<u>A –</u>	GENERAL	B5	SEISMIC LOADS PER ASCE 7/IBC SITE CLASSIFICATION = D
A1	THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH ALL OTHER CONTRACT DRAWINGS AND SPECIFICATIONS. REFER TO ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND FIRE PROTECTION DRAWINGS FOR LOCATIONS, DIMENSIONS, AND DETAILS OF OPENINGS, SLEEVES, EMBEDMENTS, INSERTS, PADS, CURBS, DEPRESSIONS, ANCHOR BOLTS, AND OTHER PROJECT REQUIREMENTS NOT SHOWN ON STRUCTURAL DRAWINGS.		SEISMIC DESIGN CATEGORY = B IMPORTANCE FACTOR = 1.00 OCCUPANCY CATEGORY = III 0.2g SPECTRAL RESPONSE, Ss = 0.321 1 SEC SPECTRAL RESPONSE, S1 = 0.078 SEISMIC RESISTING SYSTEM = STEEL FRAMING NOT SPECIFICALLY DETAILED
A2	VERIFY AND COORDINATE ALL DIMENSIONS AND ELEVATIONS RELATING TO EXISTING CONDITIONS.		FOR SEISMIC RESISTANCE RESPONSE MODIFICATION FACTOR, $R = 3.0$
A3	CODES AND STANDARDS		SEISMIC ANALYSIS PROCEDURE = EQUIVALENT LATERAL FORCE ANALYSIS
	 2009 INTERNATIONAL BUILDING CODE ASCE 7-05 — MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER 	<u>C -</u>	- FOUNDATIONS
	 ASCE 7 05 MINIMUM DESIGN LOADS FOR BOILDINGS AND OTHER STRUCTURES – "MANUAL OF STEEL CONSTRUCTION – ALLOWABLE STRESS DESIGN" AMERICAN INSTITUTE OF STEEL CONSTRUCTION – 13TH EDITION 	C1	PRIMARY FOUNDATIONS FOR THIS PROJECT CONSIST OF A MAT FOUNDATION WITH HAUNCHED SLAB.
	 "STRUCTURAL WELDING CODE – STEEL" – AMERICAN WELDING SOCIETY AWS DI.I–2008 "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" ACI 	C2	FOUNDATION DESIGN BASED ON NET ALLOWABLE BEARING CAPACITY OF 2000 POUNDS PER SQUARE FOOT. IF UNSUITABLE MATERIAL IS ENCOUNTERED AT THE INDICATED ELEVATIONS, REMOVE AND REPLACE THE UNSUITABLE MATERIAL AS DIRECTED BY THE ENGINEER.
	530-08/ASCE 5-08/TMS 402-08 - "SPECIFICATIONS FOR MASONRY STRUCTURES", ACI 530.1-08/ASCE	C3	THE CONTRACTOR SHALL DESIGN AND PROVIDE SHEETING, SHORING, BRACING,
	6–08/TMS 602–08 – "DESIGN MANUAL FOR COMPOSITE DECKS, FORM DECKS, AND ROOF DECKS" – STEEL DECK INSTITUTE – SDI		AND/OR UNDERPINNING IN ORDER TO PROTECT EXISTING UTILITIES FROM EXCESSIVE MOVEMENTS DURING THE CONSTRUCTION PERIOD.
A4	THE CONTRACTOR, OR HIS REPRESENTATIVE, SHALL DESIGN AND PROVIDE THE FOLLOWING PERMANENT ELEMENTS: - RAILINGS - STAIRS	C4	CARRY OUT CONTINUOUS CONTROL OF SURFACE AND SUBSURFACE WATER DURING CONSTRUCTION SUCH THAT FOUNDATION WORK IS DONE IN THE DRY AND ON UNDISTURBED SUBGRADE MATERIAL. NO FOUNDATION CONCRETE SHALL BE PLACED IN WATER OR ON FROZEN SUB GRADE MATERIAL.
	FOR PERFORMANCE DESIGN REQUIREMENTS OF ELEMENTS LISTED ABOVE,	<u>D</u> –	- CAST IN PLACE CONCRETE
A5	REFER TO ADDITIONAL NOTES THIS SHEET AND TECHNICAL SPECIFICATIONS. STRUCTURAL REQUIREMENTS TO ACCOMMODATE FIXED EQUIPMENT, INCLUDING BUT NOT LIMITED TO ROOF TOP UNITS, AND EMBEDMENTS, ARE INCIDENTAL	D1	UNLESS OTHERWISE NOTED, CONCRETE SHALL BE NORMAL WEIGHT AND HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH AS FOLLOWS: - SPREAD FOOTINGS, FOUNDATION WALLS, MAT SLABS4500 PSI - FILL CONCRETE, HOUSEKEEPING PADS, SIDEWALKS3000 PSI
	TO THE REQUIREMENTS OF A SPECIFIC EQUIPMENT MANUFACTURER. ALL WORK SHALL CONFORM TO APPROVED EQUIPMENT MANUFACTURER'S SHOP DRAWINGS AND INSTALLATION INSTRUCTIONS. THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR APPROVAL ANY REQUIRED MODIFICATIONS TO ACCOMMODATE APPROVED EQUIPMENT DRAWINGS. SUCH MODIFICATIONS SHALL BE MADE AT NO COST TO THE OWNER.	D2	CONCRETE WORK SHALL BE COORDINATED WITH ALL WATERPROOFING, ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL WORK. THE CONTRACTOR SHALL VERIFY INSTALLATION AND LOCATIONS OF ALL EMBEDDED ITEMS INCLUDING BUT NOT LIMITED TO INSERTS, ANCHOR BOLTS, DOWELS, BLOCKOUTS, SLEEVES, EMBEDDED PIPING, AND EMBEDDED CONDUIT PRIOR TO CONCRETE PLACEMENT.
A6	TYPICAL DETAILS AND NOTES SHOWN ON STRUCTURAL DRAWINGS SHALL BE APPLICABLE TO ALL PARTS OF THE STRUCTURAL WORK EXCEPT WHERE SPECIFICALLY REQUIRED OTHERWISE BY CONTRACT DOCUMENTS.	D3	ALL CONCRETE EXPOSED TO WEATHER SHALL INCLUDE 5% ENTRAINED AIR, $\pm 1\%$.
Α7	DETAILS NOT SPECIFICALLY SHOWN SHALL BE SIMILAR TO THOSE SHOWN FOR THE MOST NEARLY SIMILAR CONDITION AS DETERMINED BY THE ENGINEER.	D4	A MINIMUM OF 72 HOURS SHALL ELAPSE BETWEEN ADJACENT CONCRETE PLACEMENTS.
A8	STRUCTURAL WORK SHALL BE INSPECTED AND TESTED BY AN APPROVED TESTING AGENCY. THE INSPECTOR SHALL BE ENGAGED BY AND COSTS FOR ALL TESTING AND INSPECTION SHALL BE PAID BY THE CONTRACTOR.	D5	CONCRETE SLABS SHALL BE PLACED SO THAT THE SLAB THICKNESS IS AT NO POINT LESS THAN THAT INDICATED ON THE DRAWINGS.
A9	ELEVATIONS SHOWN REFER TO THE DATUM ESTABLISHED ON THE DRAWINGS.	D6	PROVIDE 3/4 IN X 3/4 IN CHAMFER ON ALL EXPOSED VERTICAL AND HORIZONTAL OUTSIDE CORNERS UNLESS OTHERWISE NOTED.
A10	THE CONTRACTOR SHALL DESIGN AND PROVIDE ALL REQUIRED SHORING AND TEMPORARY BRACING TO RESIST FORCES ON THE STRUCTURES THROUGHOUT	<u>E -</u>	- CAST IN PLACE CONCRETE REINFORCEMENT
	THE CONSTRUCTION PERIOD.	E1	REINFORCEMENT WORK OF DETAILING, FABRICATION, AND ERECTION SHALL CONFORM TO "ACI DETAILING MANUAL" – SP–66, "CRSI MANUAL OF STANDARD PRACTICE", AND "STRUCTURAL WELDING CODE – REINFORCING STEEL" – AWS D1.4.
<u>B –</u> B1	<u>GENERAL DESIGN LOADS</u> DEAD LOADS		
	(A) WEIGHT OF BUILDING COMPONENTS (B) TYPICAL PARTITION ALLOWANCE (LL \leq 80 PSF)_20 PSF	E2	STEEL REINFORCEMENT, UNLESS OTHERWISE NOTED, SHALL CONFORM TO THE FOLLOWING:
B2	(C) WEIGHT OF FIXED SERVICE EQUIPMENT		(A) BARS, TIES, AND STIRRUPSASTM A615 GRADE 60 (B) WELDED WIRE FABRICASTM A185
	(A) UNIFORM LOADS	E3	REINFORCING STEEL SHALL BE UNCOATED AND DEFORMED.
	CORRIDORS		MINIMUM CONCRETE PROTECTIVE COVERING FOR REINFORCEMENT, UNLESS NOTED OTHERWISE, SHALL BE AS FOLLOWS:
В3	SNOW LOADS		(A) SURFACES CAST AGAINST AND PERMANENTLY IN CONTACT WITH EARTH3.0"
	PER ASCE 7/IBC (A) GROUND SNOW LOAD (60 PSF)		(B) FORMED SURFACES BACKFILLED WITH EARTH OR EXPOSED TO WEATHER2.0"
	(B) FLAT ROOF SNOW LOAD (42 PSF) (C) EXPOSURE FACTOR (B) (D) IMPORTANCE FACTOR (1.0)		(C) SURFACES NOT IN CONTACT WITH EARTH OR EXPOSED TO WEATHER1.5"
B4	WIND LOADS PER ASCE 7/IBC (A) BASIC WIND SPEED (V) = 90 MPH	E5	REINFORCING STEEL SHALL BE CONTINUOUS THROUGH ALL CONSTRUCTION JOINTS, CORNERS, AND INTERSECTIONS UNLESS OTHERWISE NOTED. REINFORCING SHALL BE LAPPED AT NECESSARY SPLICES OR HOOKED AT DISCONTINUOUS ENDS, UNLESS OTHERWISE NOTED.
	 (B) IMPORTANCE FACTOR (I) = 1.0 (C) EXPOSURE CATEGORY (B) (D) DESIGN PRESSURE (P): MWFRS - WALLS: 8.6psf WINDWARD 	E6	MECHANICAL SPLICES SHALL BE PERMITTED SUBJECT TO APPROVAL BY THE ENGINEER. MECHANICAL SPLICES SHALL DEVELOP AT LEAST 125 PERCENT OF THE SPECIFIED YIELD STRENGTH OF THE BAR.
	-6.Opsf LEEWARD ROOF: ±10psf	E7	ADHESIVE ANCHORING SYSTEMS FOR DRILLED-IN REINFORCING BARS SHALL
	COMPONENTS AND CLADDING - WALLS: ±15.8psf MAIN ZONE ±19.5psf CORNERS ROOF: ±14.6psf INTERIOR ZONE		BE PERMITTED SUBJECT TO APPROVAL BY THE ENGINEER. THE ANCHORING SYSTEM SHALL BE CAPABLE OF DEVELOPING THE FULL YIELD STRENGTH OF THE BAR BASED ON THE RESULTS OF UNCONFINED PULL—OUT TESTING.
	±24.4psf END ZONES ±36.8psf CORNER ZONES	E8	DOWELS SHALL MATCH BAR SIZE AND NUMBER, UNLESS NOTED OTHERWISE.

- E9 WELDED WIRE FABRIC SHALL LAP SEE ACI 12.19 AND SHALL TOGETHER AT ALL LAPS.
- E10 REINFORCEMENT NOT DETAILED WITH WELDS SHALL NOT BE TACK WELDED.
- E11 INSTALLATION OF REINFORCEMENT SHALL BE AVAILABLE FOR INSPECTION PRIOR TO THE SCHEDULED CONCRETE PLACEMENT.

- F1 DESIGN, FABRICATION, AND ERECTION OF METAL DECK SHALL CONFORM TO THE "STRUCTURAL WELDING CODE - SHEET STEEL" (AWS D1.3-08) AND THE "DESIGN MANUAL FOR COMPOSITE DECKS, FORM DECKS, AND ROOF DECKS - STEEL DECK INSTITUTE".
- F2 STEEL DECK PANELS SHALL BE FORMED STEEL SHEETS CONFORMING TO ASTM A653 SQ GRADE 33 MINIMUM WITH COATING DESIGNATION: HOT DIP GALVANIZED G90 - ROOF DECK.
- F3 DECK ATTACHMENT SHALL BE BY MECHANICAL FASTENERS. SPACING OF FASTENERS SHALL CONFORM TO SDI SPECIFICATIONS.

G – CONCRETE MASONRY

- G1 CONCRETE MASONRY UNITS (CMU) SHALL BE PRE-INSULATED "OMNI BLOCK".
- G2 CONCRETE MASONRY CONSTRUCTION SHALL CONFORM TO THE NATIONAL CONCRETE MASONRY ASSOCIATION "SPECIFICATION FOR THE DESIGN AND CONSTRUCTION OF LOAD BEARING CONCRETE MASONRY" (TR758), "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" (ACI 530-08/ASCE 5-08/TMS 402-08), AND "SPECIFICATIONS FOR MASONRY STRUCTURES" (ACI 530.I-08/ASCE 6-08/TMS 602-08).
- /E G3 COMPRESSIVE STRENGTH SHALL BE f'm = 1500 PSI MINIMUM (UNIT) STRENGTH METHOD).
 - G4 MORTAR SHALL BE TYPE M OR S.
 - G5 GROUT SHALL HAVE A MINIMUM 28 DAY STRENGTH OF 3000 PSI.
- ED G6 GROUT SHALL CONFORM TO THE REQUIREMENTS OF ASTM C476 UNLESS OTHERWISE NOTED.
 - G7 REINFORCING STEEL SHALL BE AS FOLLOWS: A. DEFORMED BARS: ASTM A615 GR 60 UNCOATED B. JOINT REINFORCEMENT: ASTM A82 WITH DEFORMED LONGITUDINAL WIRES; HOT DIP GALVANIZED IN ACCORDANCE WITH ASTM A153
 - H COLD FORMED LIGHT GAUGE STEEL FRAMING
 - H1 COLD FORMED STEEL FRAMING MEMBERS SHALL BE OF THE TYPE, SHAPE, SIZE, GAUGE, AND SPACING AS SHOWN ON THE DRAWINGS. MEMBER TYPES AND SIZES SHOWN ON THE DRAWINGS REFER TO MEMBERS MANUFACTURED BY THE DIETRICH INDUSTRIES, INC. MEMBERS EQUIVALENT IN SHAPE, SIZE, STIFFNESS, AND STRENGTH BY OTHER MANUFACTURERS MAY BE SUBSTITUTED FOR FRAMING MEMBERS SHOWN. ALTERNATE MEMBERS SHALL BE SUBJECT TO REVIEW AND APPROVAL BY THE ENGINEER AND STRUCTURAL ENGINEER PRIOR TO FABRICATION AND ERECTION.
 - H2 ALL COLD FORMED STEEL FRAMING SHALL CONFORM TO THE ANSI SPECIFICATION FOR THE DESIGN OF COLD-FORMED STRUCTURAL MEMBERS.
 - H3 COLD FORMED STRUCTURAL TRACKS AND STUDS 12. 14 AND 16 GAUGE MATERIALS SHALL BE FORMED WITH ASTM A446, GRADE D OR ASTM A570. GRADE 50 STEEL WITH A MINIMUM FY=50 KSI. COLD-FORMED STRUCTURAL TRACKS AND STUDS 18 GAUGE AND LIGHTER SHALL BE FORMED FROM ASTM A446, GRADE A OR ASTM A570, GRADE 33 GALVANIZED. FASTENINGS SHALL BE AS SHOWN ON THE DRAWINGS. FASTENINGS NOT SHOWN SHALL BE AS RECOMMENDED BY THE MANUFACTURER.

H4 SUBMIT COMPLETE COLD FORMED SHOP DRAWINGS FOR APPROVAL.

– HANDRAIL, GUARDRAIL AND MISCELLANEOUS METALS

STRUCTURAL STEEL FABRICATION, ERECTION, AND CONNECTION DESIGN SHALL J1 CONFORM TO AISC'S "SPECIFICATION FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL" 13th EDITION.

- J2 STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING: (A) ANGLES: ASTM A36
- (B) RAILS AND POSTS: ASTM A53, GRADE B (C) ANCHOR RODS: ASTM F1554, GRADE 36
- J3 ALL STEEL SHALL BE FABRICATED AND SHIPPED AS SHOP PRIMED & PAINTED FROM ELEVATION 2'-0" AND BELOW. ALL REMAINING STEEL SHALL BE FABRICATED AND SHIPPED AS BARE STEEL UNLESS NOTED OTHERWISE.



PLEASE NOTE: THIS DOCUMENT MAY NOT ACCURATELY REPRESENT THE FINAL DOCUM ONLY AN ENGINEER, ARCHITECT OR SURVEY SIGNED, SEALED AND DATED PAPER COPY, PROVIDED BY THIS OFFICE, MAY BE UTILIZED BIDDING OR CONSTRUCTION PURPOSES.

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J4 SUBMIT COMPLETE SHOP DRAWINGS FOR REVIEW PRIOR TO FABRICATION.

J5 HANDRAILS AND GUARDRAILS SHALL BE PRIMED WITH TNEMEC SERIES 90-97 TNEME-ZINC 3 TO 4 MILS DFT. HANDRAILS AND GUARDRAILS SHALL BE PAINTED WITH TNEMEC SERIES 27 TYPOXY, 4 TO 6 MILS DFT, COLOR: SAFETY YELLOW.

> **ISSUED FOR BID** NOT FOR CONSTRUCTION JANUARY 5, 2017

							ecomaine 64 BLUEBERRY ROAD, PORTLAND, ME		
	C	RE-ISSUED FOR BID		AKG	BJB	01/05/17	SCALE HOUSE WITH INSULATED BLOCK CONSTRUCTION		
	B	RE-ISSUED FOR BID		AKG		10/28/16			
	A	ISSUED FOR BID		AKG		08/19/16	STRUCTURAL GENERAL NOTES		
	REV	REV DESCRIPTION		DWN	APP	DATE			
	47A York Street		SIZE:	AN	SI D	PROJECT NO.	DRAWING NO.		
IMENT.			DATE: 09/02/2016 DES BY: AKG DWN BY: AKG		02/2016	162.002.002			
YOR	Colby Company e n g i n e e r i n g Portland, Maine 04101 207.553.7753 colbycoengineering.com	G				S-001			
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