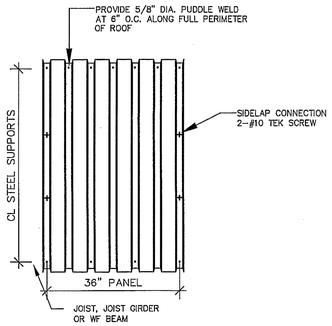
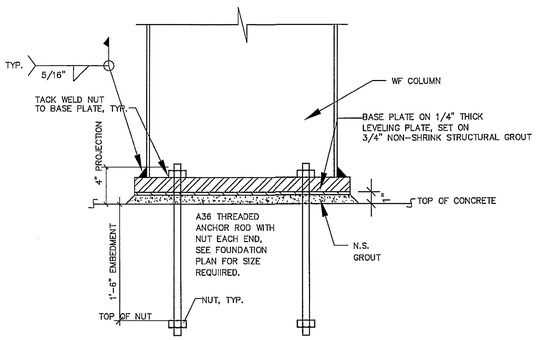


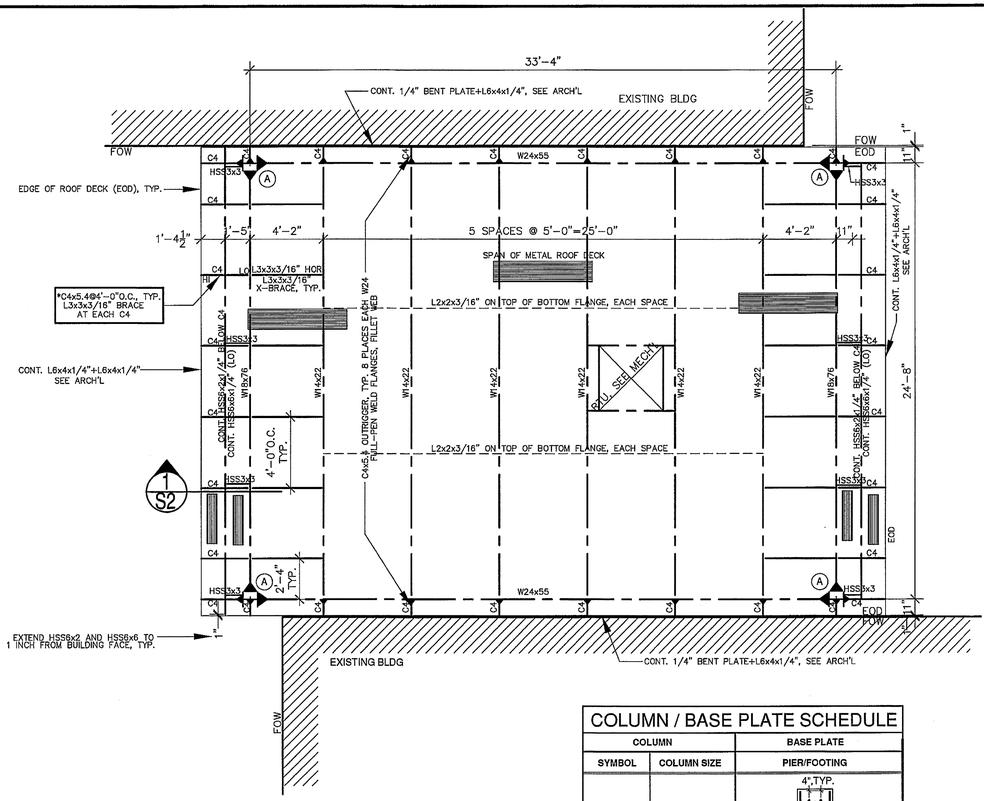
TYP. 36/4 ROOF DECK ATTACHMENT DETAIL

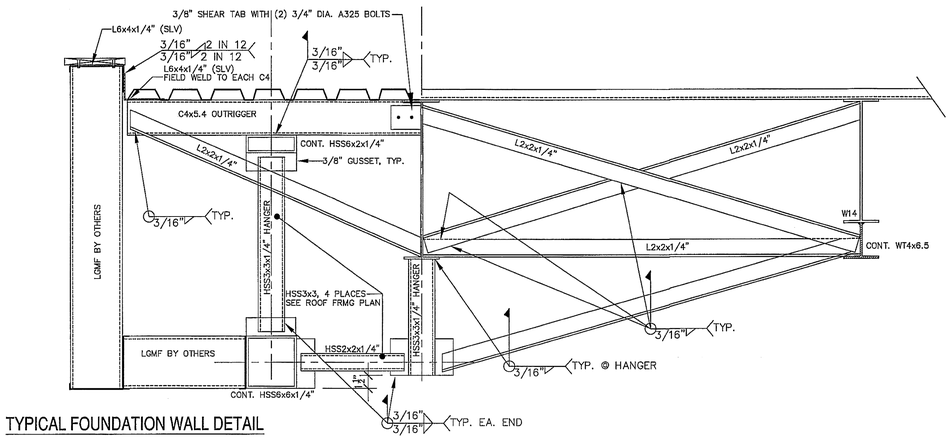
N.T.S.



TYPICAL COLUMN BASE DETAIL

N.T.S.

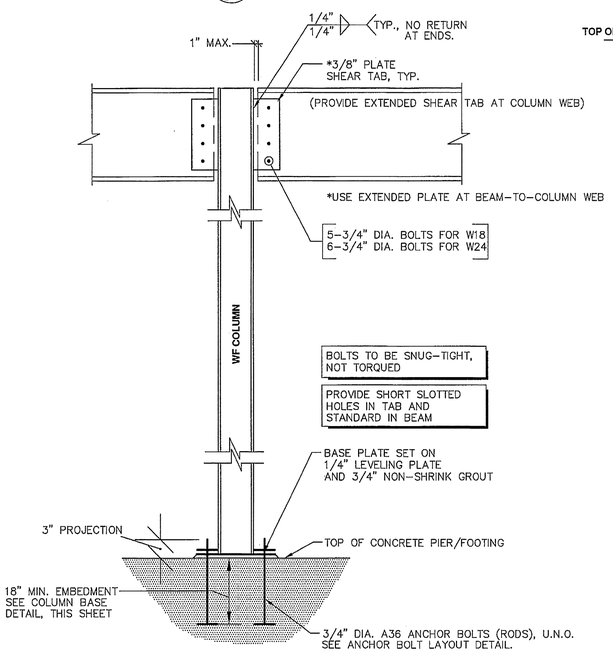




TYPICAL FOUNDATION WALL DETAIL

SECTION 1/S2

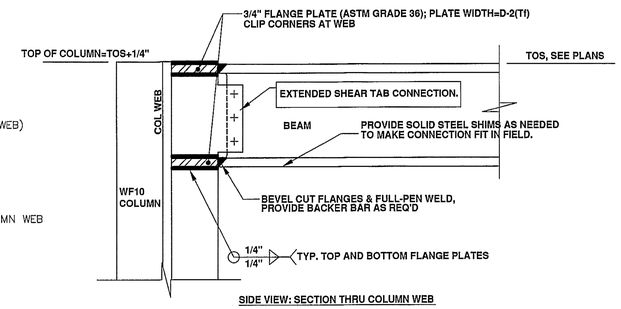
N.T.S



(ALL BEAMS TO ALIGN WITH CENTER OF COLUMN, TYP.)

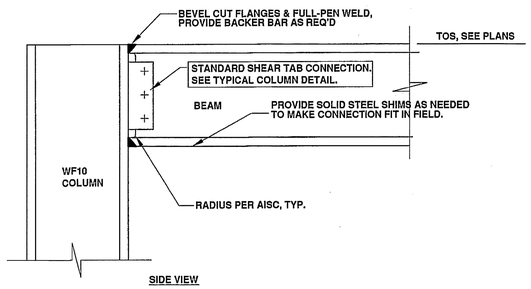
TYPICAL COLUMN DETAIL, UNLESS NOTED

N.T.S.



TYPICAL W18 MOMENT CONNECTION DETAIL

N.T.S



TYPICAL W24 MOMENT CONNECTION DETAIL

N.T.S

COLUMN / BASE PLATE SCHEDULE

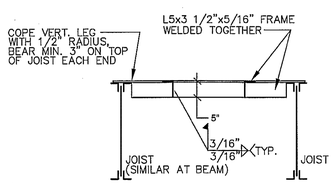
COLUMN		BASE PLATE
SYMBOL	COLUMN SIZE	PIER/FOOTING
		4" TYP.
A	W10x49	3/4" x 12" x 1'-0"

GENERAL NOTE TO CONTRACTOR

ALL STEEL FRAMING IS NON-SELF SUPPORTING AND REQUIRES INTERACTION WITH OTHER ELEMENTS NOT CLASSIFIED AS STRUCTURAL STEEL TO PROVIDE THE REQUIRED STABILITY AND RESISTANCE TO LATERAL FORCES. THE STEEL FRAMING SHALL BE TEMPORARILY BRACED BY THE CONTRACTOR UNTIL ALL BRACING, FLOOR, ROOF DECKS, AND CONCRETE HAVE BEEN INSTALLED AND ALL CONNECTIONS BETWEEN THESE ELEMENTS HAVE BEEN MADE. ALL FOUNDATION WALLS SHALL HAVE ADEQUATE TEMPORARY BRACING BEFORE BACKFILL IS PLACED AGAINST THEM. TEMPORARY BRACING SHALL NOT BE REMOVED UNTIL STEEL BRACING, ROOF DECK, AND FLOOR DECKS ARE FULLY INSTALLED.

ROOF FRAMING NOTES:

1. DESIGN CANOPY ROOF SNOW LOAD = 60psf, PER 2006 IBC; Pg=60psf, I=1.0, CE=1.2, Ct=1.2.
2. DESIGN DEAD LOAD = 25psf.
3. COLUMN SIZE IS NOTED ON PLANS, SEE COLUMN SCHEDULE.
4. REFERENCE ELEVATION TOP OF STRUCTURAL STEEL (TOS) IS NOTED ON PLANS.
5. REFER TO ARCHITECTURAL, MECHANICAL, AND ELECTRICAL PLANS FOR ALL PENETRATIONS REQUIRED.
6. STRUCTURAL STEEL TO BE ASTM A992, GRADE 50ksi. CHANNELS, PLATES, ANGLES TO BE ASTM A36.
7. STRUCTURAL STEEL TO BE NON-PRIMED AND NON-PAINTED.
8. ROOF DECK TO BE 1 1/2" DEEP x 22 GAGE GALVANIZED (G60) B-ROOF DECK.
9. ROOF DECK ATTACHMENT AS FOLLOWS USING ¾" DIAMETER PUDDLE WELDS: 36/4 WITH 2-#10 TEK SCREWS EACH SIDELAP. WELD 6" O.C. AT ALL BUILDING EDGES. USE WELDING WASHERS AS REQ'D.
10. DESIGN BUILDING CODE: 2006 IBC (INTERNATIONAL BUILDING CODE).
11. ALL STRUTURAL STEEL TO BE ASTM A572 OR A992; GRADE 50. HSS SHAPES TO BE ASTM A500, GRADE B.
12. ▶ INDICATES MOMENT CONNECTION, SEE DETAILS.
13. WELDS E70 SERIES.
14. ALL PENETRATIONS THROUGH THE STRUCTURE TO BE VERIFIED BY THE MECHANICAL, ELECTRICAL, AND PLUMBING CONTRACTORS PRIOR TO PLACING. ALL PENETRATIONS SHALL BE SUBJECT TO APPROVAL BY THE ARCHITECT/ENGINEER.
15. STRUCTURAL STEEL TO BE FURNISHED BY A FABRICATOR WHO HAS BEEN REVIEWED AND ACCEPTED BY SRG ENGINEERING PRIOR TO AWARDING CONTRACT FOR WORK.
16. ALL WELDING TO BE DONE BY AISC QUALIFIED WELDERS, SUBMIT CERTIFICATE TO SRG ENGINEERING FOR REVIEW PRIOR TO FABRICATION: NO EXCEPTION.
17. CONTRACTOR TO SUBMIT COMPLETE SHOP DRAWINGS FOR ALL PARTS OF WORK. FABRICATION OR ERECTION OF NEW STRUCTURAL ELEMENTS SHALL NOT COMMENCE WITHOUT REVIEW BY THE ARCHITECT AND ENGINEER. SUBMIT TWO (2) COPIES AND ONE (1) SEPIA; COPY WILL BE REVIEWED AND ONE COPY AND SEPIA WILL BE RETURNED.
18. THIS STRUCTURE REQUIRES "TESTING AND INSPECTION" PER CHAPTER 17 OF THE 2006 EDITION OF THE IBC ENTITLED "STRUCTURAL TESTS AND SPECIAL INSPECTIONS", NO EXCEPTION. GENERAL CONTRACTOR MUST CONTACT SRG ENGINEERING A MINIMUM OF 48 HOURS (2 BUSINESS DAYS) BEFORE ANY "SPECIAL INSPECTIONS" CAN BE PERFORMED.
19. THIS NEW STRUCTURE HAS BEEN DESIGNED FOR WIND AND SEISMIC LOADING CONDITIONS SET FORTH IN THE 2006 INTERNATIONAL BUILDING CODE; WIND CONTROLS "X-DIRECTION"---Vmax=6.2klps, SEISMIC CONTROLS "Y-DIRECTION"---Vmax=2.2k.
20. REFERENCE ELEVATION TOP OF STRUCTURAL STEEL (TOS) = 115'-0", UNLESS NOTED OTHERWISE ON PLANS.
21. STRUCTURAL STEEL FABRICATION, ERECTION AND CONNECTION DESIGN SHALL CONFORM TO THE LATEST EDITION OF AISC "SPECIFICATION FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL".



NOTE:
USE AT ALL ROOF DRAINS, BELOW CURB SUPPORTS FOR HVAC, AND OTHER ROOF OPENINGS. SEE ARCH'L AND MECH'L DWGS. FOR SIZE REQUIRED AND LOCATIONS

ROOF OPENING DETAIL

N.T.S.

SEISMIC LOADS:
S1=0.08 Ss=0.32
SEISMIC USE GROUP -- I
SEISMIC DESIGN CATEGORY - B, CATEGORY II BLDG
SITE CLASS A
Sds=0.171, Sd1=0.043, I=1.00
STRUCTURAL SYSTEM: ORDINARY MOMENT FRAMES EACH DIRECTION.
ANALYSIS PROCEDURE - EQUIVALENT LATERAL FORCE
BUILDING WEIGHT=12 KIPS

WIND LOAD:
BASIC WIND SPEED 100 MPH, EXP. B, I=1.00
MWFRS: 14.2 PSF MAX.
COMPONENTS & CLADDING: 25 PSF MAX.

ROBERT J. FOSTER - ARCHITECT
36 Groveside Road, Portland, Maine 04102
(207) 761-3822

STATE OF MAINE
STEVEN R. GRANT No. 6825
LICENSED PROFESSIONAL ENGINEER

Project Title

CROSS INSURANCE NEW CONNECTOR

Portland, Maine

Key Plan

Mark	Date	Description

Issue Date:
11.14.2008: FOR CONSTRUCTION

SRG ENGINEERING, INC. STRUCTURAL ENGINEERS
P.O. BOX 925
GRAY, ME 04039
TEL: (207) 657-7323
FAX: (207) 657-7342
EMAIL: SRG@SRGENG.COM

Drawing Title

ROOF FRAMING

Drawing Number

S2.0