installation procedures, and post installation thermal forces. DI Canning does not affect the structural integrity or overall performance of the metal panels. DI Canning is an aesthetic issue only and is not grounds for rejection of the panels.

**Parapets:**

Buildings with parapet walls and internal gutters must be furnished with rainwater overflow mechanisms (such as scuppers) to prevent the accumulation of water in the event of a gutter blockage. It is the responsibility of the Builder to make sure that the scuppers are of the appropriate size, quantity, location, and design to prevent water accumulation on the roof. Failure to do so can result in building collapse. PII accepts no responsibility for the design and installation of overflow mechanisms.

MATERIALS:	ASTM DESIGNATIONS:	YIELD STRENGTH:
Structural Steel Plate (Built-up Sections)	A529 Grades 50 & 55 A572 Grades 50 & 55 A1011 HSLAS Grades 50 & 55	50 ksi 50 ksi 50 ksi
Hot Rolled Mill Shapes (WF, Channels, Angles)	A36 A572 Grades 50 A992	36 ksi 50 ksi min. 50 ksi
Round Struct. Tubing - Pipe	A500 Grade B	42 ksi
Shaped Struct. Tubing- Tube	A500 Grade B	46 ksi
Cold Formed Shapes (Purlins, Girts, Eave Struts)	A653 (SS) Grade 50 Class 1, 2, 3 A653 (HSLAS) Grade 50, Types A or B	55 ksi min. 55 ksi min.
Roof and Wall Sheets	A653/A792 SS Grade 50 Class 1 or 2 (AZ55 Coating) A755/A792 SS Grade 50 Class 1 or 2 (AZ50 Coating)	50 ksi 50 ksi
Brace Rods	A529	50 ksi
Brace Angles	A36	36 ksi
Structural Cables (Cable Bracing)	A475 7-wire EHS Grade	
Cable Hardware	A536 Grade 65-45-12	45 ksi
Bolts	A307 Grade A SAE-J429 Grade 2 A325 Type 1	60 ksi (tensile strength) 120 ksi, 105 ksi
Nuts	A563 Grade A SAE-J995 Grade 2 A563 Grade C, D or DH (A325)	
Washers (Hardened)	F436 Type 1	
Washers (Plain)	F844	
Anchor Bolts	A307 unless otherwise noted	

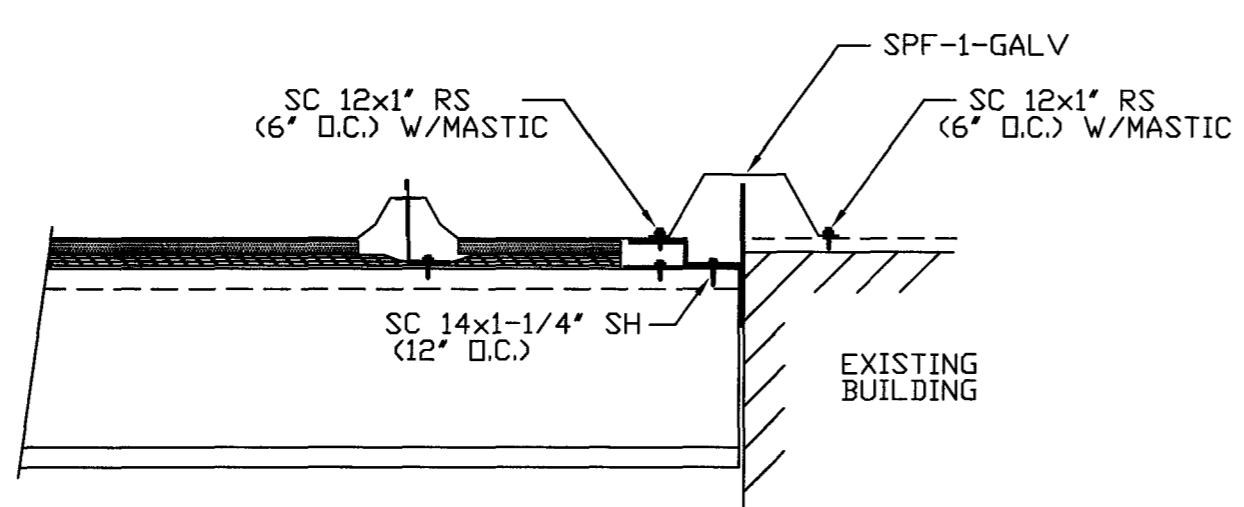
RELEASED FOR CONSTRUCTION

REV.	DESCRIPTION:	DATE:	DRAFT	ENG.
3				
2				
1				
INITIAL DRAWING: RELEASED FOR CONSTRUCTION		CURRENT REVISION: 0		
PACKAGE INDUSTRIES, INC.		Biskup Construction Inc.		
PROJECT	Hale Trailer	ANCHOR BOLT REACTIONS & NOTES		
ID	11283	DESIGN: CWE	DESIGN CHECK: CWE	
PROJECT	20 Pinetree Industrial Park	DRAFT: RPG	DRAFT CHECK: RPG	
ADDRESS	Portland, ME 04102	DATE: 1/19/11	SCALE: NONE	SHEET: ABLT-2

MEMBER TABLE	
MARK	LENGTH
P-1	10x25216 15'-7 1/2"
P-2	10x25216 15'-7 1/2"
P-3	10x25214 28'-9 1/2"
P-4	10x25212 27'-7 1/2"
P-5	10x25212 27'-7 1/2"
SI-1	10ESL114 11'-11 5/8"
SI-2	10ESL114 11'-5 3/4"
SI-3	10ESL114 20'-3'-11 5/8"
SI-4	10ESL114 20'-3'-11 5/8"
SI-5	10ESL114 20'-3'-11 5/8"
SI-6	10ESL114 20'-3'-11 5/8"
SI-7	10ESL114 20'-3'-11 5/8"
SI-8	10ESL114 20'-3'-11 5/8"
SI-9	10ESL114 20'-3'-11 5/8"
SI-10	10ESL114 20'-3'-11 5/8"
SI-11	10ESL114 20'-3'-11 5/8"
SI-12	10ESL114 20'-3'-11 5/8"
SI-13	10ESL114 20'-3'-11 5/8"
SI-14	10ESL114 20'-3'-11 5/8"
SI-15	10ESL114 20'-3'-11 5/8"
SI-16	10ESL114 20'-3'-11 5/8"
SI-17	10ESL114 20'-3'-11 5/8"
SI-18	10ESL114 20'-3'-11 5/8"
SI-19	10ESL114 20'-3'-11 5/8"
SI-20	10ESL114 20'-3'-11 5/8"
SI-21	10ESL114 20'-3'-11 5/8"
SI-22	10ESL114 20'-3'-11 5/8"
SI-23	10ESL114 20'-3'-11 5/8"
SI-24	10ESL114 20'-3'-11 5/8"
SI-25	10ESL114 20'-3'-11 5/8"
SI-26	10ESL114 20'-3'-11 5/8"
SI-27	10ESL114 20'-3'-11 5/8"
SI-28	10ESL114 20'-3'-11 5/8"
SI-29	10ESL114 20'-3'-11 5/8"
SI-30	10ESL114 20'-3'-11 5/8"
SI-31	10ESL114 20'-3'-11 5/8"
SI-32	10ESL114 20'-3'-11 5/8"
SI-33	10ESL114 20'-3'-11 5/8"
SI-34	10ESL114 20'-3'-11 5/8"
SI-35	10ESL114 20'-3'-11 5/8"
SI-36	10ESL114 20'-3'-11 5/8"
SI-37	10ESL114 20'-3'-11 5/8"
SI-38	10ESL114 20'-3'-11 5/8"
SI-39	10ESL114 20'-3'-11 5/8"
SI-40	10ESL114 20'-3'-11 5/8"
SI-41	10ESL114 20'-3'-11 5/8"
SI-42	10ESL114 20'-3'-11 5/8"
SI-43	10ESL114 20'-3'-11 5/8"
SI-44	10ESL114 20'-3'-11 5/8"
SI-45	10ESL114 20'-3'-11 5/8"
SI-46	10ESL114 20'-3'-11 5/8"
SI-47	10ESL114 20'-3'-11 5/8"
SI-48	10ESL114 20'-3'-11 5/8"
SI-49	10ESL114 20'-3'-11 5/8"
SI-50	10ESL114 20'-3'-11 5/8"
SI-51	10ESL114 20'-3'-11 5/8"
SI-52	10ESL114 20'-3'-11 5/8"
SI-53	10ESL114 20'-3'-11 5/8"
SI-54	10ESL114 20'-3'-11 5/8"
SI-55	10ESL114 20'-3'-11 5/8"
SI-56	10ESL114 20'-3'-11 5/8"
SI-57	10ESL114 20'-3'-11 5/8"
SI-58	10ESL114 20'-3'-11 5/8"
SI-59	10ESL114 20'-3'-11 5/8"
SI-60	10ESL114 20'-3'-11 5/8"
SI-61	10ESL114 20'-3'-11 5/8"
SI-62	10ESL114 20'-3'-11 5/8"
SI-63	10ESL114 20'-3'-11 5/8"
SI-64	10ESL114 20'-3'-11 5/8"
SI-65	10ESL114 20'-3'-11 5/8"
SI-66	10ESL114 20'-3'-11 5/8"
SI-67	10ESL114 20'-3'-11 5/8"
SI-68	10ESL114 20'-3'-11 5/8"
SI-69	10ESL114 20'-3'-11 5/8"
SI-70	10ESL114 20'-3'-11 5/8"
SI-71	10ESL114 20'-3'-11 5/8"
SI-72	10ESL114 20'-3'-11 5/8"
SI-73	10ESL114 20'-3'-11 5/8"
SI-74	10ESL114 20'-3'-11 5/8"
SI-75	10ESL114 20'-3'-11 5/8"
SI-76	10ESL114 20'-3'-11 5/8"
SI-77	10ESL114 20'-3'-11 5/8"
SI-78	10ESL114 20'-3'-11 5/8"
SI-79	10ESL114 20'-3'-11 5/8"
SI-80	10ESL114 20'-3'-11 5/8"
SI-81	10ESL114 20'-3'-11 5/8"
SI-82	10ESL114 20'-3'-11 5/8"
SI-83	10ESL114 20'-3'-11 5/8"
SI-84	10ESL114 20'-3'-11 5/8"
SI-85	10ESL114 20'-3'-11 5/8"
SI-86	10ESL114 20'-3'-11 5/8"
SI-87	10ESL114 20'-3'-11 5/8"
SI-88	10ESL114 20'-3'-11 5/8"
SI-89	10ESL114 20'-3'-11 5/8"
SI-90	10ESL114 20'-3'-11 5/8"
SI-91	10ESL114 20'-3'-11 5/8"
SI-92	10ESL114 20'-3'-11 5/8"
SI-93	10ESL114 20'-3'-11 5/8"
SI-94	10ESL114 20'-3'-11 5/8"
SI-95	10ESL114 20'-3'-11 5/8"
SI-96	10ESL114 20'-3'-11 5/8"
SI-97	10ESL114 20'-3'-11 5/8"
SI-98	10ESL114 20'-3'-11 5/8"
SI-99	10ESL114 20'-3'-11 5/8"
SI-100	10ESL114 20'-3'-11 5/8"

ANGLE TABLE	
MARK	LENGTH
SI-1	10'-3"
SI-2	10'-3"
SI-3	10'-3"
SI-4	10'-3"
SI-5	10'-3"
SI-6	10'-3"
SI-7	10'-3"
SI-8	10'-3"
SI-9	10'-3"
SI-10	10'-3"
SI-11	10'-3"
SI-12	10'-3"
SI-13	10'-3"
SI-14	10'-3"
SI-15	10'-3"
SI-16	10'-3"
SI-17	10'-3"
SI-18	10'-3"
SI-19	10'-3"
SI-20	10'-3"
SI-21	10'-3"
SI-22	10'-3"
SI-23	10'-3"
SI-24	10'-3"
SI-25	10'-3"
SI-26	10'-3"
SI-27	10'-3"
SI-28	10'-3"
SI-29	10'-3"
SI-30	10'-3"
SI-31	10'-3"
SI-32	10'-3"
SI-33	10'-3"
SI-34	10'-3"
SI-35	10'-3"
SI-36	10'-3"
SI-37	10'-3"
SI-38	10'-3"
SI-39	10'-3"
SI-40	10'-3"
SI-41	10'-3"
SI-42	10'-3"
SI-43	10'-3"
SI-44	10'-3"
SI-45	10'-3"
SI-46	10'-3"
SI-47	10'-3"
SI-48	10'-3"
SI-49	10'-3"
SI-50	10'-3"
SI-51	10'-3"
SI-52	10'-3"
SI-53	10'-3"
SI-54	10'-3"
SI-55	10'-3"
SI-56	10'-3"
SI-57	10'-3"
SI-58	10'-3"
SI-59	10'-3"
SI-60	10'-3"
SI-61	10'-3"
SI-62	10'-3"
SI-63	10'-3"
SI-64	10'-3"
SI-65	10'-3"
SI-66	10'-3"
SI-67	10'-3"
SI-68	10'-3"
SI-69	10'-3"
SI-70	10'-3"
SI-71	10'-3"
SI-72	10'-3"
SI-73	10'-3"
SI-74	10'-3"
SI-75	10'-3"
SI-76	10'-3"
SI-77	10'-3"
SI-78	10'-3"
SI-79	10'-3"
SI-80	10'-3"
SI-81	10'-3"
SI-82	10'-3"
SI-83	10'-3"
SI-84	10'-3"
SI-85	10'-3"
SI-86	10'-3"
SI-87	10'-3"
SI-88	10'-3"
SI-89	10'-3"
SI-90	10'-3"
SI-91	10'-3"
SI-92	10'-3"
SI-93	10'-3"
SI-94	10'-3"
SI-95	10'-3"
SI-96	10'-3"
SI-97	10'-3"
SI-98	10'-3"
SI-99	10'-3"
SI-100	10'-3"

TRIM TABLE	
MARK	LENGTH
SI-1	10'-3"
SI-2	10'-3"
SI-3	10'-3"
SI-4	10'-3"
SI-5	10'-3"
SI-6	10'-3"
SI-7	10'-3"
SI-8	10'-3"
SI-9	10'-3"
SI-10	10'-3"
SI-11	10'-3"
SI-12	10'-3"
SI-13	10'-3"
SI-14	10'-3"
SI-15	10'-3"
SI-16	10'-3"
SI-17	10'-3"
SI-18	10'-3"
SI-19	10'-3"
SI-20	10'-3"
SI-21	10'-3"
SI-22	10'-3"
SI-23	10'-3"
SI-24	10'-3"
SI-25	10'-3"
SI-26	10'-3"
SI-27	10'-3"
SI-28	10'-3"
SI-29	10'-3"
SI-30	10'-3"
SI-31	10'-3"
SI-32	10'-3"
SI-33	10'-3"
SI-34	10'-3"
SI-35	10'-3"
SI-36	10'-3"
SI-37	10'-3"
SI-38	10'-3"
SI-39	10'-3"
SI-40	10'-3"
SI-41	10'-3"
SI-42	10'-3"
SI-43	10'-3"
SI-44	10'-3"
SI-45	10'-3"
SI-46	10'-3"
SI-47	10'-3"
SI-48	10'-3"
SI-49	10'-3"
SI-50	10'-3"
SI-51	10'-3"
SI-52	10'-3"
SI-53	10'-3"
SI-54	10'-3"
SI-55	10'-3"
SI-56	10'-3"
SI-57	10'-3"
SI-58	10'-3"
SI-59	10'-3"
SI-60	10'-3"
SI-61	10'-3"
SI-62	10'-3"
SI-63	10'-3"
SI-64	10'-3"
SI-65	10'-3"
SI-66	10'-3"
SI-67	10'-3"
SI-68	10'-3"
SI-69	10'-3"
SI-70	10'-3"
SI-71	10'-3"
SI-72	10'-3"
SI-73	10'-3"
SI-74	10'-3"
SI-75	10'-3"
SI-76	10'-3"
SI-77	10'-3"
SI-78	10'-3"
SI-79	10'-3"
SI-80	10'-3"
SI-81	10'-3"
SI-82	10'-3"
SI-83	10'-3"
SI-84	10'-3"
SI-85	10'-3"
SI-86	10'-3"
SI-87	10'-3"
SI-88	10'-3"
SI-89	10'-3"
SI-90	10'-3"
SI-91	10'-3"
SI-92	10'-3"
SI-93	10'-3"
SI-94	10'-3"
SI-95	10'-3"
SI-96	10'-3"
SI-97	10'-3"
SI-98	10'-3"
SI-99	10'-3"
SI-100	10'-3"



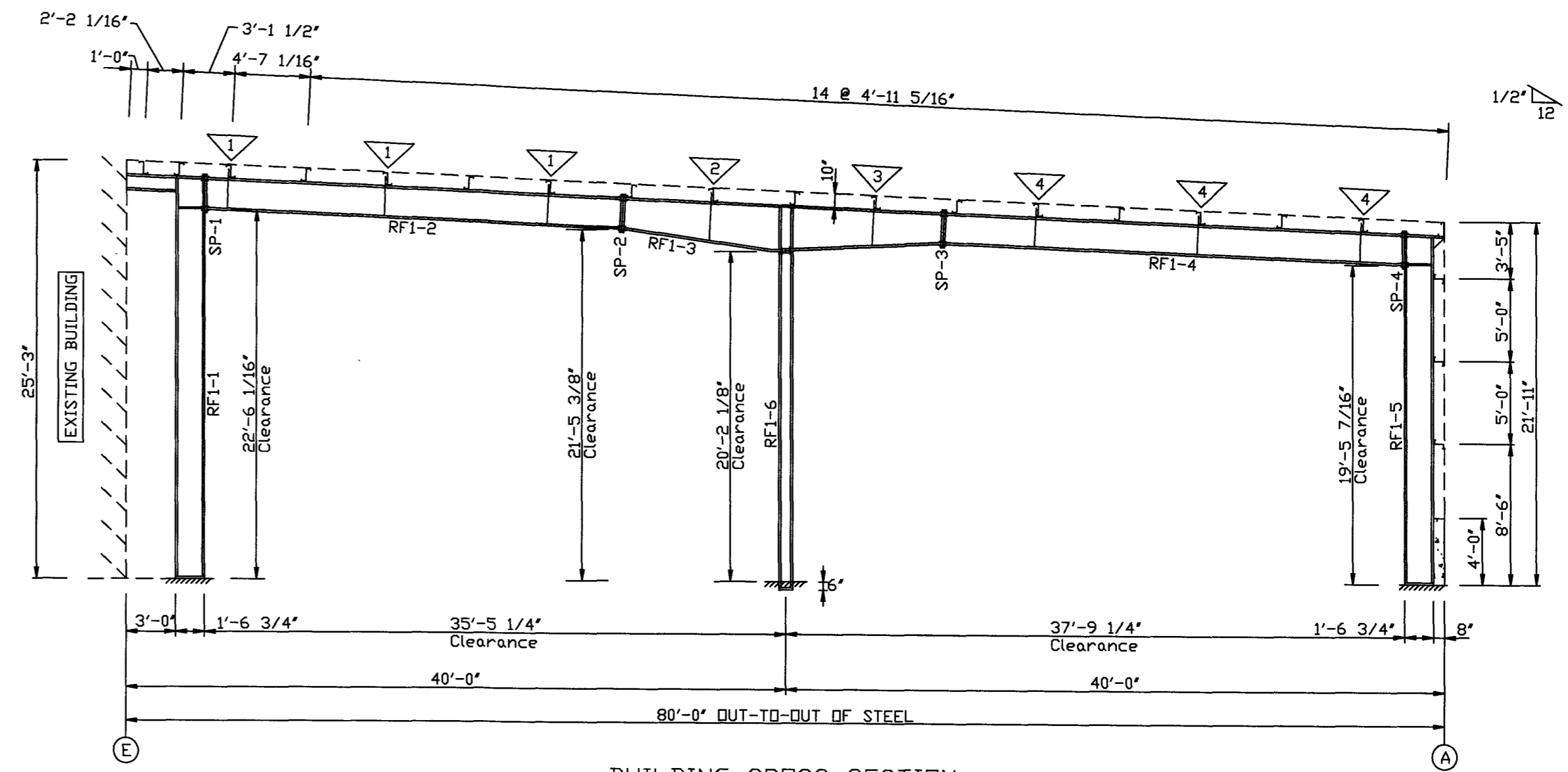
ROOF TO ROOF

SPLICE BOLTS				CAP PLATE BOLTS			
Splice Mark	Quan	Top/Bot/Int	Type Dia Length	Col Mark	Qnt	Type Dia Length	
SP-1	4	4	0 A325 0.750 2.00	RF1-6	4	A325 0.625 1.75	
SP-2	4	4	0 A325 0.750 2.25				
SP-3	4	4	0 A325 0.625 1.75				
SP-4	4	4	0 A325 0.750 2.00				

FLANGE BRACE TABLE			
V ID	MARK	LENGTH (in)	SIDES
1	FB6A	42.750	1
2	FB10A	46.380	1
3	FB9A	43.130	1
4	FB4A	42.500	1

FBxA = 2"x2"x1/8"  
FBxB = 2-1/2"x2-1/2"x3/16"

MEMBER SIZE TABLE (in)						
MARK	WEIGHT	WEB DEPTH	WEB PLATE	OUTSIDE FLANGE	INSIDE FLANGE	
		START/END THICK	THICK LENGTH	V x T x LENGTH	V x T x LENGTH	
RF1-1	1012	18.0/18.0	0.188 24.0	10 x 3/8" x 230.7	10 x 3/8" x 266.3	
RF1-2	1222	18.0/18.0	0.188 24.4	8 x 3/4" x 305.6	8 x 1/4" x 304.8	
RF1-3	865	20.0/20.0	0.250 109.4	8 x 1/4" x 233.3	8 x 3/8" x 109.9	
RF1-4	872	31.0/20.0	0.250 123.9	8 x 1/4" x 337.1	8 x 5/16" x 337.9	
RF1-5	871	20.0/20.0	0.188 97.9	10 x 3/8" x 26.4	10 x 3/8" x 229.7	
RF1-6	594	18.0/18.0	0.188 229.7	10 x 3/8" x 252.6		



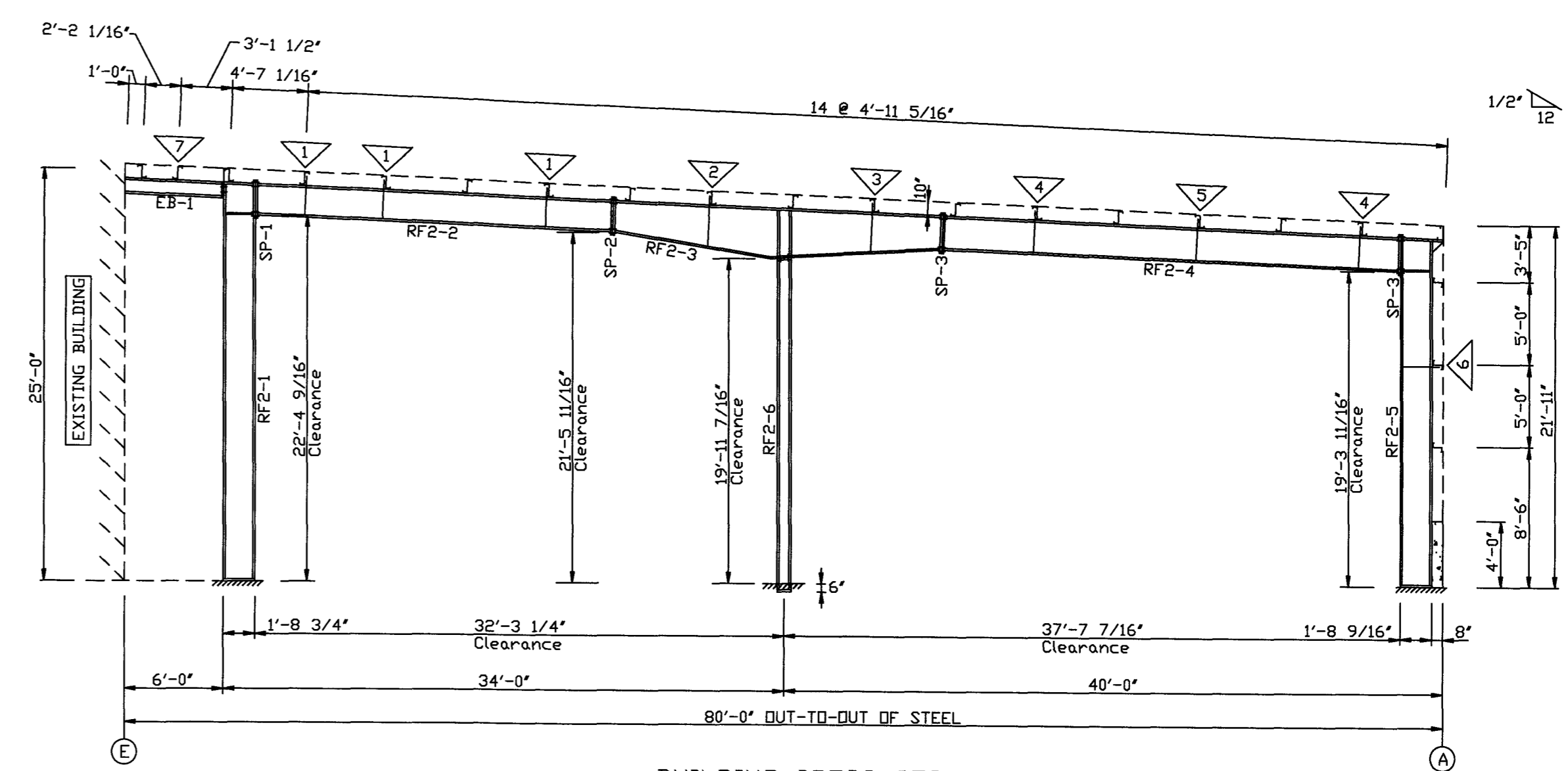
BUILDING CROSS SECTION FOR FRAME LINE 2-3

SPLICE BOLTS				CAP PLATE BOLTS			
Splice Mark	Quan	Top/Bot/Int	Type Dia Length	Col Mark	Qnt	Type Dia Length	
SP-1	4	4	0 A325 0.625 2.00	RF2-6	4	A325 0.625 1.50	
SP-2	4	4	0 A325 0.625 2.00	EB-1	4	A325 0.625 2.00	
SP-3	4	4	0 A325 0.625 1.75				

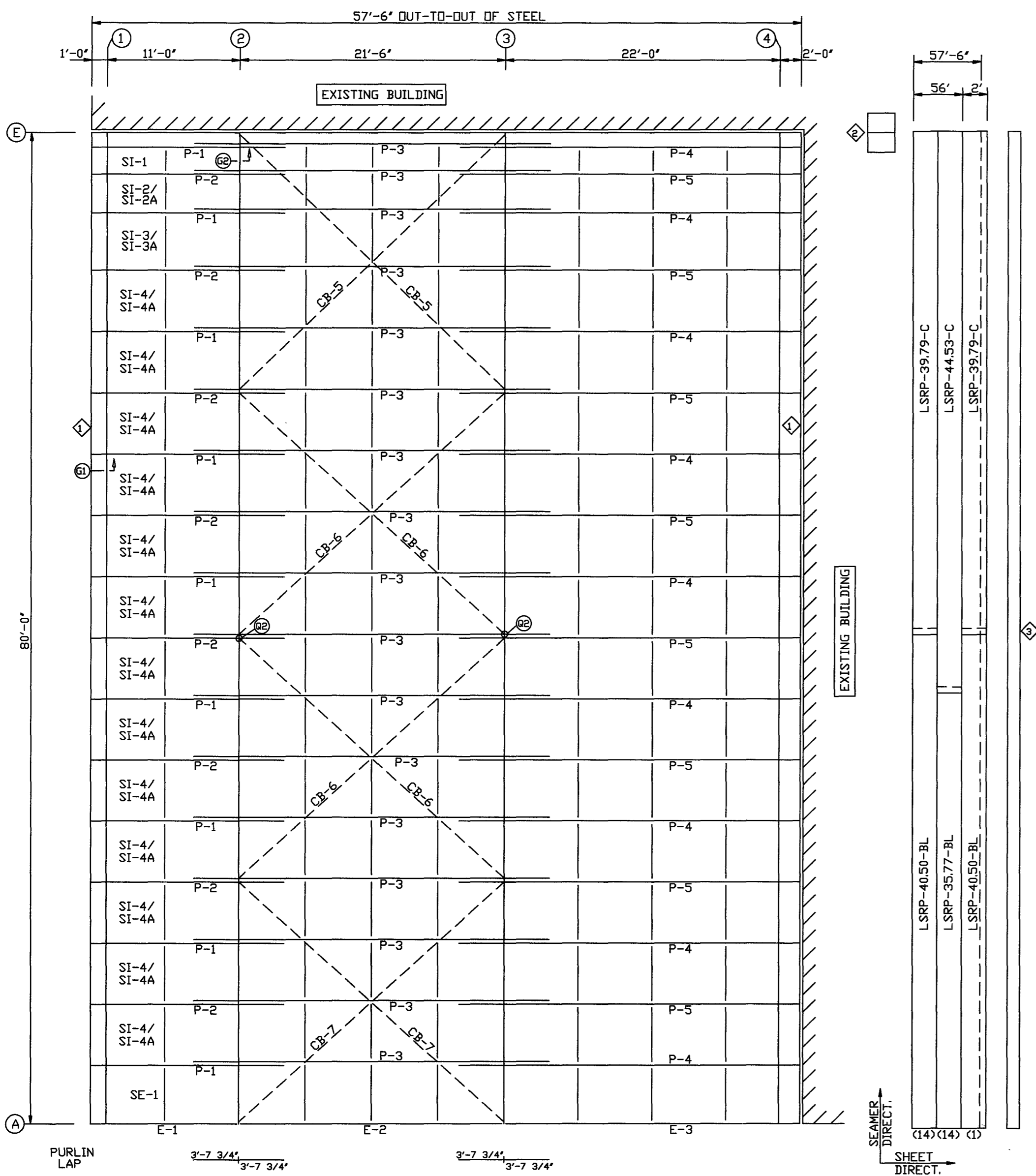
FLANGE BRACE TABLE			
V ID	MARK	LENGTH (in)	SIDES
1	FB5A	42.630	1
2	FB12A	47.750	1
3	FB11A	46.750	1
4	FB7A	43.630	1
5	FB8A	43.750	1
6	FB3A	42.000	1
7	FB2A	26.380	1

FBxA = 2"x2"x1/8"  
FBxB = 2-1/2"x2-1/2"x3/16"

MEMBER SIZE TABLE (in)						
MARK	WEIGHT	WEB DEPTH	WEB PLATE	OUTSIDE FLANGE	INSIDE FLANGE	
		START/END THICK	THICK LENGTH	V x T x LENGTH	V x T x LENGTH	
RF2-1	714	20.0/20.0	0.125 24.5	8 x 1/4" x 261.2	8 x 3/8" x 174.0	
RF2-2	681	20.0/20.0	0.188 24.5	6 x 1/2" x 262.2	6 x 1/4" x 118.3	
RF2-3	750	20.0/20.0	0.188 22.2	6 x 1/4" x 239.1	6 x 1/4" x 261.4	
RF2-4	690	22.0/22.0	0.188 120.0	6 x 1/4" x 335.4	6 x 1/4" x 116.1	
RF2-5	480	25.0/22.0	0.188 120.0	6 x 1/4" x 28.3	6 x 1/4" x 124.4	
RF2-6	503	20.0/20.0	0.125 227.9	6 x 1/4" x 252.7	6 x 5/16" x 227.9	



BUILDING CROSS SECTION FOR FRAME LINE 4



ROOF FRAMING PLAN

MEMBER TABLE		
MARK	PART	LENGTH
F-1	10ESL114	11'-11" 5/8"
F-2	10ESL114	21'-5" 3/4"
F-3	10ESL114	23'-11" 5/8"
DJ-5	8x25Z16	17'-1"
G-10	8x25Z16	13'-1" 1/2"
G-11	8x25Z16	23'-9" 1/2"
G-12	8x25Z16	24'-9" 1/2"
CB-4	CB0375	30'-2"

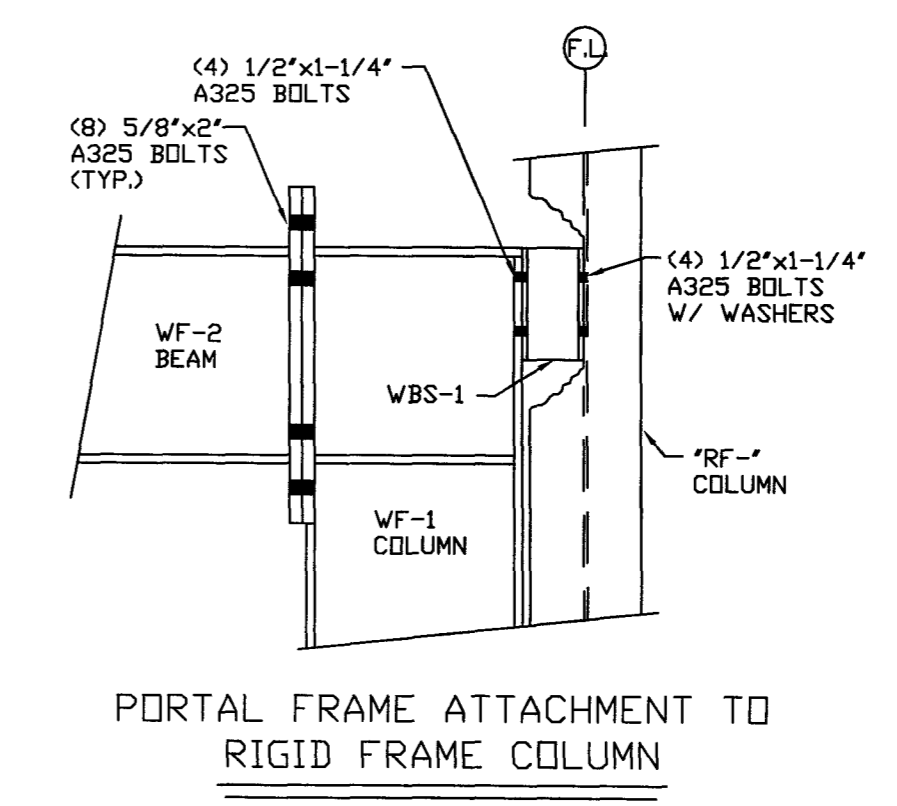
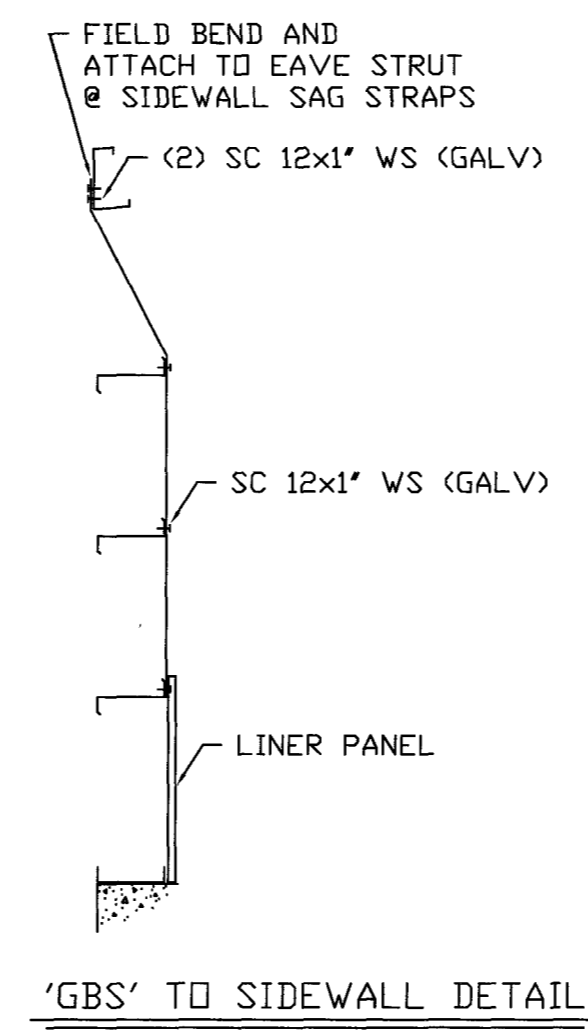
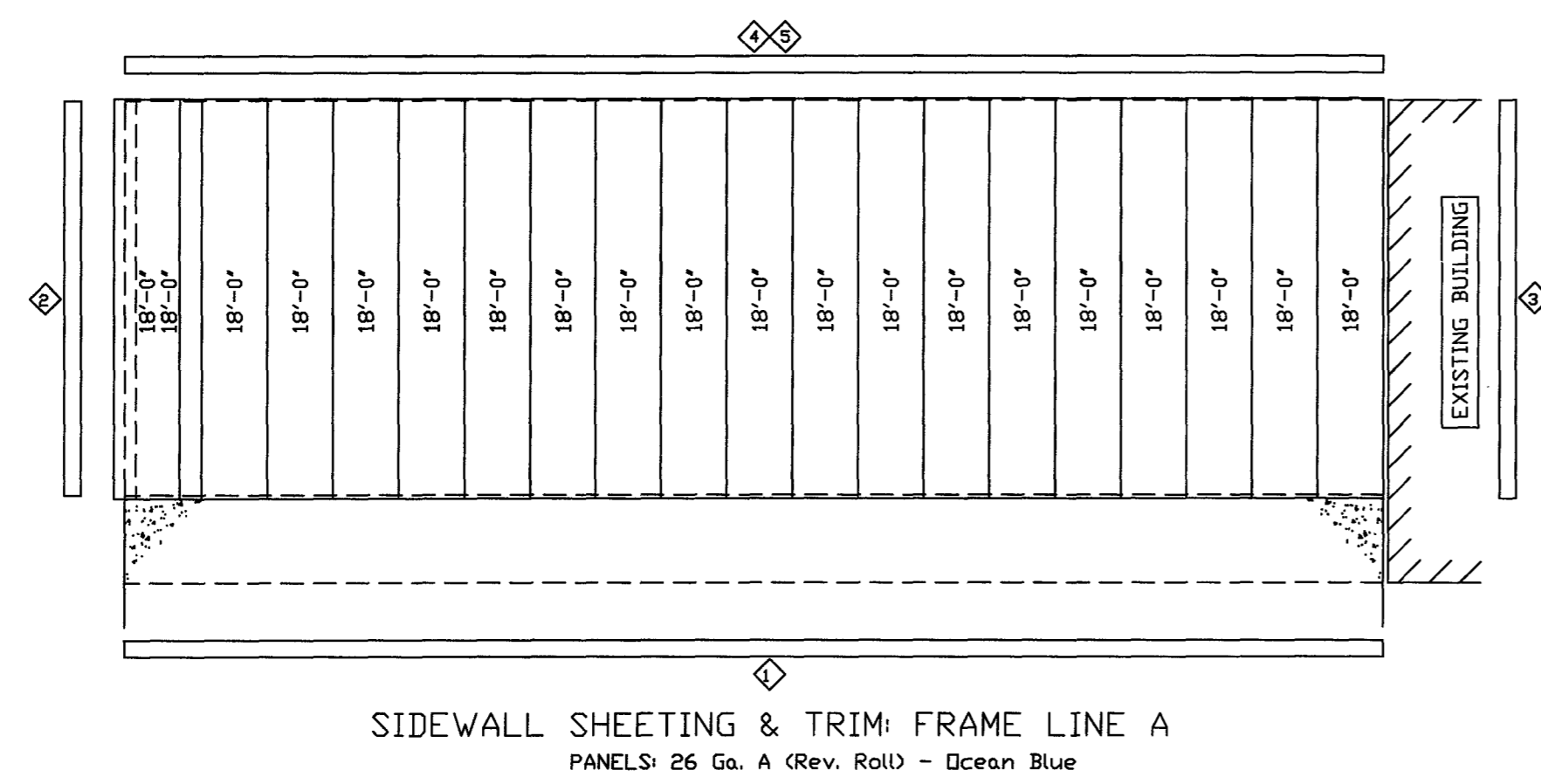
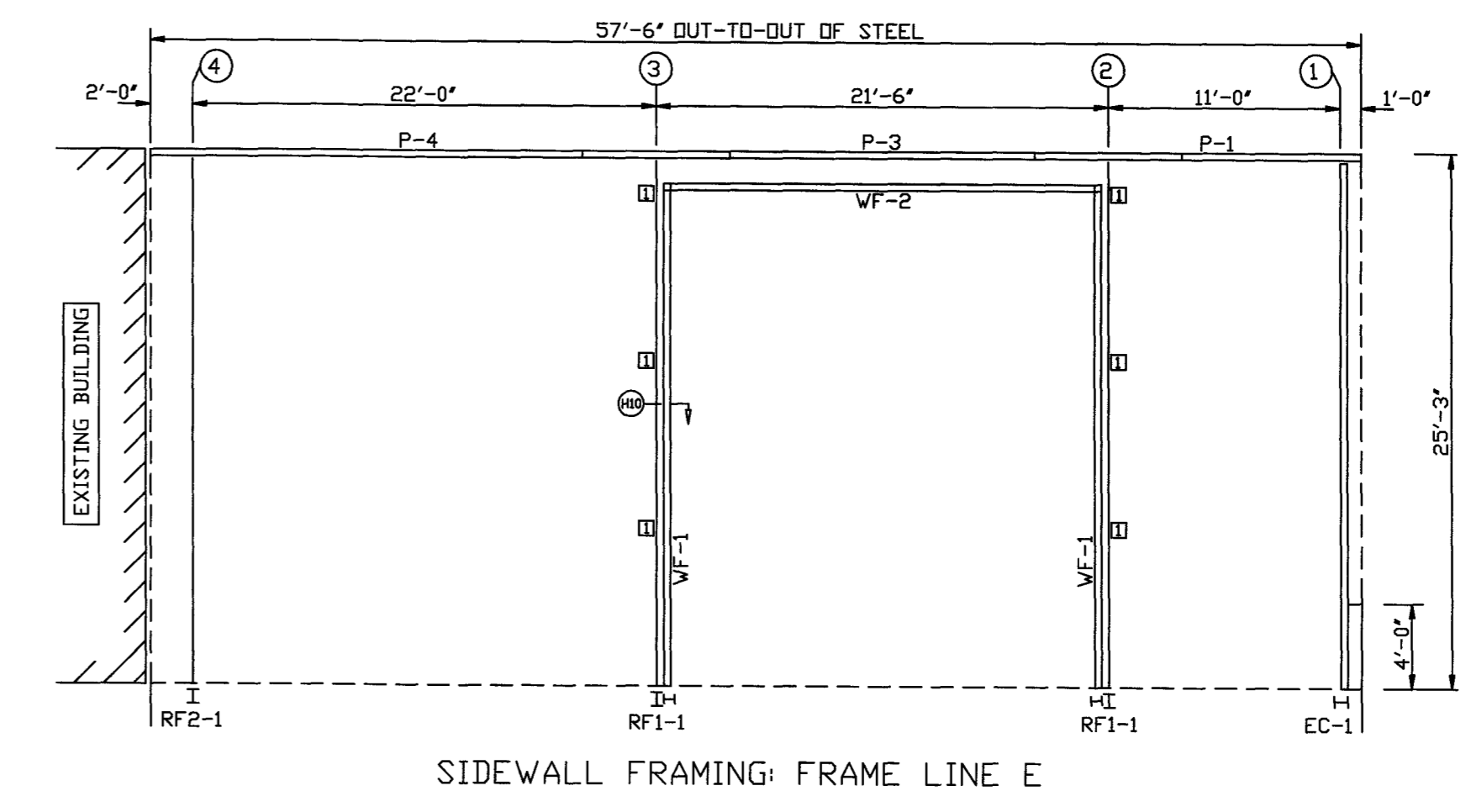
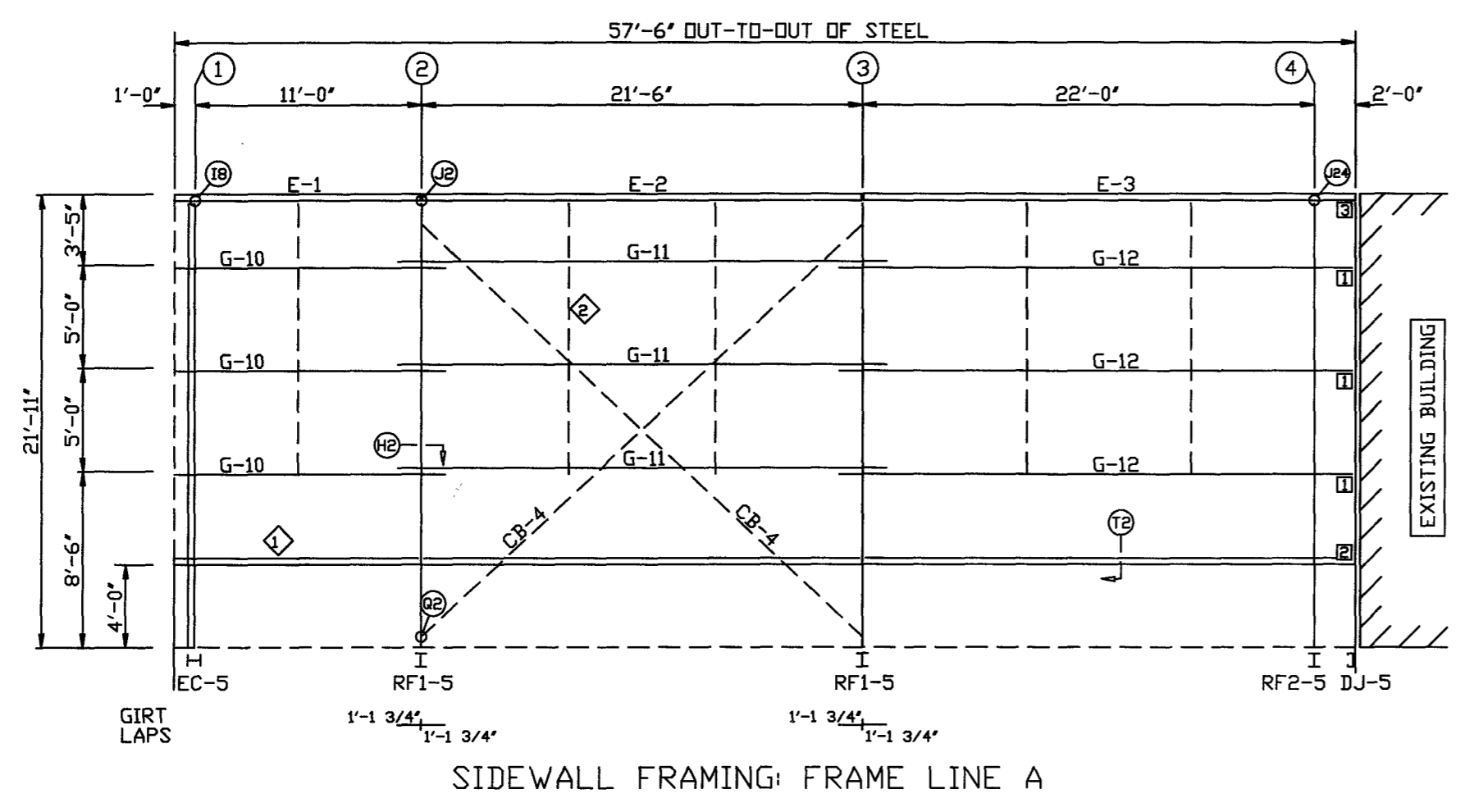
ANGLE TABLE		
MARK	PART	LENGTH
1	FC 10	10'-3"
2	GBS 1	13'-11"

CONNECTION PLATES		
MARK	PART	LENGTH
1	JGC 1	10'-3"
2	JGC 2	10'-3"

TRIM TABLE		
MARK	PART	LENGTH
1	SP 10	10'-3"
2	RROCT 10	10'-3"
3	DH 10	10'-3"
4	FG 10	10'-3"
5	SPF 2	10'-3"

MEMBER TABLE		
MARK	PART	LENGTH
WF-1	W12852	24'-1"
WF-2	W16652	18'-5" 5/8"
P-1	10x25Z16	15'-7" 1/2"
P-3	10x25Z14	28'-9" 1/2"
P-4	10x25Z12	27'-7" 1/2"

CONNECTION PLATES		
MARK	PART	LENGTH
1	6	WBS 1



PORTAL FRAME MEMBER TABLE		
MARK	PART	MATERIAL SIZE
WF-1	W12852	0.125"x11.38" WEB, 0.313" x 8" FLANGE
WF-2	W16652	0.125"x15.38" WEB, 0.313" x 6" FLANGE

RELEASED FOR CONSTRUCTION

<p>REFER TO FRAME X-SECTIONS FOR SPECIFIC FLANGE BRACE LOCATIONS.</p> <p>H2 WALL GIRTS TO FRAME COLUMN</p>	<p>H10 WIND BENT OR WIND COLUMN TO BUILDING COLUMN</p> <p>H10 WIND BENT OR WIND COLUMN TO BUILDING COLUMN</p>	<p>H18 EAVE STRUT TO ENDWALL RAFTER</p> <p>H18 EAVE STRUT TO ENDWALL RAFTER</p>	<p>H20 EAVE STRUT TO RIGID FRAME BYPASS CONDITION</p> <p>H20 EAVE STRUT TO RIGID FRAME BYPASS CONDITION</p>	<p>H24 EAVE STRUT TO RIGID FRAME BYPASS CONDITION</p> <p>H24 EAVE STRUT TO RIGID FRAME BYPASS CONDITION</p>	<p>H22 DIAGONAL CABLE, EYEBOLT END</p> <p>H22 DIAGONAL CABLE, EYEBOLT END</p>	<p>H26 SECTION THRU WALL PANEL AND CONCRETE FOUNDATION</p> <p>H26 SECTION THRU WALL PANEL AND CONCRETE FOUNDATION</p>
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REV.	DESCRIPTION	DATE	DRAFT	ENG.
3				
2				
1				
INITIAL DRAWING: RELEASED FOR CONSTRUCTION				CURRENT REVISION: 0
PACKAGE INDUSTRIES, INC.		Biskup Construction Inc.		
PROJECT	Hale Trailer	SIDEWALL FRAMING & SHEETING		
ID	11283	DESIGN: CWE	DESIGN CHECK: CWE	
PROJECT	20 Pinetree Industrial Park	DRAFT: RPG	DRAFT CHECK: TMZ	
ADDRESS	Portland, ME 04102	DATE: 5/05/11	SCALE: NONE	SHEET: FSHT-2

BOLT TABLE				
FRAME LINE 1				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-2	4	A325	5/8"	1 1/2"
ER-2/ER-3	4	A325	5/8"	1 1/2"
Columns/Raft	4	A325	1/2"	1 1/2"

MEMBER TABLE		
FRAME LINE 1		
MARK	PAR	LENGTH
FC-1	WBX13	22'-8 9/16"
FC-2	WBX13	22'-11 1/16"
FC-3	WBX13	22'-1 1/16"
FC-4	WBX13	21'-3 1/16"
FC-5	WBX13	20'-5 9/16"
ER-1	WBX13	24'-11 13/16"
ER-2	WBX13	20'-0 3/16"
ER-3	WBX13	25'-0 13/16"
DJ-1	8x25C16	9'-1 3/4"
DJ-2	8x25C16	9'-1 3/4"
DJ-3	8x25C16	14'-1 3/4"
DJ-4	8x25C16	19'-8 7/8"
DJ-5	8x25C16	20'-0 3/16"
DH-1	8x25C16	3'-4 3/8"
DH-2	8x25C16	12'-0"
DH-3	8x25C16	6'-3 1/2"
G-1	8x25Z16	25'-3 1/2"
G-2	8x25Z16	21'-3 1/2"
G-3	8x25Z16	10'-3 1/2"
G-4	8x25Z16	23'-3 1/2"
G-5	8x25Z16	23'-3 1/2"
G-6	8x25Z16	20'-11 1/2"
G-7	8x25Z16	24'-4"
G-8	8x25Z16	21'-11 1/2"
G-9	8x25Z16	22'-2"

ANGLE TABLE		
FRAME LINE 1		
ID	MARK	LENGTH (in)
1	RA 10	10'-0"
2	FC 10	10'-0"
3	GBS 2	16'-9"

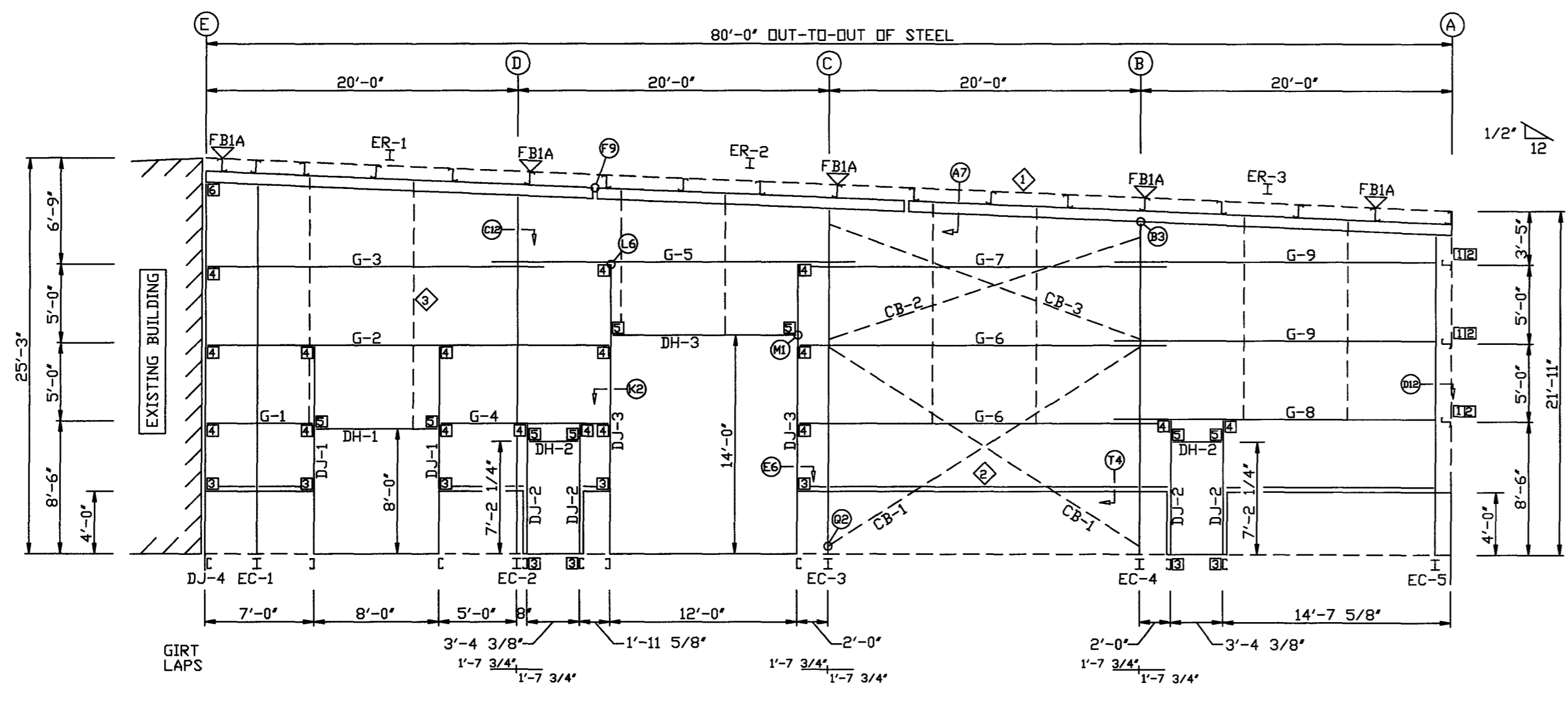
  

FLANGE BRACE TABLE		
FRAME LINE 1		
V ID	MARK	LENGTH (in)
1	FBIA	21,000

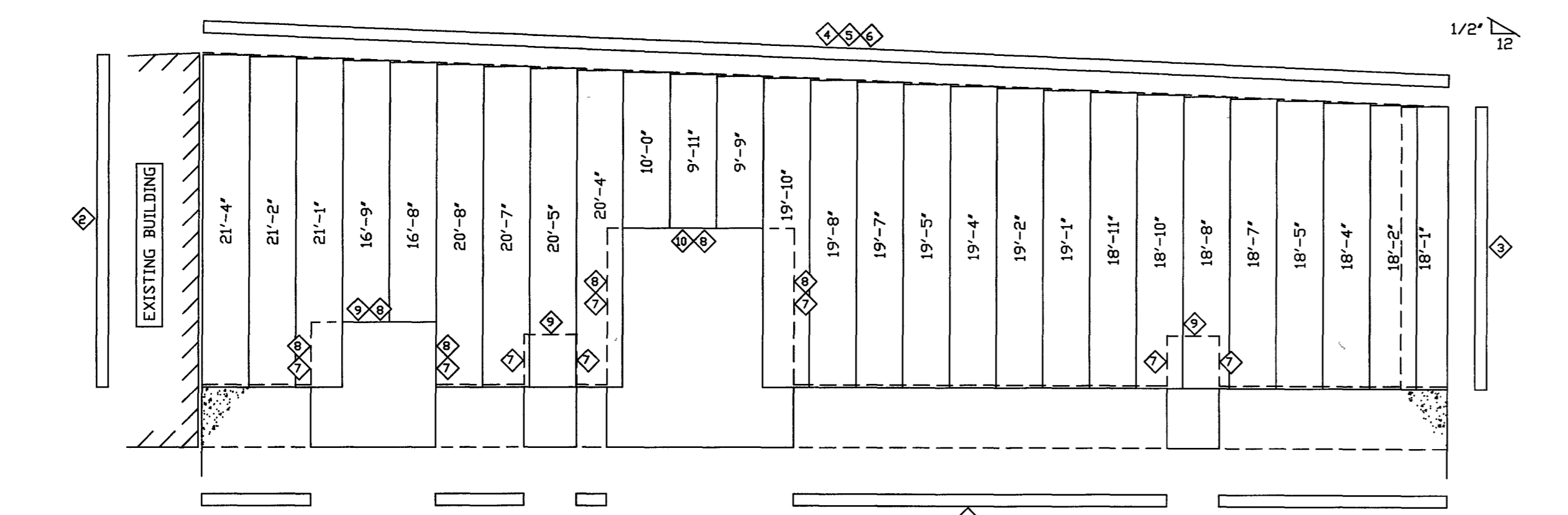
  

CONNECTION PLATES		
FRAME LINE 1		
ID	QUAN	MARK/PART
1	3	BGC 8
2	3	CFC 3
3	1	JGC 3
4	17	JGC 1
5	8	JGC 2
6	1	PL 8

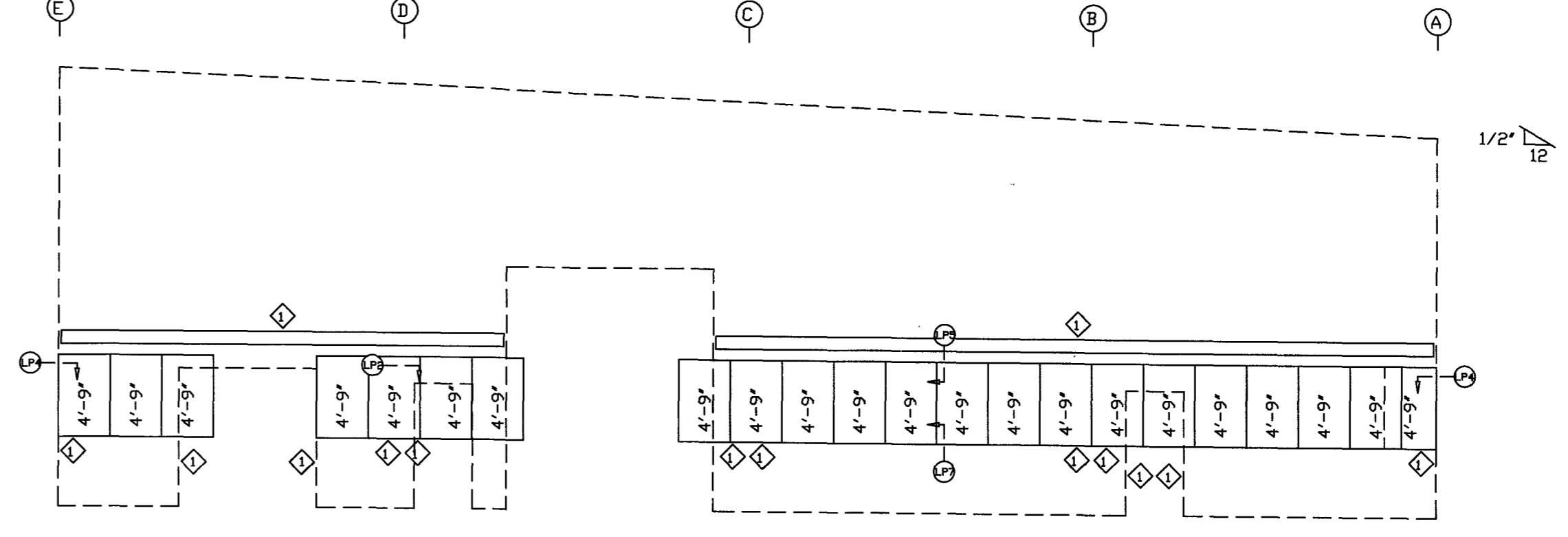
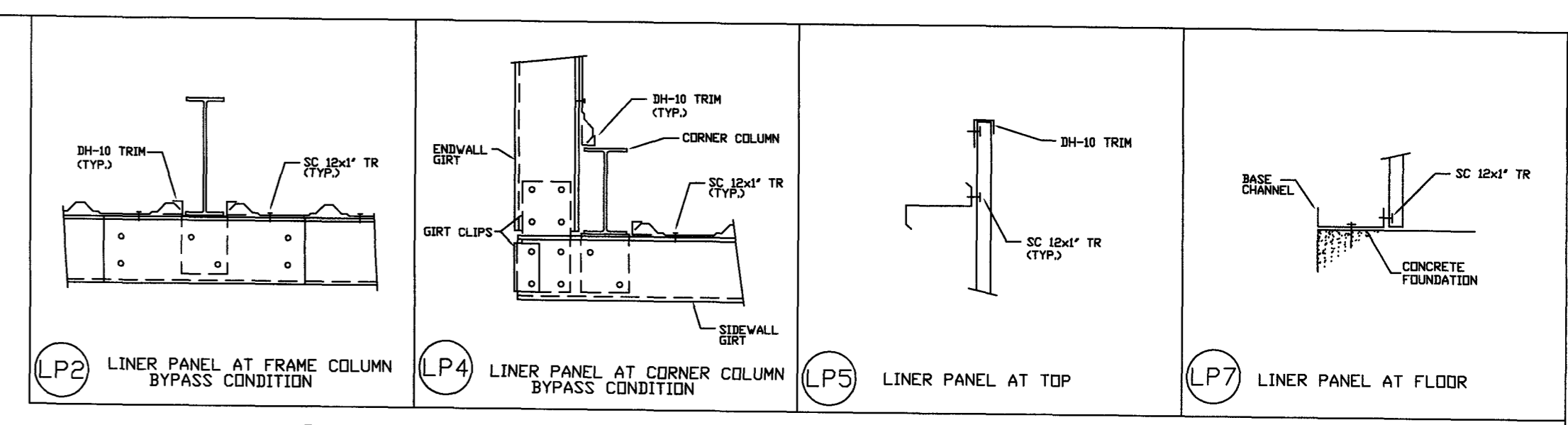
TRIM TABLE		
FRAME LINE 1		
ID	QUAN	LENGTH
1	10	10'-3"
2	10	10'-3"
3	10	10'-3"
4	10	10'-3"
5	10	10'-3"
6	10	10'-3"
7	10	10'-3"
8	10	10'-3"
9	10	10'-3"
10	12	12'-0"



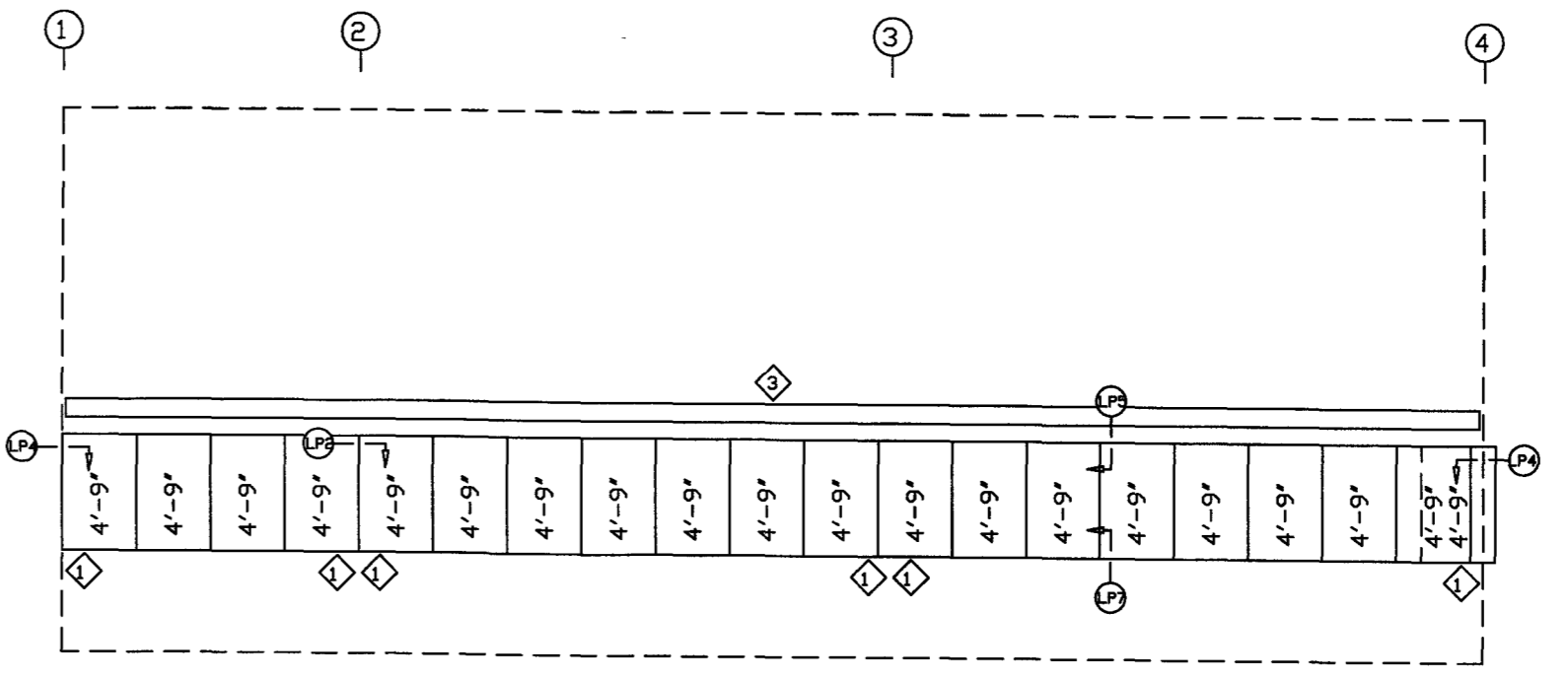
ENDWALL FRAMING: FRAME LINE 1



ENDWALL SHEETING & TRIM: FRAME LINE 1  
PANELS: 26 Ga. A (Rev. Roll) - Ocean Blue

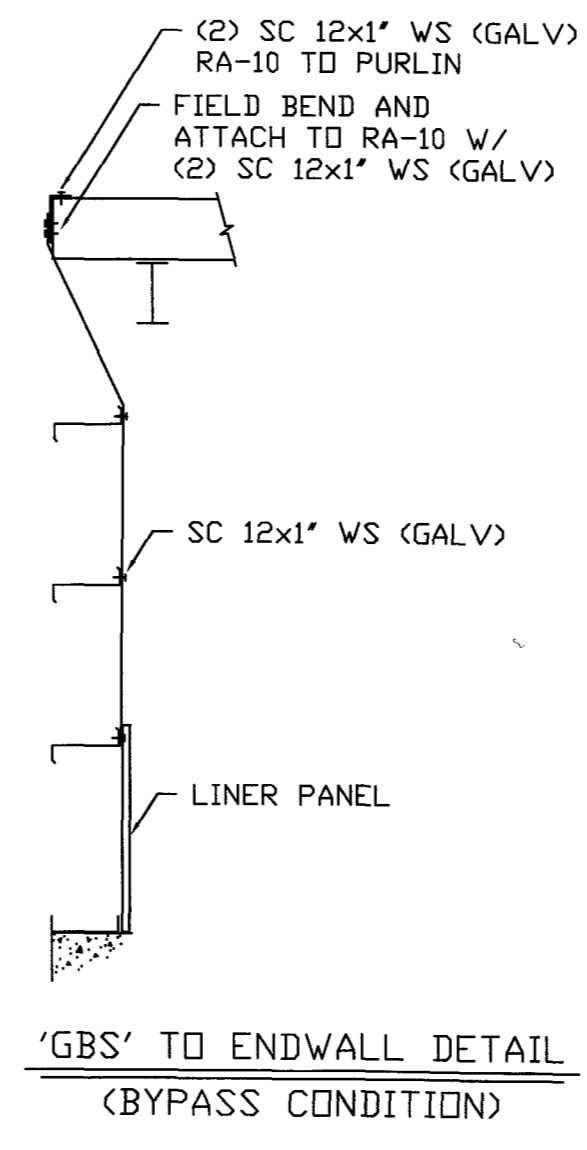
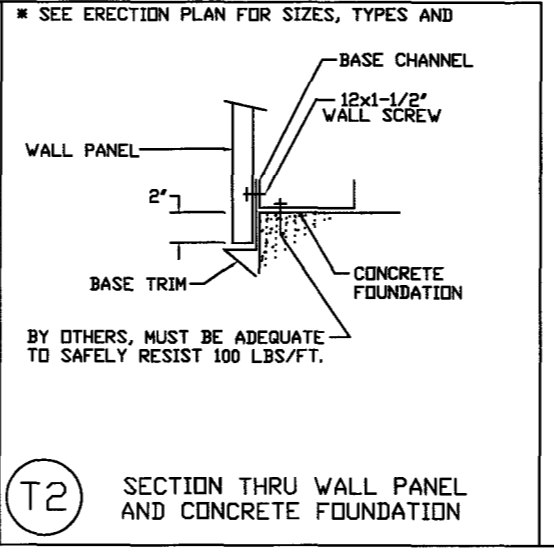
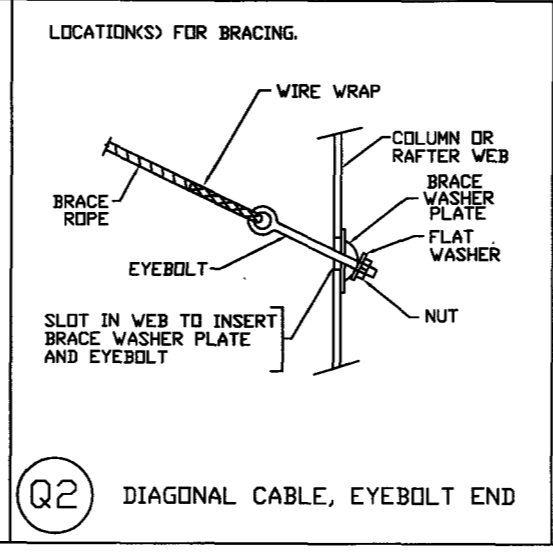
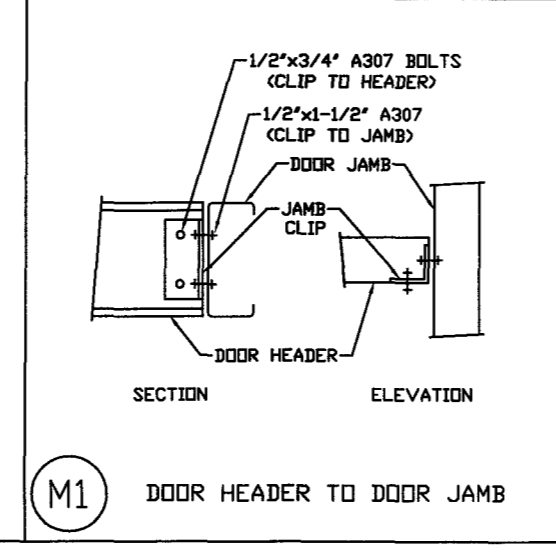
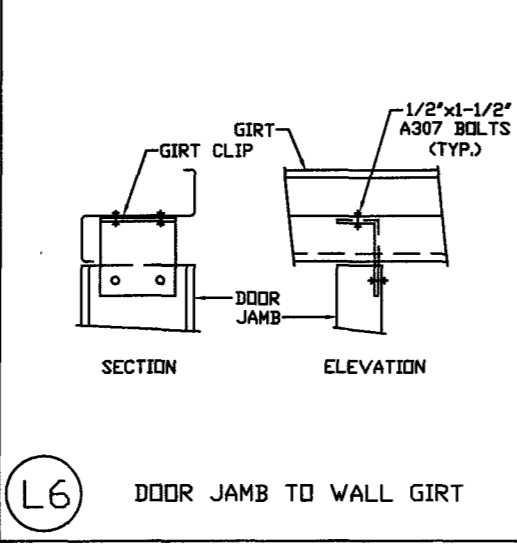
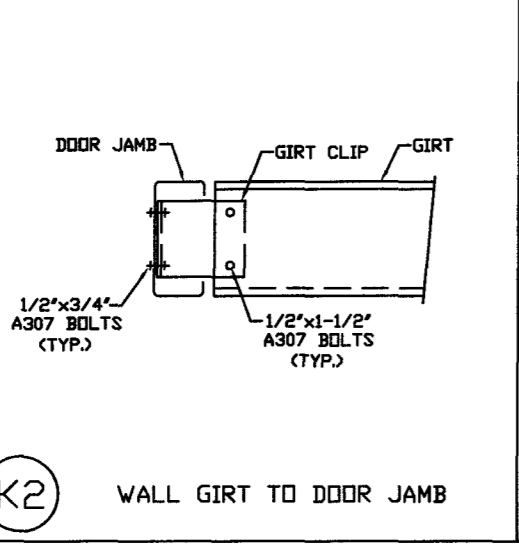
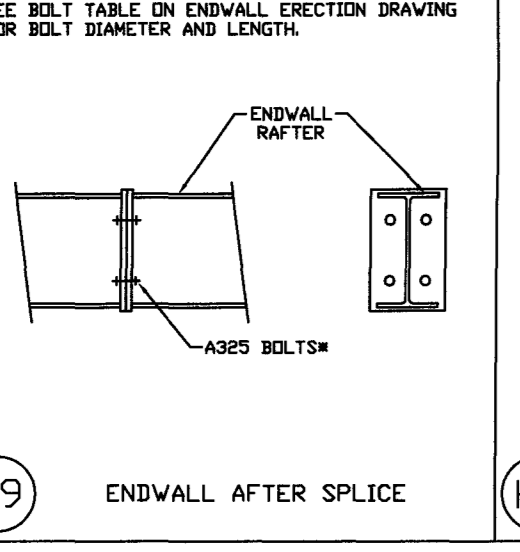
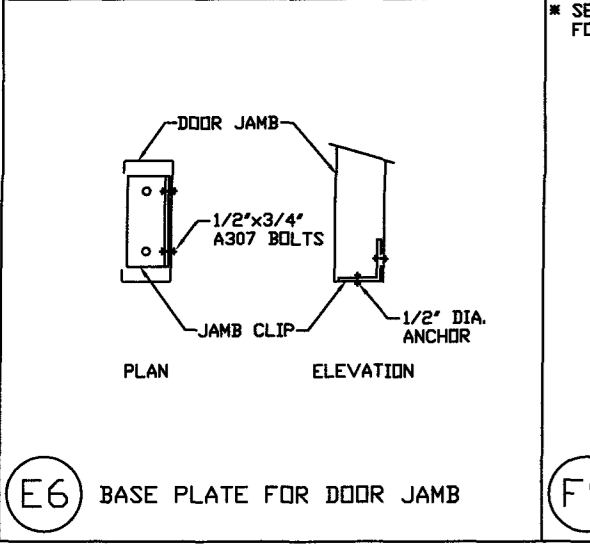
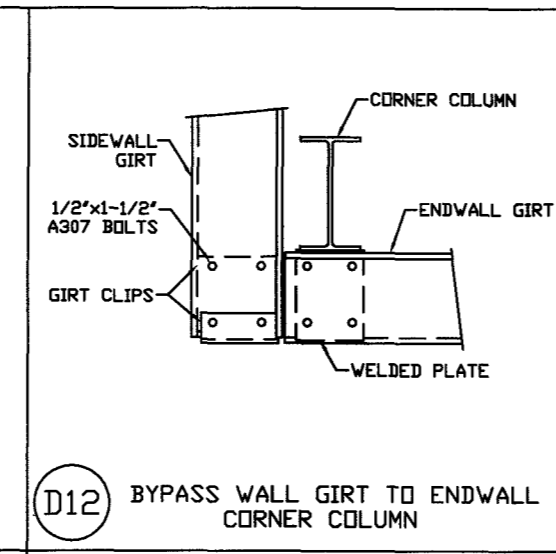
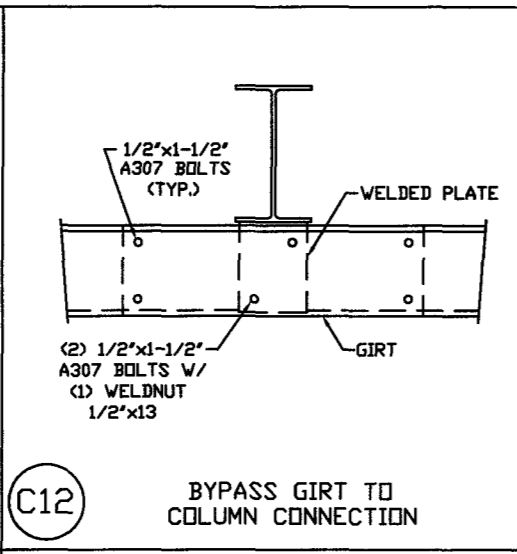
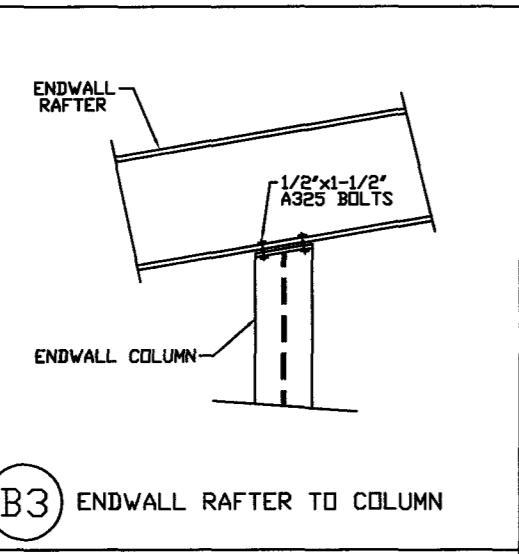
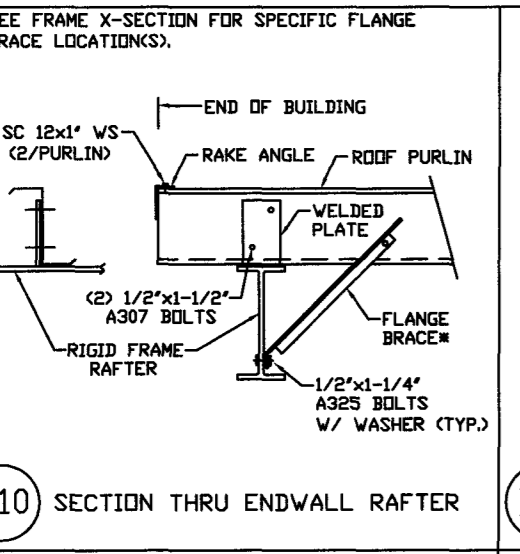
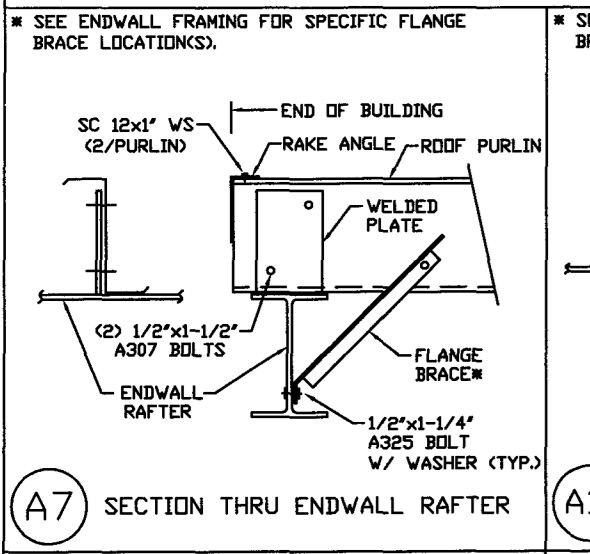


ENDWALL LINER PANEL: FRAME LINE 1  
PANELS: 26 Ga. A (Rev. Roll) - Polar White



SIDEWALL LINER PANEL: FRAME LINE A  
PANELS: 26 Ga. A (Rev. Roll) - Polar White

TRIM TABLE		
FRAME LINE 1 A		
ID	QUAN	LENGTH
1	DH 10	10'-3"



RELEASED FOR CONSTRUCTION

REV.	DESCRIPTION	DATE	DRAFT	ENG.
3				
2				
1				

INITIAL DRAWING: RELEASED FOR CONSTRUCTION		CURRENT REVISION: 0	
PACKAGE INDUSTRIES, INC.		Biskup Construction Inc.	
PROJECT: Hale Trailer	ENDWALL FRAMING & SHEETING		
ID: 11283	DESIGN: CWE	DESIGN CHECK: CWE	
PROJECT: 20 Pinetree Industrial Park	DRAFT: RPG	DRAFT CHECK: TMZ	
ADDRESS: Portland, ME 04102	DATE: 5/05/11	SCALE: NONE	SHEET: FSHT-3