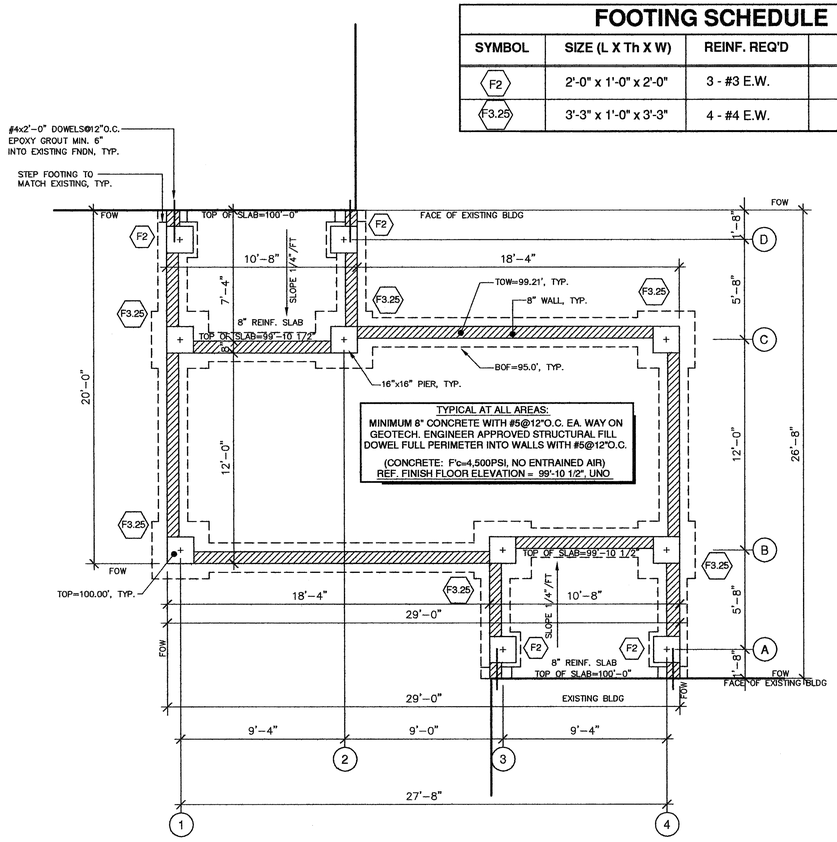
FOOTING SCHEDULE

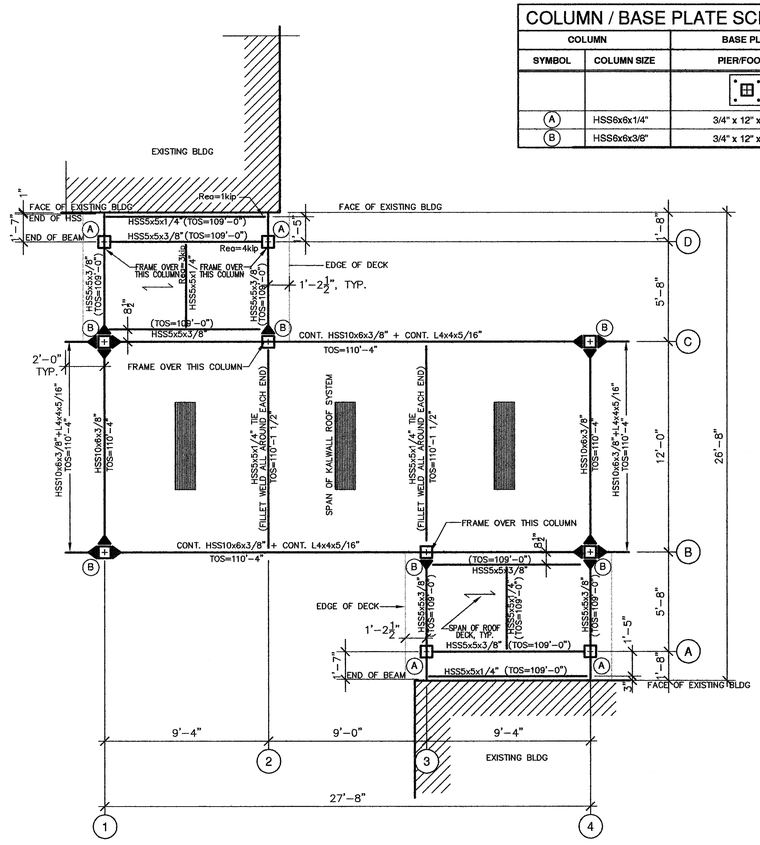
SYMBOL	SIZE (L X Th X W)	REINF. REQ'D	COMMENTS
F2	2'-0" x 1'-0" x 2'-0"	3 - #3 E.W.	BOTTOM REINF.
F3.25	3'-3" x 1'-0" x 3'-3"	4 - #4 E.W.	BOTTOM REINF.



FOUNDATION PLAN
SCALE: 1/4"=1'-0"

COLUMN / BASE PLATE SCHEDULE

COLUMN		BASE PLATE
SYMBOL	COLUMN SIZE	PIER/FOOTING
		3" TYP.
A	HSS6x6x1/4"	3/4" x 12" x 1'-0"
B	HSS6x6x3/8"	3/4" x 12" x 1'-0"



ROOF FRAMING PLAN
SCALE: 1/4"=1'-0"

FOUNDATION NOTES:

1. DESIGN MAXIMUM ASSUMED SOIL BEARING PRESSURE=1,500psf; TO BE VERIFIED BY PROJECT GEOTECHNICAL ENGINEER.
2. CONCRETE: WALLS, FOOTINGS & PIERS: F'c=4,000 P.S.I., 3/4" AGGREGATE, MAX. W/C=0.50, TYPE I OR II CEMENT; 4 TO 6% ENT. AIR, SLUMP MIN. 1", MAX. 4".
   SLABS: F'c=4,500 P.S.I., 3/4" AGGREGATE, MAX. W/C=0.40, TYPE I OR II CEMENT; 4 TO 6% ENT. AIR, SLUMP MIN. 1", MAX. 4".
   (USE A MID-RANGE WATER REDUCER IF MORE WORKABILITY IS DESIRED.)
3. CONCRETE SUPPLIER IS TO SUBMIT MIX DESIGN(S) TO ENGINEER OF RECORD FOR EACH TYPE OF CONCRETE TO BE USED.
4. REINFORCING TO BE GRADE 60, NEW DEFORMED BARS. DO NOT "WET-STICK" REBAR OR ANCHOR BOLTS, NO EXCEPTION.
5. ALL FOUNDATION WALLS ARE 8" WIDE, UNLESS NOTED OTHERWISE ON PLANS.
6. UNLESS NOTED ON PIER DETAILS, ALL ANCHOR RODS ARE 3/4" DIA. ASTM F1554 A36 THREADED ROD WITH NUT EACH END, SEE DETAILS.
7. REF. ELEV. TOP OF FOUNDATION WALL = 99.33FT, TYP. UNLESS NOTED OTHERWISE THUS "TOW=".
   REF. ELEV. TOP OF PIER = 100.0 FT, TYP. UNLESS NOTED OTHERWISE THUS "TOP=".
   REFERENCE BOTTOM OF WALL FOOTING ELEVATION = 95.0 FT, TYPICAL U.N.O. THUS "BOF="
8. ALL EXTERIOR FOOTINGS SHALL EXTEND 4'-0" BELOW FINISH GRADE, UNLESS SPECIFIED DIFFERENTLY BY PROJECT GEOTECHNICAL ENGINEER.
9. ALL FOOTINGS TO BE CENTERED BELOW COLUMN BASE ABOVE.
10. REFER TO GEOTECHNICAL ENGINEERING REPORT FOR ALL FOUNDATION, DRAINAGE, COMPACTION, BACKFILL, AND SUB-GRADE PREPARATION REQUIREMENTS.
11. G.C. TO DETERMINE SLAB PITCH REQUIRMENTS AND FIELD COORDINATE.
12. ALL STRUCTURAL FILL TO BE COMPACTED TO A MINIMUM OF 95% AS DETERMINED BY ASTM D-1557, UNO BY PROJECT GEOTECHNICAL ENGINEER.
13. CONTRACTOR TO COMPLY WITH LATEST PROVISIONS OF ACI 305 AND ACI 306 FOR HOT AND COLD WEATHER CONCRETING.
14. CONTRACTOR TO COMPLY WITH LATEST PROVISIONS OF ACI 304 FOR CONCRETE PLACEMENT.
15. CONTRACTOR TO SUBMIT DETAILED REINFORCING SHOP DRAWINGS TO SRG FOR REVIEW PRIOR TO CONSTRUCTION, NO EXCEPTION.
16. CALCIUM CHLORIDE IS NOT ALLOWED IN ANY CONCRETE AND WILL BE REJECTED IF IT IS ALLOWED IN CONCRETE MIX.
17. EACH CONCRETE TRUCK TO PROVIDE BATCH TICKET TO MATERIALS TESTING AGENCY BEFORE PLACEMENT, NO EXCEPTIONS.
18. PROVIDE SUPPORTS FOR REINFORCEMENT INCLUDING BOLSTERS, CHAIRS, SPACERS, AND OTHER DEVICES FOR SPACING, SUPPORTING, AND FASTENING REINFORCING BARS AND WELDED WIRE FABRIC IN PLACE. USE WIRE BAR TYPE SUPPORTS COMPLYING WITH CRSI RECOMMENDATIONS, UNLESS OTHERWISE ACCEPTABLE TO ENGINEER. "WET STICKING" OF FNDN. REINFORCING AND ANCHOR BOLTS IS NOT ALLOWED (EXCEPT FOR CMU STRAIGHT BAR DOWELS) AND WILL BE REJECTED.
19. ALL SLABS TO BE WET-CURED FOR AT LEAST 7 DAYS.
20. FORM RELEASE AGENT TO BE APPLIED TO FORMS PRIOR TO PLACING REINFORCING. DO NOT LET FORM RELEASE AGENT COME IN CONTACT WITH REBAR, NO EXCEPTIONS.
21. G.C. COORDINATE SLAB FINISH REQUIRED WITH ARCHITECT.

GENERAL NOTE TO CONTRACTOR

ALL STEEL FRAMING IS NON-SELF SUPPORTING AND REQUIRES INTERACTION WITH OTHER ELEMENTS NOT CLASSIFIED AS STRUCTURAL STEEL TO PROVIDE THE REQUIRED STABILITY AND RESISTANCE TO LATERAL FORCES. THE STEEL FRAMING SHALL BE TEMPORARILY BRACED BY THE CONTRACTOR UNTIL ALL BRACING, FLOOR, ROOF DECKS, AND CONCRETE HAVE BEEN INSTALLED AND ALL CONNECTIONS BETWEEN THESE ELEMENTS HAVE BEEN MADE. ALL FOUNDATION WALLS SHALL HAVE ADEQUATE TEMPORARY BRACING BEFORE BACKFILL IS PLACED AGAINST THEM. TEMPORARY BRACING SHALL NOT BE REMOVED UNTIL STEEL BRACING, ROOF DECK, AND FLOOR DECKS ARE FULLY INSTALLED.

ROOF FRAMING NOTES:

1. DESIGN CANOPY ROOF SNOW LOAD = 60psf, PER 2006 IBC; Pg=60psf, I=1.0, CE=1.2, Ct=1.2.
2. DESIGN DEAD LOAD = 15psf.
3. COLUMN SIZE IS NOTED ON PLANS, SEE COLUMN SCHEDULE.
4. REFERENCE ELEVATION TOP OF STRUCTURAL STEEL (TOS) IS NOTED ON PLANS.
5. REFER TO ARCHITECTURAL, MECHANCAL, AND ELECTRICAL PLANS FOR ALL PENETRATIONS REQUIRED.
6. ALL STEEL TO BE SHOP-PRIMED ONE COAT AND FIELD PAINTED MIN. 2 COATS WITH ARCHITECT APPROVED PAINT.
7. ALL HSS BEAM-TO-COLUMN CONNECTIONS TO HAVE SHOP FABRICATED TEMPORARY ERECTION SEAT ANGLE TO BE REMOVED FOR FINAL FIELD WELDING.
8. ROOF DECK TO BE 1 1/2" DEEP x 18/18 GAGE GALVANIZED (G60) B CELLULAR ROOF DECK.
9. ROOF DECK ATTACHMENT AS FOLLOWS USING $\frac{5}{8}$" DIAMETER PUDDLE WELDS: 36/4 WITH 1-#10 TEK SCREWS EACH SIDELAP. WELD 6" O.C. AT ALL BUILDING EDGES. USE WELDING WASHERS AS REQ'D.
10. DESIGN BUILDING CODE: 2006 IBC (INTERNATIONAL BUILDING CODE).
11. ALL STRUTURAL STEEL TO BE ASTM A572 OR A992; GRADE 50. HSS SHAPES TO BE ASTM A500, GRADE B.
12. ▶— INDICATES MOMENT CONNECTION, BEVEL CUT AND FULL PEN-WELD TOP AND BOTTOM FLANGES, FLARE-BEVEL SIDES; SEE DETAILS.
13. WELDS E70 SERIES. ALL WELDS TO BE GROUND SMOOTH.
14. ALL PENETRATIONS THROUGH THE STRUCTURE TO BE VERIFIED BY THE MECHANICAL, ELECTRICAL, AND PLUMBING CONTRACTORS PRIOR TO PLACING. ALL PENETRATIONS SHALL BE SUBJECT TO APPROVAL BY THE ARCHITECT/ENGINEER.
15. STRUCTURAL STEEL TO BE FURNISHED BY A FABRICATOR WHO HAS BEEN REVIEWED AND ACCEPTED BY SRG ENGINEERING PRIOR TO AWARDING CONTRACT FOR WORK.
16. ALL WELDING TO BE DONE BY AISC QUALIFIED WELDERS. SUBMIT CERTIFICATE TO SRG ENGINEERING FOR REVIEW PRIOR TO FABRICATION; NO EXCEPTION.
17. CONTRACTOR TO SUBMIT COMPLETE SHOP DRAWINGS FOR ALL PARTS OF WORK. FABRICATION OR ERECTION OF NEW STRUCTURAL ELEMENTS SHALL NOT COMMENCE WITHOUT REVIEW BY THE ARCHITECT AND ENGINEER. SUBMIT TWO (2) COPIES AND ONE (1) SEPIA; COPY WILL BE REVIEWED AND ONE COPY AND SEPIA WILL BE RETURNED.
18. THIS STRUCTURE REQUIRES "TESTING AND INSPECTION" PER CHAPTER 17 OF THE 2006 EDITION OF IBC ENTITLED "STRUCTURAL TESTS AND SPECIAL INSPECTIONS", NO EXCEPTION. GENERAL CONTRACTOR MUST CONTACT SRG ENGINEERING A MINIMUM OF 48 HOURS (2 BUSINESS DAYS) BEFORE ANY "SPECIAL INSPECTIONS" CAN BE PERFORMED.
19. THIS NEW STRUCTURE HAS BEEN DESIGNED FOR WIND AND SEISMIC LOADING CONDITIONS SET FORTH IN THE 2006 INTERNATIONAL BUILDING CODE; Vmax=2klps.

ROBERT J. FOSTER – ARCHITECT
36 Groveside Road, Portland, Maine 04102
(207) 761-3822

STATE OF MAINE
STEVEN R. GRANT
No. 9858
LICENSED
PROFESSIONAL ENGINEER

Project Title

CROSS INSURANCE CANOPY REPLACEMENT

Portland, Maine

Key Plan

Mark	Date	Description

Issue Date:
7.28.2006: FOR CONSTRUCTION

SRG ENGINEERING, INC.
STRUCTURAL ENGINEERS
P.O. BOX 925
GRAY, ME 04039
TEL: (207) 657-7323
FAX: (207) 657-7342
EMAIL: SRG@SRGENG.COM

Drawing Title

ROOF FRAMING AND FOUNDATION PLAN

Drawing Number

S1.0