



SRG Job#07-083

To: City of Portland Code Enforcement Department
From: Steven R. Grant, President
Date: November 20, 2007
Subject: Camp Bow Wow: Quality Assurance Plan
Project Location: 49-63 Blueberry Road, Portland

Seismic resisting lateral support will be provided by Rigid Frames. Individual footing seismic ties have been provided in accordance with the IBC Code due to Site Class E soils as determined by S.W. Cole Engineering and Seismic Design Category D per Varco-Pruden.

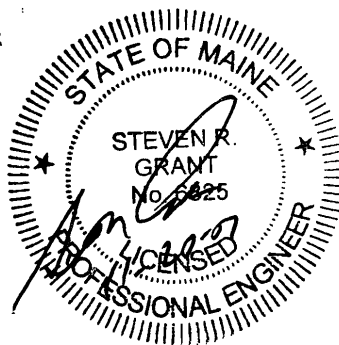
SRG Engineering has subcontracted with S.W. Cole Engineering (contact Craig Turcotte or Roger Domingo at 657-2866) to provide metal roof deck and structural steel connection review that include any diaphragm bracing at roof and walls, frame bolts, and anchor bolts. Bolts at moment connections will be checked for proper tension/torque and shear connections will be checked for all plies to be in firm contact per AISC. In addition, S.W. Cole Engineering is to provide field review of foundation reinforcing (footings/walls/piers) and anchor bolt placement. Site visits by S.W. Cole and SRG Engineering are planned to be on a limited basis throughout the construction of the foundation and building structure. In addition, SRG Engineering budgeted for a maximum of four (4) site visits to observe construction for conformance with contract documents as well.

We have asked that PATCO Construction notify SRG Engineering and S.W. Cole Engineering a minimum of 48 hours prior to all required site visits. SRG Engineering has also provided a copy of the attached check list to PATCO for their use/reference.

Please call should you have questions.

Steven R. Grant, P.E.
President

SRG:srg





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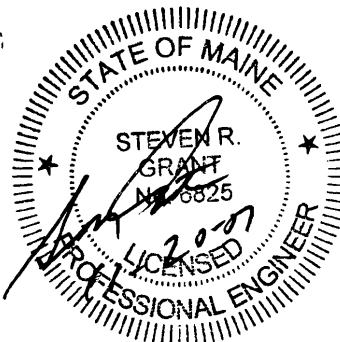
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Steven R. Grant, P.E.
President

SRG:arg



From: Jean Fraser
To: Schmuckal, Marge
Date: 9/24/2007 1:21:09 PM
Subject: Re: Blueberry Rd

Just got revised drawings late on Friday that appear to be OK and stampable (they have done everything we asked as well as a \$10,000 contribution for sidewalks and curbs) but I want to run it by Dev Rev on Wed. So it should be a matter of days...

Jean

>>> Marge Schmuckal 9/24/2007 1:11:12 PM >>>

- Camp Bow Wow has come in for a building permit. What is the status of receiving a stamped approved site plan from planning?

Marge

SRG ENGINEERING, INC.
CONSULTING STRUCTURAL ENGINEERS

FACSIMILE TRANSMITTAL SHEET

TO:	FROM:
Ron Mercier	Steven Grant, P.E.
COMPANY:	DATE:
PATCO	9/26/2007
PHONE NUMBER:	TOTAL NO. OF PAGES INCLUDING COVER:
324-5574	10
FAX NUMBER:	SENDER'S REFERENCE NUMBER:
324-1643	07-083
RE:	YOUR REFERENCE NUMBER:
Camp Bow Wow	

URGENT FOR YOUR FILES PLEASE COMMENT PLEASE REPLY PLEASE RECYCLE

NOTES/COMMENTS:

Hi Ron,

Here are the SI forms you need to get your permit...please verify address/Owner is correct. The original will be mailed directly to you once you verify the info is correct.

Also, please let me know if SWC has been contacted for scheduling their first site visit when construction starts. Also, did SWC need to do another geotech investigation or was the 2002 report sufficient? Please let me know...thanks.

Please call if you have any questions. Thanks for using SRG.

Sincerely,



Steven R. Grant, President



SRG Job#07-083

To: City of Portland Code Enforcement Department
From: Steven R. Grant, President
Date: September 26, 2007
Subject: Camp Bow Wow: Quality Assurance Plan
Project Location: 49-63 Blueberry Road, Portland

Seismic resisting lateral support will be provided by Rigid Frames.

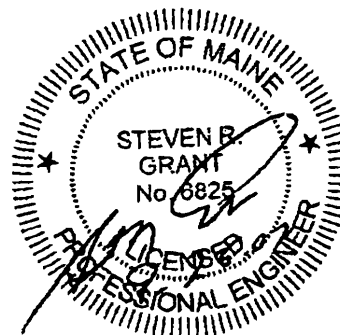
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We have asked that PATCO Construction notify SRG Engineering and S.W. Cole Engineering a minimum of 48 hours prior to all required site visits. SRG Engineering has also provided a copy of the attached check list to PATCO for their use/reference.

Please call should you have questions.

Steven R. Grant, P.E.
President

SRG:sg





**Structural Tests and "Special Inspection" Requirements For a Typical Pre-Engineered
Metal Building Structure**
(Per Chapter 17 of the 2006 International Building Code)

Site and Fill Materials:

- o Field observe sub-grade conditions prior to placement of any fill or concrete for foundations and slab
- o Field sample and perform laboratory test(s) on each soil fill material to be used
- o Observe placement and perform compaction tests on foundation and sub-slab fill materials
- o Review compliance to soils report material
- o Review lift thickness of foundation and sub-slab backfill

Reinforcing:

- o G.C. to submit reinforcing shop drawings for review prior to placement
- o G.C. to submit reinforcing and anchor bolt material certification sheet(s) for review
- o Field observe reinforcing at foundation walls for compliance with size, grade, spacing, location, and embedment.
- o Field observe reinforcing and/or WWF at structural slabs and slabs-on-grade for compliance with size, grade, spacing, location, and embedment.

Formwork:

- o Review formwork
- o Review form removal and re-shoring

Concrete:

- o G.C. to submit all mix designs to engineer for review a minimum of 10 business days before placement
- o G.C. to submit all admixtures to engineer for review a minimum of 10 business days before placement
- o G.C. to submit material certification of all slab dowels to engineer for review a minimum of 10 business days before placement
- o Review and observe field placement of all concrete: footings, walls, slabs, etc...
- o Review and observe curing techniques for footings, walls, and slabs
- o Field test concrete for slump, air, and temperature
- o Field cast four (4) cylinders for each placement to be tested for strength
- o Field observe dowel size and spacing for control and construction joints at walls and slab(s)

Steel Fabrication: (Only for structural steel not fabricated by metal building manufacturer)

- o Review and observe steel fabrication shop procedures

Steel Construction:

- o G.C. to provide material certificates for bolts, nuts, washers, and weld filler (if field welding is to be performed) material
- o Review field connections

Steel Erection:

- o G.C. to provide welders certificate for each person performing any field welding
- o Review primary steel connections
- o Verify pre-tensioning of slip-critical bolts (hanger and moment connections) by certified testing laboratory for proper bolt tension/torque.
- o Review moment connections
- o Review shear connections
- o Review bracing connections
- o Review wall girt connections
- o Review roof purlin connections
- o Review steel roof deck installation
- o Review wall siding installation

G.C. NOTE: YOU MUST NOTIFY THE MATERIALS TESTING FIRM AND THE PROJECT SPECIAL INSPECTOR A MINIMUM OF 48 BUSINESS HOURS PRIOR TO SERVICE BEING PERFORMED TO ALLOW FOR PROPER SCHEDULING OF PERSONNEL

Statement of Special Inspections

Project: *Camp Bow Wow*

Location: *49-63 Blueberry Row, Portland, ME*

Owner: *MANSEK, LLC*

Design Professional in Responsible Charge: *SRG ENGINEERING / STEVEN R. GRANT, P.E.*

This *Statement of Special Inspections* is submitted as a condition for permit issuance in accordance with the Special Inspection and Structural Testing requirements of the Building Code. It includes a schedule of Special Inspection services applicable to this project as well as the name of the Special Inspection Coordinator and the identity of other approved agencies to be retained for conducting these inspections and tests. This *Statement of Special Inspections* encompass the following disciplines:

- Structural
- Mechanical/Electrical/Plumbing
- Architectural
- Other: _____

The Special Inspection Coordinator shall keep records of all inspections and shall furnish inspection reports to the Building Official and the Registered Design Professional in Responsible Charge. Discovered discrepancies shall be brought to the immediate attention of the Contractor for correction. If such discrepancies are not corrected, the discrepancies shall be brought to the attention of the Building Official and the Registered Design Professional in Responsible Charge. The Special Inspection program does not relieve the Contractor of his or her responsibilities.

Interim reports shall be submitted to the Building Official and the Registered Design Professional in Responsible Charge.

A *Final Report of Special Inspections* documenting completion of all required Special Inspections, testing and correction of any discrepancies noted in the inspections shall be submitted prior to issuance of a Certificate of Use and Occupancy.

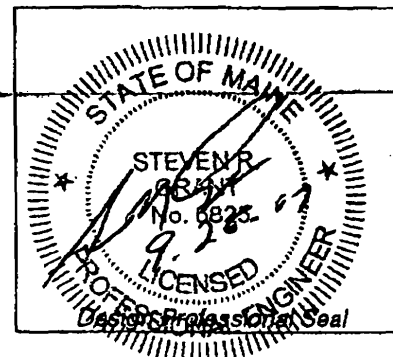
Job site safety and means and methods of construction are solely the responsibility of the Contractor.

Interim Report Frequency: _____ or per attached schedule.

Prepared by:

STEVEN R. GRANT

(type or print name)



[Handwritten Signature]

Signature

9-26-07

Date

Owner's Authorization:

Building Official's Acceptance:

Signature

Date

Signature

Date

Schedule of Inspection and Testing Agencies

This Statement of Special Inspections / Quality Assurance Plan includes the following building systems:

- | | |
|--|--|
| <input checked="" type="checkbox"/> Soils and Foundations | <input type="checkbox"/> Spray Fire Resistant Material |
| <input checked="" type="checkbox"/> Cast-in-Place Concrete | <input type="checkbox"/> Wood Construction |
| <input type="checkbox"/> Precast Concrete | <input type="checkbox"/> Exterior Insulation and Finish System |
| <input type="checkbox"/> Masonry | <input type="checkbox"/> Mechanical & Electrical Systems |
| <input checked="" type="checkbox"/> Structural Steel | <input type="checkbox"/> Architectural Systems |
| <input type="checkbox"/> Cold-Formed Steel Framing | <input type="checkbox"/> Special Cases |

Special Inspection Agencies	Firm	Address, Telephone, e-mail
1. Special Inspection Coordinator <i>STEVEN R. GRANT</i>	SRG ENGINEERING, INC. PO BOX 925 GRAY, ME 04039	207-657-7323 207-657-7342 (Fax) srge@srge19.com
2. Inspector	SRG ENGINEERING, INC. PO BOX 925 GRAY, ME 04039	SAME AS ABOVE
3. Inspector		
4. Testing Agency <i>ROGER DOMINGO</i> or <i>CRIC TURCOTE</i>	<i>J.W. COLE ENG INC</i>	286 PORTLAND ROAD GRAY, ME 04039 657-2866 rdomingo@jwcole.com
5. Testing Agency		
6. Other		

Note: The inspectors and testing agencies shall be engaged by the Owner or the Owner's Agent, and not by the Contractor or Subcontractor whose work is to be inspected or tested. Any conflict of interest must be disclosed to the Building Official, prior to commencing work.

R.D.P. IS BEING RETURNED BY PARO CONSTRUCTION.

Quality Assurance Plan

Quality Assurance for Seismic Resistance *(Per Varco-Pruden Calculations)*

Seismic Design Category *C*
 Quality Assurance Plan Required (Y/N) *yes*

Description of seismic force resisting system and designated seismic systems:
Rigid Frames at Lines 1, 2, 3, 5, 7, B and M

Quality Assurance for Wind Requirements

Basic Wind Speed (3 second gust) *100 mph*
 Wind Exposure Category *B*
 Quality Assurance Plan Required (Y/N) *N*

Description of wind force resisting system and designated wind resisting components:
SAME AS SEISMIC NOTES ABOVE

Statement of Responsibility

Each contractor responsible for the construction or fabrication of a system or component designated above must submit a Statement of Responsibility.
VARCO-PRUDEN MUST SUBMIT THIS - SEE PORTAL CONST.

Qualifications of Inspectors and Testing Technicians

The qualifications of all personnel performing Special Inspection and testing activities are subject to the approval of the Building Official. The credentials of all inspectors and testing technicians shall be provided if requested.

Key for Minimum Qualifications of Inspection Agents:

When the Registered Design Professional in Responsible Charge deems it appropriate that the individual performing a stipulated test or inspection have a specific certification or license as indicated below, such designation shall appear below the *Agency Number* on the Schedule.

PE/SE	Structural Engineer – a licensed SE or PE specializing in the design of building structures
PE/GE	Geotechnical Engineer – a licensed PE specializing in soil mechanics and foundations
EIT	Engineer-In-Training – a graduate engineer who has passed the Fundamentals of Engineering examination

American Concrete Institute (ACI) Certification

ACI-CFTT	Concrete Field Testing Technician – Grade 1
ACI-CCI	Concrete Construction Inspector
ACI-LTT	Laboratory Testing Technician – Grade 1&2
ACI-STT	Strength Testing Technician

American Welding Society (AWS) Certification

AWS-CWI	Certified Welding Inspector
AWS/AISC-SSI	Certified Structural Steel Inspector

American Society of Non-Destructive Testing (ASNT) Certification

ASNT	Non-Destructive Testing Technician – Level II or III.
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International Code Council (ICC) Certification

ICC-SMSI	Structural Masonry Special Inspector
ICC-SWSI	Structural Steel and Welding Special Inspector
ICC-SFSI	Spray-Applied Fireproofing Special Inspector
ICC-PCSI	Prestressed Concrete Special Inspector
ICC-RCSI	Reinforced Concrete Special Inspector

National Institute for Certification in Engineering Technologies (NICET)

NICET-CT	Concrete Technician – Levels I, II, III & IV
NICET-ST	Soils Technician - Levels I, II, III & IV
NICET-GET	Geotechnical Engineering Technician - Levels I, II, III & IV

Exterior Design Institute (EDI) Certification

EDI-EIFS	EIFS Third Party Inspector
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Other

Soils and Foundations

Item	Agency # (Qualif.)	Scope
1. Shallow Foundations	PE/GE (4)	Inspect soils below footings for adequate bearing capacity and consistency with geotechnical report. Inspect removal of unsuitable material and preparation of subgrade prior to placement of controlled fill
2. Controlled Structural Fill	PE/GE (4)	Perform sieve tests (ASTM D422 & D1140) and modified Proctor tests (ASTM D1557) of each source of fill material. Inspect placement, lift thickness and compaction of controlled fill. Test density of each lift of fill by nuclear methods (ASTM D2922) Verify extent and slope of fill placement.
3. Deep Foundations (N.A.)	PE/GE	Inspect and log pile driving operations. Record pile driving resistance and verify compliance with driving criteria. Inspect piles for damage from driving and plumbness. Verify pile size, length and accessories. Inspect installation of drilled pier foundations. Verify pier diameter, bell diameter, lengths, embedment into bedrock and suitability of end bearing strata.
4. Load Testing (N.A.)		
4. Other:		

Cast-in-Place Concrete

Page 5 of 6

Item	Agency # (Qualif.)	Scope
1. Mix Design	② ④ ACI-CCI ICC-RCSI	Review concrete batch tickets and verify compliance with approved mix design. Verify that water added at the site does not exceed that allowed by the mix design.
2. Material Certification		
3. Reinforcement Installation	④ ACI-CCI ICC-RCSI	Inspect size, spacing, cover, positioning and grade of reinforcing steel. Verify that reinforcing bars are free of form oil or other deleterious materials. Inspect bar laps and mechanical splices. Verify that bars are adequately tied and supported on chairs or bolsters
4. Post-Tensioning Operations NA	ICC-PCSI	Inspect placement, stressing, grouting and protection of post-tensioning tendons. Verify that tendons are correctly positioned, supported, tied and wrapped. Record tendon elongations.
5. Welding of Reinforcing NA	AWS-CWI	Visually inspect all reinforcing steel welds. Verify weldability of reinforcing steel. Inspect preheating of steel when required.
6. Anchor Rods	④	Inspect size, positioning and embedment of anchor rods. Inspect concrete placement and consolidation around anchors.
7. Concrete Placement	④ ACI-CCI ICC-RCSI	Inspect placement of concrete. Verify that concrete conveyance and depositing avoids segregation or contamination. Verify that concrete is properly consolidated.
8. Sampling and Testing of Concrete	④ ACI-CFTT ACI-STT	Test concrete compressive strength (ASTM C31 & C39), slump (ASTM C143), air-content (ASTM C231 or C173) and temperature (ASTM C1064).
9. Curing and Protection	④ ACI-CCI ICC-RCSI	Inspect curing, cold weather protection and hot weather protection procedures.
10. Other:		

Structural Steel

Item	Agency # (Qualif.)	Scope
1. Fabricator Certification/ Quality Control Procedures <input checked="" type="checkbox"/> Fabricator Exempt <i>(Vanco-Pruden)</i>	AWS/AISC-SSI ICC-SWSI	Review shop fabrication and quality control procedures. <div style="text-align: center; border: 1px solid black; border-radius: 50%; width: 100px; height: 50px; display: flex; align-items: center; justify-content: center; margin: 0 auto;"> N.A. </div>
2. Material Certification	(2)(4) AWS/AISC-SSI ICC-SWSI	Review certified mill test reports and identification markings on wide-flange shapes, high-strength bolts, nuts and welding electrodes
3. Open Web Steel Joists <div style="text-align: center; border: 1px solid black; border-radius: 50%; width: 80px; height: 50px; display: flex; align-items: center; justify-content: center; margin: 0 auto;"> NA </div>		Inspect installation, field welding and bridging of joists.
4. Bolting	(4) AWS/AISC-SSI ICC-SWSI	Inspect installation and tightening of high-strength bolts. Verify that splines have separated from tension control bolts. Verify proper tightening sequence. Continuous inspection of bolts in slip-critical connections.
5. Welding <div style="text-align: center; border: 1px solid black; border-radius: 50%; width: 80px; height: 50px; display: flex; align-items: center; justify-content: center; margin: 0 auto;"> NA </div>	AWS-CWI ASNT	Visually inspect all welds. Inspect pre-heat, post-heat and surface preparation between passes. Verify size and length of fillet welds. Ultrasonic testing of all full-penetration welds.
6. Shear Connectors <div style="text-align: center; border: 1px solid black; border-radius: 50%; width: 80px; height: 50px; display: flex; align-items: center; justify-content: center; margin: 0 auto;"> NA </div>	AWS/AISC-SSI ICC-SWSI	Inspect size, number, positioning and welding of shear connectors. Inspect suds for full 360 degree flash. Ring test all shear connectors with a 3 lb hammer. Bend test all questionable studs to 15 degrees.
7. Structural Details <div style="text-align: center; border: 1px solid black; border-radius: 50%; width: 80px; height: 50px; display: flex; align-items: center; justify-content: center; margin: 0 auto;"> # </div>	(2)(4) PE/SE	Inspect steel frame for compliance with structural drawings, including bracing, member configuration and connection details.
8. Metal Deck <i>e/roof</i>	(4) AWS-CWI	Inspect welding and side-lap fastening of metal roof and floor deck.
9. Other:		



Letter of Certification
07-16433

Date: 8/27/2007
Time: 10:40:06 AM
Page: 1 of 2

Letter of Certification

Contact: Jon Bell or Bill Rudman
Name: Patco Construction
Address: 1293 Main Street

Project: Camp Bow Wow
Builder PO #:
Jobsite: 49 Blueberry Road

City, State: Sanford, Maine 04073
Country: United States

City, State: Portland, Maine 04104
County, Country: Cumberland, United States

This is to certify that the above referenced VP BUILDINGS project has been designed for the applicable portions of the following Building Code and in accordance with the order documents which have stipulated the following applied environmental loads and conditions:

Overall Building Description

Shape	Overall Width	Overall Length	Floor Area (sq. ft.)	Wall Area (sq. ft.)	Roof Area (sq. ft.)	Max. Eave Height	Min. Eave Height 2	Max. Roof Pitch	Min. Roof Pitch	Peak Height
camp	100/0/0	80/0/0	8000	4973	8110	12/0/0	12/0/0	2.000:12	2.000:12	26/4/0
loading	15/0/0	15/0/0	225	465	228	12/0/0	9/6/0	2.000:12		
Total For All Shapes			8225	5438	8338					

Loads and Codes - Shape: camp

City: Portland County: Cumberland
Building Code: 2003 International Building Code
Building Use: Standard Occupancy Structure

State: Maine
Built Up: 89AISC
Cold Form: 04AISI

Country: United States
Rainfall: 4.00 inches per hour

Dead and Collateral Loads

Collateral Gravity: 3.00 psf
Collateral Uplift: 0.00 psf

Roof Covering + Second. Dead Load: Varies
Frame Weight (assumed for seismic): 2.50 psf

Live Load

Live Load: 20.00 psf Not Reducible

Wind Load

Wind Speed: 100.00 mph
Wind Exposure (Factor): B (0.701)
Parts Wind Exposure Factor: 0.701

Snow Load

Ground Snow Load: 60.00 psf
Design Snow (Sloped): 39.12 psf
Snow Exposure Category (Factor): 2 Partially Exposed (1.00)
Snow Importance: 1.000
Thermal Category (Factor): Heated (1.00)
Ground / Roof Conversion: 0.70
% Snow Used in Seismic: 20.00
Seismic Snow Load: 8.40 psf
Unobstructed, Slippery Roof

Seismic Load

Mapped Spectral Response - Ss: 36.90 %g
Mapped Spectral Response - S1: 9.80 %g
Seismic Hazard / Use Group: Group 1

Seismic Importance: 1.000
Seismic Performance / Design Category: C
System NOT detailed for Seismic
Framing Seismic Period: 0.2044
Bracing Seismic Period: 0.1289
Framing R-Factor: 3.0000
Bracing R-Factor: 3.0000
Soil Profile Type: Stiff soil (D, 4)
Frame Redundancy Factor: 1.0000
Brace Redundancy Factor: 1.0000
Frame Seismic Factor (Cs): 0.1234 x W
Brace Seismic Factor (Cs): 0.1234 x W

Wind Enclosure: Enclosed

Wind Importance Factor: 1.000
Topographic Factor: 1.0000
Hurricane Prone Region
NOT Windborne Debris Region
Base Elevation: 0/0/0
Primary Zone Strip Width: 9/7/3
Parts / Portions Zone Strip Width: 4/9/3
Basic Wind Pressure: 15.24 psf

Per Article 2.9 in the Builder Agreement, VP Buildings assumes that the Builder has called the local Building Official or Project Engineer to obtain all code and loading information for this specific building site.

Loads and Codes - Shape: loading

City: Portland County: Cumberland
Building Code: 2003 International Building Code
Building Use: Standard Occupancy Structure

State: Maine
Built Up: 89AISC
Cold Form: 04AISI

Country: United States
Rainfall: 4.00 inches per hour

Dead and Collateral Loads

Collateral Gravity: 3.00 psf
Collateral Uplift: 0.00 psf

Roof Covering + Second. Dead Load: Varies
Frame Weight (assumed for seismic): 2.50 psf

Live Load

Live Load: 20.00 psf Not Reducible

Wind Load

Wind Speed: 100.00 mph
Wind Exposure (Factor): B (0.701)
Parts Wind Exposure Factor: 0.701

Snow Load

Ground Snow Load: 60.00 psf
Design Snow (Sloped): 39.12 psf
Snow Exposure Category (Factor): 2 Partially Exposed (1.00)
Snow Importance: 1.000
Thermal Category (Factor): Heated (1.00)
Ground / Roof Conversion: 0.70
% Snow Used in Seismic: 20.00
Seismic Snow Load: 8.40 psf

Seismic Load

Mapped Spectral Response - Ss: 36.90 %g
Mapped Spectral Response - S1: 9.80 %g
Seismic Hazard / Use Group: Group 1

Seismic Importance: 1.000
Seismic Performance / Design Category: C
System NOT detailed for Seismic
Framing Seismic Period: 0.1872
Bracing Seismic Period: 0.1187

Wind Enclosure: Enclosed

Wind Importance Factor: 1.000
Topographic Factor: 1.0000
Hurricane Prone Region
NOT Windborne Debris Region



Letter of Certification

Date: 8/27/2007

Time: 10:40:06 AM

Page: 2 of 2

Base Elevation: 0/0/0
Primary Zone Strip Width: 8/7/3
Parts / Portions Zone Strip Width: 4/3/10
Basic Wind Pressure: 15.24 psf

Unobstructed, Slippery Roof

Framing R-Factor: 3.0000
Bracing R-Factor: 3.0000
Soil Profile Type: Stiff soil (D, 4)
Frame Redundancy Factor: 1.0000
Brace Redundancy Factor: 1.0000
Frame Seismic Factor (Cs): 0.1234 x W
Brace Seismic Factor-(Cs): 0.1234 x W

Per Article 2.9 in the Builder Agreement, VP Buildings assumes that the Builder has called the local Building Official or Project Engineer to obtain all code and loading information for this specific building site.

The steel design is in accordance with VP BUILDINGS standard design practices, which have been established based upon pertinent procedures and recommendations of the following organizations:

- American Institute of Steel Construction (AISC)
American Iron and Steel Institute (AISI)
American Welding Society (AWS)
American Society for Testing and Materials (ASTM)
Canadian Standards Association
CSA W59-Welded Steel Construction
Limit State Design of Steel Structures
Metal Building Manufacturers Association (MBMA)
VP Buildings is certified by:
- AISC-MB Certified (Design and Manufacturing)
- CSA A660 Certified (Design and Manufacturing)
- IAS Approved Fabricator
- Canadian Welding Bureau Div. I Certified

VP Buildings has designed the structural steel components of this building in accordance with the Building Code, Steel Specifications, and Standards indicated above. Steel components are designed utilizing the following steel grades unless noted otherwise:

- 3 Plate members fabricated from plate, bar, strip steel or sheets
ASTM A529, A572, A1011 - All Grade 55 ksi
Hot Rolled Shapes (W, S, C, Angles, etc)
ASTM A36, or ASTM A36Mod50, A529, A572, A588, A709, A992-All Grade 50 ksi
Tube and Pipe Sections ASTM A500, Grade B (Fy - 42 ksi pipe, Fy - 46 ksi tube)
Light Gage Sections ASTM A1011 SS Grade 55 ksi, A653 SS Grade 55 ksi
Round Rod Bracing ASTM A572 Grade 65 ksi

This certification DOES NOT apply to the design of the foundation or other on-site structures or components not supplied by VP BUILDINGS, nor does it apply to unauthorized modifications to framing systems provided by VP BUILDINGS.

Furthermore, it is understood that certification is based upon the premise that all components furnished by VP BUILDINGS will be erected or constructed in strict compliance with pertinent documents furnished by VP BUILDINGS.

Sincerely

Handwritten signature of G. Walker

P.E. Prepared by: WTC Reviewed by: WTC

VP BUILDINGS:
3200 Players Club Circle, Memphis, TN 38122-8843





Certificate of Design

Date: SEPT. 7, 2007

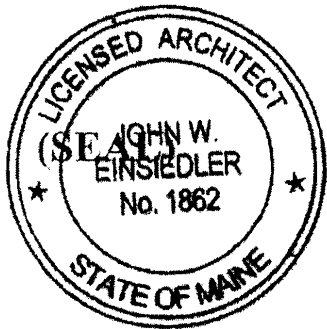
From: JOHN W. EINSIEDLER, R.A.

These plans and / or specifications covering construction work on:

CAMP BOW W/ W, 49 BLUEBERRY AVLE

Have been designed and drawn up by the undersigned, a Maine registered Architect / Engineer according to the *2003 International Building Code* and local amendments.

NOTE: STRUCTURE BY OTHERS



Signature: John W. Einsiedler

Title: ARCHITECT

Firm: JOHN W. EINSIEDLER, R.A.

Address: 148 SEA ROAD

KENNEBUNK, ME 04043

Phone: 207-985-9760

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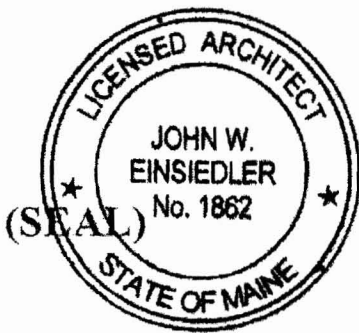
Accessibility Building Code Certificate

Designer: JOHN EINSIEDLER, R.A.

Address of Project: 49 BLUEBERRY AVE

Nature of Project: DOGIE HOTEL

The technical submissions covering the proposed construction work as described above have been designed in compliance with applicable referenced standards found in the Maine Human Rights Law and Federal Americans with Disability Act. Residential Buildings with 4 units or more must conform to the Federal Fair Housing Accessibility Standards. Please provide proof of compliance if applicable.



Signature: *John W. Einsiedler*

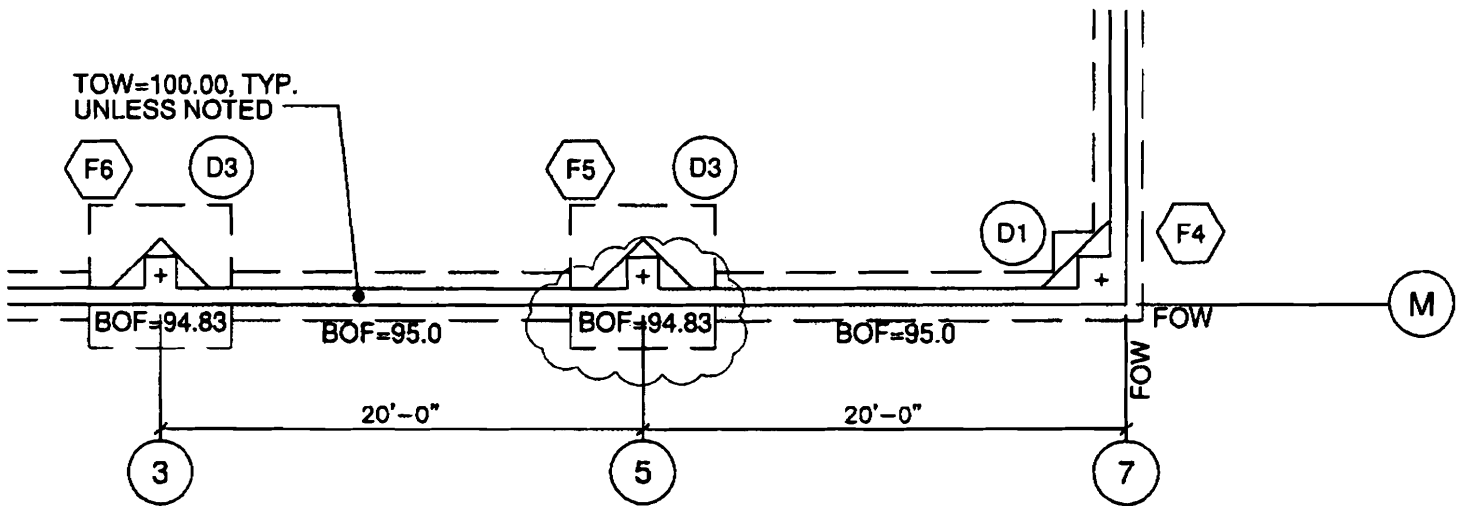
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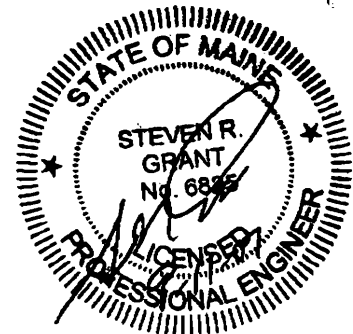
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PART FOUNDATION PLAN

SCALE: 1/8"=1'-0"



PART FOUNDATION PLAN
OF:
CAMP BOW WOW
PORTLAND, MAINE

FOR:
PATCO CONSTRUCTION, INC.
1293 MAIN STREET
SANFORD, ME 04073



P.O. BOX 925
GRAY, ME 04039
TEL: (207) 657-7323
TEL: (207) 657-7342

DESIGN BY:	SRG
DRAWN BY:	SRG
CHECKED BY:	SRG
DATE:	9.11.07
SCALE:	N.T.S.
FIELD BK:	---
PROJ. NO:	07-083
DRAWING:	07-083DT1
SHEET 1 OF 1	



Certificate of Design Application

From Designer:

JOHN EINSIEDLER, R.A.

Date:

SEPT. 7, 2007

Job Name:

CAMP BOW WOK

Address of Construction:

49 BLUEBERRY AVE

2003 International Building Code

Construction project was designed to the building code criteria listed below:

Building Code & Year 2003 IBC Use Group Classification (s) B

Type of Construction 5B

Will the Structure have a Fire suppression system in Accordance with Section 903.3.1 of the 2003 IRC NO

Is the Structure mixed use? NO If yes, separated or non separated or non separated (section 302.3)

Supervisory alarm System? YES Geotechnical/Soils report required? (See Section 1802.2)

BY OTHERS

Structural Design Calculations

Submitted for all structural members (106.1 - 106.11)

Design Loads on Construction Documents (1603)

Uniformly distributed floor live loads (1603.1.1, 1807)

Floor Area Use	Loads Shown

Wind loads (1603.1.4, 1609)

- Design option utilized (1609.1.1, 1609.6)
- Basic wind speed (1809.3)
- Building category and wind importance Factor, I_w (table 1604.5, 1609.5)
- Wind exposure category (1609.4)
- Internal pressure coefficient (ASCE 7)
- Component and cladding pressures (1609.1.1, 1609.6.2.2)
- Main force wind pressures (1603.1.1, 1609.6.2.1)

Earth design data (1603.1.5, 1614-1623)

- Design option utilized (1614.1)
- Seismic use group ("Category")
- Spectral response coefficients, S_D & S_1 (1615.1)
- Site class (1615.1.5)

BY OTHERS

- Live load reduction
- Roof live loads (1603.1.2, 1607.11)
- Roof snow loads (1603.7.3, 1608)
- Ground snow load, P_g (1608.2)
- If $P_g > 10$ psf, flat-roof snow load p_f
- If $P_g > 10$ psf, snow exposure factor, C_e
- If $P_g > 10$ psf, snow load importance factor, I_s
- Roof thermal factor, C_t (1608.4)
- Sloped roof snowload, P_s (1608.4)
- Seismic design category (1616.3)
- Basic seismic force resisting system (1617.6.2)
- Response modification coefficient, R , and deflection amplification factor, C_d (1617.6.2)
- Analysis procedure (1616.6, 1617.5)
- Design base shear (1617.4, 1617.5.1)

Flood loads (1803.1.6, 1612)

- Flood Hazard area (1612.3)
- Elevation of structure

Other loads

- Concentrated loads (1607.4)
- Partition loads (1607.5)
- Misc. loads (Table 1607.8, 1607.6.1, 1607.7, 1607.12, 1607.13, 1610, 1611, 2404)

BY OTHERS

Ron Mercier

From: Bill Rudman [brudman@patco.com]
Sent: Tuesday, November 20, 2007 10:53 AM
To: Jason Gardner; Ron Mercier; Greg Patterson ; Dennis Waters ; Rick Day; Jon Bell
Subject: FW: Camp Bow Wow

This guy is a real thorn

-----Original Message-----

From: Dave Turner [mailto:DTurner@vp.com]
Sent: Tuesday, November 20, 2007 7:11 AM
To: 'Bill Rudman'
Subject: FW: Camp Bow Wow

Bill,

Who's Mike Nugent?

Dave

-----Original Message-----

From: Mike Nugent [mailto:mjn@portlandmaine.gov]
Sent: Monday, November 19, 2007 7:47 PM
To: je@johnarchitect.com; info@patco.com; Jeanie Bourke; Dave Turner
Cc: Gregory Cass; Lannie Dobson
Subject: Camp Bow Wow

Thank you for the additional information. I have reviewed the submissions and the following items remain:

✓ 1) ~~The Seismic Q/A plan needs to be updated to reflect the upgraded Seismic Design category "D".~~

2) ~~Carl Walker from VP needs to provide an updated Letter of Certification to reflect the upgraded Seismic Design category "D".~~ *Bill working on*

~~Because it is not an A, B or C the system will have to be detailed for seismic.~~

3) ~~The updated foundation plan needs to be stamped by Steve Grant. Why does line 13 still refer to the 2002 geotechnical report, shouldn't this be changed as well?~~ *Spoke to Steve Grant*

4) ~~Please provide a copy of the Fire Marshall's Construction permit with any conditions of approval.~~ *Bill working on*

5) Please provide stamped Plumbing, Electrical and HVAC plans. *Sub-contractors*

6) ~~Captain Cass, are you all set with the egress for this project?~~ *Yes*

7) Please provide a detail of the interior wall finish in the kennel area that is a part of the stamped plan set. *Jason working on*

✓ My concern with the egress is probably due to my lack of experience with this use group. The fencing and gates creates obstacles that seem to be in conflict with Chapter 10. The "corridors" are not rated which is normally required for a non-sprinklered "B" occupancy. Finally the second means of egress goes into a storage area which appears to be in violation of Section 1013.2. *John E - responded to email*

We should probably get together and discuss this.

Thanks All!

>>> Mike Nugent 10/29 9:32 PM >>>

Question # 8



SPECIFICATIONS FOR CAMP BOW WOW® FENCING

PART 1 GENERAL

1.1 SCOPE OF WORK

The work includes the furnishing and installation of galvanized chain link, framework, fabric, accessories, gates and related hardware, concrete footings and all other work necessary to complete the installation as described in the following paragraph and drawings.

1.2 MATERIALS

A. CBW STANDARDS

1. Fabric 2" mesh 9gauge 1.2oz GAW (galvanized after weaving, no vinyl coating permitted).
2. All line post, terminal post, corner post, pull post, top rail, middle rail, bottom rail should be min. of schedule 20 galvanized pipe zinc coating 1.8oz
3. **All fittings galvanized and any protruding bolts must be grinded flush**
4. All top wire 3 strands of non-barbed galvanized wire
5. All kick-boards 10" treated wood
6. All PDS slats are top-locking (forest green)
7. All round doweled post virgin pine not treated
8. All top caps (Safety Top Cap 5.5" green)
9. Tension wire 7gauge galvanized
10. Commercial grade gate latches

PART 2 GENERAL

NOV 15 11

2.1 EXTERIOR FENCING

A. 7' high perimeter fence

1. 7' high fabric
2. 2 3/8" line post
3. 2 7/8" terminal posts (corner post, pull post, gate post etc.)

4. 1 5/8" rails (top, middle, bottom)
5. 2" x 10", 2" x 4" treated wood
6. 7' high PDS slats top-locking
7. 3 strands of non-barbed wire (facing inward)
8. 10" x 36", 12" x 36" footers

7' high perimeter fence shall be installed with line post and terminal posts in concrete footers. The frame work shall include top, middle, bottom rail using line rail clamps and rail ends (**middle rail must be installed @ 42"**). Chain link shall be installed inside of framing with 7' high PDS top-locking slats. Kick-board shall be installed on the outside of fencing secured by welded tab on bottom of posts and bolted on. Under security cage gates shall be 2" x 4" treated wood across the bottom of the gate post to secure gravel. Install 3 strands of non-barbed wire using 45 degree barb-arms facing inward.

B. 7' high divider fence inside perimeter fence

1. 7' high fabric (finished edge on bottom)
2. 2 3/8" line post
3. 2 7/8" terminal posts (corner post, pull post, gate post etc.)
4. 1 5/8" rails (top, middle, bottom)
5. 42" high PDS slats top-locking
6. Top Cap (cover fence posts)
7. 10" x 36", 12" x 36" footers

7' high divider fence shall be installed with line post and terminal posts in concrete footers. The frame work shall include top, middle, bottom rail using line rail clamps and rail ends (**middle rail must be installed @ 42"**). Chain link shall be installed inside of framing with 42" high PDS top-locking slats on the bottom of chain link fence. **Finished mesh edge at bottom and top cap must be installed on top of fence.**

NOTE:

TO INSTALL ONE TERMINAL POST IN PERIMETER FENCE LINE WHEN THE DIVIDER FENCE MEETS PERIMETER FENCE

PART 3 GENERAL

3.1 INTERIOR FENCE

- A. 5' high perimeter fence (All posts to be welded plated and anchored to concrete floor)
1. 5' high fabric (finished edge on bottom)
 2. 2 3/8" line post
 3. 2 7/8" terminal posts (corner post, pull post, gate post etc.) may needed to be offset for minimized gap.
 4. 1 5/8" rails (top)
 5. 9ga bottom tension wire
 6. Top Cap (cover fence posts)

7. No PDS slats
8. 6"x 6" plate

5' high perimeter fence shall be installed with line post and terminal posts plated and anchored. The frame work shall include top rail using rail ends and tension wire on the bottom. Chain link shall be installed inside of framing, **finished mesh edge on the bottom and top-cap installed on top of fence.**

B. 5' high divider fence inside perimeter fence (All post to be welded plated and anchored to concrete floor)

1. 5' high fabric (finished edge on bottom)
2. 2 3/8" line post
3. 2 7/8" terminal posts (corner post, pull post, gate post etc.)
4. 1 5/8" rails (top, middle, bottom)
5. Top Cap
6. 36" PDS slats
7. 6"x 6" plate

5' high divider fence shall be installed with line post and terminal posts plated and anchored 6' on center. The frame work shall include top, middle, bottom rail using line rail clamps and rail ends (**middle rail must be installed @ 36"**). Chain link shall be installed inside of framing with 36" high PDS top-locking slats on the bottom of chain link fence. **Finished mesh edge on the bottom and top cap must be installed on top of fence.**

NOTE:

TO INSTALL ONE TERMINAL POST IN PERIMETER FENCE LINE WHEN THE DIVIDER FENCE MEETS PERIMETER FENCE

C. GATES

Installation of gates should be tested and to set latching post for correct fitness if there is more than a 3" gap between cages or walls need to weld a flat plate for minimized gap. Use commercial grade gate latches only. All gates in the indoor play areas and in the kennel area are 5' with Top Cap. All gates in the outdoor play areas are 7ft. with Top Cap. Privacy slats on gates match slats on fence mesh. **CBW 4-way gate system posts between indoor play areas and cabin corridors are 4' apart and installed after all cabins are installed and in place.**

D. SPLIT RAIL LOG ACCENTS

5' high three rails doweled white pine wood (non-treated) in front of perimeter chain link fence.

1. 5' high posts 4 1/2"-5" diameter
2. 3"dia. x 8' long doweled rails
3. 6"dia. x 6" high pipe galvanized with welded tab inside to be anchored to concrete floor
4. Por-Rock