THE FOLLOWING BUILDING CODES AND STANDARDS SHALL BE REFERENCED DURING CONSTRUCTION:

2003 EDITION OF THE IBC INTERNATIONAL BUILDING CODE

AMERICAN SOCIETY OF CIVIL ENGINEERS, MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER

STRUCTURES AMERICAN CONCRETE INSTITUTE SPECIFICATION FOR STRUCTURAL CONCRETE ACI 301

AISC AMERICAN INSTITUTE OF STEEL CONSTRUCTION, MANUAL OF STEEL CONSTRUCTION, NINTH EDITION

ACI 318 AMERICAN CONCRETE INSTITUTE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE

ASTM AMERICAN SOCIETY OF TESTING AND MATERIALS

ASSOCIATION, 2001.

NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION BY NATIONAL FOREST PRODUCTS

REFERENCE ARCHITECTURAL PLANS FOR DIMENSIONS NOT SHOWN. REFERENCE MECHANICAL, ELECTRICAL, AND ARCHITECTURAL PLANS FOR SIZES AND LOCATIONS OF WALL AND SLAB OPENINGS, DUCTS. PIPING. CURBS. AND EQUIPMENT PADS. IN THE EVENT OF A CONFLICT BETWEEN THE DRAWINGS, SPECIFICATIONS, OR NOTES ON THE DRAWINGS, THE ENGINEER SHALL BE NOTIFIED PRIOR TO CONSTRUCTION.

EXISTING DIMENSIONS AND CONDITIONS ARE FOR REFERNCE ONLY. CONTRACTOR SHALL VERIFY ALL EXISTING CONSTRUCTION AND DIMENSIONS IN THE FIELD PRIOR TO CONSTRUCTION OR FABRICATION. ALL DISCREPANCIES SHALL BE REPORTED TO THE ENGINEER PRIOR TO COMMENCING WORK.

THE CONTRACTOR SHALL NOTIFY THE ENGINEER IF DEVIATIONS OR CHANGES ARE REQUIRED TO THE CONTRACT DOCUMENTS OR APPROVED SHOP DRAWINGS DUE TO INTERFERENCES, FABRICATION ERRORS, OR OTHER CAUSES.

THE STRUCTURE IS SELF-SUPPORTING AND STABLE AFTER THE ENTIRE BUILDING IS COMPLETELY CONSTRUCTED. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR ERECTION PROCEDURES AND SEQUENCING DURING CONSTRUCTION AND ERECTION TO PROVIDE AND ENSURE LOCAL AND OVERALL STABILITY OF THE BUILDING AND ITS COMPONENTS DURING CONSTRUCTION AND ERECTION. THE CONTRACTOR SHALL RETAIN A LICENSED STRUCTURAL ENGINEER TO DESIGN TEMPORARY BRACING/SHORING AND DETERMINE WHERE THE TEMPORARY BRACING/SHORING IS NEEDED.

## **GENERAL NOTES**

SCALE: NTS

LIVE LOAD:

NDS

OFFICES = 50 PSF LIVE LOAD + 20 PSF PARTITION LOAD: LOBBIES AND FIRST FLOOR CORRIDORS = 100 PSF CORRIDORS ABOVE FIRST FLOOR = 80 PSF

SNOW LOADS:

GROUND SNOW LOAD, Pg = 60 PSF SNOW EXPOSURE FACTOR, Ce = 1.0 SNOW LOAD IMPORTANCE FACTOR, I = 1.0

FLAT ROOF SNOW LOAD, Pf = 42 PSF + DRIFT

MAIN WINDFORCE-RESISTING SYSTEM

BASIC WIND SPEED = 100 MPH EXPOSURE B WIND LOADS (INCLUDES WINDWARD + LEEWARD) = 15.4 MPH COMPONENTS AND CLADDING = 23 MPH

SEISMIC CRITERIA:

SOIL SITE CLASSIFICATION = DDESIGN SPECTRAL RESPONSE ACCELERATION

Sds = .304Sd1 = .113

SEISMIC USE GROUP I SEISMIC DESIGN CATEGORY C RESPONSE MODIFICATION COEFFICIENT... R = 6.5

OCCUPANCY IMPORTANCE FACTOR.......  $I_E = 1.0$ ...... V = Cs \* W = 0.08 \* W

## DESIGN CRITERIA

SCALE: NTS

REFERENCE SUMMIT GEOENGINEERING GEOTECHNICAL REPORT DATED JANUARY 2002, FOR SUBGRADE AND SITE PREPARATION AND DETERMINATION (INCLUDING BY NOT LIMITED TO, EXCAVATION WORK, STRUCTURAL FILL GRADATION REQUIREMENTS. AND COMPACTION REQUIREMENTS) BENEATH FOOTINGS AND SLAB-ON-GRADES AND BEHIND FOUNDATION WALLS. IN THE EVENT OF A CONFLICT BETWEEN THE GEOTECHNICAL REPORT AND STRUCTURAL DRAWINGS, THE STRUCTURAL ENGINEER SHALL BE NOTIFIED PRIOR TO CONSTRUCTION.

NATIVE AND FILL SOILS WITHIN PROPOSED CONSTRUCTION AREAS SHALL BE DENSIFIED AFTER STRIPPING AND PRIOR TO PLACING FILL. PROOF ROLLING SHOULD CONSIST OF A MINIMUM OF THREE PASSES ALONG THE LONGITUDINAL AXIS OF THE BUILDING AND THEN 3 PASSES IN THE TRANSVERS DIRECTION. SUBGRADE SOILS IN FOOTING TRENCHES SHOULD BE DENSIFIED FOLLOWING FOOTING EXCATION AND PRIOR TO FOOTING CONSTRUCTION. THE SLAB ON GRADE SHOULD BE CONSTRUCTED ON A MINIMUM OF 12 INCHES OF STRUCTURAL FILL, AS SPECIFIED BELOW, OR CRUSHED SONE.

SIEVE	% FINER BY WEIGHT
1/4 INCHES	60 TO 100
#40	0 TO 50
#200	0 TO 7

STRUCTURAL FILL SHALL BE PLACED IN LAYERS NOT TO EXCEED 12 INCHES IN DEPTH AND COMPACTED TO 95% OF MAXIMUM DRY DENSITY PER ASTM TEST D 1557. FOR MORE INFORMATION SEE SUBSURFACE INVESTIGATION REPORT BY SUMMIT GEOENGINEERING DATED JANUARY 2002..

PRESUMED ALLOWABLE SOIL BEARING PRESSURE USED IN DESIGN = 3000psf MINIMUM FROST DEPTH COVER = 4'-0" FOR EXTERIOR FOOTINGS BELOW FINAL EXTERIOR GRADE. CONTRACTOR SHALL NOTIFY ENGINEER OF ANY DISCREPANCIES.

FOUNDATION WALLS SHALL BE BACKFILLED SIMULTANEOUSLY ON BOTH SIDES OF THE WALL. FOUNDATION WALLS AND SLAB-ON-GRADES SHALL REACH THEIR FULL 28 DAY COMPRESSIVE STRENGTH PRIOR TO BACKFILLING. THE THE CONTRACTOR SHALL PROVIDE TEMPORARY SHORING/BRACING FOR WALLS WHEN BACKFILL IS PLACED PRIOR TO CONCRETE ACHIEVING ITS FULL 28 DAY STRENGTH. BACKFILL FOR FOUNDATION WALLS IS BASED ON DRAINED CONDITIONS. SEE ARCHITECTURAL, CIVIL, AND MECHANICAL DRAWINGS FOR FOUNDATION DRAINAGE SYSTEM.. THE FOUNDATION SHALL BE BACKFILLED WITH FOUNDATION BACKFILL MEETING THE FOLLOWING GRADATION.

SIEVE	% FINER BY WEIGHT
1/4 INCHES	25 TO 70
#40	0 TO 30
#200	0 TO 5

PROTECT FOUNDATIONS FROM FROST AND KEEP BOTTOM OF TRENCH DRY DURING CONSTRUCTION. IF GROUNDWATER IS ENCOUNTERED NEAR OR ABOVE THE BASE OF THE FOOTINGS, EXCAVATIONS SHALL BE DEWATERED DURING CONSTRUCTION. SURFACE WATER SHALL BE DIVERTED AWAY FROM EXCAVATIONS.

CONTRACTOR SHALL BE RESPONSIBLE FOR THE SHORING AND BRACING OF EXISTING STRUCTURES DURING EXCAVATION, BACKFILLING, AND CONSTRUCTION. CONTRACTOR SHALL SLOPE EXCAVATIONS TO ACHIEVE SOIL STABILITY.

**FOUNDATION NOTES** SCALE: NTS ALL CONCRETE WORK, INCLUDING MATERIAL SELECTION, ADMIXTURES, MIXING, AND PLACEMENT OF CONCRETE SHALL BE IN CONFORMANCE WITH APPLICABLE BUILDING CODES. IN ADDITION, REFERENCE THE FOLLOWING CONCRETE STANDARDS AND SPECIFICATIONS:

AMERICAN CONCRETE INSTITUTE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE AMERICAN CONCRETE INSTITUTE SPECIFICATIONS FOR STRUCTURAL CONCRETE ACI 301

ACI 305 STANDARD SPECIFICATIONS FOR HOT WEATHER CONCRETING ACI 306 STANDARD SPECIFICATION FOR COLD WEATHER CONCRETING ACI 308 STANDARD PRACTICE FOR CURING CONCRETE

REQUIRED CONCRETE PARAMETERS ARE AS FOLLOWS:

LOCATION	MAX W/C RATIO	f'c	AIR-ENTRAINMENT
INT. CONCR./WALLS/SLABS	.52	3,000 PSI	2% +/- 1 1/2%
FOUNDATIONS, FOOTINGS & FOUNDATION WALLS	.52	3,000 PSI	5–7%
INT. SLAB-ON-GRADE	.47	4,000 PSI	2% +/- 1 1/2%
EXT. SLAB-ON-GRADE	.45	4,000 PSI	6% +/- 1 1/2%

W/C = WATER TO CEMENT RATIO ANDf'c = COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS

MAXIMUM AGGREGATE SIZE SHALL BE 3/4 INCH, IN CONFORMANCE WITH ASTM C33. USE PORTLAND CEMENT TYPE II, IN CONFORMANCE WITH ASTM 150.

AIR ENTRAINING ADMIXTURES SHALL CONFORM TO ASTM C 260. ADMIXTURES SHALL CONFORM TO "SPECIFICATION FOR CHEMICAL ADMIXTURES FOR CONCRETE" ASTM C 494. FLY ASH USED AS ADMIXTURES SHALL CONFORM TO ASTM C 618. CALCIUM CHLORIDE OR ADMIXTURES CONTAINING CALCIUM CHLORIDE IS NOT PERMITTED.

MAXIMUM SLUMP AFTER THE ADDITION OF A WATER-REDUCING ADMIXTURE IS 8 INCHES.

CONCRETE EXPOSED TO FREEZING AND THAWING, INCLUDING FOUNDATIONS, FOOTINGS, FOUNDATION WALLS, AND EXTERIOR WALKWAYS SHALL BE AIR ENTRAINED WITH AIR CONTENT BETWEEN 5% AND 6%. CONTRACTOR SHALL NOT PLACE CONCRETE ON FROZEN GROUND OR IN WATER. ADEQUATE EQUIPMENT SHALL BE PROVIDED FOR HEATING CONCRETE MATERIALS AND PROTECTING CONCRETE DURING NEAR-FREEZING OR FREEZING WEATHER. REFERENCE ACI 306. AS NOTED ABOVE, FOR RECOMMENDATIONS FOR COLD WEATHER CONCRETING.

CONTRACTOR SHALL SUBMIT PROPOSED CONCRETE MIX DESIGN AND LABORATORY TESTS OF FABRICATED CYLINDERS VERIFYING CONCRETE STRENGTH OR PERFORMANCE HISTORY OF MIX TO ENGINEER FOR ACCEPTANCE PRIOR TO PLACEMENT OF CONCRETE. CONCRETE USED ON SITE SHALL BE FIELD TESTED IN ACCORDANCE WITH AND IN THE PRESENCE OF AN APPROVED TESTING AGENCY. FIELD TESTING INFORMATION SHALL INDICATE SLUMP, AIR CONTENT, AND TEMPERATURE. COMPRESSION TEST 1 CYLINDER AT 7 DAYS AND 2 AT 28 DAYS. HOLD AN ADDITIONAL CYLINDER FOR A 56 DAY BREAK, IF NECESSARY. PROVIDE A SET OF 4 CYLINDERS FOR EACH PLACEMENT AND PER 50 CUBIC YARDS OF CONCRETE PLACED. THE OWNER SHALL PAY FOR ALL CONCRETE TESTING.

CONSTRUCTION JOINTS IN WALLS SHALL BE PERMITTED AS DETAILED ON THE STRUCTURAL DRAWINGS. SURFACES OF CONCRETE CONSTRUCTION JOINTS SHALL BE CLEANED AND LAITANCE REMOVED. IMMEDIATELY BEFORE NEW CONCRETE IS PLACED, CONSTRUCTION JOINTS SHALL BE WETTED AND STANDING WATER REMOVED. VERTICAL CONSTRUCTION JOINTS IN WALLS SHALL NOT EXCEED A SPACING OF 40 FEET

WHERE ELECTRICAL CONDUIT/ RADIANT HEATING TUBES RUN IN THE SLAB, THEY SHALL BE LOCATED AT MID-DEPTH OF THE SLAB. ALUMINUM CONDUIT AND SLEEVES ARE NOT PERMITTED. ANCHOR BOLTS SHALL CONFORM TO ASTM A307. ANCHOR BOLTS SHALL HAVE HEAVY HEX NUTS AND LOCK

## **CONCRETE NOTES**

SCALE: NTS

USE DEFORMED BILLET-STEEL REINFORCING BARS, GRADE 60, IN CONFORMANCE WITH ASTM A 615. REINFORCEMENT SHALL BE ACCURATELY PLACED AND SUPPORTED PRIOR TO CONCRETE PLACEMENT, AND SHALL BE SECURED AGAINST DISPLACEMENT.

THE CONTRACTOR SHALL SUBMIT REINFORCING SHOP DRAWINGS TO THE ENGINEER FOR REVIEW AND ACCEPTANCE PRIOR TO COMMENCING FABRICATION. REINFORCEMENT SHALL BE DETAILED IN ACCORDANCE WITH ACI "MANUAL OF STANDARD PRACTICE FOR DETAILING OF REINFORCED CONCRETE STRUCTURES". SHOP DRAWINGS SHALL SHOW REINFORCING STEEL PLACEMENT DETAILS AND SECTIONS.

MINIMUM CONCRETE COVER FOR REINFORCEMENT	
CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH	3 INCHES
CONCRETE EXPOSED TO EARTH OR WEATHER	2 INCHES
CONCRETE NOT EXPOSED TO EARTH OR WEATHER IN SLABS AND WALLS (FOR PRIMARY REINFORCEMENT, TIES, AND STIRRUPS)	1-1/2 INCHES
CONCRETE NOT EXPOSED TO EARTH OF WEATHER IN COLUMNS AND BEAMS	1-1/2 INCHES

CONCRETE NOT EXPOSED TO EARTH OF WEATHER IN COLUMNS AND BEAMS | 1-1/2 INCHES CONTINUOUS REINFORCEMENT SHALL BE TENSION LAP SPLICED PER LAP SPLICE LENGTH TABLE, UNLESS OTHERWISE NOTED.

LAP SPLICE LENGTH TABLE							
BAR SIZE	#3	#4	<b>#</b> 5	#6	<b>#</b> 7	#8	#9
MIN LAP SPLICE (INCHES)	18	24	30	36	48	64	81

REINFORCEMENT HOOKS SHALL CONFORM TO STARDARD HOOKS ACCORDING TO ACI 318, UNLESS OTHERWISE NOTED WELDING OF REINFORCEMENT IS NOT PERMITTED, UNLESS OTHERWISE NOTED.

## CONCRETE REINFORCING NOTES

SCALE: NTS

MASONRY CONSTRUCTION AND MATERIALS SHALL CONFORM TO ALL REQUIREMENTS OF BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (ACI 530/ASCE5/TMS 402) AND SPECIFICATION FOR MASONRY STRUCTURES (ACI 530.1/ASCE6/TMS 602).

MINIMUM COMPRESSIVE STRENGTH OF MASONRY SHALL BE I'm = 1500 PSI AT 28 DAYS.

ALL CONCRETE UNITS SHALL CONFORM TO ASTM C 90, GRADE N, TYPE 1, NORMAL WEIGHT. MINIMUM BLOCK COMPRESSIVE STRENGTH IS 1900 PSI. UNITS ARE SAMPLED AND TESTED IN ACCORDANCE WITH ASTM C 140. SUBMIT TEST RESULTS TO ENGINEER. SEE ARCH FOR COLOR.

MORTAR SHALL CONFORM TO ASTM C 270, TYPE S. SEE ARCH FOR COLOR.

HORIZONTAL JOINT REINFORCING SHALL BE DUR-O-WAL LADUR MEDIUM, 8 GAGE, TRUSS REINFORCEMENT OR EQUIVALENT PER ASTM 951, HOT DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A153. JOINT REINFORCING SHALL BE SPACED AT A MAXIMUM 16" O.C. VERTICAL, AT EVERY SECOND COURSE. PLACE REINFORCEMENT SUCH THAT LONGITUDINAL WIRES ARE EMBEDDED IN MORTAR WITH A MINIMUM COVER OF 5/8 INCHES WHEN EXPOSED TO EARTH AND WEATHER AND 1/2 INCHES WHEN NOT EXPOSED.

ALL UNITS SHALL BE LAID IN RUNNING BOND, UNLESS NOTED OTHERWISE. PLACE MASONRY WHILE MORTAR IS SOFT AND PLASTIC. REMOVE ANY UNIT DISTURBED TO THE EXTENT THAT THE INITIAL BOND IS BROKEN AFTER INITIAL POSITIONING AND RELAY IN FRESH MORTAR. NOTIFY THE ARCHITECT/ ENGINEER WHEN BEARING OF THE MASONRY WYTHE ON ITS SUPPORT IS LESS THAN 2/3 TIMES THE THICKNESS OF THE WYTHE.

CONSTRUCT CHASES FOR PIPES, CONDUITS, ETC, AS MASONRY UNITS ARE LAID. ALUMINUM CONDUITS, PIPES, AND ACCESSORIES IN MASONRY, GROUT, OR MORTAR ARE NOT PERMITTED UNLESS EFFECTIVELY COATED OR COVERED TO PREVENT REACTION BETWEEN ALUMINUM AND STEEL.

CONTRACTOR TO SUBMIT CONSTRUCTION PROCEDURES IF CONSTRUCTING DURING COLD OR HOT WEATHER (BELOW 40 DEGREES AND ABOVE 100 DEGREES FAHRENHEIT RESPECTIVELY).

PROVIDE VERTICAL CONTROL JOINTS IN WALLS AT A MAXIMUM SPACING OF 25'-0". REINFORCING IS CONTINUOUS THROUGH BOND BEAMS. ROUGHEN JOINT IN BOND BEAM AND APPLY SEALANT TO BOTH SIDES.

SECURE ALL CMU WALL SUPPORTED FIXTURES, EQUIPMENT, ETC. TO CMU WALL PER STRUCTURAL DRAWINGS AND MANUFACTURER'S RECOMMENDATIONS. DO NOT USE EXPANSION ANCHORS.

MASONRY NOTES SCALE: NTS ALL STRUCTURAL STEEL WORK SHALL CONFORM TO:

AISC AMERICAN INSTITUTE OF STEEL CONSTRUCTION, MANUAL OF STEEL CONSTRUCTION, NINTH EDITION AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES

STRUCTURAL STEEL MEMBERS SHALL BE IN CONFORMANCE WITH THE FOLLOWING:

ASTM A572, GRADE 50 ALL STEEL, UNO ANGLES, PLATES ASTM A36, Fy=36 KSI

STRUCTURAL TUBING ASTM A500, GRADE B, Fy=46 KSI ASTM A53, TYPE E OR S, GRADE B, Fy=35 KSI STEEL PIPE

SHOP DRAWINGS SUBMITTALS SHALL INCLUDE: CERTIFIED MILL TEST REPORTS OF STRUCTURAL STEEL (INCLUDING NAMES AND LOCATIONS OF MILLS AND SHOPS). CERTIFIED MILL TEST REPORTS OF BOLTS, NUTS AND WASHERS (INCLUDING NAMES AND LOCATIONS OF MILLS AND

SHOP DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW PRIOR TO COMMENCING FABRICATION.

STRUCTURAL STEEL FABRICATION AND ERECTION DRAWINGS WHICH INCLUDE BOLTED CONNECTIONS (SHOP AND FIELD) AND WELDED CONNECTIONS (SHOP AND FIELD) DEPICTING AWS WELDING SYMBOLS. METAL DECK SHOP DRAWINGS DEPICTING SHEAR STUD LAYOUT ON BEAMS AND GIRDERS.

OWNER SHALL RETAIN A QUALIFIED TESTING AGENCY TO PERFORM AND VERIFY THE FOLLOWING:

VISUAL INSPECTION OF ALL WELDS. ULTRASONIC TESTING, IN ACCORDANCE WITH ASTM E-164, ON 100% OF ALL FIELD FULL PENETRATION WELDS. PROVIDE RANDOM VERIFICATION VIA ULTRASONIC TESTING OF SHOP FULL PENETRATION WELDS.

FIELD BOLTED CONNECTIONS, INCLUDING VERIFICATION OF BOLT GRADES. SHEAR STUD QUANTITY, PROPER INSTALLATION, SIZE, AND SPACING. SHEAR STUDS SHALL CONFORM TO AWS D1.1.

BOLTED CONNECTIONS

FIELD CONNECTIONS SHALL UTILIZE MINIMUM 3/4-INCH DIAMETER A325 HIGH STRENGTH BOLTS, UNO. BOLTED CONNECTION SHALL BE SLIP CRITICAL (SC) AT ALL MOMENT FRAMES, BRACED FRAMES, AND AT ADDITIONAL LOCATIONS INDICATED IN THE DRAWINGS. SLIP CRITICAL CONNECTIONS SHALL UTILIZE LOAD INDICATOR WASHERS OR TENSION CONTROL BOLTS. BOLT HOLES SHALL BE STANDARD SIZE, UNO.

HIGH STRENGTH BOLTS SHALL BE INSTALLED AND TIGHTENED PER AISC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 BOLTS.

ANCHOR BOLTS SHALL CONFORM TO ASTM A307, GRADE A, STANDARD HEX HEAD FURNISHED WITH HEAVY HEX NUTS AND LOCK WASHERS.

CONTRACTOR SHALL DESIGN CONNECTIONS NOT ALREADY DETAILED ON STRUCTURAL DRAWINGS. DESIGN SHALL BE STAMPED BY A LICENSED STRUCTURAL ENGINEER AND SUBMITTED PRIOR TO COMMENCING FABRICATION.

WELDED CONNECTIONS WELDING SHALL CONFORM TO AWS D1.1. USE LOW-HYDROGEN SMAW ELECTRODES WITH MINIMUM TENSILE STRENGTH OF 70 KSI.

STRUCTURAL STEEL SHALL RECEIVE THE FOLLOWING PROTECTIVE COATINGS:

DO NOT PAINT SURFACES TO RECEIVE METAL DECK AND/ OR SHEAR CONNECTORS FASTENED BY WELDING, CONTACT SURFACES OF HIGH STRENGTH BOLTED CONNECTIONS, FINISHED BEARING SURFACES, AND SURFACES TO BE WELDED IN THE FIELD. IF REQUIRED, PROTECT THESE SURFACES BY RUST-INHIBITING COATING THAT CAN BE REMOVED EASILY PRIOR TO ERECTION

UNEXPOSED STRUCTURAL STEEL SHALL BE CLEANED IN ACCORDANCE WITH SSPC-SP3 AND PAINTED WITH PRIMER PAINT, TNEMEC 10-99, OR EQUIVALENT, UNO.

EXPOSED STRUCTURAL STEEL TO RECEIVE ZINC-RICH EPOXY PAINT SHALL BE FIRST CLEANED IN ACCORDANCE WITH SSPC-SP6 ,COMMERCIAL BLAST CLEANING. USE TNEMEC ZIN-RICH EPOXY PAINT, OR EQUIVALENT. APPLY FINISH COAT PER ARCHITECT.

EXPOSED STRUCTURAL STEEL TO BE HOT-DIPPED GALVANIZED SHALL BE IN ACCORDANCE WITH ASTM A123. SHEAR CONNECTOR STUDS

SHEAR CONNECTOR STUDS SHALL BE NELSON, OR EQUIVALENT, 3/4-INCH DIAMETER, UNO. WELD STUDS PER STUD MANUFACTURER'S RECOMMENDATIONS THROUGH METAL DECKING. STUD LENGTH SHALL BE 1-INCH BELOW

STANDARDS: WORK SHALL MEET THE REQUIREMENTS OF THE FOLLOWING STANDARDS:

AMERICAN SOCIETY FOR TESTING AND MATERIALS (A.S.T.M.)

AND BE OF TYPE AND SIZE AS SHOWN ON THE STRUCTURAL DRAWINGS.

STRUCTURAL ENGINEER. REPORT DISCREPANCIES TO STRUCTURAL ENGINEER.

MINIMUM REQUIREMENTS OF A.I.S.I. STANDARDS.

SHALL BE COMPATIBLE WITH LIGHT GAGE FRAMING.

WILL NOT BE ACCESSIBLE TO THE INSULATION CONTRACTOR.

LIGHT GAUGE STUD FRAMING

ALL WELDING SHALL BE TOUCHED UP WITH A ZINC RICH PAINT.

AMERICAN INSTITUTE OF STEEL CONSTRUCTION (A.I.S.C.)

TOP OF CONCRETE SLAB ON DECK.

ACTIVITIES.

2. GALVANIZED MATERIAL:

AGAINST ROTATION.

MEMBERS IS NOT PERMITTED, UNO.

STRUCTURAL STEEL NOTES

STRUCTURAL MEMBERS.

SHEAR STUDS, WHERE REQUIRED, ARE INDICATED ON THE DRAWINGS AS [XX], WHERE XX IS THE NUMBER OF STUDS EQUALLY SPACED BETWEEN SUPPORTS ON A BEAM OR GIRDER.

QUALITY ASSURANCE: CONTRACTOR SHALL PROVIDE QUALITY CONTROL OVER ALL FABRICATION AND ERECTION

AMERICAN IRON AND STEEL INSTITUTE (A.I.S.I.), LATEST EDITION OF "DESIGN OF COLD FORMED STEEL

AMERICAN WELDING SOCIETY (A.W.S.) D.1.3. LATEST EDITION OF "STRUCTURAL WELDING CODE- STEEL

PRODUCT HANDLING (PROTECTION OF PRODUCTS): MATERIAL SHALL BE PROTECTED FROM RAIN AND SNOW.

1. ALL FRAMING MATERIALS SHALL BE MANUFACTURED AND SUPPLIED BY DIETRICH INDUSTRIES (OR EQUIV)

A. ALL GALVANIZED STUDS AND JOISTS SHALL BE FORMED FROM STEEL THAT CORRESPONDS TO THE

B. ALL GALVANIZED STUDS, JOISTS, TRACKS, BRIDGING, AND ACCESSORIES SHALL BE FORMED FROM

3. THE PHYSICAL AND STRUCTURAL PROPERTIES LISTED BY DIETRICH INDUSTRIES SHALL BE CONSIDERED THE

POWER ACTUATED FASTENERS IN CONCRETE SHALL BE 0.145" DIAMETER HILTI X-DNI OR X-EDNI FOR STEEL,

UNO. PROVIDE MINIMUM 1-1/8" EMBEDMENT INTO CONCRETE. PROTECTIVE COATINGS ON SCREW FASTENERS

PERMANENT BRACING AND BRIDGING SHALL BE IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS

AND APPLICABLE CODES. BRIDGING SHALL BE SPACED PER MANUFACTURER'S RECOMMENDATIONS. END

BLOCKING SHALL BE PROVIDED FOR STEEL JOISTS WHERE JOIST ENDS ARE NOT OTHERWISE RESTRAINED

ALL TRACK BUTT JOINTS, ABUTTING PIECES OF TRACK SHALL BE SECURELY ANCHORED TO A COMMON

JACK STUDS SHALL BE INSTALLED BELOW WINDOW SILLS, ABOVE WINDOW AND DOOR HEADS TO FURNISH

STEEL TRUSSES SHALL BE LATERALLY SUPPORTED AT ALL PANEL POINTS AND AS SHOWN ON PLANS.

REQUIRED X-BRACING SHALL BE INSTALLED IMMEDIATELY AFTER ERECTION OF EACH ROOF TRUSS.

SUPPORT AND SHALL BE SECURELY ATTACHED TO SUPPORTING MEMBERS. PROVIDE INSULATION EQUAL TO

THAT SPECIFIED IN ARCHITECTURAL DRAWINGS IN ALL DOUBLE OR MORE JAMB STUDS OR HEADERS WHICH

STRUCTURAL ELEMENT, SUCH AS A STUD, OR THEY SHALL BE BUTT WELDED TOGETHER. SPLICING OF OTHER

STEEL HAVING A GALVANIZED COATING MEETING THE REQUIREMENTS OF A.S.T.M. A653.

MINIMUM PERMITTED FOR FRAMING MEMBERS. ANY SUBSTITUTIONS SHALL BE SUBMITTED TO THE

TEMPORARY BRACING SHALL BE PROVIDED BY AND BE THE RESPONSIBILITY OF THE CONTRACTOR.

SCALE: NTS

SLOPE DESIGNATION

ELEVATION MARK

ROOF PITCH

SPAN DIRECTION

LEGEND

SCALE: NTS

SLOPE

SECTION No. ———

DWG. WHERE SHOWN-

AB ADDL ARCH &	ANCHOR BOLT ADDITIONAL ARCHITECT AND	L LL LB LF LLH LLV	ANGLE DOUBLE ANGLE POUND LINEAR FOOT LONG LEGS HORIZONTAL LONG LEGS VERTICAL
BLDG BM BOT BRG	BOTTOM OF FOOTING BUILDING BEAM BOTTOM BEARING BETWEEN	MAX MECH MFR MIN MISC	MAXIMUM MECHANICAL MANUFACTURER MINIMUM MISCELLANEOUS
C CANT CIP CJ CL	STRUCTURAL STEEL CHANNEL CANTILEVER CAST-IN-PLACE CONCRETE CONTROL JOINT CENTERLINE	NF NO NS NTS	NEAR FACE NUMBER NEAR SIDE NOT TO SCALE
CLR CMU CNJ COL CONC	CLEAR CONCRETE MASONRY UNIT CONSTRUCTION JOINT COLUMN CONCRETE	OC OF OPNG OPP	ON CENTER OUTSIDE FACE OPENING OPPOSITE
CONN CONT CONTR CP CTR, CEN	CONNECTION CONTINUOUS CONTRACTOR COMPLETE PENETRATION WELD CENTER CUBIC YARD	P PL PP PREFAB PSF PSI	PIER DESIGNATION PLATE PARTIAL PENETRATION WELD PREFABRICATED POUNDS PER SQUARE FOOT POUNDS PER SQUARE INCH
DIM DISCONT	DIAMETER DIMENSION DISCONTINUOUS DRAWING	REINF REQ, REQD RD	REINFORCING STEEL REQUIRED ROOF DRAIN
EF EL, ELEV EQ EQUIP ES	EXISTING EACH EACH FACE ELEVATION EQUAL EQUIPMENT EACH SIDE EACH WAY EXPANSION EXTERIOR	SC SECT SHEATH SIM SOG SPAC SPECS SS STD STIFF STL	SLIP CRITICAL SECTION SHEATHING SIMILAR SLAB ON GRADE SPACING SPECIFICATIONS STAINLESS STEEL STANDARD STIFFENER STEEL
F FDN FF FLG FLR	FOOTING DESIGNATION FOUNDATION FINISH FLOOR FLANGE FLOOR	STR STRUCT T T&B	STRAIGHT STRUCTURAL TOP TOP AND BOTTOM
FT FTG FV GA GALV	FOOT FOOTING FIELD VERIFY GAGE GALVANIZED	TOC, T/CONC T/FTG, TOF TEMP T/SHELF T/SLAB	TOP OF CONCRETE TOP OF FOOTING TEMPERATURE TOP OF SHELF TOP OF SLAB
HOR, HORIZ HSS HT	HORIZONTAL HOLLOW STRUCTURAL SHAPE HEIGHT	T/STL T/WALL TS TYP	TOP OF STEEL TOP OF WALL STRUCTURAL TUBING TYPICAL
IF IN INFO	INSIDE FACE INCH INFORMATION	UNO VER, VERT	UNLESS NOTED OTHERWISE VERTICAL
JT K	JOINT KIP (1 KIP = 1000 LBS)	VIF	VERIFY IN FIELD
Ksi	KIPS PER SQUARE INCH	W W/O WP WT WWF	STRUCTURAL STEEL WIDE FLA WITH WITHOUT WORK POINT WEIGHT WELDED WIRE FABRIC

\_ANGE **ABBREVIATIONS** 

UNDISTURBED EARTH

COMPACTED STRUCTURAL FILL

LEDGE

CONCRETE

GROUT

BRICK

SCALE: NTS

SCALE: NTS

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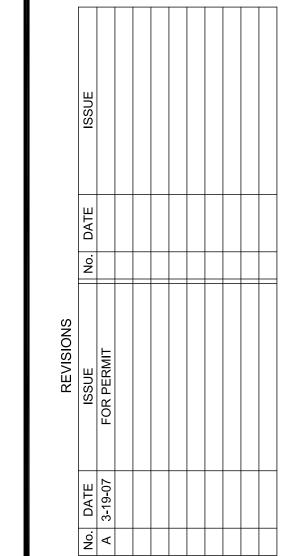
KENNEBUNKPORT, MAINE

CLIENT:

DIDONATO

134 GUINEA ROAD

ENGINEERING



SHEET TITLE: **NOTES** 

DESIGNED: SM DRAWN РM DATE: 2-7-07 CADD FILE: 7016-S1.DWG PROJECT NUMBER: 7016