MASONRY

1. ALL MASONRY WALLS AND PARTITIONS SHALL BE REINFORCED.
2. COMPRESSIVE STRENGTH OF MASONRY, F'm, SHALL BE MIN. 1500 PSI.
3. COMPRESSIVE STRENGTH OF LOAD BEARING CONCRETE MASONRY UNITS SHALL BE IN ACCORDANCE WITH ASTM C90.
4. MORTAR SHALL BE TYPE S WITH A MINIMUM COMPRESSIVE STRENGTH OF 1800 PSI AT 28 DAYS.
5. GROUT SHALL HAVE A COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS.
6. COMPRESSIVE STRENGTH OF LOAD BEARING BRICK SHALL BE IN ACCORDANCE WITH ASTM C55.
7. EXPANSION BOLTS IN HOLLOW MASONRY WALL SHALL BE LOCATED AT SOLID BLOCK OR BOND BEAM.
8. SMU INDICATES SOLID MASONRY UNITS, OR HOLLOW CONCRETE UNITS WITH ALL VOIDS COMPLETELY FILLED WITH 3000 PSI CONCRETE OR GROUT.
9. WOOD NAILERS ON MASONRY SHALL BE ANCHORED WITH 1/2" DIA. ANCHOR BOLTS @ 2'-8" O.C. AND EXTENDED 6" INTO SOLID MASONRY UNLESS NOTED OTHERWISE.
10. WHERE BEAMS, JOISTS, OR COLUMNS BEAR ONTO MASONRY, THE MASONRY SHALL BE SOLID OR FILLED SOLID, A MINIMUM OF 2'-0" TO EACH SIDE OF THE CENTERLINE OF BEARING, FOUR COURSES HIGH, OR A CONTINUOUS CONCRETE FILLED BOND BEAM SHALL BE LOCATED UNDER THE BEARING.
11. MASONRY LOCATED BELOW TOP OF SLAB ON FILL SHALL BE SOLID OR FILLED SOLID.
12. ALL CMU SHALL BE REINFORCED.

STRUCTURAL STEEL

1. STUCTURAL STEEL SHALL BE ASTM A992, Fy = 50 KSI. SQUARE AND RECTANGULAR HSS SECTIONS SHALL BE ASTM A500, GRADE B, Fy = 46 KSI. ROUND HSS SECTIONS SHALL BE ASTM A500, GRADE B, Fy = 42 KSI, UNLESS NOTED OTHERWISE.
2. FABRICATION SHALL NOT BEGIN UNTIL SHOP DRAWINGS HAVE BEEN APPROVED,
3. CONNECTIONS NOT DETAILED ON DRAWINGS SHALL BE DESIGNED BY THE STEEL FABRICATOR.
4. BASE PLATES AND BEARING PLATES SHALL BE GROUTED WITH NON-SHRINK GROUT AND AT PROPER GRADE, BEFORE PLACING STEEL.
5. CONTRACTOR SHALL APPLY 2 BRUSH COATS OF ASPHALT TO COLUMNS AND BASE PLATES EXPOSED TO FILL AFTER COLUMN IS IN PLACE.
6. STEEL BEAMS ENCASED IN CONCRETE SHALL RECEIVE CLIPS OR BE WRAPPED WITH WIRE MESH, UNLESS NOTED OTHERWISE.
7. STEEL COLUMNS ENCASED IN CONCRETE SHALL BE WRAPPED WITH WIRE MESH, UNLESS NOTED OTHERWISE.
8. STRUCTURAL STEEL NOT ENCASED OR EMBEDDED IN CONCRETE SHALL RECEIVE ONE SHOP COAT OF PAINT.
9. STEEL BEAMS TO RECEIVE WOOD NAILERS SHALL HAVE BOLT HOLES DRILLED FOR 1/2" DIA. BOLTS @ 2'- 8"+/- O.C. STAGGERED.
10. VERIFY WITH MECHANICAL DRAWINGS FOR LOCATION OF DUCTS, PIPING, OR THROUGH FLOOR AND ROOF CONSTRUCTION BEFORE SPACING FRAMING.

NON-COMPOSITE FLOOR DECK

1. WHERE NOTED ON PLAN, STEEL FLOOR FORM SHALL BE 1 1/2" - 22 GA STEEL DECK

2. FASTENING PATTERN:
   AT EACH SUPPORT WELD DECK TO SUPPORT AT 12" OC MAX, 36/4 PATTERN, WITH 5/8" DIAMETER PUDDLE WELDS.
   SIDE LAPS TO BE SCREWED WITH #10 TEKS AT 2'-0" OC MAX.

APPROVED DECK MANUFACTURERS ARE VULCRAFT, CANAM, AND UNITED STEEL DECK. USE OF A DECK BY A MANUFACTURER OTHER THAN THOSE LISTED WILL REQUIRE VERIFICATION BY AN ENGINEER HIRED BY THE DECK FABRICATOR THAT THE ALTERNATE DECK CAN SUPPORT ALL IMPOSED LOADS. THIS VERIFICATION MUST BE STAMPED BY A PROFESSIONAL ENGINEER AND SUBMITTED PRIOR TO RECIEPT OF THE DECK SHOP DRAWINGS.

STEEL ROOF DECK

1. WHERE NOTED ON PLAN, ROOF DECK SHALL BE TYPE B (WIDE RIB), 1 1/2" - 22 GA

   DECK ATTACHMENT - 5/8" PUDDLE WELD
   PATTERN - 36/5
   SIDELAPS: 9 - #10 TEK SCREWS PER SPAN

APPROVED DECK MANUFACTURERS ARE VULCRAFT, CANAM, AND UNITED STEEL DECK. USE OF A DECK BY A MANUFACTURER OTHER THAN THOSE LISTED WILL REQUIRE VERIFICATION BY AN ENGINEER HIRED BY THE DECK FABRICATOR THAT THE ALTERNATE DECK CAN SUPPORT ALL IMPOSED LOADS. THIS VERIFICATION MUST BE STAMPED BY A PROFESSIONAL ENGINEER AND SUBMITTED PRIOR TO RECEIPT OF THE DECK SHOP DRAWINGS.

MISCELLANEOUS

1. CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND EXISTING CONDITIONS BEFORE PROCEEDING WITH THE WORK.
2. CONTRACTOR SHALL REPORT ANY VARIATIONS FOUND AT SITE BEFORE PROCEEDING WITH THAT PART OF THE WORK.

DESIGN LOADS (IBC 2003)

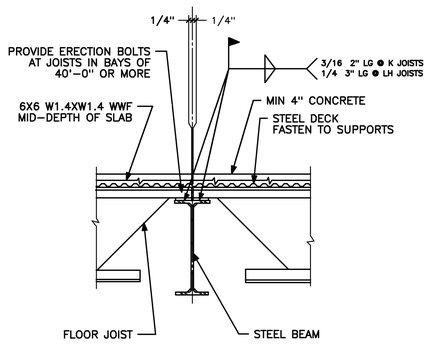
LIVE LOAD - CORRIDORS (ABOVE FIRST FLOOR) = 80 PSF
OFFICE = 50 PSF
LOBBIES AND STAIRS = 100 PSF
MECHANICAL ROOMS = 150 PSF
STORAGE = 80 PSF

SNOW LOAD - Pg = 60 PSF
Ce = 1.0
I = 1.0
Pf = 42 PSF + DRIFTING

WIND LOAD - BASIC WIND SPEED = 100 MPH
I = 1.0
EXPOSURE B
P = 17.9 PSF
NET UPLIFT PRESSURE = 15 PSF

SEISMIC LOAD - SEISMIC USE GROUP - II
Sds = .522
Sd1 = .231
SITE CLASS - E
S.F.R. SYSTEM - CONCENTRIC BRACED FRAMES
ANALYSIS PROCEDURE - EQUIVALENT LATERAL FORCE
BASE SHEAR - V = .104W

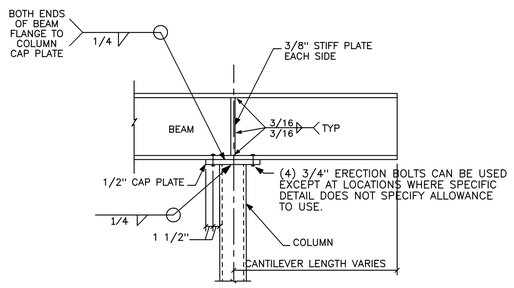
GENERAL NOTES — NO SCALE — REF: NA — A4



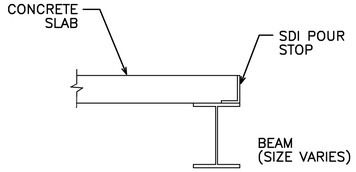
(DETAIL SIMILAR AT ROOF)

TYPICAL FLOOR JOISTS BEARING ON STEEL BEAM — NOT TO SCALE — REF: NA — C3

NOTE: PROVIDE WEB STIFFENERS PER THE INSTRUCTIONS OF THIS DETAIL FOR ALL BEAM OVER BEAM, BEAM OVER COLUMN, AND COLUMN SUPPORTED BY BEAM CONDITIONS.



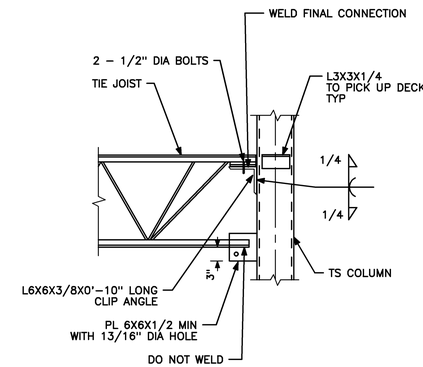
TYPICAL BEAM OVER COLUMN DETAIL — NO SCALE — REF: NA — C2



NOTE:

SEE ARCHITECTURAL SECTIONS FOR LOCATIONS OF SLAB ENDS

TYPICAL POUR STOP DETAIL — NO SCALE — REF: NA — C1



TYPICAL TIE JOIST BEARING — NOT TO SCALE — REF: NA — B3

EXTERIOR BRICK VENEER LINTEL SCHEDULE		
MASONRY OPENING	SIZE	BEARING
LESS THAN 11'-0"	L6x6x3/8	8" MIN.
11'-0" TO 12'-8"	BENT PLATE 8x6x3/8	8" MIN.

NOTES:

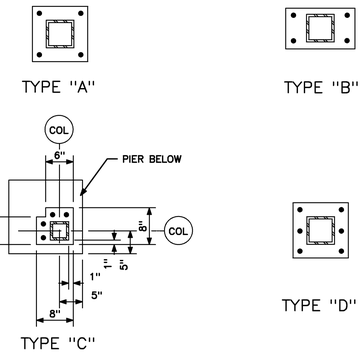
1. LINTEL FOR EXTERIOR MASONRY SHALL BE HOT-DIPPED GALVANIZED.
2. LINTEL IS SIZED FOR 4" WIDTH OF MASONRY.

CONCRETE MASONRY LINTEL SCHEDULE		
MASONRY OPENING	SIZE	BEARING
4'-10" OR LESS	L4x3 1/2x5/16 (LLV)	8" MIN. ON SGMU
4'-10" TO 7'-0"	L5x3 1/2x5/16 (LLV)	8" MIN. ON SGMU
7'-0" TO 9'-0"	L6x3 1/2x3/8 (LLV)	8" MIN. ON SGMU

NOTES:

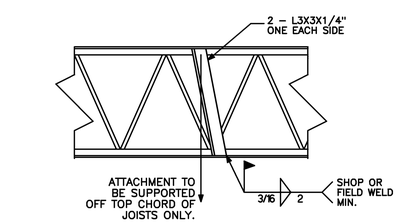
1. ONE ANGLE REQUIRED FOR EVERY 4" OF MASONRY.
2. MULTIPLE ANGLES SHALL BE WELDED BACK TO BACK.
3. "SGMU" INDICATES SOLID GROUTED MASONRY UNIT.

STEEL LINTEL SCHEDULES — NO SCALE — REF: NA — B2



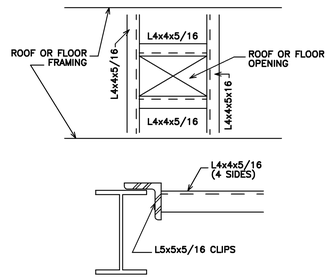
NOTES:
1. THE FOLLOWING ANCHOR RODS ARE REQUIRED AT ALL COLUMNS UNLESS NOTED OTHERWISE ON THE FOUNDATION PLANS.
   4-3/4" DIA. X 9" EMBEDMENT
   HEADED ANCHOR RODS (ASTM F1554, GR. 36)

BASE PLATE TYPES — NO SCALE — REF: S20.2, S30.1 — B1



THIS JOIST REINFORCING DETAIL SHALL BE USED AT ALL LOCATIONS WHERE MECHANICAL EQUIPMENT, DUCTWORK, PIPING, AND ELECTRICAL CONDUITS ARE EITHER HUNG FROM OR BEARING ON THE TOP CHORD OF THE JOISTS. WHEN NOT AT A JOIST PANEL POINT. G.C. TO COORDINATE LOCATIONS OF ABOVE ITEMS TO MINIMIZE REINFORCEMENTS NECESSARY.

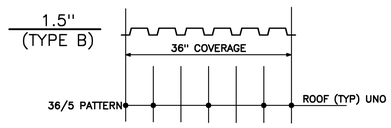
ROOF JOIST REINFORCING DETAIL — NO SCALE — REF: NA — A3



NOTES:

1. PROVIDE AT ALL OPENINGS 6" OR LARGER IN NEW FLOORS AND ROOF AND EXG FLOOR AND ROOF.
2. ALL CONNECTIONS TO BE WELDED.
3. COORDINATE OPENING SIZE WITH MECHANICAL, ELECTRICAL AND PLUMBING TRADES.

TYPICAL FLOOR AND ROOF FRAME OPENING — NO SCALE — REF: S30.1 — A2



ROOF DECK FASTENER LAYOUT — NO SCALE — REF: S20.2, S30.1 — A1

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Project Title

WOODARD & CURRAN OFFICE ADDITION

PORTLAND, MAINE

HA Project No.	05178

Key Plan

Mark	Date	Description
–	05-15-06	ISSUED FOR CONSTRUCTION
–	03-14-06	FINAL REVIEW
–	02-03-06	DD REVIEW

Issue Dates

Drawing Status

Drawing Title

FRAMING NOTES AND DETAILS

PA / PE: JCF	Drawn By: SJF

Drawing Number

S60.1