

August 8, 2014

Ms. Jean Fraser City of Portland Planning Department 389 Congress Portland, ME 04101

Subject: Congress Street Convenience Store –Portland, Maine Comment Response # 1 Response to City of Portland Comments of Jean Fraser Dated August 1, 2014

Dear Ms. Fraser:

We have received and reviewed the email review comments dated August 1, 2014 regarding the Amended Site Plan for the 2282 Congress Street Convenience Store project. We have completed plan revisions related to these comments as well as revisions based on the client's value engineering requests. For ease of reference, we have repeated the comments (*italics*) followed by our response.

Comment 1

The parking spaces on the Congress St. side need to be set back 10' from the MTA property line.

Response

The accompanying plans have been modified to show the shifted parking spaces meeting the setback requirements from the MTA property line.

Comment 2

Add the "future" electric charging station to the plan (at those Congress St. side parking area)

Response

The accompanying plans now include a "future" electric charging station at the Congress Street side parking area.

Comment 3

Just put the building box "future CNG building" back on the plan (does not need to work grade wise)

Response

The "future CNG building" has been re-added to the plans per the comment.

FAY, SPOFFORD & THORNDIKE

Ms. Jean Fraser August 8, 2014 Page 2

Comment 4

David needs to review the stormwater management plan closer because it is a change of direction in scope.

Response

See response to our peer review comments from David Senus dated August 5, below.

Comment 5

Architecture-Marge needs to see the revise elevation

Response

The Applicant's architect is completing updated building elevations which we will forward to you under separate cover in a few days.

Comment 6

Floor plan- Marge needs to see the revised elevations

Response

Same as above.

Comment 7

Site Lighting- Are there alternative fixtures available?-less institutional

Response

At this time, the applicant proposes no changes to the lighting fixtures that were originally approved.

Comment 8

Waiting for Tom Errico to confirm the off-site improvement plan

Response

We have included the offsite improvement plans in the plans accompanying this letter to allow Mr. Errico's final review. In addition to addressing City Staff's comments, the plan set has also been revised to include a list of value engineering items at the request of the developer. The following responses are provided to David Senus comments dated August 5, 2014:

Comment 1:

The project will disturb over one acre; as such, filing a notice of intent to comply with the Maine Construction General Permit is required. The Applicant should clarify the status of this filing.

FAY, SPOFFORD & THORNDIKE

Ms. Jean Fraser August 8, 2014 Page 3

Response:

The Applicant intends to complete the Notice of Intent to Comply prior to construction. Evidence of this filing will be provided to the City Planning Department for your records.

Comment 2:

The Applicant obtained a Waste Discharge Permit from the MaineDEP and the Long Creek Watershed District in October of 2013, and has noted that an amended approval is being sought concurrent with the City submission. The Applicant provides a copy of the revised permit upon receipt.

Response:

It is the Applicant's intent to supply the City with evidence of MaineDEP's approval of the Waste Discharge Permit upon receipt.

Comment 3:

In accordance with Section 5 of the City of Portland's Technical Manual, a Level III Site Plan project is required to submit a stormwater management plan pursuant to the regulations of MaineDEP Chapter 50 Stormwater Management Rules, including conformance with the Basic, General, and Flooding Standards. We offer the following comments:

- a) Basic Standard: Plans, notes, and details have been provided to address erosion and sediment control requirements, inspection and maintenance requirements, and good housekeeping practices in general accordance with Appendix A, B, C, of MaineDEP Chapter 500.
- *b) General Standard: The proposed project will provide adequate stormwater quality control in compliance with the General Standards.*
- *c)* Flooding Standard: The Applicant has sufficiently demonstrated compliance with Flooding Standard for the 2, 10, & 25-year storm events.

Response:

- a) No further response is required
- b) No further response is required
- c) No further response is required

Comment 4:

The Applicant should provide a detail for the reinforced turf spillway channel.

Response:

Detail F on Sheet C-8.6 is intended to describe the installation measures for the reinforced turf spillway.

FAY, SPOFFORD & THORNDIKE

Ms. Jean Fraser August 8, 2014 Page 4

In addition to addressing City Staff comments, the plan set has also been revised to include a list of value engineering items requested by the applicant.

Below is a list of these plan revisions.

- The underground storage fuel tanks have been re-arranged to be stacked side by side to reduce the size of the reinforced concrete pad. See Sheet C-2.0.
- Curbing has been reduced around the perimeter of site. Grading alterations have been made to Sheet C-3.0 to accommodate these curb reductions.
- The location of the farm stand and picnic areas have been altered to reduce the size of the stone dust path. See Sheet C-2.0.
- Curb has been reduced in the parking stall island behind the ATM machine. Grading alterations have been made to Sheet C-3.0 to accommodate curb reductions.
- Curbing has been removed from the eight parking spaces along Congress Street.
- Heavy-duty pavement has been reduced in areas that will not see large truck traffic. These areas included passenger vehicle parking areas and area between the store and gas dispenser island. See Sheet C-2.0 for changes to heavy-duty pavement.
- As an option, a Focal PointTM biofiltration system has been added to the plan set. Please see attached supplemental Stormwater Report regarding what effect changing to a Focal PointTM system would have on the stormwater management system. We are including the Focal PointTM system simply as an optional treatment measure that would replace the underdrained soil filter median section of the proposed stormwater management basin.

If you have any questions regarding these responses, or require additional information, please contact this office.

Sincerely,

FAY, SPOFFORD & THORNDIKE

Stephen R. Bushey, P.E. Senior Principal Engineer

SRB/cmd

Attachments:

- Supplemental Stormwater Report and Revised Post Development HydroCAD Results
- Focal PointTM Sizing Computations
- Revised Plans Full Set

c: David Latulippe (electronically submitted)

SUPPLEMENTAL STORMWATER REPORT AND REVISED POST DEVELOPMENT HYDROCAD RESULTS

SUPPLEMENTAL STORMWATER MANAGEMENT REPORT

CONGRESS STREET CONVENIENCE STORE AND FUEL STATION PORTLAND, MAINE

1.0 <u>INTRODUCTION</u>

The applicant is considering a second stormwater treatment option for the Congress Street convenience store project. This supplemental report is intended to analyze the impacts of using a Focal Point[™] Bio-filtration system versus the originally proposed Grassed Underdrained Soil Filter. A Focal PointTM system has been proposed in the plan set as a Value Engineering option. The applicant may use a Focal PointTM system to reduce overall water quality filter area and overall site impacts. According to computations, using a Focal Point[™] system would reduce the total filter area from 3,000 SF to 36 SF. The Focal PointTM system is designed to treat the entire water quality volume at a rate of 100 in/hr, which allows for a significant reduction in filter area. The stormwater detention basin is still designed to provide the same channel protection storage meeting the MeDEP General Standards Requirements. The open detention basin system will meet the flooding standards and manage the release of the 2, 10, and 25-year storm event to below predevelopment conditions as required by the City of Portland Stormwater Regulations. Both options consisting of a grassed underdrained soil filter or Focal PointTM filter have been designed to meet stormwater quality standards required under the general standards as outlined in the adopted MaineDEP Chapter 500 Stormwater Management Technical Manual.

2.0 WATER QUALITY DEVICE – FOCAL POINTTM BIO-FILTRATION SYSTEM

Focal PointTM Bio-Filtration systems are relatively new treatment systems. Focal PointTM systems utilize 18" inches of filter media that is placed below a specified planting arrangement. Focal PointTM systems are in the process of being able to be implemented without the need of a StormTechTM Isolator row (as comparatively required in a FilterraTM system) according to the MaineDEP. This is due to the ability of a Focal PointTM system to remove potential containments as effectively as the StormTechTM pretreatment system. The Focal PointTM system has the ability to force water through the system at a rate of 100 in/hr. This allows the water quality volume to go through the system efficiently while still providing the same level of treatment.

3.0 WATER QUALITY IMPACTS OF FOCAL POINTTM USE

The project requires 5,077 CF of water quality volume treatment. This is calculated based on 1" of tributary impervious area and 0.4" of tributary pervious area. The following table highlights the differences between the grassed underdrained soil filter and the Focal PointTM System with respect to water quality sizing:

FOCA	WATEI L POINT™ AND	TABLE 1 R QUALITY CO GRASSED UND		OIL FILTER	
Water Quality System	Pervious Area Tributary to Treatment (SF)	Impervious Area Tributary to Treatment (SF)	Water Quality Volume (CF)	Filter Area Required* (SF)	Water Quality Depth (')
Focal Point TM	24,454	50,748	5,077	36	1.2'
Grassed Underdrained Soil Filter	24,454	50,748	5,077	3,046	0.5'

*Filter area for Grassed Underdrained Soil Filter computed using 2% of pervious area plus 5% of impervious area. Filter Area for Focal PointTM computed assuming system can flow at a rate of 100 in/hr and thus being able to treat more volume over a smaller area.

Based on the treatment area remaining, the same the Focal Point[™] system will still meet required treatment percentages according to MaineDEP Chapter 500 Stormwater Management Technical Manual. See previous report for treatment percent breakdown.

4.0 <u>PEAK FLOW IMPACTS</u>

The Focal PointTM system requires a maximum water ponding depth of 0.5' in the stormwater detention basin. This requires water release controls inside of the outlet control structure A-1 to be lowered from 89.62' to 88.92'. The change in outlet control elevations required a re-design of the outlet control structure. In order to control the 2-year peak flow a 3" orifice was computed to control the flow at the water quality elevation of 88.92'. This resulted in the following Detention Pond "0" impacts:

TABLE 2 DETENTION POND '0' FOCAL POINT™ AND GRASSED UNDERDRAINED SOIL FILTER COMPARISON						
Water Quality System	Water Quality System25-year DET '0' Peak Pond Outflow (CFS)25-year DET '0' Peak Stage (')25-Year (') '5					
Focal Point TM	1.87	90.97'	12,830			
Grassed Underdrain Filter	3.02	90.96'	12,745			

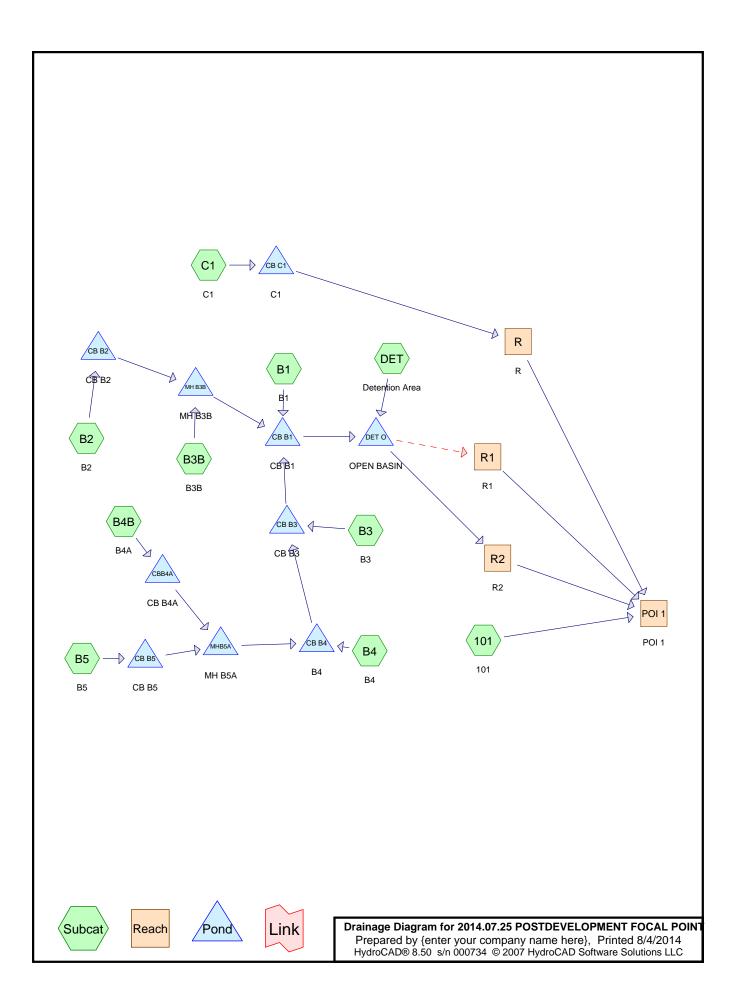
Based on the ability of the Focal PointTM system to release flow at a lower rate from the outlet control structure the following impacts on POI 1 were computed:

FOCAL PO	DINT TM AND	PEAK F	TABLE 3 FLOWS AT PO IDERDRAINI	-	TER COM	PARISON
Water Quality System	2-Year Pre development Flow (CFS)	10-Year Pre development Flow (CFS)	25-Year Pre development Flow (CFS)	2-Year Storm Flow (CFS	10-Year Storm Flow (CFS)	25-Year Storm Flow (CFS)
Focal Point TM Option	2.86	7.18	9.42	2.35	5.38	7.07
Grassed Underdrain Filter Option	2.86	7.18	9.42	2.18	6.34	8.84

As shown both designed treatment systems will meet the flooding standards and manage the release of the 2, 10 and 25-year storm event to below predevelopment conditions as required by the City of Portland Stormwater Regulations.

The Applicant is requesting the right to make a selection between the grassed underdrained soil filter on the Focal PointTM system once full project pricing is practicable.

REVISED POST DEVELOPMENT HYDROCAD RESULTS



Prepared by {enter your company name here} HydroCAD® 8.50 s/n 000734 © 2007 HydroCAD Software Solutions LLC

Area Listing (selected nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
0.439	70	Woods, Good, HSG C (101,B2,B5)
1.016	71	Meadow, non-grazed, HSG C (101,B2,B5)
0.307	74	>75% Grass cover, Good, HSG C (B2,B4B,B5,DET)
0.743	77	Woods, Good, HSG D (101)
0.241	78	Meadow, non-grazed, HSG D (101)
0.099	89	Gravel roads, HSG C (101)
0.294	98	Paved parking & roofs (B3B,B4B)
1.160	98	Paved roads w/curbs & sewers (101,B1,B2,B3,B4,B5,C1)
4.299		TOTAL AREA

Prepared by {enter your company name here} HydroCAD® 8.50 s/n 000734 © 2007 HydroCAD Software Solutions LLC

Soil Listing (selected nodes)

Area	Soil	Subcatchment
(acres)	Goup	Numbers
0.000	HSG A	
0.000	HSG B	
1.861	HSG C	101, B2, B4B, B5, DET
0.984	HSG D	101
1.454	Other	101, B1, B2, B3, B3B, B4, B4B, B5, C1
4.299		TOTAL AREA

Prepared by {enter your company name here} HydroCAD® 8.50 s/n 000734 © 2007 HydroCAD Software Solutions LLC

Time span=0.00-30.00 hrs, dt=0.01 hrs, 3001 points Runoff by SCS TR-20 method, UH=SCS Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 101: 101	Runoff Area=106,667 sf 7.69% Impervious Runoff Depth=1.02" Flow Length=102' Tc=16.1 min CN=76 Runoff=2.03 cfs 0.207 af
Subcatchment B1: B1	Runoff Area=8,879 sf 100.00% Impervious Runoff Depth=2.77" Tc=6.0 min CN=98 Runoff=0.59 cfs 0.047 af
Subcatchment B2: B2	Runoff Area=22,113 sf 55.19% Impervious Runoff Depth=1.66" Tc=6.0 min CN=86 Runoff=0.99 cfs 0.070 af
Subcatchment B3: B3	Runoff Area=4,242 sf 100.00% Impervious Runoff Depth=2.77" Tc=6.0 min CN=98 Runoff=0.28 cfs 0.022 af
Subcatchment B3B: B3B	Runoff Area=4,562 sf 100.00% Impervious Runoff Depth=2.77" Tc=6.0 min CN=98 Runoff=0.30 cfs 0.024 af
Subcatchment B4: B4	Runoff Area=2,843 sf 100.00% Impervious Runoff Depth=2.77" Tc=6.0 min CN=98 Runoff=0.19 cfs 0.015 af
Subcatchment B4B: B4A	Runoff Area=8,504 sf 96.88% Impervious Runoff Depth=2.66" Tc=6.0 min CN=97 Runoff=0.56 cfs 0.043 af
Subcatchment B5: B5	Runoff Area=14,208 sf 68.83% Impervious Runoff Depth=1.98" Tc=6.0 min CN=90 Runoff=0.75 cfs 0.054 af
Subcatchment C1: C1	Runoff Area=4,401 sf 100.00% Impervious Runoff Depth=2.77" Tc=6.0 min CN=98 Runoff=0.29 cfs 0.023 af
Subcatchment DET: Detention	n Area Runoff Area=10,851 sf 0.00% Impervious Runoff Depth=0.91" Tc=6.0 min CN=74 Runoff=0.25 cfs 0.019 af
Reach POI 1: POI 1	Inflow=2.35 cfs 0.511 af Outflow=2.35 cfs 0.511 af
Reach R: R	Avg. Depth=0.04' Max Vel=0.19 fps Inflow=0.29 cfs 0.023 af n=0.200 L=354.0' S=0.0593 '/' Capacity=18.75 cfs Outflow=0.14 cfs 0.023 af
Reach R1: R1	Avg. Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.200 L=196.0' S=0.0306 '/' Capacity=64.45 cfs Outflow=0.00 cfs 0.000 af
Reach R2: R2	Avg. Depth=0.06' Max Vel=0.21 fps Inflow=0.30 cfs 0.282 af =0.200 L=138.0' S=0.0435 '/' Capacity=121.42 cfs Outflow=0.30 cfs 0.281 af
Pond CB B1: CB B1	Inflow=3.67 cfs 0.276 af Primary=3.67 cfs 0.276 af
Pond CB B2: CB B2	Inflow=0.99 cfs 0.070 af Primary=0.99 cfs 0.070 af

2014.07.25 POSTDEVELOPMENT FOCAL POINT Prepared by {enter your company name here}	Type III 24-hr 2 YR Rainfall=3.00" Printed 8/4/2014
HydroCAD® 8.50 s/n 000734 © 2007 HydroCAD Software Solutions LL	
Pond CB B3: CB B3	Inflow=1.78 cfs 0.135 af
	Primary=1.78 cfs 0.135 af
Pond CB B4: B4	Inflow=1.50 cfs 0.112 af
	Primary=1.50 cfs 0.112 af
Pond CB B5: CB B5	Inflow=0.75 cfs 0.054 af
	Primary=0.75 cfs 0.054 af
	Inflow=0.29 cfs_0.023 af
Pond CB C1: C1	Primary=0.29 cfs 0.023 af
	1 hindry=0.29 cis 0.025 ai
Pond CBB4A: CB B4A	Inflow=0.56 cfs 0.043 af
	Primary=0.56 cfs 0.043 af
	Storage=6,232 cf Inflow=3.91 cfs 0.295 af
Primary=0.30 cfs 0.282 af Secondary=0.0	00 cfs 0.000 af Outflow=0.30 cfs 0.282 af
Pond MH B3B: MH B3B	Inflow=1.29 cfs_0.094 af
	Primary=1.29 cfs 0.094 af
Pond MHB5A: MH B5A	Inflow=1.31 cfs 0.097 af
	Primary=1.31 cfs 0.097 af
Total Runoff Area = 4.299 ac Runoff Volume = 66.17% Pervious = 2.3	

Prepared by {enter your company name here} HydroCAD® 8.50 s/n 000734 © 2007 HydroCAD Software Solutions LLC

Summary for Subcatchment 101: 101

Runoff = 2.03 cfs @ 12.24 hrs, Volume= 0.207 af, Depth= 1.02"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2 YR Rainfall=3.00"

_	A	rea (sf)	CN	Description		
_		8,201	98	Paved road	s w/curbs &	& sewers
		4,313	89	Gravel road	ls, HSG C	
		13,704	70	Woods, Go	od, HSG C	
		37,589	71	Meadow, no	on-grazed,	HSG C
		10,508		Meadow, no		
_		32,352	77	Woods, Go	od, HSG D	
	1	06,667	76	Weighted A	verage	
		98,466		Pervious Ar	ea	
		8,201		Impervious	Area	
	_				- ·	
	Tc	Length	Slope		Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	15.9	90	0.0370	0.09		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.00"
	0.2	12	0.0400	1.00		Shallow Concentrated Flow,
-						Woodland Kv= 5.0 fps
	16 1	100	Total			

16.1 102 Total

Summary for Subcatchment B1: B1

Runoff = 0.59 cfs @ 12.08 hrs, Volume= 0.047 af, Depth= 2.77"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2 YR Rainfall=3.00"

Area (sf)	CN	Description		
8,879	98	Paved road	s w/curbs &	& sewers
8,879		Impervious	Area	
Tc Length (min) (feet)	Slope (ft/ft)		Capacity (cfs)	Description
6.0				Direct Entry,

Summary for Subcatchment B2: B2

Runoff = 0.99 cfs @ 12.09 hrs, Volume= 0.070 af, Depth= 1.66"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2 YR Rainfall=3.00"

Prepared by {enter your company name here} HydroCAD® 8.50 s/n 000734 © 2007 HydroCAD Software Solutions LLC

Area (sf)	CN Description
12,204	98 Paved roads w/curbs & sewers
3,238	70 Woods, Good, HSG C
5,811	71 Meadow, non-grazed, HSG C
860	74 >75% Grass cover, Good, HSG C
22,113	86 Weighted Average
9,909	Pervious Area
12,204	Impervious Area
Tc Length	
(min) (feet)	
6.0	Direct Entry,
	Summary for Subcatchment B3: B3
Runoff =	0.28 cfs @ 12.08 hrs, Volume= 0.022 af, Depth= 2.77"
Rupoff by SCS TE	R-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs
	(R Rainfall=3.00")
Area (sf)	CN Description
4,242	98 Paved roads w/curbs & sewers
4,242	Impervious Area
Tc Length	Slope Velocity Capacity Description

IC Length Siope velocity apacity escription (min) (feet) (ft/ft) (ft/sec) (cfs) 6.0

Direct Entry,

Summary for Subcatchment B3B: B3B

Runoff 0.30 cfs @ 12.08 hrs, Volume= 0.024 af, Depth= 2.77" =

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2 YR Rainfall=3.00"

A	rea (sf)	CN	Description		
	4,562	98	Paved park	ing & roofs	
	4,562		Impervious	Area	
Tc (min)	Length (feet)	Slop (ft/ft		Capacity (cfs)	Description
6.0					Direct Entry,

Summary for Subcatchment B4: B4

Runoff 0.19 cfs @ 12.08 hrs, Volume= 0.015 af, Depth= 2.77" =

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2 YR Rainfall=3.00"

Prepared by {enter your company name here} HydroCAD® 8.50 s/n 000734 © 2007 HydroCAD Software Solutions LLC

Area	a (sf) CN	Description
2	2,843 98	Paved roads w/curbs & sewers
2	2,843	Impervious Area
Tc L (min)	ength Slor (feet) (ft/	
6.0	\$ <i>i</i> \$	Direct Entry,
		Summary for Subcatchment B4B: B4A
Runoff	= 0.56	6 cfs @ 12.08 hrs, Volume= 0.043 af, Depth= 2.66"
	SCS TR-20 m -hr 2 YR Rai	nethod, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs infall=3.00"
Area	a (sf) CN	Description
3	3,239 98	Paved parking & roofs
	265 74	>75% Grass cover, Good, HSG C
8	3,504 97	Weighted Average
	265	Pervious Area
٤	3,239	Impervious Area
Tc L _(min)	ength Slop (feet) (ft/	
6.0		Direct Entry,
		Summary for Subcatchment B5: B5
		•
Runoff	= 0.75	5 cfs @ 12.09 hrs, Volume= 0.054 af, Depth= 1.98"
	SCS TR-20 m -hr 2 YR Rai	nethod, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs infall=3.00"
Area	a (sf) CN	Description
ç	9,780 98	Paved roads w/curbs & sewers
2	2,175 70	Woods, Good, HSG C
	000 74	

A	rea (sf)	CN	Description				
	9,780	98	Paved road	s w/curbs &	& sewers		
	2,175	70	Woods, Go	od, HSG C			
	868		Meadow, no				
	1,385	74	>75% Gras	s cover, Go	ood, HSG C		
	14,208	90	00 Weighted Average				
	4,428		Pervious Ar	ea			
	9,780		mpervious	Area			
_							
Tc	Length	Slope		Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
6.0					Direct Entry,		

Prepared by {enter your company name here} HydroCAD® 8.50 s/n 000734 © 2007 HydroCAD Software Solutions LLC

Summary for Subcatchment C1: C1

Runoff = 0.29 cfs @ 12.08 hrs, Volume= 0.023 af, Depth= 2.77"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2 YR Rainfall=3.00"

A	rea (sf)	CN E	Description					
	4,401	98 F	98 Paved roads w/curbs & sewers					
	4,401	l	mpervious	Area				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
6.0					Direct Entry,			

Summary for Subcatchment DET: Detention Area

Runoff = 0.25 cfs @ 12.10 hrs, Volume= 0.019 af, Depth= 0.91"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2 YR Rainfall=3.00"

Area (sf)	CN	Description					
10,851	74	>75% Grass cover, Good, HSG C					
10,851		Pervious A	rea				
Tc Length (min) (feet)	Slop (ft/l		Capacity (cfs)	Description			
6.0				Direct Entry,			

Summary for Reach POI 1: POI 1

Inflow Area =	4.299 ac, 33.83% Impervious, Inflov	w Depth > 1.43" for 2 YR event	
Inflow =	2.35 cfs @ 12.25 hrs, Volume=	0.511 af	
Outflow =	2.35 cfs @ 12.25 hrs, Volume=	0.511 af, Atten= 0%, Lag= 0.0 min	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Reach R: R

Inflow Area	a =	0.101 ac,100	0.00% Impervic	ous, Inflow Depth	h = 2.77"	for 2 YR event
Inflow	=	0.29 cfs @	12.08 hrs, Vol	ume= 0.0	023 af	
Outflow	=	0.14 cfs @	12.24 hrs, Vol	ume= 0.0	023 af, Atte	en= 53%, Lag= 9.4 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Max. Velocity= 0.19 fps, Min. Travel Time= 31.3 min Avg. Velocity = 0.07 fps, Avg. Travel Time= 90.2 min

t

Inflow

=

Prepared by {enter your company name here} HydroCAD® 8.50 s/n 000734 © 2007 HydroCAD Software Solutions LLC

Peak Storage= 261 cf @ 12.24 hrs, Average Depth at Peak Storage= 0.04' Bank-Full Depth= 0.50', Capacity at Bank-Full= 18.75 cfs

18.00' x 0.50' deep channel, n= 0.200 Sheet flow: Woods+light brush Side Slope Z-value= 54.0 '/' Top Width= 72.00' Length= 354.0' Slope= 0.0593 '/' Inlet Invert= 95.00', Outlet Invert= 74.00'

Outflow	=	0.00 cfs @	0.00 hrs, Volume=	0.000 af, Atten= 0%, Lag= 0.0 min
Routina b	v Dvn-St	tor-Ind method	l. Time Span= 0.00-30.00	hrs. dt= 0.01 hrs

0.00 hrs, Volume=

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

0.00 cfs @

Peak Storage= 0 cf @ 0.00 hrs, Average Depth at Peak Storage= 0.00' Bank-Full Depth= 1.00', Capacity at Bank-Full= 64.45 cfs

18.00' x 1.00' deep channel, n= 0.200 Sheet flow: Woods+light brush Side Slope Z-value= 54.0 '/' Top Width= 126.00' Length= 196.0' Slope= 0.0306 '/' Inlet Invert= 80.00', Outlet Invert= 74.00'



Summary for Reach R1: R1

0.000 af

Summary for Reach R2: R2

Inflow Area	a =	1.749 ac, 6	6.60% Imp	ervious,	Inflow D	epth >	1.94"	for 2 Y	R event	
Inflow	=	0.30 cfs @	13.37 hrs,	Volume	=	0.282	af			
Outflow	=	0.30 cfs @	13.52 hrs,	Volume	=	0.281	af, At	ten= 0%,	Lag= 8.7 m	in

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Max. Velocity= 0.21 fps, Min. Travel Time= 11.2 min Avg. Velocity = 0.14 fps, Avg. Travel Time= 16.3 min

Peak Storage= 200 cf @ 13.52 hrs, Average Depth at Peak Storage= 0.06' Bank-Full Depth= 1.00', Capacity at Bank-Full= 121.42 cfs

‡

Prepared by {enter your company name here} HydroCAD® 8.50 s/n 000734 © 2007 HydroCAD Software Solutions LLC

Type III 24-hr 2 YR Rainfall=3.00" Printed 8/4/2014 Page 11

18.00' x 1.00' deep channel, n= 0.200 Sheet flow: Woods+light brush Side Slope Z-value= 100.0 '/' Top Width= 218.00' Length= 138.0' Slope= 0.0435 '/' Inlet Invert= 80.00', Outlet Invert= 74.00'

Summary for Pond CB B1: CB B1

Inflow Area =	1.500 ac, 77.66% Impervious, Infl	ow Depth = 2.21" for 2 YR event
Inflow =	3.67 cfs @ 12.09 hrs, Volume=	0.276 af
Primary =	3.67 cfs @ 12.09 hrs, Volume=	0.276 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Pond CB B2: CB B2

Inflow Area =	0.508 ac, 55.19% Impervious, Infl	ow Depth = 1.66"	for 2 YR event
Inflow =	0.99 cfs @ 12.09 hrs, Volume=	0.070 af	
Primary =	0.99 cfs @ 12.09 hrs, Volume=	0.070 af, Atte	en= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Pond CB B3: CB B3

Inflow Area =	=	0.684 ac, 8	84.25% Impe	ervious,	Inflow Depth	= 2.3	6" for 2 YR	event
Inflow =	=	1.78 cfs @	12.08 hrs,	Volume	= 0.1	35 af		
Primary =	=	1.78 cfs @	12.08 hrs,	Volume	= 0.1	35 af,	Atten= 0%, La	ag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Pond CB B4: B4

Inflow Area	1 =	0.587 ac, 8	31.64% Imp	ervious,	Inflow Depth	= 2.30)" for 2 YR event
Inflow	=	1.50 cfs @	12.09 hrs,	Volume	= 0.1	12 af	
Primary	=	1.50 cfs @	12.09 hrs,	Volume	= 0.1	12 af, /	Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Pond CB B5: CB B5

Inflow Area =	0.326 ac, 68.83%	Impervious, Inflow Depth	= 1.98" for 2 YR event
Inflow =	0.75 cfs @ 12.09	hrs, Volume= 0.0	54 af
Primary =	0.75 cfs @ 12.09	hrs, Volume= 0.0	54 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Prepared by {enter your company name here} HydroCAD® 8.50 s/n 000734 © 2007 HydroCAD Software Solutions LLC

Summary for Pond CB C1: C1

Inflow Area	a =	0.101 ac,10	0.00% Impe	ervious,	Inflow Depth	= 2.77	7" for 2 YF	R event
Inflow	=	0.29 cfs @	12.08 hrs,	Volume=	= 0.0	23 af		
Primary	=	0.29 cfs @	12.08 hrs,	Volume=	= 0.0	23 af, 7	Atten= 0%,	Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Pond CBB4A: CB B4A

Inflow Area =	0.195 ac, 96.88% Impervie	ous, Inflow Depth = 2.66" for 2 YR event	
Inflow =	0.56 cfs @ 12.08 hrs, Vol	lume= 0.043 af	
Primary =	0.56 cfs @ 12.08 hrs, Vol	lume= 0.043 af, Atten= 0%, Lag= 0.0 mir	٦

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Pond DET O: OPEN BASIN

Inflow Area =	1.749 ac, 66.60% Impervious, Inflow De	epth = 2.02" for 2 YR event
Inflow =	3.91 cfs @ 12.09 hrs, Volume=	0.295 af
Outflow =	0.30 cfs @ 13.37 hrs, Volume=	0.282 af, Atten= 92%, Lag= 77.3 min
Primary =	0.30 cfs @ 13.37 hrs, Volume=	0.282 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 89.80' @ 13.37 hrs Surf.Area= 5,095 sf Storage= 6,232 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 251.8 min (1,043.5 - 791.7)

Volume	Invert	Avail.Sto	rage Stor	age Description	
#1	88.42'	19,67	77 cf Cus	tom Stage Data (Pi	rismatic)Listed below (Recalc)
Elevatio		ırf.Area (sq-ft)	Inc.Store (cubic-feet		
88.4	42	3,965	() 0	
89.0		4,433	2,43	·	
90.0		5,263	4,848		
91.0		6,192	5,728	·	
92.0	00	7,141	6,667	7 19,677	
Device	Routing	Invert	Outlet Dev	vices	
#1	Primary	85.32'		•	CPP, projecting, no headwall, Ke= 0.900
					637 '/' Cc= 0.900 n= 0.012
#2	Secondary	91.10'			road-Crested Rectangular Weir
			· ·	/	0.80 1.00 1.20 1.40 1.60
			• •		70 2.65 2.64 2.65 2.65 2.63
#3	Device 1	90.50'			ad-Crested Rectangular Weir
				t) 0.20 0.40 0.60	
щл	Davias 1	00.00		glish) 2.80 2.92 3.	
#4	Device 1	88.92'	3.0° Vert.	Orifice/Grate C= (0.000

Prepared by {enter your company name here} HydroCAD® 8.50 s/n 000734 © 2007 HydroCAD Software Solutions LLC

#5 85.42' 1.3" Vert. Orifice/Grate C= 0.600 Device 1

Primary OutFlow Max=0.30 cfs @ 13.37 hrs HW=89.80' TW=80.06' (Dynamic Tailwater) **1=Culvert** (Passes 0.30 cfs of 22.27 cfs potential flow) -3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-4=Orifice/Grate (Orifice Controls 0.20 cfs @ 4.18 fps)

-5=Orifice/Grate (Orifice Controls 0.09 cfs @ 10.01 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=88.42' TW=80.00' (Dynamic Tailwater) -2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond MH B3B: MH B3B

Inflow Area	a =	0.612 ac, 62.85% Impervious, Inflow Depth = 1.85" for 2 YR	event
Inflow	=	1.29 cfs @ 12.09 hrs, Volume= 0.094 af	
Primary	=	I.29 cfs @ 12.09 hrs, Volume= 0.094 af, Atten= 0%, La	ag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Pond MHB5A: MH B5A

Inflow Area =	0.521 ac, 79.34% Impervious, Inflow	Depth = 2.24" for 2 YR event
Inflow =	1.31 cfs @ 12.09 hrs, Volume=	0.097 af
Primary =	1.31 cfs @ 12.09 hrs, Volume=	0.097 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Prepared by {enter your company name here} HydroCAD® 8.50 s/n 000734 © 2007 HydroCAD Software Solutions LLC

Time span=0.00-30.00 hrs, dt=0.01 hrs, 3001 points Runoff by SCS TR-20 method, UH=SCS Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment101:101	Runoff Area=106,667 sf 7.69% Impervious Runoff Depth=2.29" Flow Length=102' Tc=16.1 min CN=76 Runoff=4.82 cfs 0.467 af
Subcatchment B1: B1	Runoff Area=8,879 sf 100.00% Impervious Runoff Depth=4.46" Tc=6.0 min CN=98 Runoff=0.94 cfs 0.076 af
Subcatchment B2: B2	Runoff Area=22,113 sf 55.19% Impervious Runoff Depth=3.19" Tc=6.0 min CN=86 Runoff=1.88 cfs 0.135 af
Subcatchment B3: B3	Runoff Area=4,242 sf 100.00% Impervious Runoff Depth=4.46" Tc=6.0 min CN=98 Runoff=0.45 cfs 0.036 af
Subcatchment B3B: B3B	Runoff Area=4,562 sf 100.00% Impervious Runoff Depth=4.46" Tc=6.0 min CN=98 Runoff=0.48 cfs 0.039 af
Subcatchment B4: B4	Runoff Area=2,843 sf 100.00% Impervious Runoff Depth=4.46" Tc=6.0 min CN=98 Runoff=0.30 cfs 0.024 af
Subcatchment B4B: B4A	Runoff Area=8,504 sf 96.88% Impervious Runoff Depth=4.35" Tc=6.0 min CN=97 Runoff=0.89 cfs 0.071 af
Subcatchment B5: B5	Runoff Area=14,208 sf 68.83% Impervious Runoff Depth=3.59" Tc=6.0 min CN=90 Runoff=1.33 cfs 0.098 af
Subcatchment C1: C1	Runoff Area=4,401 sf 100.00% Impervious Runoff Depth=4.46" Tc=6.0 min CN=98 Runoff=0.46 cfs 0.038 af
Subcatchment DET: Detention	Dn AreaRunoff Area=10,851 sf0.00% ImperviousRunoff Depth=2.13"Tc=6.0 minCN=74Runoff=0.62 cfs0.044 af
Reach POI 1: POI 1	Inflow=5.38 cfs 0.991 af Outflow=5.38 cfs 0.991 af
Reach R: R	Avg. Depth=0.05' Max Vel=0.23 fps Inflow=0.46 cfs 0.038 af n=0.200 L=354.0' S=0.0593 '/' Capacity=18.75 cfs Outflow=0.24 cfs 0.038 af
Reach R1: R1	Avg. Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.200 L=196.0' S=0.0306 '/' Capacity=64.45 cfs Outflow=0.00 cfs 0.000 af
Reach R2: R2	Avg. Depth=0.10' Max Vel=0.28 fps Inflow=0.86 cfs 0.488 af n=0.200 L=138.0' S=0.0435 '/' Capacity=121.42 cfs Outflow=0.84 cfs 0.486 af
Pond CB B1: CB B1	Inflow=6.26 cfs 0.478 af Primary=6.26 cfs 0.478 af
Pond CB B2: CB B2	Inflow=1.88 cfs 0.135 af Primary=1.88 cfs 0.135 af

2014.07.25 POSTDEVELOPMENT FOCAL POINT Prepared by {enter your company name here}	Type III 24-hr 10 YR Rainfall=4.70" Printed 8/4/2014
HydroCAD® 8.50 s/n 000734 © 2007 HydroCAD Software Solution	ons LLC Page 15
Pond CB B3: CB B3	Inflow=2.97 cfs 0.229 af
	Primary=2.97 cfs 0.229 af
Pond CB B4: B4	Inflow=2.52 cfs 0.193 af
	Primary=2.52 cfs 0.193 af
Pond CB B5: CB B5	Inflow=1.33 cfs 0.098 af
	Primary=1.33 cfs 0.098 af
Pond CB C1: C1	Inflow=0.46 cfs 0.038 af
	Primary=0.46 cfs 0.038 af
Pond CBB4A: CB B4A	Inflow=0.89 cfs_0.071 af
	Primary=0.89 cfs 0.071 af
	········, ······
Pond DET O: OPEN BASIN Peak Elev=90.	72' Storage=11,338 cf Inflow=6.87 cfs 0.523 af
Primary=0.86 cfs 0.488 af Seconda	ary=0.00 cfs 0.000 af Outflow=0.86 cfs 0.488 af
Pond MH B3B: MH B3B	Inflow=2.36 cfs 0.174 af
	Primary=2.36 cfs 0.174 af
Pond MHB5A: MH B5A	Inflow=2.22 cfs 0.168 af
	Primary=2.22 cfs 0.168 af
Total Runoff Area = 4.299 ac Runoff Volu 66.17% Pervious	me = 1.028 af Average Runoff Depth = 2.87" s = 2.845 ac 33.83% Impervious = 1.454 ac

Prepared by {enter your company name here} HydroCAD® 8.50 s/n 000734 © 2007 HydroCAD Software Solutions LLC

Summary for Subcatchment 101: 101

Runoff = 4.82 cfs @ 12.22 hrs, Volume= 0.467 af, Depth= 2.29"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10 YR Rainfall=4.70"

_	A	rea (sf)	CN	Description		
		8,201	98	Paved road	s w/curbs &	& sewers
		4,313	89	Gravel road	ls, HSG C	
		13,704	70	Woods, Go	od, HSG C	
		37,589	71	Meadow, no	on-grazed,	HSG C
		10,508	78	Meadow, no	on-grazed,	HSG D
_		32,352	77	Woods, Go	od, HSG D	
	1	06,667	76	Weighted A	verage	
		98,466		Pervious Ar	ea	
		8,201		Impervious	Area	
	Тс	Length	Slope		Capacity	Description
_	(min)	(feet)	(ft/ft	(ft/sec)	(cfs)	
	15.9	90	0.0370	0.09		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.00"
	0.2	12	0.0400	1.00		Shallow Concentrated Flow,
_						Woodland Kv= 5.0 fps
	16 1	102	Total			

16.1 102 Total

Summary for Subcatchment B1: B1

Runoff = 0.94 cfs @ 12.08 hrs, Volume= 0.076 af, Depth= 4.46"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10 YR Rainfall=4.70"

Area (sf)	CN	Description		
8,879	98	Paved road	ls w/curbs &	& sewers
8,879		Impervious	Area	
Tc Length (min) (feet)	Slop (ft/f		Capacity (cfs)	Description
6.0				Direct Entry,

Summary for Subcatchment B2: B2

Runoff = 1.88 cfs @ 12.09 hrs, Volume= 0.135 af, Depth= 3.19"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10 YR Rainfall=4.70"

Type III 24-hr 10 YR Rainfall=4.70" Printed 8/4/2014 Prepared by {enter your company name here}

HydroCAD® 8.50 s/n 000734 © 2007 HydroCAD Software Solutions LLC						
Area (sf)	CN	Description				
12,204	98	Paved roads w/curbs & sewers				
3,238	70	Woods, Good, HSG C				
5,811	71	Meadow, non-grazed, HSG C				
000	74	TEN/ Cross asver Cood HSC C				

	860	74	>75% Grass cover, Good, HSG C				
22	2,113	86 Weighted Average					
ç	9,909	Pervious Area					
12	2,204	Impervious Area					
Tc L	.ength	Slop	be Velocity Capacity Description				

(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		•
6.0					Direct Entry,	

Summary for Subcatchment B3: B3

Runoff	=	0.45 cfs @	12.08 hrs, Volume	e= 0.036 af, Depth= 4.46"
Runon	_	0.45 015 @		= 0.030 al, Depti = 4.40

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10 YR Rainfall=4.70"

A	rea (sf)	CN [Description						
	4,242	98 F	98 Paved roads w/curbs & sewers						
	4,242	I	Impervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
6.0					Direct Entry,				

Summary for Subcatchment B3B: B3B

Runoff 0.48 cfs @ 12.08 hrs, Volume= 0.039 af, Depth= 4.46" =

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10 YR Rainfall=4.70"

Α	rea (sf)	CN	Description				
	4,562	98	98 Paved parking & roofs				
	4,562		Impervious				
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description		
6.0					Direct Entry,		

Summary for Subcatchment B4: B4

Runoff 0.30 cfs @ 12.08 hrs, Volume= 0.024 af, Depth= 4.46" =

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10 YR Rainfall=4.70"

Prepared by {enter your company name here} HydroCAD® 8.50 s/n 000734 © 2007 HydroCAD Software Solutions LLC

A	rea (sf)	CN [Description						
-	2,843 98 Paved roads w/curbs & sewers								
	2,843	2,843 Impervious Area							
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
6.0	6.0 Direct Entry,								
Summary for Subcatchment B4B: B4A									
Runoff	=	0.89 cf	ⁱ s @ 12.0	8 hrs, Volu	ume= 0.071 af, Depth= 4.35"				
	Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10 YR Rainfall=4.70"								

Α	rea (sf)	CN	Description							
	8,239	98	Paved park	ing & roofs						
	265	74	>75% Ġras	>75% Grass cover, Good, HSG C						
	8,504	97	Weighted Average							
	265		Pervious Area							
	8,239		Impervious	Area						
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description					
6.0			· · · · · ·		Direct Entry,					
			-	_						

Summary for Subcatchment B5: B5

Runoff 1.33 cfs @ 12.09 hrs, Volume= 0.098 af, Depth= 3.59" =

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10 YR Rainfall=4.70"

Area	a (sf)	CN	Description				
g	9,780	98	Paved road	s w/curbs &	& sewers		
2	2,175	70	Woods, Go	od, HSG C			
	868		Meadow, no				
1	,385	74	74 >75% Grass cover, Good, HSG C				
14	,208	90	Weighted A	verage			
4	I,428		Pervious Ar	ea			
g	9,780		Impervious	Area			
	ength.	Slope		Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
6.0					Direct Entry,		

Type III 24-hr 10 YR Rainfall=4.70" Printed 8/4/2014 Page 18

Prepared by {enter your company name here} HydroCAD® 8.50 s/n 000734 © 2007 HydroCAD Software Solutions LLC

Summary for Subcatchment C1: C1

Runoff = 0.46 cfs @ 12.08 hrs, Volume= 0.038 af, Depth= 4.46"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10 YR Rainfall=4.70"

Α	rea (sf)	CN [Description						
	4,401	98 F	98 Paved roads w/curbs & sewers						
	4,401	I	mpervious	Area					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
6.0					Direct Entry,				

Summary for Subcatchment DET: Detention Area

Runoff = 0.62 cfs @ 12.09 hrs, Volume= 0.044 af, Depth= 2.13"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10 YR Rainfall=4.70"

A	rea (sf)	CN D	escription					
	10,851	74 >	74 >75% Grass cover, Good, HSG C					
	10,851	P	ervious Ar	ea				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
6.0					Direct Entry,			

Summary for Reach POI 1: POI 1

Inflow Area =	4.299 ac, 33.83% Impervious, Ir	nflow Depth > 2.77" for 10 YR event
Inflow =	5.38 cfs @ 12.22 hrs, Volume=	0.991 af
Outflow =	5.38 cfs @ 12.22 hrs, Volume=	0.991 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Reach R: R

Inflow Area	a =	0.101 ac,100.00% Impervious, Inflow Depth = 4.46" for 10 YR event
Inflow	=	0.46 cfs @ 12.08 hrs, Volume= 0.038 af
Outflow	=	0.24 cfs @ 12.22 hrs, Volume= 0.038 af, Atten= 48%, Lag= 7.9 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Max. Velocity= 0.23 fps, Min. Travel Time= 25.9 min Avg. Velocity = 0.07 fps, Avg. Travel Time= 80.8 min

t

Inflow

Outflow

=

=

Prepared by {enter your company name here} HydroCAD® 8.50 s/n 000734 © 2007 HydroCAD Software Solutions LLC

Peak Storage= 372 cf @ 12.22 hrs, Average Depth at Peak Storage= 0.05' Bank-Full Depth= 0.50', Capacity at Bank-Full= 18.75 cfs

18.00' x 0.50' deep channel, n= 0.200 Sheet flow: Woods+light brush Side Slope Z-value= 54.0 '/' Top Width= 72.00' Length= 354.0' Slope= 0.0593 '/' Inlet Invert= 95.00', Outlet Invert= 74.00'

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

0.00 hrs, Volume=

0.00 hrs, Volume=

Peak Storage= 0 cf @ 0.00 hrs, Average Depth at Peak Storage= 0.00' Bank-Full Depth= 1.00', Capacity at Bank-Full= 64.45 cfs

0.00 cfs @

0.00 cfs @

18.00' x 1.00' deep channel, n= 0.200 Sheet flow: Woods+light brush Side Slope Z-value= 54.0 '/' Top Width= 126.00' Length= 196.0' Slope= 0.0306 '/' Inlet Invert= 80.00', Outlet Invert= 74.00'



Summary for Reach R1: R1

0.000 af

0.000 af, Atten= 0%, Lag= 0.0 min

Summary for Reach R2: R2

Inflow Area	a =	1.749 ac, 6	6.60% Imp	ervious,	Inflow D	epth >	3.35	5" for 10	YR event
Inflow	=	0.86 cfs @	12.67 hrs,	Volume	=	0.488	af		
Outflow	=	0.84 cfs @	12.83 hrs,	Volume	=	0.486	af, A	Atten= 2%,	Lag= 9.8 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Max. Velocity= 0.28 fps, Min. Travel Time= 8.2 min Avg. Velocity = 0.16 fps, Avg. Travel Time= 14.1 min

Peak Storage= 413 cf @ 12.83 hrs, Average Depth at Peak Storage= 0.10' Bank-Full Depth= 1.00', Capacity at Bank-Full= 121.42 cfs

‡

Prepared by {enter your company name here} HydroCAD® 8.50 s/n 000734 © 2007 HydroCAD Software Solutions LLC

Type III 24-hr 10 YR Rainfall=4.70" Printed 8/4/2014 C Page 21

18.00' x 1.00' deep channel, n= 0.200 Sheet flow: Woods+light brush Side Slope Z-value= 100.0 '/' Top Width= 218.00' Length= 138.0' Slope= 0.0435 '/' Inlet Invert= 80.00', Outlet Invert= 74.00'

Summary for Pond CB B1: CB B1

Inflow Area =	1.500 ac, 77.66% Impervious, Inflow I	Depth = 3.83" for 10 YR event
Inflow =	6.26 cfs @ 12.08 hrs, Volume=	0.478 af
Primary =	6.26 cfs @ 12.08 hrs, Volume=	0.478 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Pond CB B2: CB B2

Inflow Area	=	0.508 ac, 5	5.19% Imper	vious, I	nflow Depth =	= 3.19	" for 10	YR event
Inflow	=	1.88 cfs @	12.09 hrs, V	/olume=	0.13	5 af		
Primary	=	1.88 cfs @	12.09 hrs, V	/olume=	0.13	5 af, A	tten= 0%,	Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Pond CB B3: CB B3

Inflow Area	a =	0.684 ac, 8	84.25% Impe	rvious, Inflow D	epth = 4.01"	for 10 YR event
Inflow	=	2.97 cfs @	12.08 hrs, \	Volume=	0.229 af	
Primary	=	2.97 cfs @	12.08 hrs, \	Volume=	0.229 af, Atte	en= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Pond CB B4: B4

Inflow Area	a =	0.587 ac, 8	31.64% Imp	ervious,	Inflow Dep	oth = 3	8.94"	for 10	YR event
Inflow	=	2.52 cfs @	12.08 hrs,	Volume	= C).193 af	f		
Primary	=	2.52 cfs @	12.08 hrs,	Volume	= 0).193 af	f, Atte	en= 0%,	Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Pond CB B5: CB B5

Inflow Area	=	0.326 ac, 6	8.83% Imp	ervious,	Inflow De	epth =	3.59"	for 10	YR event
Inflow	=	1.33 cfs @	12.09 hrs,	Volume	=	0.098	af		
Primary	=	1.33 cfs @	12.09 hrs,	Volume)=	0.098	af, Att	en= 0%,	Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Pond CB C1: C1

Inflow Area	a =	0.101 ac,10	0.00% Impervi	ous, Inflow De	epth = 4.46"	for 10 YR event
Inflow	=	0.46 cfs @	12.08 hrs, Vo	lume=	0.038 af	
Primary	=	0.46 cfs @	12.08 hrs, Vo	lume=	0.038 af, Atte	en= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Pond CBB4A: CB B4A

Inflow Area	a =	0.195 ac, 96.88% Imperviou	s, Inflow Depth = 4.35"	for 10 YR event
Inflow	=	0.89 cfs @ 12.08 hrs, Volur	ne= 0.071 af	
Primary	=	0.89 cfs @ 12.08 hrs, Volur	ne= 0.071 af, At	tten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Pond DET O: OPEN BASIN

Inflow Area =	1.749 ac, 66.60% Impervious, Inflow De	epth = 3.58" for 10 YR event
Inflow =	6.87 cfs @ 12.09 hrs, Volume=	0.523 af
Outflow =	0.86 cfs @ 12.67 hrs, Volume=	0.488 af, Atten= 88%, Lag= 34.9 min
Primary =	0.86 cfs @ 12.67 hrs, Volume=	0.488 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 90.72' @ 12.67 hrs Surf.Area= 5,936 sf Storage= 11,338 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 252.8 min (1,033.8 - 780.9)

Volume	Invert	Avail.Stor	rage Storage	Description	
#1	88.42'	19,67	77 cf Custom	Stage Data (P	rismatic)Listed below (Recalc)
Elevatio		Irf.Area	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	
88.4	12	3,965	0	0	
89.0	00	4,433	2,435	2,435	
90.0)0	5,263	4,848	7,283	
91.0	00	6,192	5,728	13,011	
92.0	00	7,141	6,667	19,677	
Device	Routing	Invert	Outlet Device	S	
#1	Primary	85.32'	24.0" x 60.0'	long Culvert	CPP, projecting, no headwall, Ke= 0.900
	-		Outlet Invert=	81.50' S= 0.06	637 '/' Cc= 0.900 n= 0.012
#2	Secondary	91.10'	13.0' long x	14.0' breadth B	road-Crested Rectangular Weir
	-		Head (feet) 0	.20 0.40 0.60	0.80 1.00 1.20 1.40 1.60
			Coef. (English	n) 2.64 2.67 2.	70 2.65 2.64 2.65 2.65 2.63
#3	Device 1	90.50'	1.5' long x 0	.5' breadth Bro	ad-Crested Rectangular Weir
			Head (feet) 0	.20 0.40 0.60	0.80 1.00
			Coef. (English	n) 2.80 2.92 3.	08 3.30 3.32
#4	Device 1	88.92'	3.0" Vert. Ori	fice/Grate C= 0	0.600

Prepared by {enter your company name here} HydroCAD® 8.50 s/n 000734 © 2007 HydroCAD Software Solutions LLC

#5 Device 1 85.42' **1.3" Vert. Orifice/Grate** C= 0.600

Primary OutFlow Max=0.86 cfs @ 12.67 hrs HW=90.72' TW=80.10' (Dynamic Tailwater) 1=Culvert (Passes 0.86 cfs of 25.06 cfs potential flow)

3=Broad-Crested Rectangular Weir (Weir Controls 0.45 cfs @ 1.33 fps)

-4=Orifice/Grate (Orifice Controls 0.31 cfs @ 6.24 fps)

-5=Orifice/Grate (Orifice Controls 0.10 cfs @ 11.03 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=88.42' TW=80.00' (Dynamic Tailwater) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond MH B3B: MH B3B

Inflow Are	a =	0.612 ac, 62.85% Im	pervious, Inflow D	epth = 3.41"	for 10 YR event
Inflow	=	2.36 cfs @ 12.09 hrs	s, Volume=	0.174 af	
Primary	=	2.36 cfs @ 12.09 hrs	s, Volume=	0.174 af, Atte	en= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Pond MHB5A: MH B5A

Inflow Area	=	0.521 ac, 7	9.34% Imp	ervious,	Inflow De	pth = 3	3.87"	for 10	YR event
Inflow	=	2.22 cfs @	12.08 hrs,	Volume	=	0.168 a	ıf		
Primary	=	2.22 cfs @	12.08 hrs,	Volume	=	0.168 a	if, Atte	en= 0%,	Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Prepared by {enter your company name here} HydroCAD® 8.50 s/n 000734 © 2007 HydroCAD Software Solutions LLC

Time span=0.00-30.00 hrs, dt=0.01 hrs, 3001 points Runoff by SCS TR-20 method, UH=SCS Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 101: 101	Runoff Area=106,667 sf 7.69% Impervious Runoff Depth=2.95" Flow Length=102' Tc=16.1 min CN=76 Runoff=6.25 cfs 0.603 af
Subcatchment B1: B1	Runoff Area=8,879 sf 100.00% Impervious Runoff Depth=5.26" Tc=6.0 min CN=98 Runoff=1.10 cfs 0.089 af
Subcatchment B2: B2	Runoff Area=22,113 sf 55.19% Impervious Runoff Depth=3.94" Tc=6.0 min CN=86 Runoff=2.30 cfs 0.167 af
Subcatchment B3: B3	Runoff Area=4,242 sf 100.00% Impervious Runoff Depth=5.26" Tc=6.0 min CN=98 Runoff=0.52 cfs 0.043 af
Subcatchment B3B: B3B	Runoff Area=4,562 sf 100.00% Impervious Runoff Depth=5.26" Tc=6.0 min CN=98 Runoff=0.56 cfs 0.046 af
Subcatchment B4: B4	Runoff Area=2,843 sf 100.00% Impervious Runoff Depth=5.26" Tc=6.0 min CN=98 Runoff=0.35 cfs 0.029 af
Subcatchment B4B: B4A	Runoff Area=8,504 sf 96.88% Impervious Runoff Depth=5.15" Tc=6.0 min CN=97 Runoff=1.05 cfs 0.084 af
Subcatchment B5: B5	Runoff Area=14,208 sf 68.83% Impervious Runoff Depth=4.36" Tc=6.0 min CN=90 Runoff=1.60 cfs 0.119 af
Subcatchment C1: C1	Runoff Area=4,401 sf 100.00% Impervious Runoff Depth=5.26" Tc=6.0 min CN=98 Runoff=0.54 cfs 0.044 af
Subcatchment DET: Detenti	on Area Runoff Area=10,851 sf 0.00% Impervious Runoff Depth=2.77" Tc=6.0 min CN=74 Runoff=0.81 cfs 0.057 af
Reach POI 1: POI 1	Inflow=7.07 cfs 1.239 af Outflow=7.07 cfs 1.239 af
Reach R: R	Avg. Depth=0.06' Max Vel=0.24 fps Inflow=0.54 cfs 0.044 af n=0.200 L=354.0' S=0.0593 '/' Capacity=18.75 cfs Outflow=0.29 cfs 0.044 af
Reach R1: R1	Avg. Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.200 L=196.0' S=0.0306 '/' Capacity=64.45 cfs Outflow=0.00 cfs 0.000 af
Reach R2: R2	Avg. Depth=0.15' Max Vel=0.35 fps Inflow=1.87 cfs 0.594 af n=0.200 L=138.0' S=0.0435 '/' Capacity=121.42 cfs Outflow=1.79 cfs 0.592 af
Pond CB B1: CB B1	Inflow=7.48 cfs 0.575 af Primary=7.48 cfs 0.575 af
Pond CB B2: CB B2	Inflow=2.30 cfs 0.167 af Primary=2.30 cfs 0.167 af

2014.07.25 POSTDEVELOPMENT FOCAL POINT Prepared by {enter your company name here} HydroCAD® 8.50 s/n 000734 © 2007 HydroCAD Software Solutions LL	Type III 24-hr 25 YR Rainfall=5.50" Printed 8/4/2014
	_C Page 25
Pond CB B3: CB B3	Inflow=3.52 cfs 0.274 af
	Primary=3.52 cfs 0.274 af
Pond CB B4: B4	Inflow=3.00 cfs 0.231 af
	Primary=3.00 cfs 0.231 af
	Inflow 1.00 efc. 0.110 ef
Pond CB B5: CB B5	Inflow=1.60 cfs 0.119 af
	Primary=1.60 cfs 0.119 af
Pond CB C1: C1	Inflow=0.54 cfs_0.044 af
	Primary=0.54 cfs 0.044 af
Pond CBB4A: CB B4A	Inflow=1.05 cfs 0.084 af
	Primary=1.05 cfs 0.084 af
	Storage=12,830 cf Inflow=8.29 cfs 0.633 af
Primary=1.87 cfs 0.594 af Secondary=0.	.00 cfs 0.000 af Outflow=1.87 cfs 0.594 af
Pond MH B3B: MH B3B	Inflow=2.86 cfs 0.212 af
	Primary=2.86 cfs 0.212 af
Pond MHB5A: MH B5A	Inflow=2.64 cfs_0.202 af
	Primary=2.64 cfs 0.202 af
	:a.y=2.01 010 0.202 a
Total Runoff Area = 4.299 ac Runoff Volume = 66.17% Pervious = 2.	

Prepared by {enter your company name here} HydroCAD® 8.50 s/n 000734 © 2007 HydroCAD Software Solutions LLC

Summary for Subcatchment 101: 101

Runoff = 6.25 cfs @ 12.22 hrs, Volume= 0.603 af, Depth= 2.95"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25 YR Rainfall=5.50"

_	A	rea (sf)	CN	Description		
		8,201	98	Paved road	s w/curbs &	& sewers
		4,313	89	Gravel road	ls, HSG C	
		13,704	70	Woods, Go	od, HSG C	
		37,589	71	Meadow, no	on-grazed,	HSG C
		10,508		Meadow, no		
_		32,352	77	Woods, Go	od, HSG D	
	1	06,667	76	Weighted A	verage	
		98,466		Pervious Ar	ea	
		8,201		Impervious	Area	
	_				- ·	
	Tc	Length	Slope		Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	15.9	90	0.0370	0.09		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.00"
	0.2	12	0.0400	1.00		Shallow Concentrated Flow,
_						Woodland Kv= 5.0 fps
	16 1	102	Total			

16.1 102 Total

Summary for Subcatchment B1: B1

Runoff = 1.10 cfs @ 12.08 hrs, Volume= 0.089 af, Depth= 5.26"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25 YR Rainfall=5.50"

Area (sf)	CN	Description		
8,879	98	Paved road	s w/curbs &	& sewers
8,879		Impervious	Area	
Tc Length (min) (feet)	Slope (ft/ft	,	Capacity (cfs)	Description
6.0				Direct Entry,

Summary for Subcatchment B2: B2

Runoff = 2.30 cfs @ 12.09 hrs, Volume= 0.167 af, Depth= 3.94"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25 YR Rainfall=5.50"

Prepared by {enter your company name here}

HydroCAD® 8.50 s/n 000734 © 2007 HydroCAD Software Solutions LLC	i lopaloa by tolltol your o	empany name nerej
	HydroCAD® 8.50 s/n 000734	© 2007 HydroCAD Software Solutions LLC

Area (sf) CN Description								
12,204 98 Paved roads w/curbs & sewers								
3,238 70 Woods, Good, HSG C								
5,811 71 Meadow, non-grazed, HSG C								
860 74 >75% Grass cover, Good, HSG C								
22,113 86 Weighted Average 9.909 Pervious Area								
9,909 Pervious Area 12,204 Impervious Area								
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)								
6.0 Direct Entry,								
Summary for Subcatchment B3: B3								
Runoff = 0.52 cfs @ 12.08 hrs, Volume= 0.043 af, Depth= 5.26"								
Runoff = $0.52 \text{ cfs} @ 12.08 \text{ hrs}$, Volume= 0.043 af , Depth= 5.26°								
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs								
Type III 24-hr 25 YR Rainfall=5.50"								
Area (sf) CN Description								
4,242 98 Paved roads w/curbs & sewers								
4,242 Impervious Area								
Tc Length Slope Velocity Capacity Description								
(min) (feet) (ft/ft) (ft/sec) (cfs)								
6.0 Direct Entry,								
Summary for Subcatchment B3B: B3B								
Runoff = 0.56 cfs @ 12.08 hrs, Volume= 0.046 af, Depth= 5.26"								
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs								

Runott by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25 YR Rainfall=5.50"

Ar	ea (sf)	CN	Description		
	4,562	98	Paved park	ing & roofs	
	4,562		Impervious	Area	
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description
6.0					Direct Entry,

Summary for Subcatchment B4: B4

Runoff 0.35 cfs @ 12.08 hrs, Volume= 0.029 af, Depth= 5.26" =

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25 YR Rainfall=5.50"

Prepared by {enter your company name here} HydroCAD® 8.50 s/n 000734 © 2007 HydroCAD Software Solutions LLC

A	rea (sf)	CN	Description					
	2,843	98	Paved road	s w/curbs &	sewers			
	2,843		Impervious	Area				
Tc (min)	Length (feet)	Slop (ft/f		Capacity (cfs)	Description			
6.0					Direct Entry,			
	Summary for Subcatchment B4B: B4A							
Runoff	=	1.05	cfs @ 12.08	8 hrs, Volu	me= 0.0	084 af, Depth= 5.	15"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25 YR Rainfall=5.50"								
A	rea (sf)	CN	Description					
	8,239	98	Paved park	ing & roofs				
	265	74	>75% Grass	s cover. Go	od, HSG C			

Α	rea (sf)	CN	Description		
	8,239		Paved park		
	265	74	>75% Ġras	s cover, Go	bod, HSG C
	8,504	97	Weighted A	verage	
	265		Pervious Ar	rea	
	8,239		Impervious	Area	
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description
6.0					Direct Entry,
			-	_	

Summary for Subcatchment B5: B5

Runoff 1.60 cfs @ 12.08 hrs, Volume= 0.119 af, Depth= 4.36" =

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25 YR Rainfall=5.50"

Area	(sf) CN	Description	l	
9,	780 98	Paved road	ls w/curbs &	& sewers
2,	175 70	Woods, Go	od, HSG C	
	868 71		on-grazed,	
1,	385 74	>75% Gras	s cover, Go	bod, HSG C
14,	208 90	Weighted A	verage	
4,	428	Pervious A	rea	
9,	780	Impervious	Area	
		pe Velocity	Capacity	Description
(min) ((feet) (f	/ft) (ft/sec)	(cfs)	
6.0				Direct Entry,

Prepared by {enter your company name here} HydroCAD® 8.50 s/n 000734 © 2007 HydroCAD Software Solutions LLC

Summary for Subcatchment C1: C1

Runoff = 0.54 cfs @ 12.08 hrs, Volume= 0.044 af, Depth= 5.26"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25 YR Rainfall=5.50"

Α	rea (sf)	CN [Description						
	4,401	98 F	98 Paved roads w/curbs & sewers						
	4,401	I	mpervious	Area					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
6.0					Direct Entry,				

Summary for Subcatchment DET: Detention Area

Runoff = 0.81 cfs @ 12.09 hrs, Volume= 0.057 af, Depth= 2.77"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25 YR Rainfall=5.50"

Area (sf) CN	Description		
10,8	51 74	>75% Gras	s cover, Go	ood, HSG C
10,8	51	Pervious A	rea	
Tc Ler (min) (fe	igth Sloj eet) (ft/		Capacity (cfs)	Description
6.0				Direct Entry,

Summary for Reach POI 1: POI 1

Inflow Area	a =	4.299 ac, 33.83% Impervious, Inflow Depth >	3.46" for 25 YR event
Inflow	=	7.07 cfs @ 12.24 hrs, Volume= 1.239	af
Outflow	=	7.07 cfs @ 12.24 hrs, Volume= 1.239	af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Reach R: R

Inflow Area	a =	0.101 ac,100.00% Impervious, Inflow Depth = 5.26" for 25 YR event	
Inflow	=	0.54 cfs @ 12.08 hrs, Volume= 0.044 af	
Outflow	=	0.29 cfs @ 12.21 hrs, Volume= 0.044 af, Atten= 47%, Lag= 7.6 min	۱

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Max. Velocity= 0.24 fps, Min. Travel Time= 24.2 min Avg. Velocity = 0.08 fps, Avg. Travel Time= 77.1 min

Prepared by {enter your company name here} HydroCAD® 8.50 s/n 000734 © 2007 HydroCAD Software Solutions LLC

Peak Storage= 421 cf @ 12.21 hrs, Average Depth at Peak Storage= 0.06' Bank-Full Depth= 0.50', Capacity at Bank-Full= 18.75 cfs

18.00' x 0.50' deep channel, n= 0.200 Sheet flow: Woods+light brush Side Slope Z-value= 54.0 '/' Top Width= 72.00' Length= 354.0' Slope= 0.0593 '/' Inlet Invert= 95.00', Outlet Invert= 74.00'

Outflow=0.00 cfs @0.00 hrs, Volume=0.000 af, Atten= 0%, Lag= 0.0 minRouting by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrsMax. Velocity= 0.00 fps, Min. Travel Time= 0.0 min

0.00 hrs, Volume=

Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

0.00 cfs @

t

Inflow

=

Peak Storage= 0 cf @ 0.00 hrs, Average Depth at Peak Storage= 0.00' Bank-Full Depth= 1.00', Capacity at Bank-Full= 64.45 cfs

18.00' x 1.00' deep channel, n= 0.200 Sheet flow: Woods+light brush Side Slope Z-value= 54.0 '/' Top Width= 126.00' Length= 196.0' Slope= 0.0306 '/' Inlet Invert= 80.00', Outlet Invert= 74.00'



Summary for Reach R1: R1

0.000 af

Summary for Reach R2: R2

Inflow Area	a =	1.749 ac, 6	6.60% Imp	ervious,	Inflow De	epth >	4.08"	for 25	YR event
Inflow	=	1.87 cfs @	12.49 hrs,	Volume	=	0.594 a	af		
Outflow	=	1.79 cfs @	12.58 hrs,	Volume	=	0.592 a	af, At	ten= 4%,	Lag= 5.4 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Max. Velocity= 0.35 fps, Min. Travel Time= 6.6 min Avg. Velocity = 0.17 fps, Avg. Travel Time= 13.5 min

Peak Storage= 713 cf @ 12.58 hrs, Average Depth at Peak Storage= 0.15' Bank-Full Depth= 1.00', Capacity at Bank-Full= 121.42 cfs

‡

Prepared by {enter your company name here} HydroCAD® 8.50 s/n 000734 © 2007 HydroCAD Software Solutions LLC

Type III 24-hr 25 YR Rainfall=5.50" Printed 8/4/2014 C Page 31

18.00' x 1.00' deep channel, n= 0.200 Sheet flow: Woods+light brush Side Slope Z-value= 100.0 '/' Top Width= 218.00' Length= 138.0' Slope= 0.0435 '/' Inlet Invert= 80.00', Outlet Invert= 74.00'

Summary for Pond CB B1: CB B1

Inflow Area	=	1.500 ac, 7	7.66% Impervious	, Inflow Depth =	4.60"	for 25 YR event
Inflow	=	7.48 cfs @	12.08 hrs, Volum	e= 0.575	af	
Primary	=	7.48 cfs @	12.08 hrs, Volum	e= 0.575	af, Atte	en= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Pond CB B2: CB B2

Inflow Area	a =	0.508 ac, 55.19% Impervious, Inflow Depth = 3.94" for 25 YR event	•
Inflow	=	2.30 cfs @ 12.09 hrs, Volume= 0.167 af	
Primary	=	2.30 cfs @ 12.09 hrs, Volume= 0.167 af, Atten= 0%, Lag= 0.0) min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Pond CB B3: CB B3

Inflow Area	a =	0.684 ac, 8	34.25% Impe	ervious, Inflow D	epth = $4.80"$	for 25 YR event
Inflow	=	3.52 cfs @	12.08 hrs,	Volume=	0.274 af	
Primary	=	3.52 cfs @	12.08 hrs,	Volume=	0.274 af, Atte	en= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Pond CB B4: B4

Inflow Area	a =	0.587 ac, 8	1.64% Impe	ervious,	Inflow Dep	oth = 4.7	72" for	25 YR event
Inflow	=	3.00 cfs @	12.08 hrs,	Volume	= (0.231 af		
Primary	=	3.00 cfs @	12.08 hrs,	Volume	= (0.231 af,	Atten= C	0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Pond CB B5: CB B5

Inflow Area =	0.326 ac, 68.83% Impervious, Infl	ow Depth = 4.36" for 25 YR event
Inflow =	1.60 cfs @ 12.08 hrs, Volume=	0.119 af
Primary =	1.60 cfs @ 12.08 hrs, Volume=	0.119 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Pond CB C1: C1

Inflow Area	a =	0.101 ac,10	0.00% Imperv	vious, Inflow De	epth = $5.26"$	for 25 YR event
Inflow	=	0.54 cfs @	12.08 hrs, Vo	olume=	0.044 af	
Primary	=	0.54 cfs @	12.08 hrs, Vo	olume=	0.044 af, Atte	en= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Pond CBB4A: CB B4A

Inflow Area	a =	0.195 ac, 9	6.88% Imperv	vious, Inflow De	epth = $5.15''$	for 25 YR event
Inflow	=	1.05 cfs @	12.08 hrs, Vo	olume=	0.084 af	
Primary	=	1.05 cfs @	12.08 hrs, Vo	olume=	0.084 af, Atte	en= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Pond DET O: OPEN BASIN

Inflow Area =	1.749 ac, 66.60% Impervious, Inflow De	epth = 4.34" for 25 YR event
Inflow =	8.29 cfs @ 12.09 hrs, Volume=	0.633 af
Outflow =	1.87 cfs @ 12.49 hrs, Volume=	0.594 af, Atten= 77%, Lag= 24.2 min
Primary =	1.87 cfs @ 12.49 hrs, Volume=	0.594 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 90.97' @ 12.49 hrs Surf.Area= 6,165 sf Storage= 12,830 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 223.4 min (1,000.7 - 777.3)

Volume	Invert	Avail.Stor	rage Storage	Description	
#1	88.42'	19,67	7 cf Custom	Stage Data (P	rismatic)Listed below (Recalc)
Elevatio		Irf.Area	Inc.Store	Cum.Store	
(fee		(sq-ft)	(cubic-feet)	(cubic-feet)	
88.4	12	3,965	0	0	
89.0	00	4,433	2,435	2,435	
90.0	00	5,263	4,848	7,283	
91.0	00	6,192	5,728	13,011	
92.0	00	7,141	6,667	19,677	
		,	,	,	
Device	Routing	Invert	Outlet Devices	S	
#1	Primary	85.32'	24.0" x 60.0'	long Culvert	CPP, projecting, no headwall, Ke= 0.900
			Outlet Invert=	81.50' S= 0.06	637 '/' Cc= 0.900 n= 0.012
#2	Secondary	91.10'	13.0' long x '	14.0' breadth B	road-Crested Rectangular Weir
	,				0.80 1.00 1.20 1.40 1.60
					70 2.65 2.64 2.65 2.65 2.63
#3	Device 1	90.50'			ad-Crested Rectangular Weir
<i>"</i> 0	Device 1	00.00		.20 0.40 0.60	
			· · · ·	n) 2.80 2.92 3.	
#4	Device 1	88.92'		fice/Grate C= (
#4	Device I	00.92	3.0 vert. On		J.000

Prepared by {enter your company name here} HydroCAD® 8.50 s/n 000734 © 2007 HydroCAD Software Solutions LLC

#5 Device 1 85.42' **1.3" Vert. Orifice/Grate** C= 0.600

Primary OutFlow Max=1.87 cfs @ 12.49 hrs HW=90.97' TW=80.15' (Dynamic Tailwater) 1=Culvert (Passes 1.87 cfs of 25.75 cfs potential flow) 2 Proved Created Pastengular Wair (Weir Controls 1.44 cfs @ 2.04 fps)

3=Broad-Crested Rectangular Weir (Weir Controls 1.44 cfs @ 2.04 fps)

-4=Orifice/Grate (Orifice Controls 0.33 cfs @ 6.68 fps)

-5=Orifice/Grate (Orifice Controls 0.10 cfs @ 11.29 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=88.42' TW=80.00' (Dynamic Tailwater) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond MH B3B: MH B3B

Inflow Area	a =	0.612 ac, 62.8	5% Impervious, Inflow D	epth = 4.16"	for 25 YR event
Inflow	=	2.86 cfs @ 12.	.09 hrs, Volume=	0.212 af	
Primary	=	2.86 cfs @ 12.	.09 hrs, Volume=	0.212 af, Atte	en= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Pond MHB5A: MH B5A

Inflow Area =	. 0.	521 ac, 7	9.34% Imp	ervious,	Inflow De	pth = 4.	.65" for	25 YR event
Inflow =	2.6	64 cfs @	12.08 hrs,	Volume	=	0.202 af		
Primary =	2.0	64 cfs @	12.08 hrs,	Volume	=	0.202 af	, Atten= 0	0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

FOCAL POINTTM SIZING COMPUTATIONS

FOCALPOINT SIZING AND CALCULATION SHEET



PROJECT: 2282 CONGRESS ST DEVELOPMENT

PREPARED BY: ROB WOODMAN, PE – FABCO INDUSTRIES, INC.

PREPARED FOR: BO KENNEDY, PE – FAY, SPOFFORD AND THORNDIKE

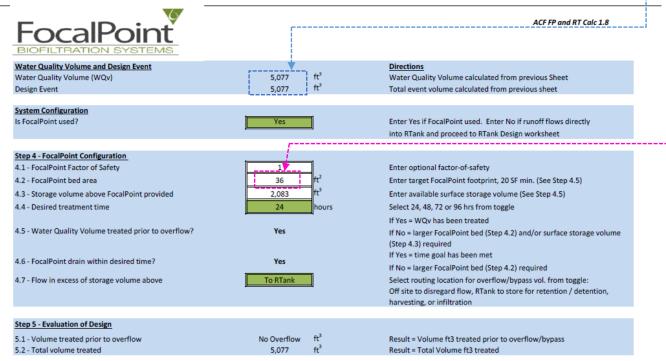
DATE: JULY 24, 2014 revised JULY 25, 2014

Based on the following data provided by the project engineer, Fabco Industries has calculated the proposed sizing of the FocalPoint system and the ability of the system to treat the Water Quality Volume (W_{QV}) prior to overflow/bypass.

• Water Quality Volume (W_{QV})* = 5,077 cf ---- use **5,077 cf** for water quality goal -----

* The Water Quality Volume is based on the Maine DEP Chapter 500 requirement to treat 1.0" of runoff from impervious areas and 0.4" from pervious areas.

Using the *ACF FP and RT Calc version 1.8*, with a type III rainfall distribution. The proposed size of the FocalPoint unit shall be **36 sf** with a minimum ponding volume of **2,083.25 cf** above the unit prior to overflow. The chart below summarizes the associated calculation and performance verification.



Based on the elevation data provided, the top of the FocalPoint could be set at elevation 88.42 with an invert out/bottom of system at elevation 85.42. Another option would be to raise the bottom of the basin up so that the top of FocalPoint is 6 inches below the first outlet control device. Note: the rate of 100 in/hr over the 36 sf FocalPoint point can be converted to a flowrate of 0.083 cfs.