Form # P 04

DISPLAY THIS CARD ON PRINCIPAL FRONTAGE OF WORK

CITY OF PORTLAND

BUILDING INSPECTION

PERMIT

Please Read Application And Notes, If Any, Attached

Permit Number: 030668

This is to certify that Transport Leasing Corp/M.H anborn, Inc.

has permission to addition of 7,500 sq. Ft. One ory Met n Lot 3

AT 9 Johnson Rd 214A A001001

provided that the person or persons rm or on a epting this permit shall comply with all of the provisions of the Statutes of ine and of the O ances of the City of Portland regulating the construction, maintenance and e of buildings uctures, and of the application on file in this department.

Apply to Public Works for street line and grade if nature of work requires such information.

ification Inspe on mus e n and w en perm on proc d re this lding or t there s ed or erwis sed-in 4 UR NO QUIRED.

A certificate of occupancy must be procured by owner before this building or part thereof is occupied.

OTHER REQUIRED APPROVALS

Fire Dept. OK mc

Health Dept. _____

Appeal Board _____

Other _____

Department Name

7/15/03

Director - Building & Inspection Services

PENALTY FOR REMOVING THIS CARD

City of Portland, Maine - Building or Use Permit Application

389 Congress Street, 04101 Tel: (207) 874-8703, Fax: (207) 874-8716

Permit No:	Issue Date:	CBL:
03-0668		214A A001001

Location of Construction:	Owner Name:	Owner Address:	Phone:
9 Johnson Rd	Transport Leasing Corp	9 Johnson St B7	775-6045
Business Name:	**Contractor Name:**	**Contractor Address:**	**Phone**
	M.H. Sanborn, Inc.	807 Turnpike Street Andover	
Lessee/Buyer's Name	**Phone:**	**Permit Type:**	**Zone:**
		Additions - Commercial	B4

Past Use:	Proposed Use:	Permit Fee:	Cost of Work:	CEO District:
Parking	Office Space	$3,173.00	$450,000.00	3

FIRE DEPT: ☑ Approved ☐ Denied

Signature:

INSPECTION: Use Group: B Type: 2B

7/15/03

Signature:

Proposed Project Description:
addition of 7,500 sq. Ft. One Story Metal Building on Lot 3

PEDESTRIAN ACTIVITIES DISTRICT (P.A.D.)

Action: ☐ Approved ☐ Approved w/Conditions ☐ Denied

Signature: Date:

Permit Taken By:	Date Applied For:
gad	06/06/2003

1. This permit application does not preclude the Applicant(s) from meeting applicable State and Federal Rules.
2. Building permits do not include plumbing, septic or electrical work.
3. Building permits are void if work is not started within six (6) months of the date of issuance. False information may invalidate a building permit and stop all work..

Zoning Approval

Special Zone or Reviews	Zoning Appeal	Historic Preservation
☐ Shoreland N/A	☐ Variance	☑ Not in District or Landmark
☐ Wetland Separate permit required for any new signage	☐ Miscellaneous	☐ Does Not Require Review
☐ Flood Zone	☐ Conditional Use	☐ Requires Review
☐ Subdivision	☐ Interpretation	☐ Approved
☒ Site Plan ± 2003-0087	☐ Approved	☐ Approved w/Conditions
Maj ☐ Minor ☐ MM ☐	☐ Denied	☐ Denied
Date: OK with conditions 6/17/03	Date:	Date:

CERTIFICATION

I hereby certify that I am the owner of record of the named property, or that the proposed work is authorized by the owner of record and that I have been authorized by the owner to make this application as his authorized agent and I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in the application is issued, I certify that the code official's authorized representative shall have the authority to enter all areas covered by such permit at any reasonable hour to enforce the provision of the code(s) applicable to such permit.

SIGNATURE OF APPLICANT | ADDRESS | DATE | PHONE

RESPONSIBLE PERSON IN CHARGE OF WORK, TITLE | DATE | PHONE

William E. Whited, Inc.
Architecture/Engineering/Interiors

William E. Whited, President

July 15, 2003

Michael J. Nugent, Inspection Service Mgr.
Department of Planning & Development
City of Portland
389 Congress Street
Portland, ME 04101

VIA FAX: 207-874-8716

RE: Transport Leasing Corp/Time Warner
Addition to Building at 9 Johnson Road

Dear Mr. Nugent:

The existing wall separating the proposed addition from the existing building is a 12 inch thick concrete masonry wall, extending above the proposed addition by 42 inches. This wall meets the requirements of a two-hour fire resistance Design No. U905.

Doors through this wall connecting the addition to the existing building will be rated 90 minute fire doors and frames.

Sincerely,

William E. Whited
P.E., R.A.

cc: Dwight Sanborn, Transport Leasing Corp.
Chris Dahlgren, Dahlgren Const.

1321 Washington Avenue, Portland, ME 04103
(207) 878-4530 Fax (207) 878-4533

City of Portland, Maine - Building or Use Permit
389 Congress Street, 04101 Tel: (207) 874-8703, Fax: (207) 874-8716

Permit No:	Date Applied For:	CBL:
03-0668	06/06/2003	214A A001001

Location of Construction:	Owner Name:	Owner Address:	Phone:
9 Johnson Rd	Transport Leasing Corp	9 Johnson St B7	( ) 775-6045
Business Name:	**Contractor Name:**	**Contractor Address:**	**Phone**
	M.H. Sanborn, Inc.	807 Turnpike Street Andover	
Lessee/Buyer's Name	**Phone:**	**Permit Type:**	
		Additions - Commercial	

Proposed Use:	Proposed Project Description:
Office Space	addition of 7,500 sq. Ft. One Story Metal Building on Lot 3

Dept: Zoning **Status:** Approved with Conditions **Reviewer:** Marge Schmuckal **Approval Date:** 06/17/2003
Note: **Ok to Issue:** ☑

1) Separate permits shall be required for any new signage.
2) This permit is being approved on the basis of plans submitted. Any deviations shall require a separate approval before starting that work.

Dept: Building **Status:** Approved with Conditions **Reviewer:** Mike Nugent **Approval Date:** 07/15/2003
Note: **Ok to Issue:** ☑

1) Special inspection required for fabricated steel from the engineer for the fabricator, a report w/satisfactory results must be received prior to the receipt and erection of the steel. William Whited notified and agreed this date.

Dept: Fire **Status:** Approved with Conditions **Reviewer:** Lt. MacDougal **Approval Date:** 06/19/2003
Note: **Ok to Issue:** ☑

1) fire extinquishers shall be installed in accordance with NFPA 10 standards
2) the fire alarm system shall be maintained to NFPA 72 standards
3) the sprinkler system shall be maintained to NFPA 13 standards

Comments:
06/26/2003-mjn: Need engineering docs, geo technical, Special Inspection info & Certifications

William E. Whited, Inc.
Architecture/Engineering/Interiors

William E. Whited, President

July 11, 2003

Michael J. Nugent, Inspection Service Mgr.
Department of Planning & Development
City of Portland
389 Congress Street
Portland, ME 04101

VIA FAX: 207-874-8716

RE: Transport Leasing Corp/Time Warner
Addition to Building at 9 Johnson Road

Dear Mr. Nugent:

S. W. Cole Engineering will be performing the special inspections for soil compaction, concreting, and structural steel erection.

Butler Manufacturing will be shop fabricating the structural steel off-site and is being required to provide the special inspections of this fabrication. The professional engineer who prepared the design shall verify that the fabrication is in accordance with the design.

Sincerely,

William E. Whited
P.E., R.A.

cc: Dwight Sanborn, Transport Leasing Corp.
Chris Dahlgren, Dahlgren Const.
Butler Manufacturing, Inc.
S. W. Cole Engineering

1321 Washington Avenue, Portland, ME 04103
(207) 878-4530 Fax (207) 878-4533

Post-it® Fax Note 7671	Date 6-27-03	# of pages ▶ 1
To Mike Nugent	From Chris Dahlgren	
Co./Dept. Bldg. Inspection	Co. Dahlgren Const.	
Phone # 874 8700	Phone # 846 3505	
Fax # 874 8716	Fax # 846 4481	

CITY OF PORTLAND MAINE

389 Congress St., Rm 315
Portland, ME 04101
Tel. – 207-874-8704
Fax – 207-874-8716

TO: Inspector of Buildings City of Portland, Maine
Planning & Urban Development
Division of Housing & Community Services

FROM DESIGNER: Butler Manufacturing Company
400 N. Lexington St. Annville, PA 17003

DATE: 6/27/03

Job Name: Time Warner Lot 3 Addition

Address of Construction: Jet Port Business Park, 9 Johnson Road Portland, ME

THE BOCA NATIONAL BUILDING CODE/1999 Fourteenth EDITION

Construction project was designed according to the building code criteria listed below:

Building Code and Year: NBC (BOCA) 1999 Use Group Classification(s): B

Type of Construction: 2B Bldg. Height: 14' Eave Bldg. Sq. Footage: 7,480 sf

Seismic Zone: Av=Aa=.10g Group Class: C

Roof Snow Load Per Sq. Ft.: 49 psf Dead Load Per Sq. Ft.: 27psf + Frame Weight

Basic Wind Speed (mph): 90 mph Effective Velocity Pressure Per Sq. Ft.: 20.7 psf

Floor Live Load Per Sq. Ft.: PER FOUNDATION DRAWINGS / Conc. Slab on Grade, 3000 PSI, 5" Thickness

Structure has full sprinkler system? Yes ____ No X Alarm System? Yes X No ____
Sprinkler & Alarm systems must be installed according to BOCA and NFPA Standards with approval from the Portland Fire Department.

Is structure being considered unlimited area building: Yes ____ No X

If unlimited use, what subsection of 312 is being considered: No

List Occupant loading for each room or space, designed into this Project. PER FOUNDATION PLANS

(Designers Stamp & Signature)

PRINT DATE:

Department of Planning & Development
Lee D. Urban, Director

CITY OF PORTLAND

Division Directors
Mark B. Adelson
Housing & Neighborhood Services

Alexander Q. Jaegerman, AICP
Planning

John N. Lufkin
Economic Development

July 10, 2003

William Whited
1321 Washington Ave.
Portland, ME 04103

RE: Time Warner Addition, 9 Johnson Road
CBL: 214A A001001

Dear Mr. Whited:

On June 27, 2003, the Portland Planning Authority granted minor site plan for the building expansion and parking reconfiguration at 9 Johnson Road, as shown on the approved plan.

The approval is based on the submitted site plan. If you need to make any modifications to the approved site plan, you must submit a revised site plan for staff review and approval.

Please note the following provisions and requirements for all site plan approvals:
site plan, you must submit a revised site plan for staff review and approval.

Please note the following provisions and requirements for all site plan approvals:

1. The site plan approval will be deemed to have expired unless work in the development has commenced within one (1) year of the approval or within a time period agreed upon in writing by the City and the applicant. A one year extension may be granted by this department if requested by the applicant in writing prior to the expiration date of the site plan.

2. A performance guarantee in a form acceptable to the City of Portland and an inspection fee equal to 2.0% of the performance guarantee will have to be posted before beginning any site construction or issuance of a building permit.

3. A defect guarantee, consisting of 10% of the performance guarantee, must be posted before the performance guarantee will be released.

4. Prior to construction, a pre-construction meeting shall be held at the project site with the contractor, development review coordinator, Public Work's representative and owner to review the construction schedule and critical aspects of the site work. At that time, the site/building contractor shall provide three (3) copies of a detailed construction schedule to the attending City representatives. It shall be the contractor's responsibility to arrange a mutually agreeable time for the pre-construction meeting.

389 Congress Street • Portland, Maine 04101 • (207) 874-8721 • FAX 756-8258 • TTY 874-8936

William E. Whited, Inc.
Architecture/Engineering/Interiors

William E. Whited, President

May 5, 2003

City of Portland
Department of Planning & Development
ATTN: Sara Hopkins
389 Congress Street
Portland, ME 04101

RE: Time Warner Addition, Jetport Business Park

Dear Sara:

This addition to the building on Lot #3 in the Jetport Business Park in Portland and South Portland, will provide office space and studio space for Time Warner Cable of Maine. This addition is a 7,500 sq. ft. one floor building. Exterior facade will be similar to the Time Warner building on Lot 1B across the street.

Forty-two parking spaces are planned for this building and the existing building. The existing building is used for storage and service call vans. It is separated from the proposed addition by a one-hour fire wall. Opening between the buildings will be through one-hour rated doors.

Site plan checklist comments are included in the attached Site Plan Application Booklet.

Sincerely,

William E. Whited
P.E., R.A.

1321 Washington Avenue, Portland, ME 04103
(207) 878-4530 Fax (207) 878-4533

N.A. — Not applicable

ADMINISTRATION (Chapter 1)

✓ Complete construction documents *(107.5, 107.6, 107.7)*

✓ Signed/sealed construction documents *(107.7, 114.1)*

BUILDING PLANNING (Chapters 3, 4, 5, 6)

USE OR OCCUPANCY CLASSIFICATION (302.0-313.0)

✓ Single Use Group

____ Mixed Use Groups

____ Specific occupancy areas *(302.1.1)*

____ Accessory areas *(302.1.2)*

GENERAL BUILDING LIMITATIONS (Chapters 5 & 6)

Apply Case 1 to determine the allowable height and area and permitted types of construction for a building containing a single use group or nonseparated mixed use groups. Apply Case 2 to determine the allowable height and area and permitted types of construction for a building containing separated mixed use groups.

AREA MODIFICATIONS TO TABLE 503

% of Allowable tabular area *(Table 503)*	100%
% Reduction for height *(Table 506.4)*	- 0 %
% Increase for open perimeter *(506.2)*	+ 94 %
% Increase for automatic sprinklers *(506.3)*	+ 0 %
Total percentage factor	= 194 %

Conversion factor 194/100 = 1.94
(Total percentage factor/100%)

Open perimeter *(506.2)*: 128 (North) / N/A (East) / 238 (South) / 132 (West)

Open perim. 498 ft. Perimeter 688 ft.

% Open perimeter = .72 x 100% = 72%
(Open perim./perim.) x 100%

% Tab. area increase *(506.2)* = 2 x 47 = 94%
2x(% Open perim. -25%)

CASE 1 — SINGLE USE OR NONSEPARATED MIXED USE GROUPS (313.1.1, 503.0)

Using Table 503, identify the allowable height and area of the single use group or the most restrictive of the nonseparated mixed use groups. Construction types that provide an allowable tabular area equal to or greater than the adjusted floor area and allowable heights (as modified by Section 504.0) equal to or greater than the actual building height are permitted.

PROPOSED Actual floor area 20,200 ft.²

ALLOWABLE Adjusted floor area* 27,936 ft.²

Actual building height 18 feet 1 stories

Allowable building height 40 feet ~~4~~ 3 stories

*Adjusted floor area = actual floor area/conversion factor

Permitted types of construction 1, 2, 3

Type of construction assumed for review (602.3) 2C

ATRIUMS

- Automatic sprinkler system *(404.2)*
- Occupancy *(404.3)*
- Smoke control *(404.4)*
- Enclosure *(404.5)*
- Fire alarm system *(404.6)*
- Travel distance *(404.7)*

OTHER SPECIAL USE AND OCCUPANCY

- Underground structures *(405.0)*
- Open parking structures *(406.0)*
- Private garages *(407.0)*
- Public garages *(408.0)*
- Use Group I-2 *(409.0)*
- Use Group I-3 *(410.0)*
- Stages and platforms *(412.0)*
- Special amusement buildings *(413.0)*
- HPM facilities *(416.0)*
- Hazardous materials *(307.8, 417.0)*
- Use Groups H-1, H-2, H-3 and H-4 *(418.0)*
- Swimming pools *(421.0)*

FIRE PROTECTION (Chapters 6, 7, 8, 9)

FIRERESISTANT MATERIALS AND CONSTRUCTION (Chapter 7 and Table 602)

Note: Entry in ☐ indicates required rating in hours. NC indicates noncombustible construction required.

COMBUSTIBILITY *(603.0, 604.0, 605.0, 606.0)*

- O Exterior walls
- O Interior elements
- O Roof

CONSTRUCTION DOCUMENTS *(703.0)*

- Fire tests *(704.0)*

EXTERIOR WALLS *(507.2, 705.0, 716.5)*

	North	East	South	West
Fire separation distance	ALL IN EXCESS OF 30			
Loadbearing	☐ N/A	☐	☐	☐
Nonloadbearing	☐	☐	☐	☐

- ☐ N/A Exterior opening protectives *(705.3, 706.0)*
- ☐ N/A Parapet walls *(705.6)*

FIRE SEPARATION ASSEMBLIES

- ☐ NONE Exit enclosures *(709.0, 710.0, 1014.11)*
- ☐ N/A Other shafts *(709.0, 710.0)*
- ☐ N/A Mixed use and fire area separations *(313.1.2)*
- ☐ N/A Other separation assemblies *(302.1.1, Table 602)*

FIRE PARTITIONS

- ☐ N/A Exit access corridors *(711.0,1011.4)*
- ☐ N/A Tenant separations *(711.0)*
- ☐ N/A Dwelling unit separations *(711.0)*
- ☐ Guestroom separations *(711.0)*

OTHER FIRERESISTANT CONSTRUCTION

- ☐ N/A Fire and party walls *(707.0 and Table 707.1)*
- ☐ N/A Smoke barriers *(712.0)*
- ☐ N/A Nonloadbearing partitions *(Table 602)*
- ☐ Interior loadbearing walls, columns, girders, trusses *(716.0)*
- ☐ Supporting construction *(716.0)*
- ☐ Floor construction *(713.0, 1006.3.1)*
- ☐ Roof construction *(713.0, 715.0)*
- Penetrations *(714.0)*
- Opening protectives *(717.0, 719.0, 720.0)*
- Fire dampers *(718.0)*
- Fireblocking/draftstopping *(721.0)*
- Thermal and sound-insulating materials *(723.0)*

1.

03-0668

All Purpose Building Permit Application

If you or the property owner owes real estate or personal property taxes or user charges on any property within the City, payment arrangements must be made before permits of any kind are accepted.

Location/Address of Construction: 9 Johnson Road, Jetport Business Park		
Total Square Footage of Proposed Structure: 7500 SF new plans 12,700 existing	Square Footage of Lot: 73180 SF	
Tax Assessor's Chart, Block & Lot — Chart# 214A, Block# A, Lot# 1	Owner: Transport Leasing Corp. 9 Johnson Rd. Portland, Me 04102	Telephone: ~~775~~ 775-6045
Lessee/Buyer's Name (If Applicable): Time Warner Cable of Maine	Applicant name, address & telephone: Transport Leasing Corp. 9 Johnson Rd. Portland, Me 04102 775-6045	Cost Of Work: $450,000.00 Fee: $ 3173.00

no Cof O paid for

Current use: Paved Parking

If the location is currently vacant, what was prior use: _____

Approximately how long has it been vacant: _____

Proposed use: Office space

Project description:
One story metal building addition to existing masonry structure.

Contractor's name, address & telephone: M.H. Sanborn, Inc. 807 Turnpike St. North Andover, Ma. 01845

Who should we contact when the permit is ready: Penney Booker @ Transport Leasing

Mailing address: Transport Leasing Corp 9 Johnson Rd. Portland, Me. 04102

We will contact you by phone when the permit is ready. You must come in and pick up the permit and review the requirements before starting any work, with a Plan Reviewer. A stop work order will be issued and a $100.00 fee if any work starts before the permit is picked up. **PHONE:** 775-6045

note - dng. cert. for building on the works

IF THE REQUIRED INFORMATION IS NOT INCLUDED IN THE SUBMISSIONS THE PERMIT WILL BE AUTOMATICALLY DENIED AT THE DISCRETION OF THE BUILDING/PLANNING DEPARTMENT, WE MAY REQUIRE ADDITIONAL INFORMATION IN ORDER TO APROVE THIS PERMIT.

DEPT. OF BUILDING INSPECTION CITY OF PORTLAND

JUN 6 2003

RECEIVED

I hereby certify that I am the Owner of record of the named property, or that the owner of record authorizes the proposed work and that I have been authorized by the owner to make this application as his/her authorized agent. I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in this application is issued, I certify that the Code Official's authorized representative shall have the authority to enter all areas covered by this permit at any reasonable hour to enforce the provisions of the codes applicable to this permit.

Signature of applicant:	Date: 6-5-03

This is NOT a permit, you may not commence ANY work until the permit is issued.
If you are in a Historic District you may be subject to additional permitting and fees with the Planning Department on the 4th floor of City Hall

City of Portland, Maine - Building or Use Permit
389 Congress Street, 04101 Tel: (207) 874-8703, Fax: (207) 874-8716

Permit No:	Date Applied For:	CBL:
03-0668	06/06/2003	214A A001001

Location of Construction:	Owner Name:	Owner Address:	Phone:
9 Johnson Rd	Transport Leasing Corp	9 Johnson St B7	( ) 775-6045
Business Name:	**Contractor Name:**	**Contractor Address:**	**Phone**
	M.H. Sanborn, Inc.	807 Turnpike Street Andover	
Lessee/Buyer's Name	**Phone:**	**Permit Type:**	
		Additions - Commercial	

Proposed Use:	Proposed Project Description:
Office Space	addition of 7,500 sq. Ft. One Story Metal Building on Lot 3

Dept: Zoning **Status:** Approved with Conditions **Reviewer:** Marge Schmuckal **Approval Date:** 06/17/2003
Note: **Ok to Issue:** ☑

1) Separate permits shall be required for any new signage.
2) This permit is being approved on the basis of plans submitted. Any deviations shall require a separate approval before starting that work.

Dept: Building **Status:** Pending **Reviewer:** Mike Nugent **Approval Date:**
Note: **Ok to Issue:** ☐

Dept: Fire **Status:** Approved with Conditions **Reviewer:** Lt. MacDougal **Approval Date:** 06/19/2003
Note: **Ok to Issue:** ☑

1) fire extinquishers shall be installed in accordance with NFPA 10 standards
2) the fire alarm system shall be maintained to NFPA 72 standards
3) the sprinkler system shall be maintained to NFPA 13 standards

Comments:
06/26/2003-mjn: Need engineering docs, geo technical, Special Inspection info & Certifications

CITY OF PORTLAND, MAINE
DEVELOPMENT REVIEW APPLICATION
PLANNING DEPARTMENT PROCESSING FORM
Planning Copy

2003-0087
Application I. D. Number

Transport Leasing Corp
Applicant

05/05/2003
Application Date

9 Johnson St B7, Portland, ME 04102
Applicant's Mailing Address

Time Warner Addition
Project Name/Description

Consultant/Agent

9 - 9 Johnson Rd, Portland, Maine
Address of Proposed Site

Agent Ph: **Agent Fax:**
Applicant or Agent Daytime Telephone, Fax

214A A001001
Assessor's Reference: Chart-Block-Lot

Proposed Development (check all that apply): ☐ New Building ☑ Building Addition ☑ Change Of Use ☐ Residential ☐ Office ☐ Retail ☐ Manufacturing ☐ Warehouse/Distribution ☐ Parking Lot ☐ Other (specify)

7,500 s.f.
Proposed Building square Feet or # of Units

Acreage of Site

B4
Zoning

Check Review Required:

☑ Site Plan (major/minor)
☐ Subdivision # of lots
☐ PAD Review
☐ 14-403 Streets Review
☐ Flood Hazard
☐ Shoreland
☐ HistoricPreservation
☐ DEP Local Certification
☐ Zoning Conditional Use (ZBA/PB)
☐ Zoning Variance
☐ Other

Fees Paid: Site Plan **$400.00** Subdivision Engineer Review **$946.66** Date **07/09/2003**

[redacted] Approval Status: Reviewer **Sarah Hopkins**

☐ **Approved** [redacted] ☐ **Denied**
See Attached

Approval Date Approval Expiration Extension to ☐ Additional Sheets Attached

[redacted] signature [redacted] date

Performance Guarantee ☑ **Required*** ☐ **Not Required**

* No building permit may be issued until a performance guarantee has been submitted as indicated below

☑ Performance Guarantee Accepted	**07/08/2003** date	**$47,333.00** amount	**09/01/2004** expiration date
☑ Inspection Fee Paid	**07/07/2003** date	**$946.66** amount	
☐ Building Permit Issue	date		
☐ Performance Guarantee Reduced	date	remaining balance	signature
☐ Temporary Certificate of Occupancy	date	☐ Conditions (See Attached)	expiration date
☐ Final Inspection	date	signature	
☐ Certificate Of Occupancy	date		
☐ Performance Guarantee Released	date	signature	
☐ Defect Guarantee Submitted	submitted date	amount	expiration date
☐ Defect Guarantee Released	date	signature	

CITY OF PORTLAND
ACCESSIBILITY CERTIFICATE

Designer: William E. Whited, Inc.

Address of Project Jetport Business Park, Johnson Road

Nature of Project Addition to Time Warner

Date 06-06-03

The technical submissions covering the proposed construction work as described above have been have been designed in compliance with applicable referenced standards found in the Maine Human Rights Law and Federal Americans with Disability Act.

(SEAL)

STATE OF MAINE
WILLIAM E. WHITED 1283 REGISTERED
PROFESSIONAL ENGINEER

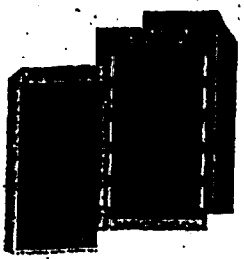
Signature William Whited

Title P.E., R.A.

Firm William E. Whited, Inc.

Address 1321 Washington Ave.,
Portland, ME 04103

Telephone 207-878-4530



CITY OF PORTLAND
BUILDING CODE CERTIFICATE
389 Congress St., Rm 315
Portland, ME 04101

TO: Inspector of Buildings City of Portland, Maine
Department of Planning & Urban Development
Division of Housing & Community Service

FROM: William E. Whited, Inc.

RE: Certificate of Design

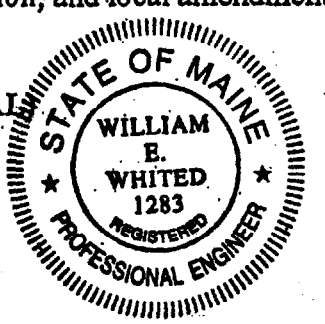
DATE: 06-06-03

These plans and/or specifications covering construction work on:

Addition to Time Warner, Jetport Business Park, Portland, ME 04102

Have been designed and drawn up by the undersigned, a Maine registered architect/engineer according to the *BOCA National Building Code/1999 Fourteenth* Edition, and local amendments.

(SEAL)



Signature William E. Whited

Title P.E., R.A.

Firm William E. Whited, Inc.

Address 1321 Washington Ave., Portland, ME 04103

As per Maine State Law:

$50,000.00 or more in new construction, repair, expansion, addition, or modification for Building or Structures, shall be prepared by a registered design Professional.

PSH 6/20/2k

From: Marge Schmuckal
To: Sarah Hopkins
Date: Wed, May 28, 2003 12:08 PM
Subject: 9 Johnson Rd - Time Warner addition & ch of use

Sarah,
I have reviewed the most recent submittal that I received from you today at the site plan review meeting.

It is meeting all the requiremtents of the B-4 zone in which it is located. This includes setbacks, parking, F.A.R., and impervious surface maximums.

Marge Schmuckal
Zoning Administrator
5/28/03

FAX MEMO

William E. Whited, Inc.
Architecture/Engineering/Interiors
1321 Washington Ave.
Portland, ME 04103

Tel: (207) 878-4530
Fax: (207) 878-4533

TO: Michael J. Nugent, City of Portland
FAX - 207-874-8716

FROM: William Whited
FAX - 207-878-4533

DATE: 06-27-03

RE: Addition to Time Warner
Jetport Business Park, Johnson Road

PAGES: 11

As you requested, attached is info from S.W. Cole regarding Geotechnical Investigation.

The allowable soil bearing capacity is 4,000 lbs. per sq. ft. per the S. W. Cole report.

The maximum footing bearing pressure as designed is 3,200 lbs. per sq. ft.

S. W. COLE
ENGINEERING, INC.
GEOTECHNICAL CONSULTANT

SIX LIBERTY DRIVE
BANGOR, MAINE 04401
TEL. 848-5714

87-278 S

November 16, 1987

Transport Leasing Corp.
c/o Dearborn/Whited Architects-Engineers
Attn: Mr. William Whited
P.O. Box 127
Portland, Maine 04112

Subject: Geotechnical Investigation
Proposed Johnson Road Subdivision
Portland/South Portland, Maine

Gentlemen:

1.0 INTRODUCTION:

1.1 Scope of Work - In accordance with our Agreement dated September 28, 1987, we have made a subsurface investigation for the proposed industrial subdivision. The investigation has included the making of backhoe-dug test pit explorations, laboratory testing of recovered samples and a geotechnical evaluation of the findings as they relate to the proposed project.

1.2 Proposed Construction - We understand that the proposed project consists of the development of the undeveloped portions of a 13+/- acre parcel of land for use as an industrial subdivision.

2.0 EXPLORATION AND TESTING:

2.1 Exploration - Six backhoe-dug test pit explorations were made at the site on October 27, 1987 by a locally hired contractor under the direction of S. W. COLE ENGINEERING, INC. personnel. Exploration locations were selected and located in the field by S. W. COLE ENGINEERING, INC. personnel using a site plan provided by Dearborn/Whited Architects-Engineers and cloth tape measurements from existing site features. The approximate locations of the explorations are shown on the "Exploration Location Plan and General Soils Map", attached as Sheet 1. Logs of the explorations, based on observations made in the field and modified after laboratory testing, are attached as Sheets 2 through 4. Sheet 5 is a key defining the symbols used on the log sheets.

2.2 Laboratory Testing - Laboratory testing was performed on selected samples recovered from the test pits during exploration work. Moisture content and Atterberg Limit results have been noted on the individual log

sheets. The result of one grain size analysis is presented graphically on Sheet 6.

3.0 SITE AND SUBSURFACE CONDITIONS:

3.1 Site Location and Surficial Conditions - The site of the proposed subdivision is a parcel of land located in the northeasterly quadrant of the intersection of Johnson Road and the Portland International Jetport Access Road. The parcel is approximately 13 acres and triangular in shape. The site is bordered to the northeast by Brooklawn Memorial Park, to the south by the Jetport Access Road and to the northwest by Johnson Road.

The ground surface at the site is relatively flat although a small knoll, a few low ares, swales and ditches were observed. At present, a majority of the site is occupied by building structures or paved areas with the remaining portion being fill areas, swales or open grass field.

3.2 Subsurface Conditions - Published surficial soils mapping indicates that the soils at the site are glacial marine deposits consisting of silt, clay and sand. The soils encountered during the test pit exploration program generally consisted of a mottled stiff grayish-brown silty clay. The test pit explorations ranged in depth from about 11 to 13 feet below the existing ground surface. Three of the Test Pits (1, 4 and 5) encountered gravelly fill overlying silty clay. The fill appeared to be about 2 to 2.5 feet thick. The remaining three Test Pits (2, 3 and 6) encountered up to about 14 inches of topsoil and organics overlying the silty clay. Test Pit 2 encountered a refusal surface (presumed to be bedrock) at about 11.5 feet below the existing ground surface. Test Pit 2 encountered layers of brown sand, gray sand and gray silty clay at about 10 feet overlying till and bedrock at about 11.5 feet. Test Pits 1, 4, 5 and 6 were terminated in the silty clay soil at depths ranging from 11 to 13 feet below the ground surface. Test Pit 3, made near the knoll area encountered light brown sand with silt and gravel throughout its depth to the bottom of the exploration (11 +/- feet). See the log sheets for more detailed soils descriptions.

3.3 Groundwater - Water was observed ponded at the surface in many of the low swale areas at the site. Test Pits 2 and 5 were observed to have small amounts of water seeping from the exploration sidewalls. No free water was observed in the other explorations, however, the soils did appear saturated at or near the ground surface. Based on observations made during the exploration program, groundwater at the site is likely at or near the ground surface throughout much of the year in the low areas and is at the surface during periods of heavy precipitation and snowmelt.

S. W. COLE ENGINEERING, INC.
GEOTECHNICAL CONSULTANT
87-278 S
November 16, 1987

4.0 EVALUATION AND RECOMMENDATIONS:

4.1 Site Suitability - Based on the findings at the exploration locations and visual observations made of the site, it appears that the site is suitable for the proposed development project. The preliminary data developed in this investigation indicates that the native stiff silty clay or sandy soils are suitable for support of one to two story structures with moderate to heavy intensity floor loadings utilizing conventional spread footings and slab-on-grade type construction. The fill soils encountered at the site are not suitable for this purpose. Some site limitations include: 1) groundwater at or near the surface in the low areas and 2) required fill for proposed construction over low areas.

4.2 Recommendations - We offer the following general comments and recommendations for your consideration during the planning and preliminary design phases of your project:

1. Site Preparation - Site preparation should include the removal of all topsoil and organics from areas of construction. We also recommend that swales, ditches and other drainage structures be constructed to promote water movement away from paved areas and building structures.

2. Seasonal Frost Depth - The design freezing index for the Portland, Maine area is approximately 1250 degree days. Therefore, we recommend that exterior footings be placed at least 4.5 feet below exterior finished grade to provide frost protection. Interior footings may be placed at a shallower depth provided that the interior is heated during winter months.

3. Foundations - Footings should be placed on undisturbed native stiff brown silty clay, brown sand or on a clean compacted granular fill placed upon the native soils. Footings should be designed for a net bearing contact pressure of 4 ksf or less, unless further design specific testing and evaluation indicates a higher allowable bearing pressure.

4. Control Joints - The native soils are relatively incompressible. However, we recommend that design incorporate control joints in foundation walls and floor slabs to accommodate minor post-construction movements. Control joints in foundation walls and masonry block walls should coincide (where applicable).

5. Excavation - It should be anticipated that groundwater will be encountered during excavation work. However, the clayey soils at the site are poorly drained, therefore, quantities should be small. Minor

caving and sloughing should be anticipated in the granular soils in the high area near Test Pit 3.

6. Foundation Drainage - We recommend that foundation drainage be provided for proposed building structures. Exterior underdrains should be placed near footing grade and be provided with a positive gravity outlet. In addition, weepholes should be provided through foundation walls near footing grade to allow drainage from beneath the slabs during periods of seasonal high water.

7. Backfill Requirements - The silty clay soils at the site are poorly drained and medium to highly frost susceptible. Therefore, the existing soils are not suitable for foundation backfill or sub-slab fill.

Because of the low permeability of the silty soils, some ponding of water may occur at or near the ground surface following periods of heavy precipitation and/or snowmelt. Therefore, site grading should be planned to promote surface drainage away from areas of construction.

8. Entrances - Entrance approaches should be designed to reduce the effects of differential frost action between doorways and entrances. We recommend that 3 feet of clean granular soil meeting the backfill gradation specification be provided below entrance slabs. Sub-slab fill should be placed in lifts and compacted to at least 92 percent of its maximum dry density as determined by ASTM D-1557. Drainage should be provided below entrance slabs.

9. Summary - In summary, the site appears to be well suited for the proposed development from a soils standpoint. However, it should be noted that the scope of this investigation has been limited to the undeveloped areas of the site and to the development of general soils information for these undeveloped areas. We have reviewed the soils relative to hydrologic classification. We recommend that Hydrologic Soil Group C be considered for this site.

5.0 CLOSURE:

This report has been prepared for the exclusive use of B & D Realty for specific application to the proposed Johnson Road Subdivision, Portland/South Portland, Maine in accordance with generally accepted soil and foundation engineering practices. No other warranty, expressed or implied, is made. The conclusions and recommendations presented in this report are based upon the data obtained from the explorations performed at the locations indicated on Sheet 1. This report does not reflect undetected variations which may occur between these explorations. This report is general in that it does not address specific structures. We

S. W. COLE ENGINEERING, INC.
GEOTECHNICAL CONSULTANT
87-278 S
November 16, 1987

recommend that a general review of final design and specifications be made by us in order that the earthwork and foundation recommendations may be properly interpreted and implemented in the design and specifications. It is recommended that the soils engineer be retained to provide soils engineering services during the design and foundation phases of work at the site. This is to observe compliance with the preliminary design concepts and to allow design changes in the event that subsurface conditions are found to differ from those anticipated prior to start of construction.

It has been a pleasure to be of assistance to you with this phase of your project. If you have any questions or if we may be of further assistance, please do not hesitate to contact us.

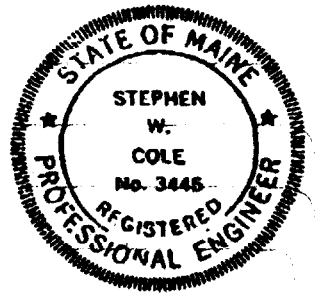
Very truly yours,

S. W. COLE ENGINEERING, INC.

Paul F. Kohler, Soils Engineer

Stephen W. Cole, P.E.

PFK-SWC:slh



Job No. 87-278 S

LOG OF TEST PIT 1

DATE 10-27-87 SURFACE ELEV. 66± LOCATION See Sheet 1

Depth (ft.)	Symbol	Sample	Soil Description	Test Results
0			Note: Filled Area	
			Brown silty sand w/gravel, cobbles (Fill)	
			Dark brown silty clay w/organics	←old ground surface
			Grayish-brown mottled silty clay	qp = 2.5+3.5 ksf @ 2.5'
5				qp = 9+ ksf @ 3.5'
10			Brownish-gray silty clay w/grayish-brown sand layers becoming	
			Brown silty sand w/gravel & cobbles	
			Bottom of exploration	

Completion Depth 13±' (Not Refusal)

Depth to Water No free water observed, however, soils appeared saturated at surface.

LOG OF TEST PIT 2

DATE 10-27-87 SURFACE ELEV. 67± LOCATION See Sheet 1

Depth (ft.)	Symbol	Sample	Soil Description	Test Results
0			12 to 14" Topsoil w/organics	
	X	S-1	Grayish-brown mottled silty clay (blocky, desiccated)	qp = 9.0± ksf @ 2'
5				qp = 9+ ksf @ 4' w = 25.6% W_L = 40.1 W_P = 24.0
10			Brown sand w/gray silty clay	qp = 1.0± ksf @ 10'-11' (clay)
			Brown silty sand w/gravel & cobbles	
			Bedrock	

Completion Depth 11.5±' (Refusal)

Depth to Water Water seeping @ 10±'. Soils saturated near surface.

Job No. 87-278 S

LOG OF TEST PIT 3

DATE 10-27-87 SURFACE ELEV. 71± LOCATION See Sheet 1

Depth(ft.)	Symbol	Sample	Soil Description	Test Results
0			12"± Topsoil w/organics	
			Light brown sand w/trace of silt & some gravel	
5	X	S-1	(more gravelly w/depth)	w = 8.2%
10			Cobbles and small boulders	
			Bottom of exploration	

Completion Depth 11+' (Not Refusal)

Depth to Water No free water observed. Soils appeared saturated @ 2.5±'.

LOG OF TEST PIT 4

DATE 10-27-87 SURFACE ELEV. 67± LOCATION See Sheet 1

Depth(ft.)	Symbol	Sample	Note: Filled Area Soil Description	Test Results
0			Brown silty sand w/gravel, cobbles, small boulders,	brick (Fill)
			Dark brown fine sandy clayey silt w/organics	old ground surface
5			Grayish-brown mottled silty clay	qp = 9+ ksf @ 4'
			(blocky, desiccated)	
10				qp = 9+ ksf @ 10'
			Bottom of exploration	

Completion Depth 11.5±' (Not Refusal)

Depth to Water No free water observed, however, soils appeared saturated @ 1'.

Job No. 87-278 S

LOG OF TEST PIT 5

DATE 10-27-87 SURFACE ELEV. 66± LOCATION See Sheet 1

Depth (ft.)	Symbol	Sample	Soil Description	Test Results
0			Note: Filled Area	
	X	S-1	Brown silty sand w/gravel (Fill)	w = 14.0%
			Dark brown fine sandy clayey silt w/organics — old ground surface	qp = 4.5± ksf @ 2'
			Grayish-brown mottled silty clay	qp = 9+ ksf @ 3.5'
5	X	S-2	(blocky, desiccated)	w = 27.5% W_L = 44.3 W_P = 27.2
10			(tough digging)	qp = 9+ ksf @ 10'
			Bottom of exploration	

Completion Depth 11.5±' (Not Refusal)

Depth to Water Water seeping @ 4±'. Soils saturated @ 2±'.

LOG OF TEST PIT 6

DATE 10-27-87 SURFACE ELEV. 65± LOCATION See Sheet 1

Depth (ft.)	Symbol	Sample	Soil Description	Test Results
0			14"± Topsoil w/organics	
	X	S-1	Grayish-brown mottled silty clay (blocky, desiccated) (occasional cobble)	w = 27.1%
5				
10			(tough digging)	
			Bottom of exploration	

Completion Depth 11±' (Not Refusal)

Depth to Water No free water observed, however, soils were saturated at surface.

KEY TO NOTES AND SYMBOLS

W - Water Content, percent - dry weight basis.

q_p - Unconfined Compressive Strength, kips/sq. ft. Based on Pocket Penetrometer

W_L - Liquid Limit

W_p - Plastic Limit

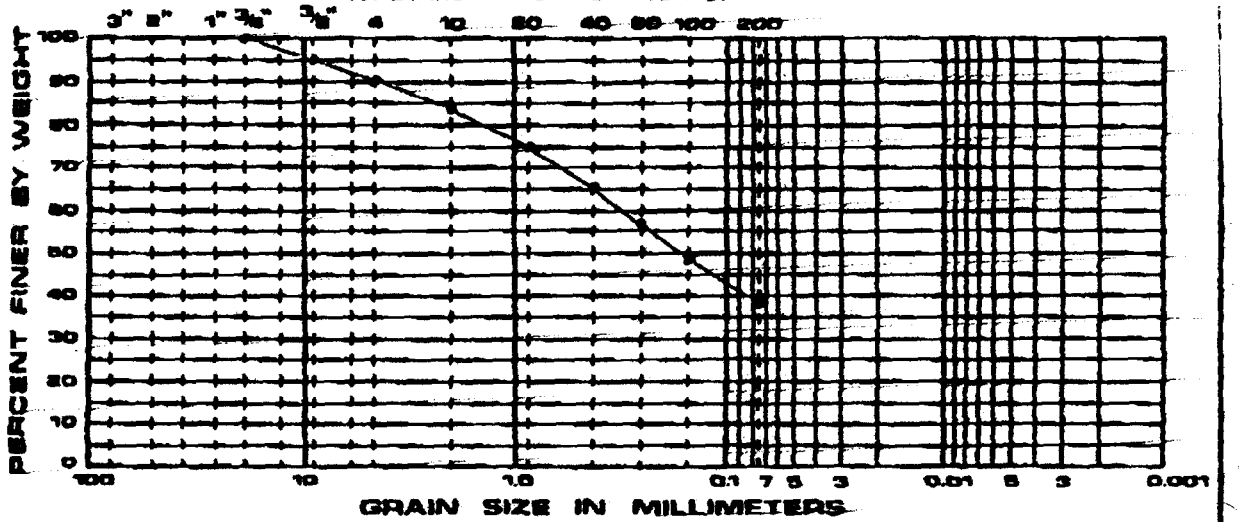
——— All stratification lines represent the approximate boundary between soil types and the transition may be gradual.

Refusal - Refusal depth indicates that depth at which sufficient resistance to the advance of the backhoe bucket was encountered to render further advance impossible or impracticable by the procedures and equipment being used. Although refusal may indicate the encountering of the bedrock surface, it may indicate the striking of large cobbles, boulders, very dense or cemented soil, or other buried natural or man-made objects or it may indicate the encountering of a harder zone after penetrating a considerable depth through a weathered or disintegrated zone of the bedrock.

GRAIN SIZE ANALYSIS

COBBLE	GRAVEL		SAND			SILT OR CLAY
	COARSE	FINE	COA.	MEDIUM	FINE	

U.S. STANDARD SIEVE SIZE NO.

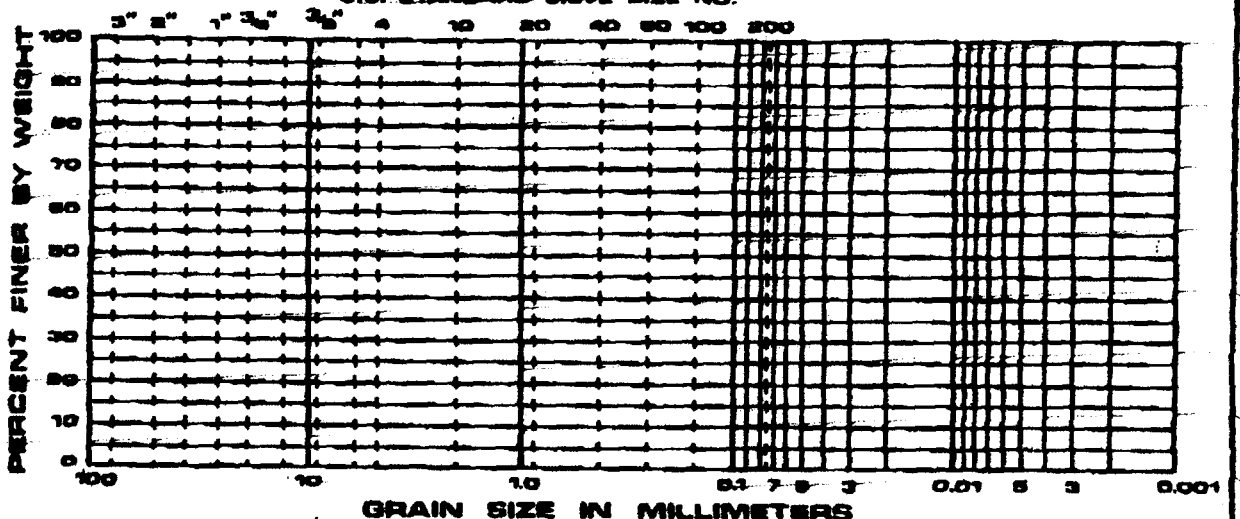


PLOT	SOURCE	SAMP.	DEPTH	CLASSIFICATION	LL	W%
•	TPS	S1	1'1"	Sand with Silt and some gravel		14.0

GRAIN SIZE ANALYSIS

COBBLE	GRAVEL		SAND			SILT OR CLAY
	COARSE	FINE	COA.	MEDIUM	FINE	

U.S. STANDARD SIEVE SIZE NO.



PLOT	SOURCE	SAMP.	DEPTH	CLASSIFICATION	LL	W%

BUTLER MANUFACTURING COMPANY
Northeast Region
400 North Weaber Street
Annville, Pennsylvania 17003
Phone: (717) 867-3201 Engineering
(717) 867-4651 Manufacturing
(717) 867-4606 Sales

June 23, 2003

Rick Warner
InterSpec Construction
8 Railroad Avenue
Rowley, MA 1969

68' x 110' x 14' MRF 0.50:12
Time Warner
Portland, Maine
BMC Order No. 104605
Builder Order No. 3001

To Whom It May Concern:

Please accept this letter as our certification that the Butler components of the subject building are designed in accordance with the 1989 Edition of the AISC Specification for the Design, Fabrication and Erection of Structural Steel and the 1996 Edition of the AISI Specification for the Design of Cold-Formed Steel Structural Members. The basic loads of the subject building meet or exceed the County Climatic Data as published in the 1996 edition of the MBMA Low Rise Buildings System Manual.

The governing design code is the 1999 Edition of the BOCA National Building Code. The following loads are applied in accordance with the governing code:

Dead Load		1.20 psf + Frame Weight
Collateral Load		5.00 psf
Roof Live Load	0-200 sf. Trib. Area	20 psf
	200-600 sf. Trib. Area	16 psf
	Over 600 sf. Trib. Area	12 psf
Roof Snow Load		49 psf
Snow Exposure		0.7
Exposure of Roof		Partially Exposed Roof
Thermal Condition		Normal
Wind Speed		90 mph (Includes Importance Factor
Wind Exposure		B
Seismic Acceleration, Aa		0.10 g
Seismic Velocity, Av		0.10 g
Category Use		1 (GENERAL USE)

Collateral load is included with snow load in determining critical stresses. Load combinations are in accordance with the governing code.

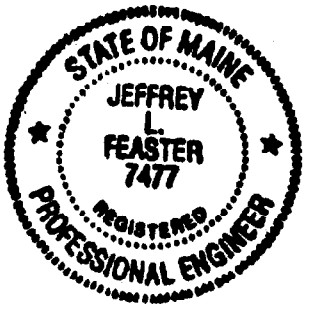
These Butler components, when properly erected on an adequate foundation in accordance with the erection drawings as supplied and using the components as furnished, will meet the above loading requirements. The design of this building for wind load assumes that doors not supplied by Butler are designed to sustain the same wind pressures and suctions as the walls in which they are installed. This certification does not cover field modifications or design of material not furnished by Butler Manufacturing Company. The design of this building was performed in one or more of Butler Manufacturing Company's facilities located in Annville, PA, Birmingham, AL, Burlington, ONT, Galesburg, IL, Kansas City, MO, Laurinburg, NC, San Marcos, TX and Visalia, CA. This building is produced in one or more Butler Mfg. Company's facilities located in Annville, PA, Birmingham, AL, Galesburg, IL, Laurinburg, NC, San Marcos, TX, and Visalia, CA. All listed facilities are Category MB certified by the American Institute of Steel Construction.

Cordially Yours,

Jeffrey L Feaster, P.E.
Div. Builder Services Manger

JLF/bas

cc: Order File



BUTLER MANUFACTURING COMPANY
400 North Weaber, Annville, PA 17003
Tel / Fax # (717)-867-4651 / (717)-867-3248

104605
InterSpec Construction
8 Railroad Avenue
Rowley, MA 1969

June 23, 2003

Rick Warner

End Customer: **Transport Leasing Corporation**
Location: **Portland, Maine**
Description: **68' 0" x 110' 0" x 14' 2 1/2"**

Butler Order No: **104605**
Builder Order No: **3001**

TO WHOM IT MAY CONCERN:

Please accept this letter as our certification that we furnished the roofing material including panels, purlins and fasteners for the above subject building, for a U L Class 90 roof as described in Construction Number(s)."

See the U L Building Material Directory for further description of Construction Number(s):

Panel Type	Construction Number
MR-24 Coated or Aluminized Steel 24 or 22 gage	No. 62

The roof material furnished by us for this order is marked by U L Label on the carton and is the same material as that tested by U L and described under the Construction Number(s) listed above.

Cordially yours,

Barry A. Scanlan, EIT

Design Engineer

717.867.3221

cc: Order File

Applicant: Transport Leasing Co/ Date: 5/28

Address: 9 Johnson Rd C-B-L: 214A-A-001

CHECK-LIST AGAINST ZONING ORDINANCE

Date - ~~Existing Dev~~

Zone Location - B-4

Interior or corner lot - change of use of old Bldg - office & storage for New Addition

Proposed Use/Work - Bldg Addition for Time Warner ~~Existing bldg = storage & service call vans~~

Sewage Disposal - City

Lot Street Frontage - 60' min = 251.82' shown

Front Yard - 20' or average - 51' scaled

Rear Yard - 20' min No change shown - existing Bldg

Side Yard - 10' req - 96' & 130' scaled

1 & 2 stories

Projections -

Width of Lot - 60' min - 200+ shown

Height - 65' max - 1 story prop. - 17' scaled to top of flat roof

Lot Area - 10,000 ☐' min 73,180 ☐' given

Lot Coverage/ Impervious Surface - 80% max of 58,544 ☐' max - previous given { Bldgs = 19,970 ☐' Pavement = 34,483 ☐' } 54,453

Area per Family - N/A

Off-street Parking - 7500 ÷ 400 = 18.75 spaces
12700 ÷ 1000 = 12.70 spaces
31.45 - req - 42 spaces shown

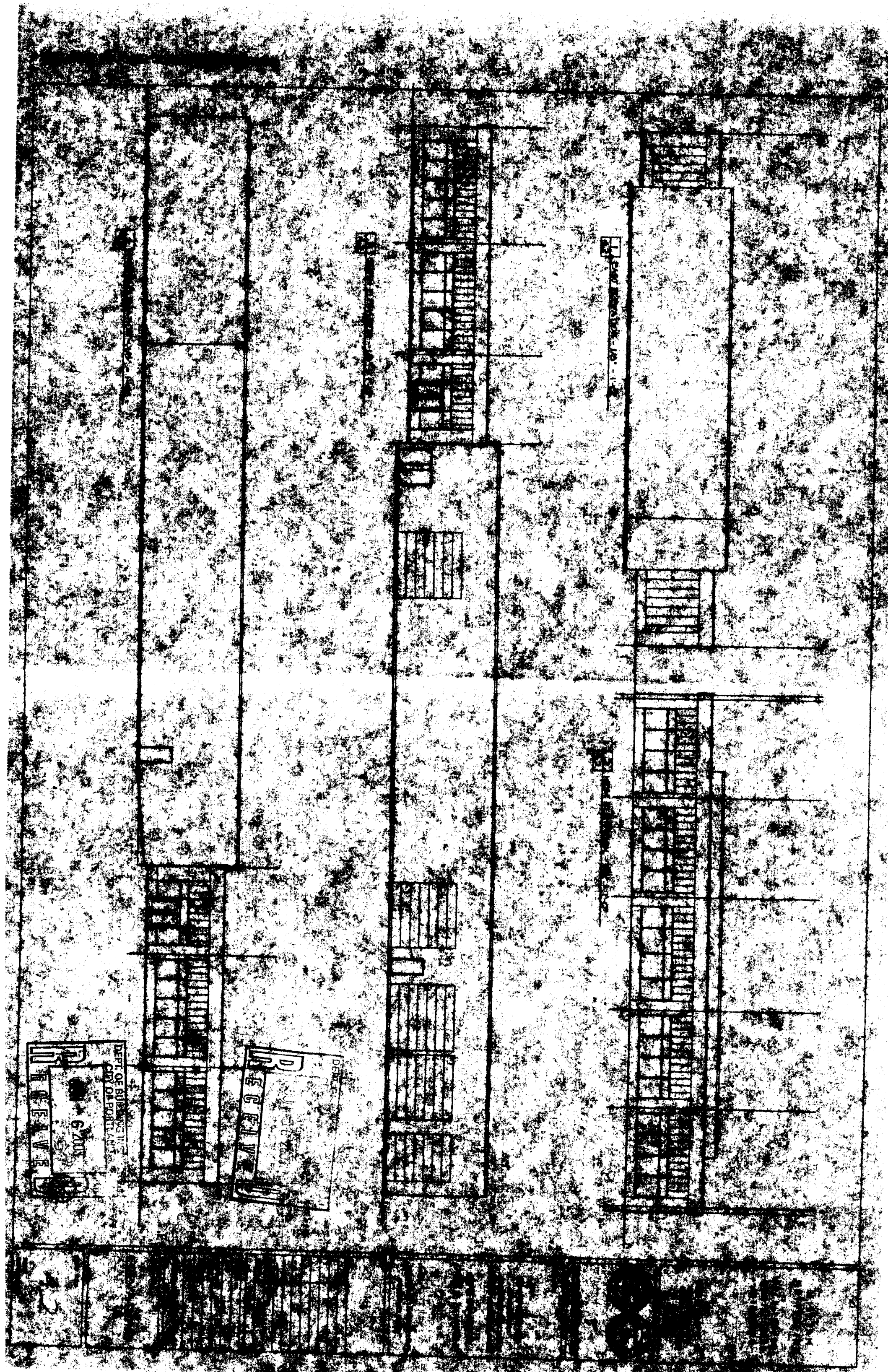
Loading Bays - 5 shown existing

Site Plan - minor #2003-0087

Shoreland Zoning/ Stream Protection - N/A

Flood Plains - panel 12 Zone X

Floor Area Ratio - closest R Zone = R-1 (.45 max) of 20,200 / 73180 = .276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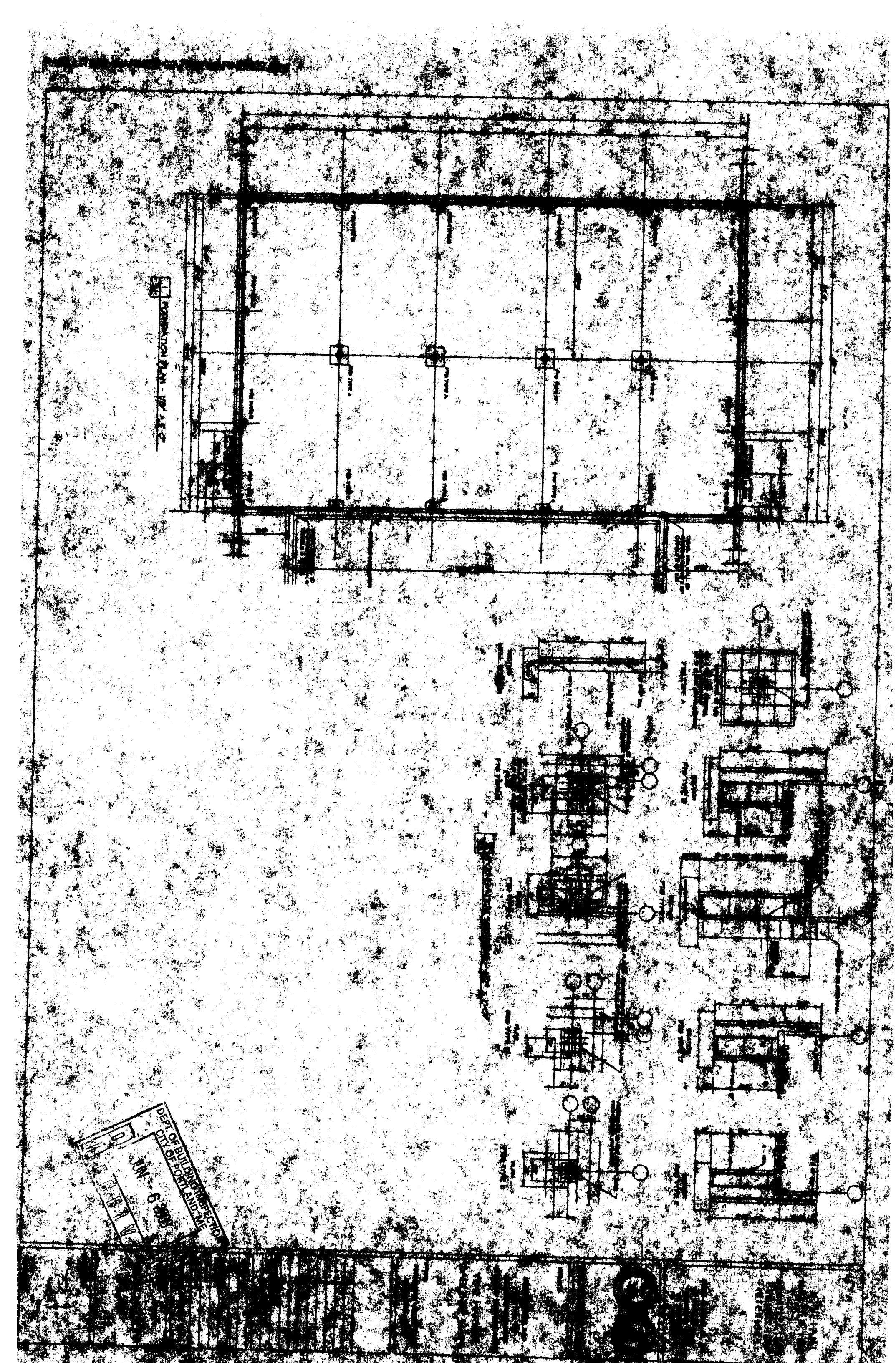
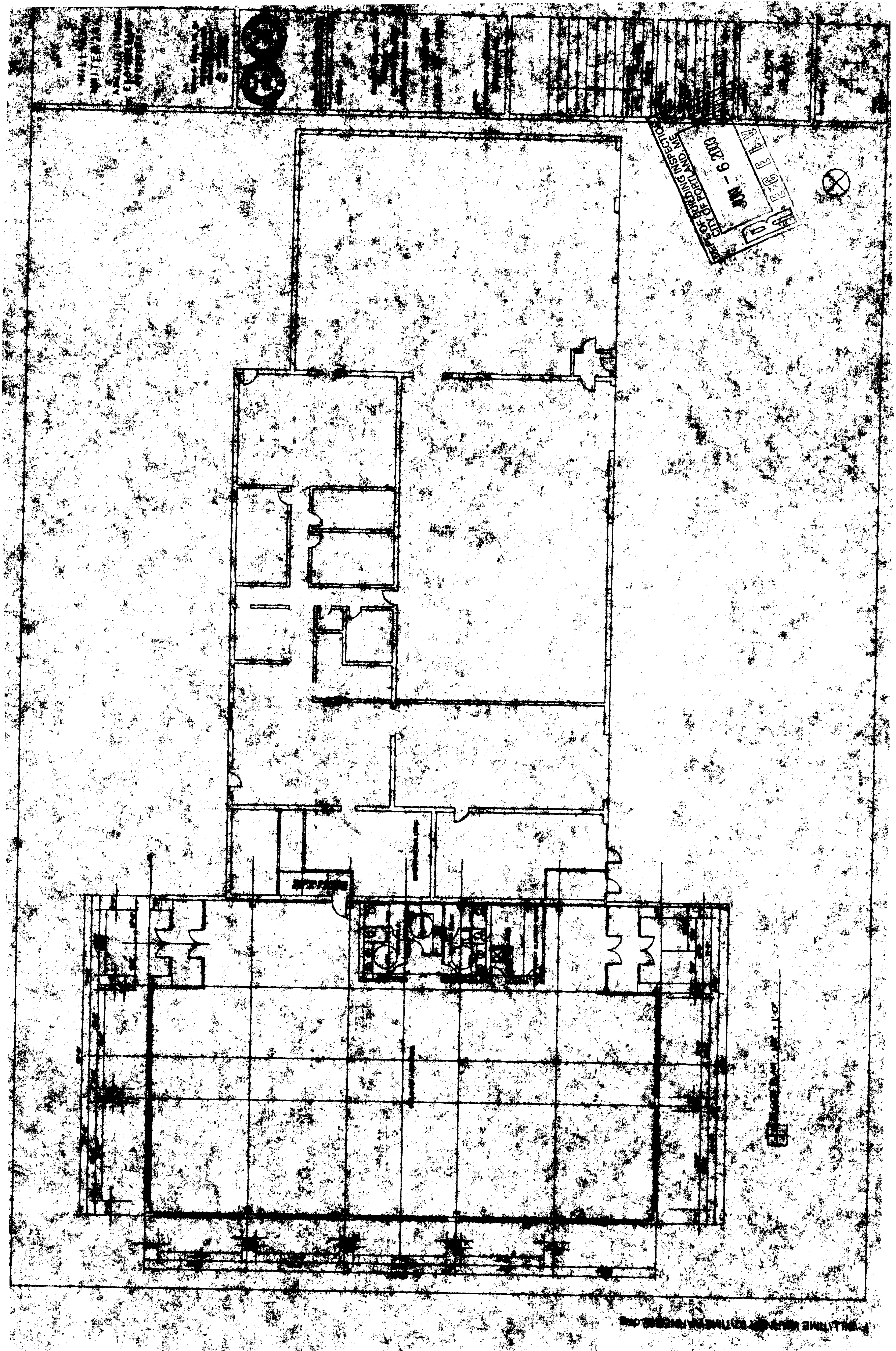
DEPT OF BUILDING
CITY OF PORTLAND
6 2005

SITE PLAN
SCALE 1" = 20'

FOUNDATION PLAN - 1/8" = 1'-0"

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CITY OF PORTLAND, ME
JUN - 6 2003
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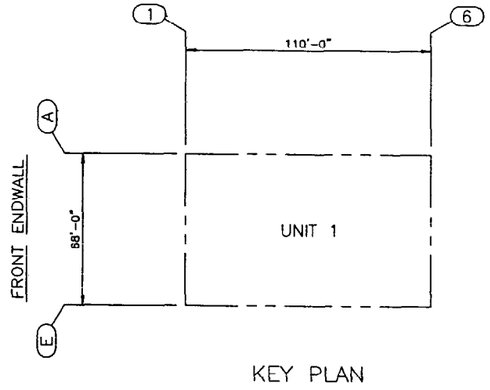

KEY PLAN

SPECIFIC ERECTION DRAWING LIST

THE DRAWINGS LISTED BELOW HAVE BEEN CREATED BY COMPUTER SPECIFICALLY FOR YOUR ORDER TO ASSIST YOU IN PUTTING UP YOUR BUILDING. THESE SPECIFIC ERECTION DRAWINGS ARE THE SAME SIZE AS THIS SHEET AND CAN BE IDENTIFIED INDIVIDUALLY FROM THE TITLES AND DRAWING SEQUENCE NUMBERS THAT APPEAR IN THE LOWER RIGHT HAND CORNER OF EACH DRAWING.

DRAWING TITLE:		AG FILE NO.:
ROOF SECONDARY STRUCTURAL FRAMING PLAN		DATE: 02/16/83
	DRAWING NUMBER: D 83-123456-05L	REVISION 02

TITLE

THE YEAR DURING WHICH YOUR ORDER WAS ENTERED

THE BUTLER ORDER NUMBER OF YOUR ORDER

DRAWING SEQUENCE NUMBER

REVISION LEVEL OF THE DRAWING. IF THE DRAWING HAS NEVER BEEN REVISED, 00 APPEARS HERE.

D03-104605-00 COVER DRAWING - SHEET 1
D03-104605-00A SHAKEOUT PLAN - SHEET 2
D03-104605-01 SPECIFIC ANCHOR BOLT DRAWING - SHEET 1
D03-104605-01A SPECIFIC REACTION DRAWING - SHEET 2
D03-104605-01B ANCHOR BOLT DETAIL SHEET - SHEET 3
D03-104605-02 CROSS SECTION ERECTION DRAWING - SHEET 1
D03-104605-02A CROSS SECTION ERECTION DRAWING - SHEET 2
D03-104605-02B CROSS SECTION ERECTION DRAWING - SHEET 3
D03-104605-02C CROSS SECTION ERECTION DRAWING - SHEET 4
D03-104605-02D CROSS SECTION ERECTION DRAWING DETAIL SHEET - SHEET 5
D03-104605-03 WIND BRACING DRAWING - SHEET 1
D03-104605-03A WIND BRACING DETAIL SHEET - SHEET 2
D03-104605-04 ROOF SECONDARY STRUCTURAL FRAMING PLAN - SHEET 1
D03-104605-04A ROOF SECONDARY STRUCTURAL DETAIL SHEET - SHEET 2
D03-104605-05 WALL SECONDARY STRUCTURAL ELEVATION - SHEET 1
D03-104605-05A WALL SECONDARY STRUCTURAL ELEVATION - SHEET 2
D03-104605-05B WALL SECONDARY STRUCTURAL ELEVATION DETAIL SHEET - SHEET 3
D03-104605-07 ROOF PANEL DRAWING - SHEET 1
D03-104605-07A EDGE OF ROOF DETAIL SHEET - SHEET 2

SUPPLEMENTING THE SPECIFIC ERECTION DRAWINGS LISTED ARE SEVERAL GENERAL ERECTION DRAWINGS CALLED "PLANOGRAPHS". SEE THE DRAWING MANIFEST FOR A COMPLETE LIST OF THESE DRAWINGS.

DESIGN CRITERIA SUMMARY

The 1999 edition of the "BOCA National Building Code" by the Building Officials and Code Administrators International Inc. (BOCA)

Building Use Category	1 (GENERAL USE)		
Collateral Load	5.00 psf		
Roof Panel Dead Load	1.20 psf		
Live Load			
Roof Live Load	20.00 psf		
Tributary Load Reduction (Y/N)	Y		
Snow Load			
Ground Snow Load (Pg)	70.00 psf		
Snow Importance Factor (I)	1.00		
Snow Exposure Factor (Ce)	0.7		
Exposure of Roof	Partial Exposed Rf		
Terrain Category	B		
Thermal Condition	Normal		
Rain on Snow Load	0.00 psf		
Roof Snow Load (Pf)	49.00 psf		
Wind Load			
Wind Speed	90 mph (Includes Importance Factor)		
Wind Importance Factor	1.10		
Enclosure Classification	Enclosed		
Wind Exposure Factor	B		
Seismic Load			
Peak Acceleration (Aa)	.10 g		
Peak Velocity (Av)	.10 g		
Seismic Performance Category	C		
Soil Profile Type	S4		
	Basic Struc. System	Seismic Res. System	Response Mod. Factor	Deflection Amp. Factor
Bracing				
	Moment-Res. Frm. System	Ordinary moment frames of steel	4.50	4.00
Main Frames				
	Moment-Res. Frm. System	Ordinary moment frames of steel	4.50	4.00
Analysis Procedure	Equivalent Lateral Force Procedure			

COVER DRAWING NOTES

STANDARD NOTES:

CD0001 ATTACH PATENT PLATE 007849 TO THE WEB OF AN INTERMEDIATE FRAME COLUMN AT EYE LEVEL.

CD0002 PARTS SHOWN MAY BE UPGRADED DUE TO STANDARDIZED FABRICATION. REFER TO THE SHIPPING MANIFEST FOR POSSIBLE SUBSTITUTIONS.

CD0013 IF THESE DRAWINGS ARE SEALED, THE SEAL APPLIES ONLY TO THE MATERIALS SUPPLIED BY BUTLER MFG. CO. AND IS NOT INTENDED AS THE SEAL OF THE ENGINEER OF RECORD FOR THE ENTIRE PROJECT.

CD0017 MATERIALS

	ASTM DESIGNATION	
STRUCT PLATE 1" & LESS	A529 AND A572	FY = 55 KSI (GRADE 55)
STRUCT PLATE OVER 1"	A529 AND A572	FY = 50 KSI (GRADE 50)
LIGHT GAGE/COLD FORMED	EQUIV TO A1011	FY = 60 KSI (GRADE 60)
ROOF AND WALL PANELS	A653 AND A792	GRADE 50, 60, 80
ROUND TUBE	A500 GRADE B	FY = 42 KSI
SQUARE TUBE	A500 GRADE B	FY = 46 KSI
BRACE RODS 3/4" & LESS	A510 GRADE 1018	
BRACE RODS OVER 3/4"	A572 GRADE 50 OR GREATER	

CD0026 1/2" DIA. BOLTS AND NUTS ARE FURNISHED AS AN ASSEMBLY. THE DRAWINGS CALL OUT THESE BOLTS AND NUTS BY THEIR COMPONENT NO'S.

ASSEMBLY	CONTAINS
0097461	1/2X1-1/4 BOLT (0095085) AND NUT (0095032)
0097462	1/2X1-1/4 THN HD BOLT (0096836) AND NUT (0095032)
0097463	1/2X1-1/2 BOLT (0095195) AND NUT (0095032)
0097464	1/2X1-3/4 GALV BOLT (0095331) AND (0095032)
0097465	1/2X2 A325T BOLT (0097280) AND (0095230)

CD0027 HIGH STRENGTH BOLTING:

ALL HIGH STRENGTH BOLTS ARE A-325-T WITH HEAVY HEX NUTS AND ARE TO BE INSTALLED USING THE SNUG TIGHT METHOD SPECIFIED IN THE "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS", PUBLISHED BY RCSC, DATED JUNE 23, 2000. SNUG TIGHT CONDITION IS ATTAINED WITH A FEW IMPACTS OF AN IMPACT WRENCH OR THE FULL EFFORT OF AN IRON WORKER USING AN ORDINARY SPUD WRENCH TO BRING THE PLIES INTO FIRM CONTACT.

STATE OF MAINE
JEFFREY L. FEASTER
7475
REGISTERED
PROFESSIONAL ENGINEER

STATE OR LOCAL
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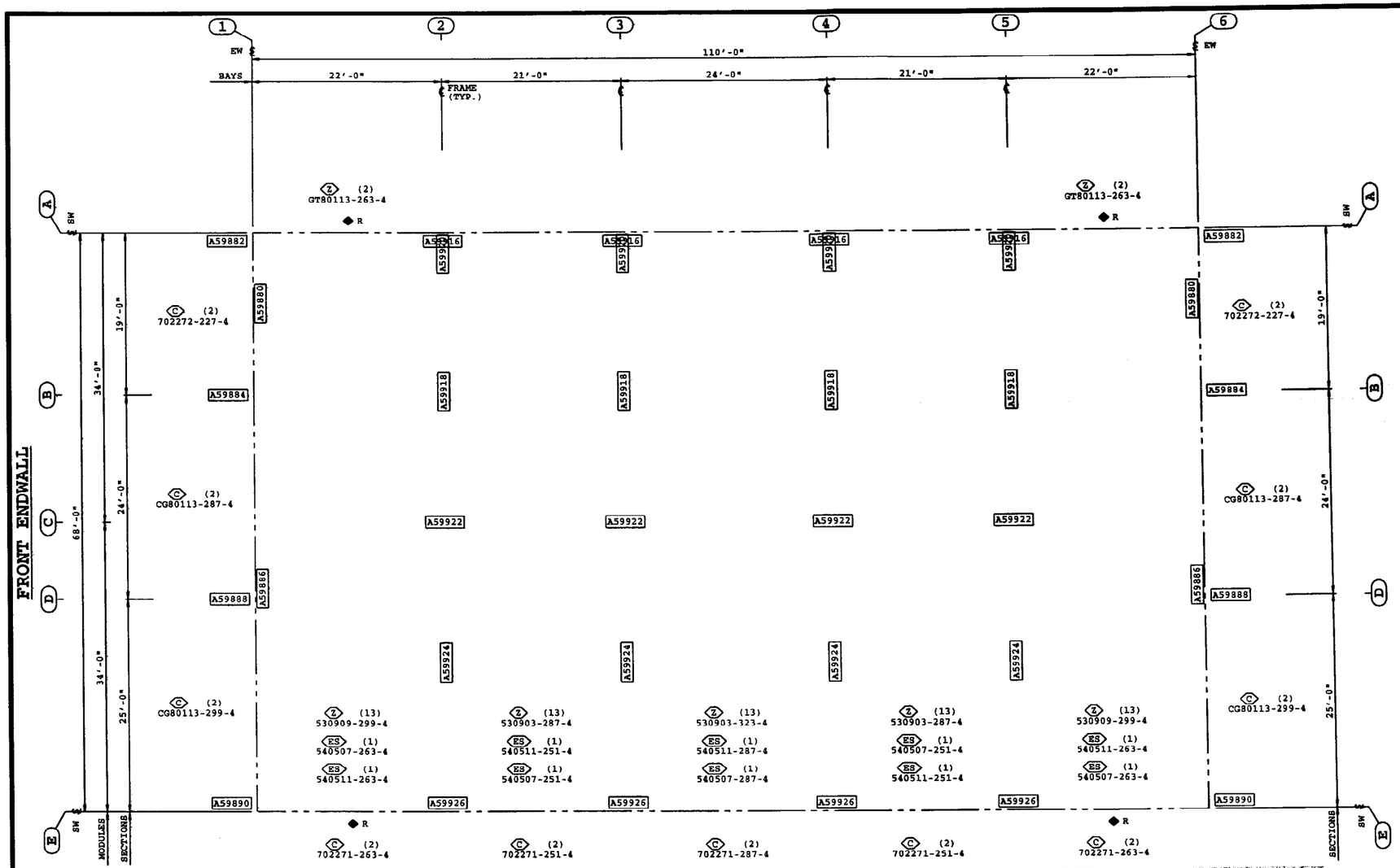
USERNAME: bascanlan 06-25-2003 10:49:30 O:\Orders\104605\Erecdwg\104605-01.dwg

REVISION NO. 1		REVISION NO. 2									
DATE:		DATE:		BUTLER MANUFACTURING COMPANY GENERAL OFFICES-KANSAS CITY, MISSOURI	PRODUCT OF THE PRONTO® ORDER ENGINEERING SYSTEM	BUILDER: M.H. SANBORN, INC. D/B/ ROWLEY, MA	PROJECT: TIME WARNER PROJECT- CUMBERLAND, MAINE	BUILDING ORDER DESCRIPTION: 68X110X14 MRSS 49# Roof SL + 5#CLL 90 MPH EXP B NBC 99	DRAWING TITLE: COVER DRAWING	DRW: BASCANLAN	ENG: BAS
DRAWN BY:		DRAWN BY:								CHK:	DATE: 06/23/03
CHECKED BY:		CHECKED BY:								DRAWING NUMBER: D03-104605-00	REV. 00



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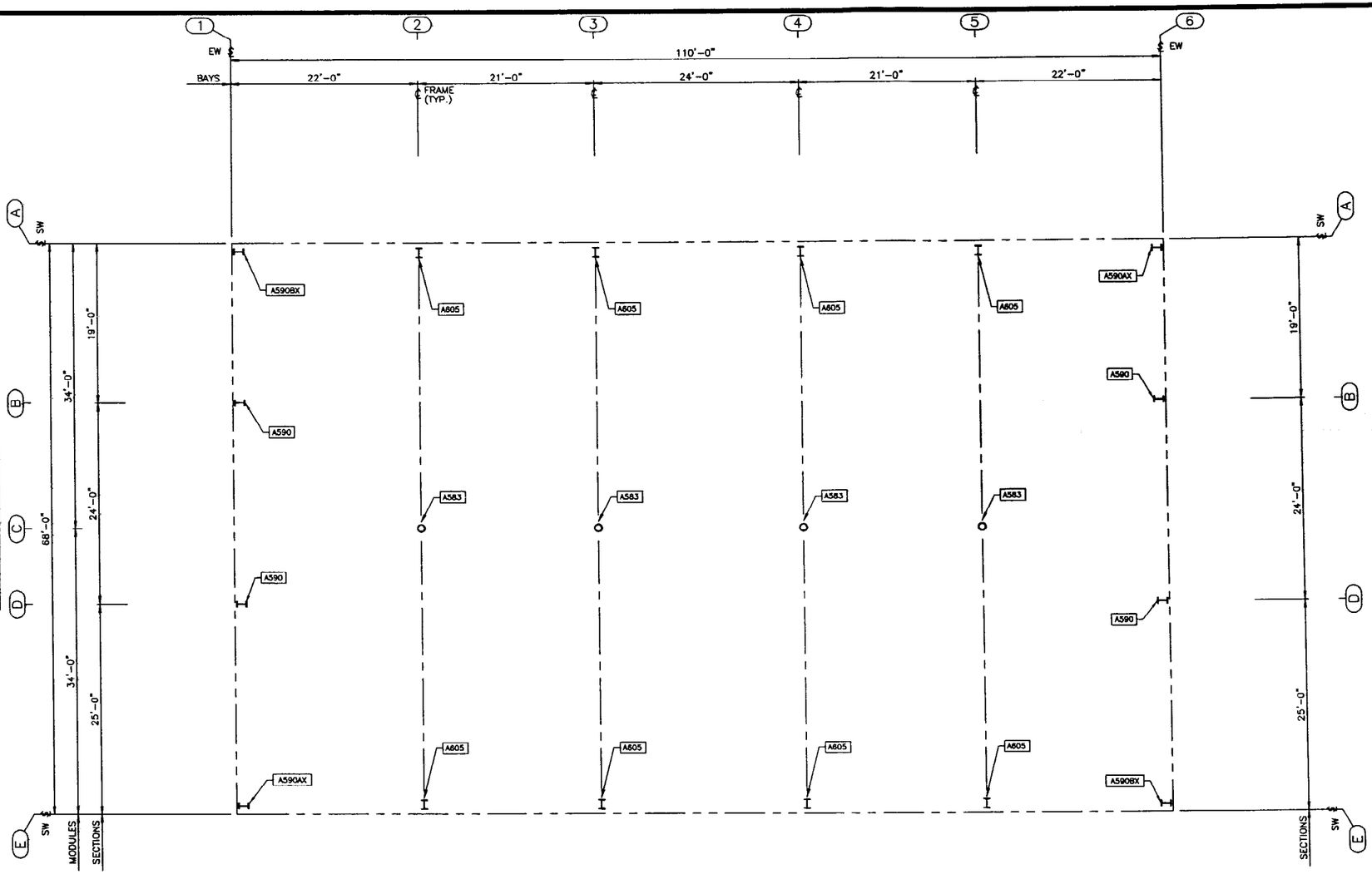
ONLY THE FOLLOWING TYPES OF PARTS ARE CALLED OUT OF THE SHAKEOUT PLAN. OTHER PARTS ARE SPECIFIED ON THE ERECTION DRAWINGS:
COLUMNS AND ROOF BEAMS SECONDARY PARTS ARE: ES EAVE STRUT, TP TRUSS PURLIN, C C-SECTION, Z Z-SECTION

* ◆ R - BRACE BAY OR SECTION [* = W, WP, or PF (W - WALL RODS) (WP - WIND POST) (PF - PORTAL FRAME)] [R = Roof]

REVISION NO. 1	REVISION NO. 2							
DATE:	DATE:	BUTLER MANUFACTURING COMPANY GENERAL OFFICES-KANSAS CITY, MISSOURI	PRODUCT OF THE PRONTO® ORDER ENGINEERING SYSTEM	BUILDER: M.H. SANBORN, INC. D/B/ ROWLEY, MA	PROJECT: TIME WARNER PROJECT- CUMBERLAND, MAINE	BUILDING ORDER DESCRIPTION: 68X110X14 MRSS 49# Roof SL + 5#CLL 90 MPH EXP B NBC 99	DRAWING TITLE: SHAKEOUT PLAN	DRW: BASCANLAN ENG: BAS CHK: DATE: 06/25/03 DRAWING NUMBER: D03-104605-00A REV. 00
DRAWN BY:	DRAWN BY:							
CHECKED BY:	CHECKED BY:							



STANDARD NOTES:

AB0007 GIVE TOP OF FOUNDATION A TROWEL FINISH. THE FOUNDATION MUST BE SQUARE, LEVEL, AND SMOOTH.

AB0009 ANCHOR BOLTS MUST BE LOCATED BY MEANS OF A TEMPLATE. DO NOT HAND SET ANCHOR BOLTS.

AB0010 ANCHOR BOLTS, NUTS AND WASHERS MUST BE ASTM F1554.

AB0013 USE REINFORCING STEEL AS REQUIRED, PROVIDE FOR HORIZONTAL LOAD ON ANCHOR BOLTS.

AB0014 C A U T I O N

RIGID FRAMES HAVE BOTH HORIZONTAL AND VERTICAL REACTIONS ACTING AT THE BASE OF THE COLUMN. IN SOME CASES THE HORIZONTAL FORCE IS GREATER THAN THE VERTICAL. A FOUNDATION DESIGNED FOR A CONVENTIONAL STRUCTURE WILL NOT BE SATISFACTORY. FAILURE TO MAKE ADEQUATE PROVISION FOR THE HORIZONTAL THRUST CAN RESULT IN FOUNDATION FAILURE.

AB0015 FOUNDATION DESIGN AND ANCHOR BOLTS, NUTS AND WASHERS ARE NOT FURNISHED BY BUTLER. PROPER EMBEDMENT LENGTH MUST BE DESIGNED BY FOUNDATION DESIGN ENGINEER.

AB0018 REFER TO WALL PANEL ANCHOR BOLT DRAWING FOR SPECIFIC EDGE OF FOUNDATION DETAILS.

AB0017 THE SOLID BLACK <> SYMBOL DESIGNATES A BRACED BAY OR SECTION.

UNIT 1 ANCHOR BOLT PLAN

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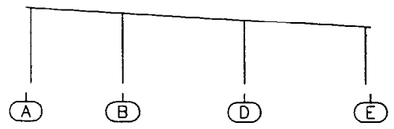
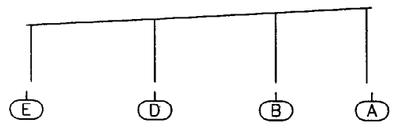
REVISION NO. 1	REVISION NO. 2							
DATE:	DATE:	BUTLER MANUFACTURING COMPANY GENERAL OFFICES-KANSAS CITY, MISSOURI	PRODUCT OF THE PRO/TO® ORDER ENGINEERING SYSTEM	BUILDER: M.H. SANBORN, INC. D/B/ ROWLEY, MA	PROJECT: TIME WARNER PROJECT- CUMBERLAND, MAINE	BUILDING ORDER DESCRIPTION: 68X110X14 MRSS 49# Roof SL + 5#CLL 90 MPH EXP B NBC 99	DRAWING TITLE: SPECIFIC ANCHOR BOLT DRAWING	DRW: BASCANLAN ENG: BAS CHK: DATE: 06/23/03 DRAWING NUMBER: D03-104605-01 REV. 00
DRAWN BY:	DRAWN BY:							
CHECKED BY:	CHECKED BY:							



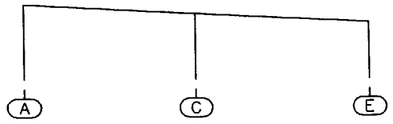
UNIT 1 – ENDWALL FRAME AT SIDEWALL GRID 1

		A				B			D			E			
		H	V	Z	ZM	V	Z	ZM	V	Z	ZM	H	V	Z	ZM
DL	DEAD LOAD	0.00	0.55	0.00	0.	1.17	0.00	0.	1.39	0.00	0.	0.00	0.59	0.00	0.
CLL	COLLATERAL LOAD	0.00	0.43	0.00	0.	1.32	0.00	0.	1.58	0.00	0.	0.00	0.61	0.00	0.
SL	SNOW LOAD	0.00	4.21	0.00	0.	12.80	0.00	0.	15.34	0.00	0.	0.00	5.93	0.00	0.
WLL	WIND LOAD LEFT	-0.89	-0.72	-0.83	-39.	-2.31	-1.85	-95.	-2.76	-1.98	-96.	-0.45	-1.09	-1.05	-58.
WLR	WIND LOAD RIGHT	0.55	-0.84	-0.83	-39.	-2.46	-1.85	-95.	-2.95	-1.98	-96.	0.73	-1.10	-1.05	-58.
IP	WIND INT PRESSURE	0.00	-0.16	-0.01	22.	-0.50	-0.24	8.	-0.60	-0.29	2.	0.00	-0.23	-0.14	1.
IS	WIND INT SUCTION	0.00	0.16	0.01	-22.	0.50	0.24	-8.	0.60	0.29	-2.	0.00	0.23	0.14	-1.
EQ	SEISMIC LOAD	0.00	0.00	+/-0.64	+/-127.	0.00	+/-1.21	+/-235.	0.00	+/-1.46	+/-263.	0.00	0.00	+/-0.78	+/-131.
SLR	SNOWLOAD RIGHT SLOPE	0.00	4.21	0.00	0.	12.80	0.00	0.	15.34	0.00	0.	0.00	5.93	0.00	0.
EWP	EW WIND PRESSURE	0.00	-0.75	1.57	154.	-2.30	3.14	279.	-2.76	3.42	289.	0.00	-1.07	1.70	137.
EWS	EW WIND SUCTION	0.00	-0.75	-1.15	-127.	-2.31	-2.27	-227.	-2.77	-2.46	-232.	0.00	-1.07	-1.23	-110.
	MAX. POS. REACTION	0.55	5.19	1.58	132.	15.29	3.37	271.	18.31	3.70	287.	0.73	7.13	1.84	136.
	MAX. NEG. REACTION	-0.89	-0.46	-1.16	-127.	-1.79	-2.50	-235.	-2.16	-2.75	-263.	-0.45	-0.74	-1.37	-131.



UNIT 1 – ENDWALL FRAME AT SIDEWALL GRID 6

		E				D			B			A			
		H	V	Z	ZM	V	Z	ZM	V	Z	ZM	H	V	Z	ZM
DL	DEAD LOAD	-0.00	0.59	0.00	0.	1.39	0.00	0.	1.17	0.00	0.	-0.00	0.55	0.00	0.
CLL	COLLATERAL LOAD	-0.00	0.61	0.00	0.	1.58	0.00	0.	1.32	0.00	0.	-0.00	0.43	0.00	0.
SL	SNOW LOAD	-0.00	5.93	0.00	0.	15.34	0.00	0.	12.80	0.00	0.	-0.00	4.21	0.00	0.
WLR	WIND LOAD RIGHT	0.45	-1.09	-1.05	-58.	-2.76	-1.98	-96.	-2.31	-1.85	-95.	0.89	-0.72	-0.83	-39.
WLL	WIND LOAD LEFT	-0.73	-1.10	-1.05	-58.	-2.95	-1.98	-96.	-2.46	-1.85	-95.	-0.55	-0.84	-0.83	-39.
IP	WIND INT PRESSURE	-0.00	-0.23	-0.14	1.	-0.60	-0.29	2.	-0.50	-0.24	8.	-0.00	-0.16	-0.01	22.
IS	WIND INT SUCTION	-0.00	0.23	0.14	-1.	0.60	0.29	-2.	0.50	0.24	-8.	-0.00	0.16	0.01	-22.
EQ	SEISMIC LOAD	-0.00	0.00	+/-0.78	+/-131.	0.00	+/-1.46	+/-263.	0.00	+/-1.21	+/-235.	-0.00	0.00	+/-0.64	+/-127.
SLL	MIN SNOW LOAD LEFT	-0.00	5.93	0.00	0.	15.34	0.00	0.	12.80	0.00	0.	-0.00	4.21	0.00	0.
EWP	EW WIND PRESSURE	-0.00	-1.07	1.70	137.	-2.76	3.42	289.	-2.30	3.14	279.	-0.00	-0.75	1.57	154.
EWS	EW WIND SUCTION	-0.00	-1.07	-1.23	-110.	-2.77	-2.46	-232.	-2.31	-2.27	-227.	-0.00	-0.75	-1.15	-127.
	MAX. POS. REACTION	-0.73	7.13	1.84	136.	18.31	3.70	287.	15.29	3.37	271.	-0.55	5.19	1.58	132.
	MAX. NEG. REACTION	0.45	-0.74	-1.37	-131.	-2.16	-2.75	-263.	-1.79	-2.50	-235.	0.89	-0.46	-1.16	-127.

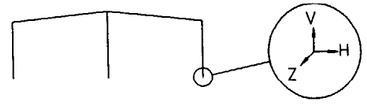


UNIT 1 – FRAME AT SIDEWALL GRIDS 2, 3, 4 & 5

		A		C		E	
		H	V	H	V	H	V
DL	DEAD LOAD	0.17	1.53	0.00	3.67	-0.17	1.30
CLL	COLLATERAL LOAD	0.22	1.61	0.00	4.40	-0.22	1.61
SL	SNOW LOAD	2.15	15.80	0.00	43.33	-2.15	15.80
WLL	WIND LOAD LEFT	-4.13	-3.66	0.00	-8.74	-4.14	-1.07
WLR	WIND LOAD RIGHT	2.01	-2.51	0.00	-7.76	4.21	-4.15
IP	WIND INT PRESSURE	0.21	-0.61	0.00	-1.73	-0.27	-0.59
IS	WIND INT SUCTION	-0.21	0.61	0.00	1.73	0.27	0.59
EQ	SEISMIC LOAD	-0.86	-0.29	0.00	-0.53	-1.65	0.82
SLR	MIN SNOW LOAD RIGHT	2.15	15.80	0.00	43.33	-2.15	15.80
EWP	EW WIND PRESSURE	0.96	-2.82	0.00	-7.95	-1.25	-2.69
EWS	EW WIND SUCTION	0.96	-2.82	0.00	-7.95	-1.25	-2.69
PDE	P Delta Unit Story S	-0.09	-0.10	0.50	-0.22	-0.41	0.32
	MAX. POS. REACTION	3.66	18.93	0.00	51.40	4.30	18.71
	MAX. NEG. REACTION	-4.17	-2.74	0.00	-6.79	-5.31	-3.43

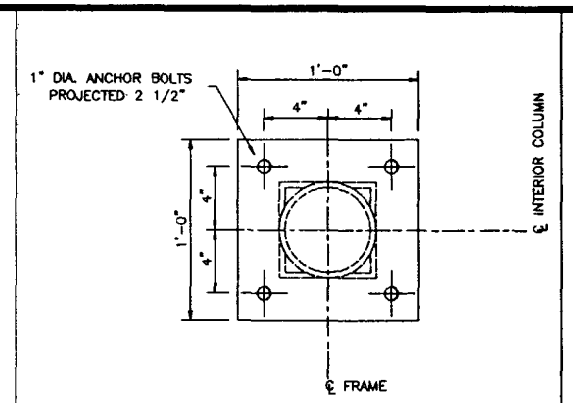
REACTION NOTES:

1. THE VERTICAL (V) AND HORIZONTAL (H AND Z) REACTION ARE STATED IN KIPS. (1 KIP = 1000 POUNDS) POSITIVE DIRECTION IS AS SHOWN IN THE FOLLOWING DIAGRAM.
2. MOMENT REACTIONS ARE STATED IN INCH-KIPS AND ARE POSITIVE IN A COUNTERCLOCKWISE DIRECTION.
3. THE FORCE ON THE ANCHOR BOLTS OR FOUNDATION WILL BE IN THE OPPOSITE DIRECTION TO THAT SHOWN.
4. MAXIMUM REACTION SUMMARY IS THE MAXIMUM POSITIVE AND NEGATIVE REACTIONS BASED ON THE REQUIRED LOAD COMBINATIONS.
5. SEISMIC REACTIONS SHOWN ARE CONSISTENT WITH THE BASE SHEAR (V) FROM THE SPECIFIED BUILDING CODE WITHOUT ADJUSTMENT FACTORS.
6. THE WIND LOAD BRACING REACTIONS DO NOT INCLUDE EWP, EWS AND IP REACTIONS FROM THE INTERMEDIATE FRAME. THESE REACTIONS SHOULD BE ADDED TO GET THE TOTAL REACTIONS FOR THE FOUNDATION DESIGN.

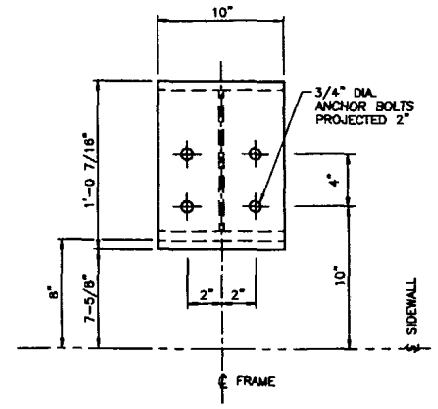


REVISION NO. 1	REVISION NO. 2					
DATE:	DATE:	BUTLER MANUFACTURING COMPANY GENERAL OFFICES-KANSAS CITY, MISSOURI	PRODUCT OF THE PROMOD® ORDER ENGINEERING SYSTEM	BUILDER: M.H. SANBORN, INC. D/B/ ROWLEY, MA	PROJECT: TIME WARNER PROJECT- CUMBERLAND, MAINE	BUILDING ORDER DESCRIPTION: 68X110X14 MRSS 49# Roof SL + 5#CLL 90 MPH EXP B NBC 99
DRAWN BY:	DRAWN BY:					
CHECKED BY:	CHECKED BY:					

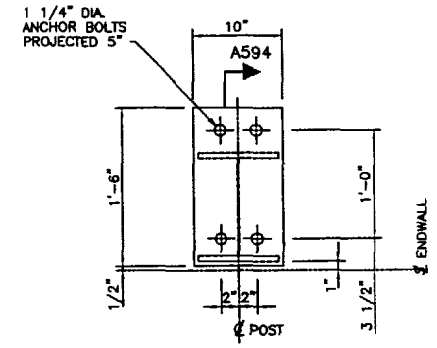
DRAWING TITLE:	DRW: BASCANLAN	ENG: BAS
SPECIFIC REACTION DRAWING	CHK:	DATE: 06/25/03
	DRAWING NUMBER: D03-104605-01A	REV. 00



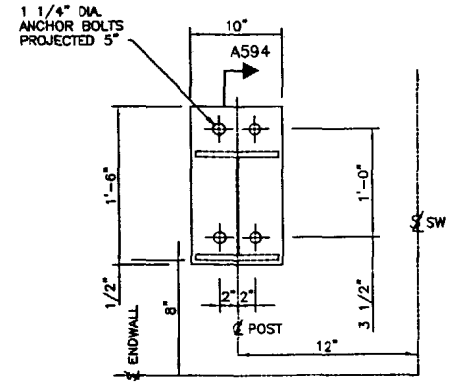
DETAIL A583 | INTERIOR PIPE OR TUBE COLUMN



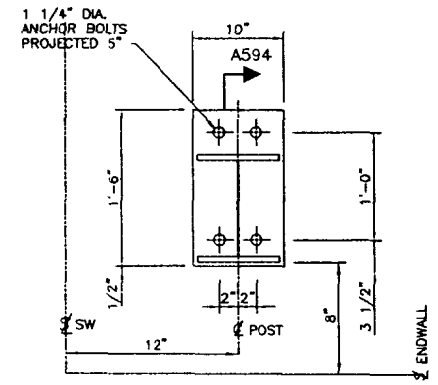
DETAIL A605 | SIDEWALL COLUMN



REV. DATED 10/13/98 | REV. NO. 00
DETAIL A590 | 12" EW POST FIXED BASE ENDWALL



REV. DATED | REV. NO.
DETAIL A590AX | 12" CORNER POST FIXED BASE ENDWALL



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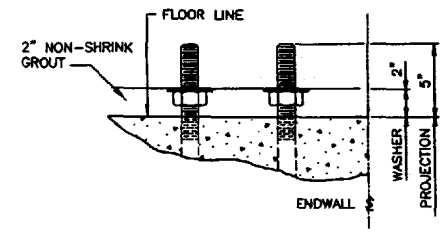
DETAIL A590BX | 12" CORNER COLUMN FIXED BASE ENDWALL



REV. DATED 10/13/98 | REV. NO. 00
SECTION A594 | FIXED BASE SECTION WITH NO POCKET

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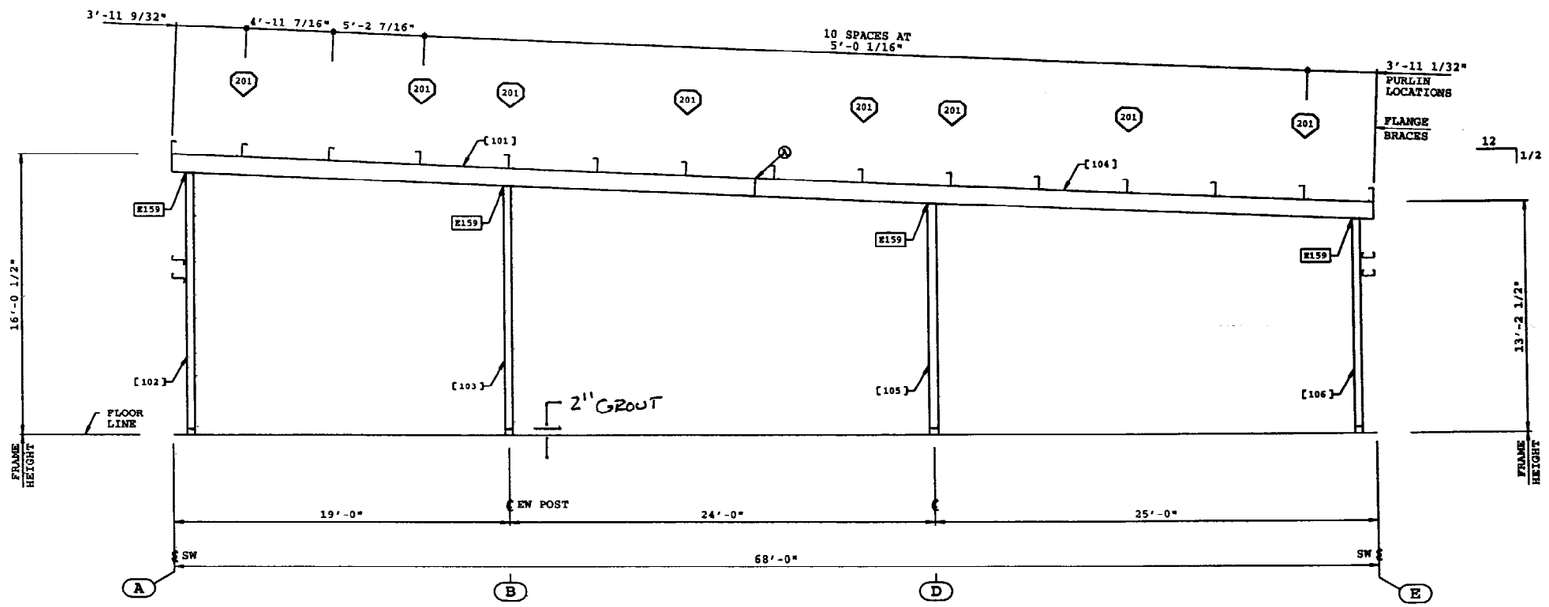
REVISION NO. 1		REVISION NO. 2								
DATE:		DATE:		BUTLER MANUFACTURING COMPANY GENERAL OFFICES-KANSAS CITY, MISSOURI	PRODUCT OF THE PRONTO® ORDER ENGINEERING SYSTEM	BUILDER: M.H. SANBORN, INC. D/B/ ROWLEY, MA	PROJECT: TIME WARNER PROJECT- CUMBERLAND, MAINE	BUILDING ORDER DESCRIPTION: 68X110X14 MRSS 49# Roof SL + 5#CLL 90 MPH EXP B NBC 99	DRAWING TITLE: ANCHOR BOLT DETAIL SHEET	DRW: BASCANLAN CHK:	ENG: RAS DATE: 06/23/03
DRAWN BY:		DRAWN BY:								DRAWING NUMBER: D03-104605-01B	REV. 00
CHECKED BY:		CHECKED BY:									



UNIT 1 FRONT EW ELEVATION AT GRID 1

PART SCHEDULE				
[ ]	PART NAME	PART NUMBER	PART LENGTH	FIELD WORK ▲
101	EW ROOF BEAM	A59880	~~39'-0 19/32"~~	
102	EW INTERM POST	A59882	~~14'-11 3/4"~~	
103	EW INTERM POST	A59884	14'-3 3/4"	
104	EW ROOF BEAM	A59886	~~24'-11 7/8"~~	
105	EW INTERM POST	A59888	~~13'-2 3/4"~~	
106	EW INTERM POST	A59890	~~12'-3 3/4"~~	

BOLTED CONNECTION SCHEDULE					
○	QUANTITY	BOLT NO.	DESCRIPTION	NUT NO.	DETAIL
A	04	097282	5/8X2-1/4 BOLT A325T	095233	

FLANGE BRACE SCHEDULE					
○	PART NO. FRONT/LEFT	PART NO. REAR/RIGHT	DIM. "Y"	DETAIL	FIELD WORK ▲
201	545100		10 1/2"	C347	

STANDARD NOTES:

IF0009 1/2" DIA. BOLTS AND NUTS ARE FURNISHED AS AN ASSEMBLY. THE DRAWINGS CALL OUT THESE BOLTS AND NUTS BY THEIR COMPONENT NO'S.

ASSEMBLY	CONTAINS
0097461	1/2X1-1/4 BOLT (0095085) AND NUT (0095032)
0097462	1/2X1-1/4 THN HD BOLT (0096636) AND NUT (0095032 1
0097463	1/2X1-1/2 BOLT (0095195) AND NUT (0095032)
0097464	1/2X1-3/4 GALV BOLT (0095331) AND (0095032)
0097465	1/2X2 A325T BOLT (0097280) AND (0095230)

IF0010 ALL HIGH STRENGTH BOLTS ARE A-325-T WITH HEAVY HEX NUTS AND ARE TO BE INSTALLED USING THE SNUG TIGHT METHOD SPECIFIED IN THE "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS", PUBLISHED BY RCSC, DATED JUNE 23, 2000. SNUG TIGHT CONDITION IS ATTAINED WITH A FEW IMPACTS OF AN IMPACT WRENCH OR THE FULL EFFORT OF AN IRON WORKER USING AN ORDINARY SPUD WRENCH TO BRING THE PLIES INTO FIRM CONTACT.

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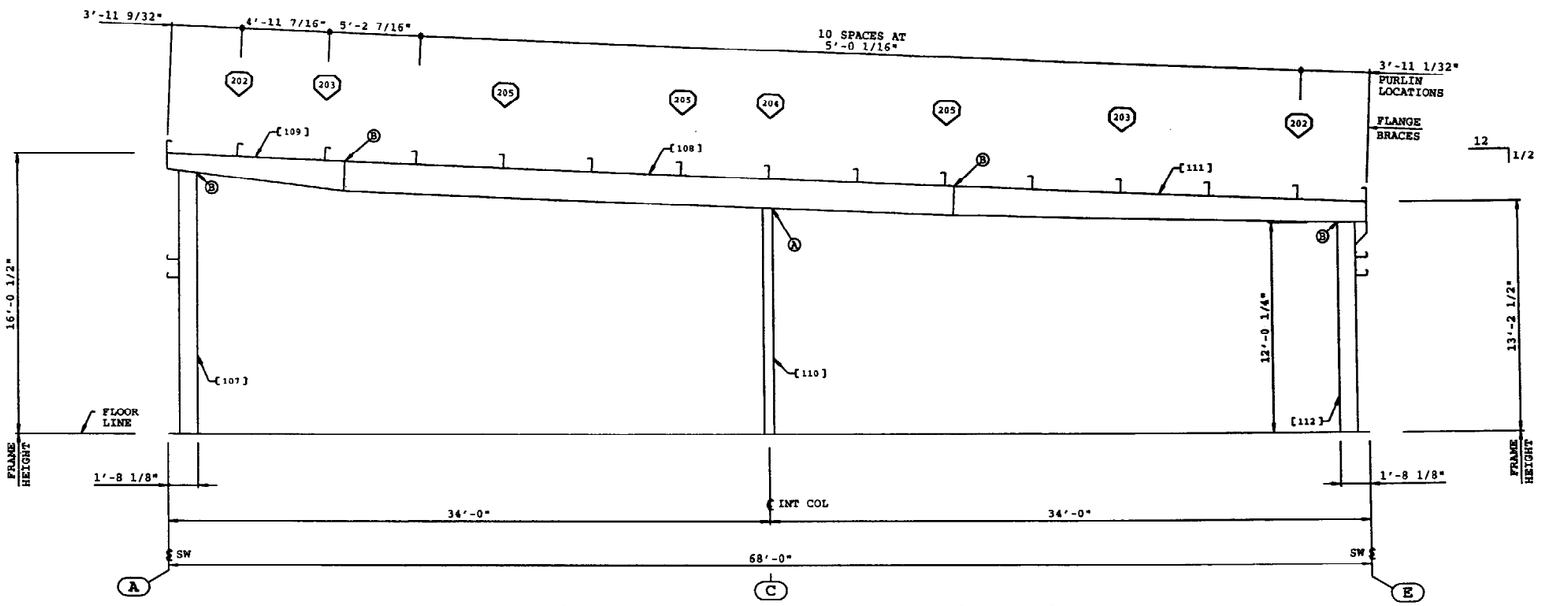
REVISION NO. 1	REVISION NO. 2								
DATE:	DATE:	BUTLER MANUFACTURING COMPANY GENERAL OFFICES-KANSAS CITY, MISSOURI	PRODUCT OF THE PRONTO® ORDER ENGINEERING SYSTEM	BUILDER:	PROJECT:	BUILDING ORDER DESCRIPTION:	DRAWING TITLE:	DRW: BASCANLAN	ENG: BAS
DRAWN BY:	DRAWN BY:			M.H. SANBORN, INC. D/B/ ROWLEY, MA	TIME WARNER PROJECT- CUMBERLAND, MAINE	68X110X14 MRSS 49# Roof SL + 5#CLL 90 MPH EXP B NBC 99	CROSS SECTION ERECTION DRAWING	CHK:	DATE: 06/25/03
CHECKED BY:	CHECKED BY:							DRAWING NUMBER: D03-104605-02	REV. 00



UNIT 1 FRAME ELEVATION AT GRIDS 2 AND 5

PART SCHEDULE				
[ ]	PART NAME	PART NUMBER	PART LENGTH	FIELD WORK ▲
107	EXTERIOR COLUMN	A59916	15'-1 3/32"	
108	ROOF BEAM	A59918	34'-7 15/32"	
109	ROOF BEAM	A59920	9'-11 31/32"	
110	INTERIOR COLUMN	A59922	12'-11 11/32"	
111	ROOF BEAM	A59924	23'-5"	
112	EXTERIOR COLUMN	A59926	12'-0 1/4"	

BOLTED CONNECTION SCHEDULE					
○	QUANTITY	BOLT NO.	DESCRIPTION	NUT NO.	DETAIL
A	04	097282	5/8X2-1/4 BOLT A325T	095233	
B	08	097282	5/8X2-1/4 BOLT A325T	095233	

FLANGE BRACE SCHEDULE					
◇	PART NO. FRONT/LEFT	PART NO. REAR/RIGHT	DIM. "Y"	DETAIL	FIELD WORK ▲
202	545101		1'-4 1/2"	C346	
203	545101		1'-4 1/2"	C347	
204	545102	545102	1'-10 1/2"	C344	
205	545102		1'-10 1/2"	C346	

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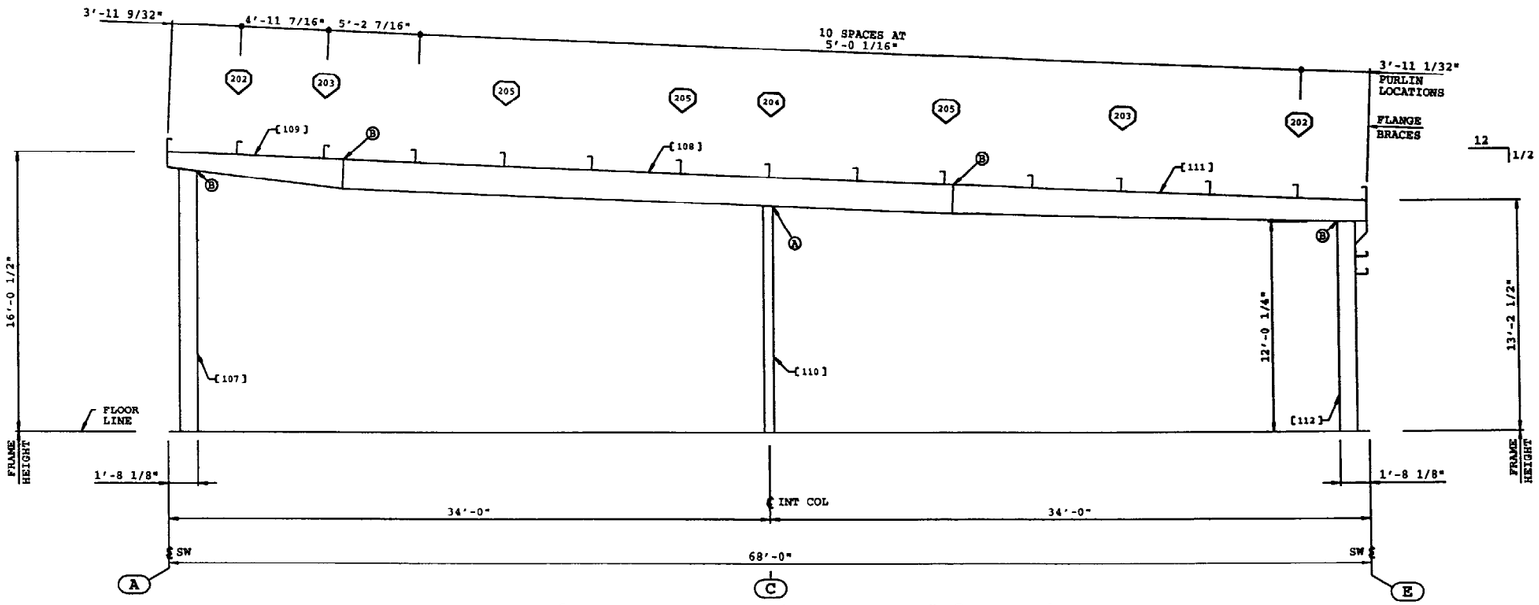
REVISION NO. 1		REVISION NO. 2		BUTLER MANUFACTURING COMPANY GENERAL OFFICES-KANSAS CITY, MISSOURI	PRODUCT OF THE PROMO ORDER ENGINEERING SYSTEM	BUILDER:	PROJECT:	BUILDING ORDER DESCRIPTION:	DRAWING TITLE:	DRW: BASCANLAN	ENG: RAS
DATE:		DATE:				M.H. SANBORN, INC. D/B/ ROWLEY, MA	TIME WARNER PROJECT- CUMBERLAND, MAINE	68X110X14 MRSS 49# Roof SL + 5#CLL 90 MPH EXP B NBC 99	CROSS SECTION ERECTION DRAWING	CHK:	DATE: 06/25/03
DRAWN BY:		DRAWN BY:								DRAWING NUMBER: D03-104605-02A	REV. 00
CHECKED BY:		CHECKED BY:									



UNIT 1 FRAME ELEVATION AT GRIDS 3 AND 4

PART SCHEDULE				
[ ]	PART NAME	PART NUMBER	PART LENGTH	FIELD WORK ▲
107	EXTERIOR COLUMN	A59916	15'-1 3/32"	
108	ROOF BEAM	A59918	34'-7 15/32"	
109	ROOF BEAM	A59920	9'-11 31/32"	
110	INTERIOR COLUMN	A59922	12'-11 11/32"	
111	ROOF BEAM	A59924	23'-5"	
112	EXTERIOR COLUMN	A59926	12'-0 1/4"	

BOLTED CONNECTION SCHEDULE					
○	QUANTITY	BOLT NO.	DESCRIPTION	NUT NO.	DETAIL
A	04	097282	5/8X2-1/4 BOLT A325T	095233	
B	08	097282	5/8X2-1/4 BOLT A325T	095233	

FLANGE BRACE SCHEDULE					
◇	PART NO. FRONT/LEFT	PART NO. REAR/RIGHT	DIM. "Y"	DETAIL	FIELD WORK ▲
202	545101		1'-4 1/2"	C346	
203	545101		1'-4 1/2"	C347	
204	545102	545102	1'-10 1/2"	C344	
205	545102		1'-10 1/2"	C346	

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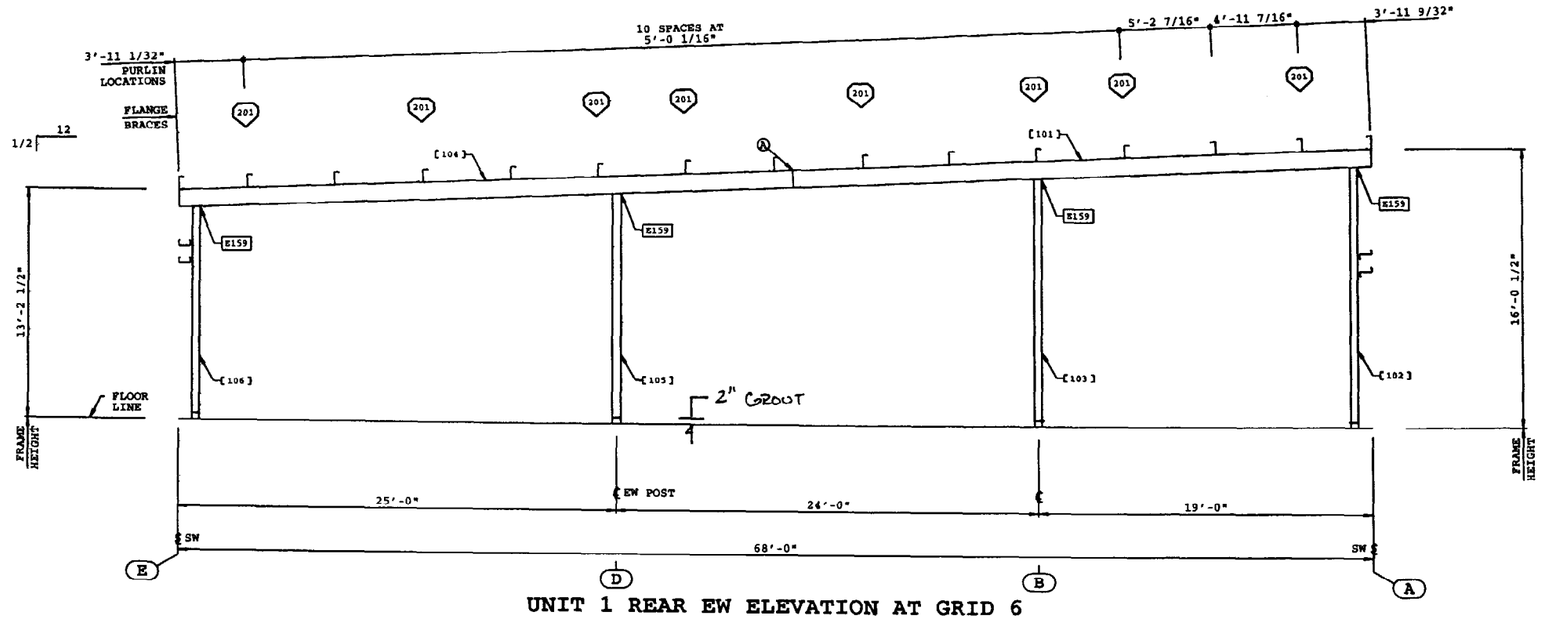
REVISION NO. 1		REVISION NO. 2									
DATE:		DATE:		BUTLER MANUFACTURING COMPANY GENERAL OFFICES-KANSAS CITY, MISSOURI	PRODUCT OF THE PRO/ENGINEER® ORDER ENGINEERING SYSTEM	BUILDER:	PROJECT:	BUILDING ORDER DESCRIPTION:	DRAWING TITLE:	DRW: BASCANLAN	ENG: BAS
DRAWN BY:		DRAWN BY:				M.H. SANBORN, INC. D/B/ ROWLEY, MA	TIME WARNER PROJECT- CUMBERLAND, MAINE	68X110X14 MRSS 49# Roof SL + 5#CLL 90 MPH EXP B NBC 99	CROSS SECTION ERECTION DRAWING	CHK:	DATE: 06/25/03
CHECKED BY:		CHECKED BY:								DRAWING NUMBER: D03-104605-02B	REV. 00



UNIT 1 REAR EW ELEVATION AT GRID 6

PART SCHEDULE				
[ ]	PART NAME	PART NUMBER	PART LENGTH	FIELD WORK ▲
101	EW ROOF BEAM	A59880	33'-0 19/32"	
102	EW INTERM POST	A59882	14'-11 3/4"	
103	EW INTERM POST	A59884	14'-2 3/4"	
104	EW ROOF BEAM	A59886	34'-11 7/8"	
105	EW INTERM POST	A59888	13'-2 3/4"	
106	EW INTERM POST	A59890	12'-2 3/4"	

BOLTED CONNECTION SCHEDULE					
○	QUANTITY	BOLT NO.	DESCRIPTION	NUT NO.	DETAIL
A	04	097282	5/8X2-1/4 BOLT A325T	095233	

FLANGE BRACE SCHEDULE					
○	PART NO. FRONT/LEFT	PART NO. REAR/RIGHT	DIM. "Y"	DETAIL	FIELD WORK ▲
201	545100		10 1/2"	C347	

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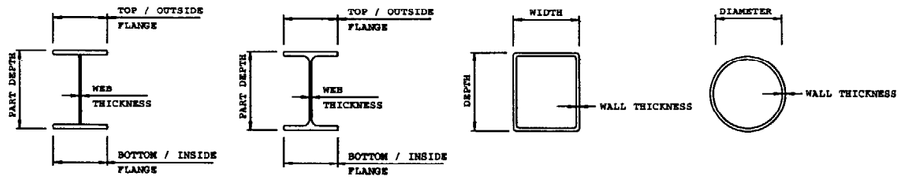
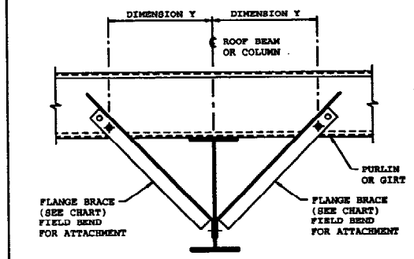
DATE:	DATE:	BUTLER MANUFACTURING COMPANY GENERAL OFFICES-KANSAS CITY, MISSOURI	PRODUCT OF THE PRONTO® ORDER ENGINEERING SYSTEM	BUILDER:	PROJECT:	BUILDING ORDER DESCRIPTION:	DRAWING TITLE:	DRW: BASCANLAN	ENG: BAS
DRAWN BY:	DRAWN BY:			M.H. SANBORN, INC. D/B/ ROWLEY, MA	TIME WARNER PROJECT- CUMBERLAND, MAINE	68X110X14 MRSS 49# Roof SL + 5#CLL 90 MPH EXP B NBC 99	CROSS SECTION ERECTION DRAWING	CHK:	DATE: 06/25/03
CHECKED BY:	CHECKED BY:							DRAWING NUMBER: D03-104605-02C	REV. 00

NOT ALL SECTION TYPES SHOWN MAY APPLY TO THIS ORDER.



FRAME DESIGN INFORMATION

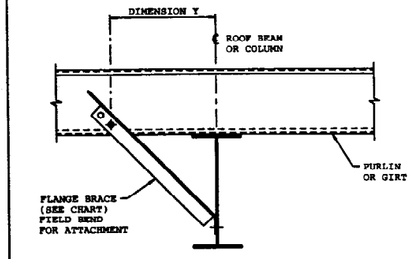
PART	DESCRIPTION
A59880 396-5 EW ROOF BEAM	TOP FLANGE = 5" X .250"
	BOTTOM FLANGE = 5" X .250"
	WEB THICKNESS = .100"
	PART DEPTH = 12"
A59882 179-6 EW INTERM POST	TOP FLANGE = 5" X .250"
	BOTTOM FLANGE = 5" X .250"
	WEB THICKNESS = .100"
	PART DEPTH = 12"
A59884 170-6 EW INTERM POST	TOP FLANGE = 5" X .250"
	BOTTOM FLANGE = 5" X .250"
	WEB THICKNESS = .100"
	PART DEPTH = 12"
A59886 419-7 EW ROOF BEAM	TOP FLANGE = 5" X .250"
	BOTTOM FLANGE = 5" X .250"
	WEB THICKNESS = .100" - LOW END TO 19'-11 7/8" FROM LOW END .120" - 19'-11 7/8" FROM LOW END TO HIGH END
	PART DEPTH = 12"
A59888 158-6 EW INTERM POST	TOP FLANGE = 6" X .250"
	BOTTOM FLANGE = 6" X .250"
	WEB THICKNESS = .100"
	PART DEPTH = 12"
A59890 146-6 EW INTERM POST	TOP FLANGE = 5" X .250"
	BOTTOM FLANGE = 5" X .250"
	WEB THICKNESS = .100"
	PART DEPTH = 12"
A59916 181-1 EXTERIOR COLUMN	TOP FLANGE = 6" X .250"
	BOTTOM FLANGE = 6" X .375"
	WEB THICKNESS = .120"
	PART DEPTH = 12 1/8"
A59918 415-4 ROOF BEAM	TOP FLANGE = 6" X .500"
	BOTTOM FLANGE = 6" X .500"
	WEB THICKNESS = .219" - LOW END TO 20'-0 1/8" FROM LOW END .160" - 20'-0 1/8" FROM LOW END TO HIGH END
	PART DEPTH = 20"
A59920 120-0 ROOF BEAM	TOP FLANGE = 5" X .250"
	BOTTOM FLANGE = 5" X .250"
	WEB THICKNESS = .160"
	PART DEPTH = VARIES 20" TO 10 9/32"
A59922 155-3 INTERIOR COLUMN R6.625X.156	PIPE DIAMETER = 6 5/8"
	WALL THICKNESS = .156"
	-
	-
A59924 281-0 ROOF BEAM	TOP FLANGE = 6" X .313"
	BOTTOM FLANGE = 6" X .250"
	WEB THICKNESS = .160" - LOW END TO 9'-11 25/32" FROM LOW END .140" - 9'-11 25/32" FROM LOW END TO HIGH END
	PART DEPTH = VARIES 15" TO 20"
A59926 144-2 EXTERIOR COLUMN	TOP FLANGE = 6" X .250"
	BOTTOM FLANGE = 6" X .375"
	WEB THICKNESS = .100"
	PART DEPTH = 12 1/8"



PART USAGE
8 OR 9-1/2 PURLINS
8 Z-GIRTS

USE (3) 1/2" X 1 1/4" HEX. BOLT (095085) AND 1/2" HEX. NUT (095032)

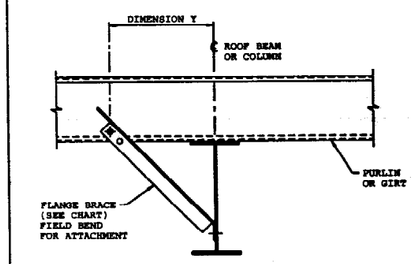
DETAIL C344 — DOUBLE FLANGE BRACE CONNECTION
USE FIRST HOLE AT PURLIN OR GIRT CONNECTION



PART USAGE
8 OR 9-1/2 PURLINS
8 Z-GIRTS

USE (2) 1/2" X 1 1/4" HEX. BOLT (095085) AND 1/2" HEX. NUT (095032)

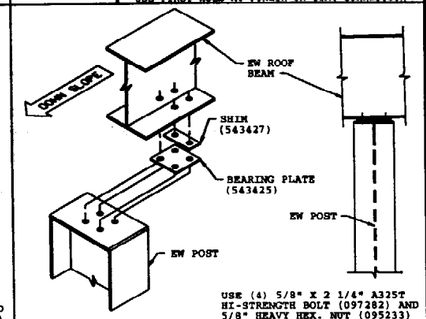
DETAIL C346 — SINGLE FLANGE BRACE CONNECTION
USE FIRST HOLE AT PURLIN OR GIRT CONNECTION



PART USAGE
8 OR 9-1/2 PURLINS
8 Z-GIRTS

USE (2) 1/2" X 1 1/4" HEX. BOLT (095085) AND 1/2" HEX. NUT (095032)

DETAIL C347 — SINGLE FLANGE BRACE CONNECTION
USE SECOND HOLE AT PURLIN OR GIRT CONNECTION



USE (4) 5/8" X 2 1/4" A325T HI-STRENGTH BOLT (097282) AND 5/8" HEAVY HEX. NUT (095233)

DETAIL E159 — ENDWALL POST TOP CONNECTION
1/2:12 SLOPE (.375 THRU .625)

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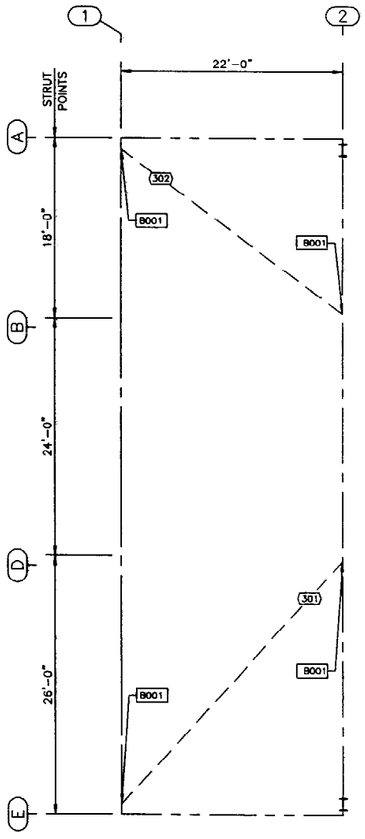
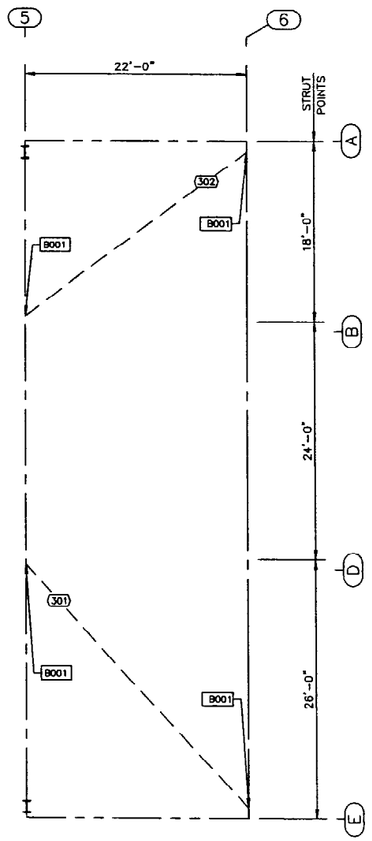
REVISION NO. 1		REVISION NO. 2								
DATE:		DATE:		BUTLER MANUFACTURING COMPANY GENERAL OFFICES-KANSAS CITY, MISSOURI	PRODUCT OF THE PROMOD® ORDER ENGINEERING SYSTEM	BUILDER: M.H. SANBORN, INC. D/B/ ROWLEY, MA	PROJECT: TIME WARNER PROJECT- CUMBERLAND, MAINE	BUILDING ORDER DESCRIPTION: 68X110X14 MRSS 49# Roof SL + 5#CLL 90 MPH EXP B NBC 99	DRAWING TITLE: CROSS SECTION ERECTION DRAWING DETAIL SHEET	DRW: BASCANLAN CHK: ENG: BAS DATE: 06/25/03 DRAWING NUMBER: D03-104605-02D REV. 00
DRAWN BY:		DRAWN BY:								
CHECKED BY:		CHECKED BY:								



UNIT 1 ROOF BRACING



UNIT 1 ROOF BRACING

BRACE ROD SCHEDULE				
⬭	PART NUMBER	DESCRIPTION	THREAD	TURNBUCKLE
301	544048 -394	5/8 RH-RH BR ROD	RH-RH	NONE
302	544048 -326	5/8 RH-RH BR ROD	RH-RH	NONE

STANDARD NOTES:

WB0003 RED THREAD PROTECTORS INDICATE 5/8" BRACE RODS

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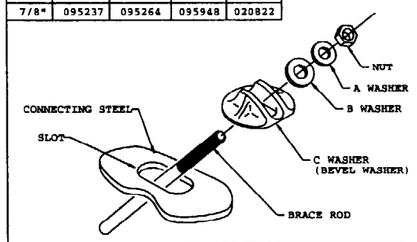
REVISION NO. 1		REVISION NO. 2	
DATE:		DATE:	
DRAWN BY:		DRAWN BY:	
CHECKED BY:		CHECKED BY:	

BUTLER MANUFACTURING COMPANY
GENERAL OFFICES-KANSAS CITY, MISSOURI

PRODUCT OF THE PRONTO® ORDER ENGINEERING SYSTEM

BUILDER:	PROJECT:	BUILDING ORDER DESCRIPTION:	DRAWING TITLE:	DRW: BASCANLAN	ENG: BAS
M.H. SANBORN, INC. D/B/ ROWLEY, MA	TIME WARNER PROJECT- CUMBERLAND, MAINE	68X110X14 MRSS 49# Roof SL + 5#CLL 90 MPH EXP B NBC 99	WIND BRACING DRAWING	CHK:	DATE: 06/23/03
				DRAWING NUMBER: D03-104605-03	REV. 00

ROD DIA.	NUT	WASHERS		
		A	B	C
1/2"	095032	095133	095165	020821
5/8"	095231	095298	095165	020821
3/4"	095235	095264	095948	020822
7/8"	095237	095264	095948	020822



DETAIL B001 — TYPICAL BEVEL WASHER CONN.

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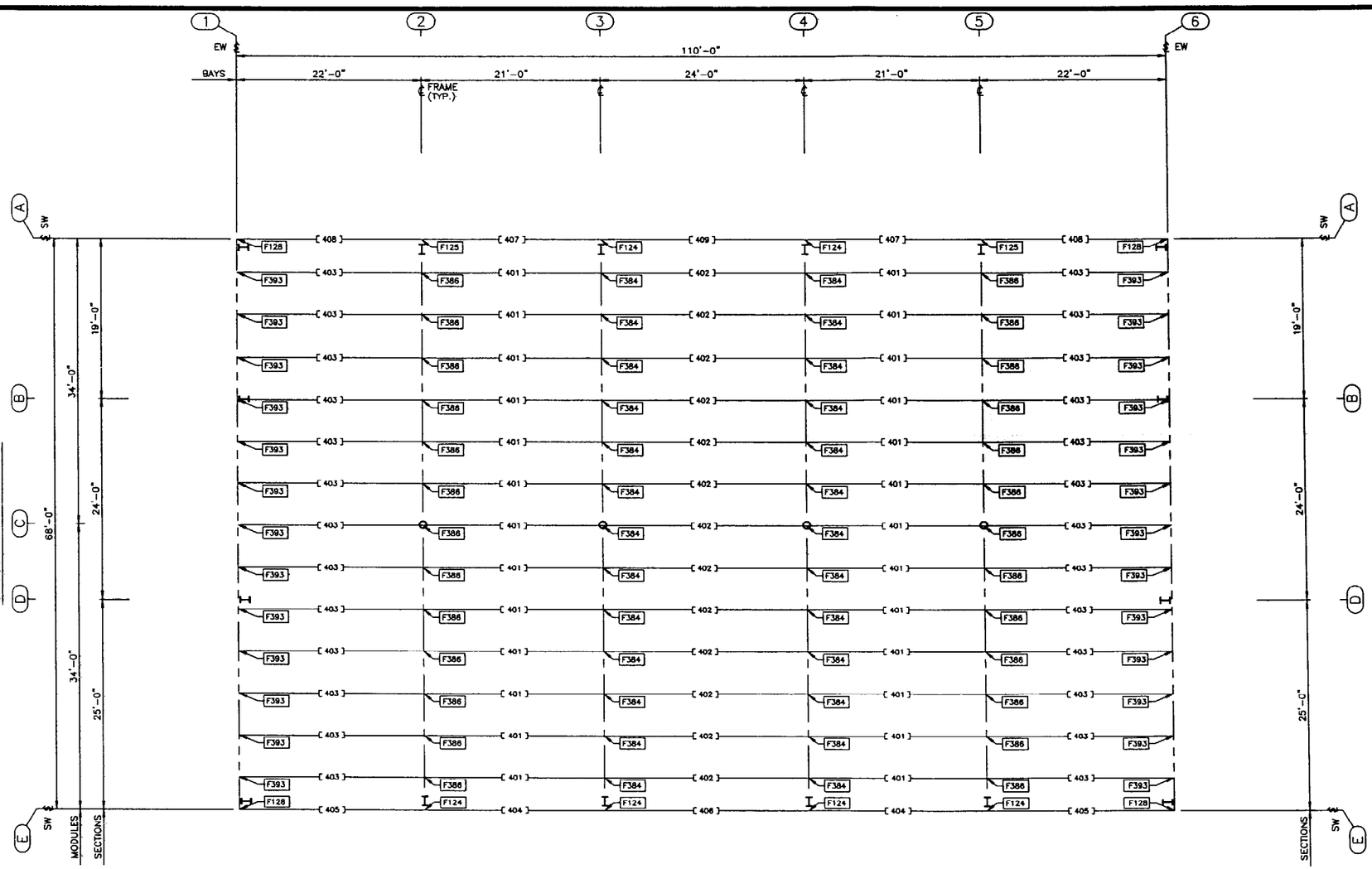
REVISION NO. 1	REVISION NO. 2		BUILDER:	PROJECT:	BUILDING ORDER DESCRIPTION:	DRAWING TITLE:	DRW: BASCANLAN	ENG: BAS
DATE:	DATE:	BUTLER MANUFACTURING COMPANY GENERAL OFFICES-KANSAS CITY, MISSOURI PRODUCT OF THE PROMO® ORDER ENGINEERING SYSTEM	M.H. SANBORN, INC. D/B/ ROWLEY, MA	TIME WARNER PROJECT- CUMBERLAND, MAINE	68X110X14 MRSS 49# Roof SL + 5#CLL 90 MPH EXP B NBC 99	WIND BRACING DETAIL SHEET	CHK:	DATE: 06/23/03
DRAWN BY:	DRAWN BY:						DRAWING NUMBER: D03-104605-03A	REV. 00
CHECKED BY:	CHECKED BY:							



UNIT 1 ROOF SECONDARY STRUCTURAL FRAMING PLAN

PART SCHEDULE

[ ]	PART NAME	PART NUMBER	PART LENGTH	FIELD WORK ▲
401	PUR P 9.5X.098 2CP	530903 287-4	23'-11 1/2"	
402	PUR P 9.5X.098 4CP		26'-11 1/2"	
403	PUR P 9.5X.098 4CP	530909 299-4	24'-11 1/2"	
404	LS ES P 9.5X.087	540507 251-4	20'-11 1/2"	
405	LS ES P 9.5X.087	540507 263-4	21'-11 1/2"	
406	LS ES P 9.5X.087	540507 287-4	23'-11 1/2"	
407	HS ES P 9.5X.087	540511 251-4	20'-11 1/2"	
408	HS ES P 9.5X.087	540511 263-4	21'-11 1/2"	
409	HS ES P 9.5X.087	540511 287-4	23'-11 1/2"	

PURLIN BRACE LOCATION SCHEDULE

BETWEEN GRIDS	LOCATION DETAIL	BRACE SECTION TOP	BRACE SECTION BOTTOM	RIDGE BRACE
1 AND 2	F444	(3) B	(2) A	(0)
2 AND 3	F439	(2) B	-0-	(0)
3 AND 4	F440	(2) B	(2) A	(0)
4 AND 5	F439	(2) B	-0-	(0)
5 AND 6	F444	(3) B	(2) A	(0)

STANDARD NOTES:

RS0002 REFER TO DRAWING B-1080945 FOR PURLIN BRACE PART NUMBERS AND TYPICAL INSTALLATION FOR SLOPE LESS THAN OR EQUAL TO 0.75:12.

RS0028 1/2" DIA. BOLTS AND NUTS ARE FURNISHED AS AN ASSEMBLY. THE DRAWINGS CALL OUT THESE BOLTS AND NUTS BY THEIR COMPONENT NO'S.

ASSEMBLY	CONTAINS
0097461	1/2X1-1/4 BOLT (0095085) AND NUT (0095032)
0097462	1/2X1-1/4 THN HD BOLT (0096636) AND NUT (0095032)
0097463	1/2X1-1/2 BOLT (0095195) AND NUT (0095032)
0097464	1/2X1-3/4 GALV BOLT (0095331) AND (0095032)
0097465	1/2X2 A325T BOLT (0097280) AND (0095230)

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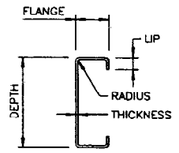
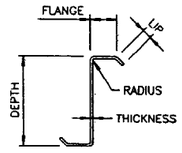
REVISION NO. 1		REVISION NO. 2								
DATE:		DATE:		BUTLER MANUFACTURING COMPANY GENERAL OFFICES-KANSAS CITY, MISSOURI	PRODUCT OF THE PROMIS® ORDER ENGINEERING SYSTEM	BUILDER: M.H. SANBORN, INC. D/B/ ROWLEY, MA	PROJECT: TIME WARNER PROJECT- CUMBERLAND, MAINE	BUILDING ORDER DESCRIPTION: 68X110X14 MRSS 49# Roof SL + 5#CLL 90 MPH EXP B NBC 99	DRAWING TITLE: ROOF SECONDARY STRUCTURAL FRAMING PLAN	DRW: BASCANLAN ENG: BAS CHK: DATE: 06/24/03 DRAWING NUMBER: D03-104605-04 REV. 00
DRAWN BY:		DRAWN BY:								
CHECKED BY:		CHECKED BY:								



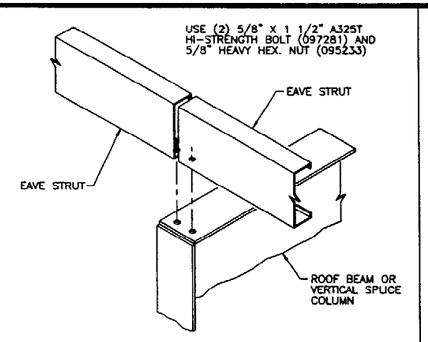
C – SECTION DESIGN INFORMATION

PART	DEPTH	FLANGE	LIP	THICKNESS	RADIUS
HS ES P 9.5X.087 540511	9 1/2"	3 1/2"	1 1/8"	.087"	5/16"
LS ES P 9.5X.087 540507	9 1/2"	3 1/2"	1 1/8"	.087"	5/16"

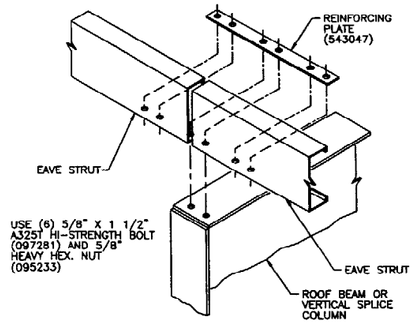


Z – SECTION DESIGN INFORMATION

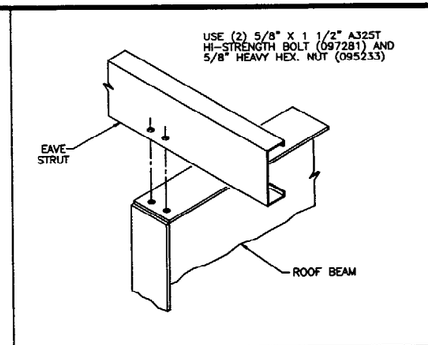
PART	DEPTH	FLANGE	LIP	THICKNESS	RADIUS
PUR P 9.5X.082 2CP 530902	9 1/2"	2 3/4"	1 1/8"	.082"	1/4"
PUR P 9.5X.082 4CP 530908	9 1/2"	2 3/4"	1 1/8"	.082"	1/4"



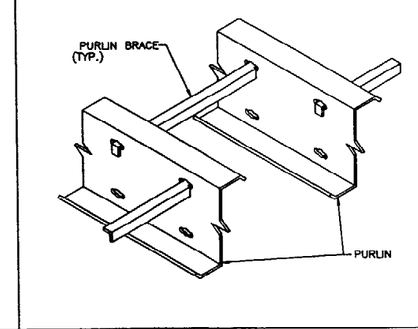
DETAIL F124 — EAVE STRUT CONN. TO INTERM. FRAME



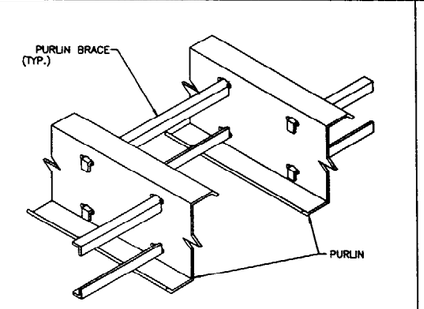
DETAIL F125 — EAVE STRUT CONN. TO INTERM. FRAME
STRUT CONNECTION



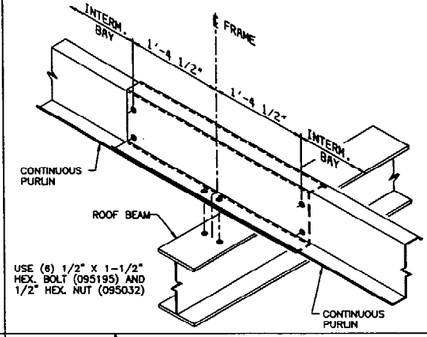
DETAIL F128 — EAVE STRUT CONN. TO ENDWALL FRAME



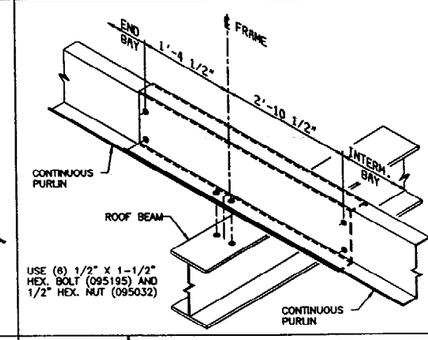
DETAIL F152 — PURLIN BRACE
TOP ROW



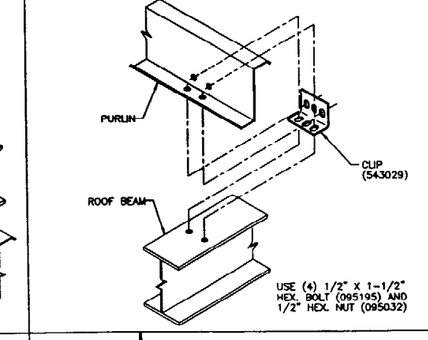
DETAIL F154 — PURLIN BRACE
TOP AND BOTTOM ROW



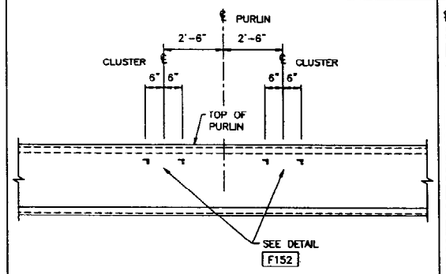
DETAIL F384 — CONTINUOUS SPAN PURLIN CONNECTION
INTERMEDIATE LOCATION – 9-1/2" PURLIN



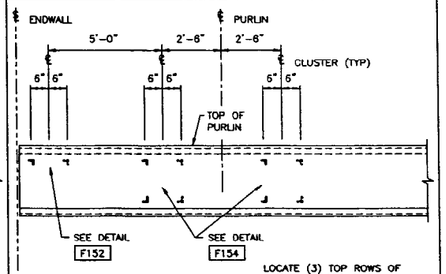
DETAIL F386 — CONTINUOUS SPAN PURLIN CONNECTION
END BAY LOCATION – 9-1/2" PURLIN



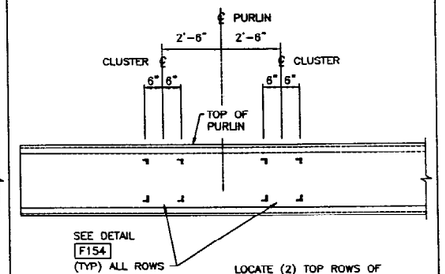
DETAIL F393 — PURLIN CONNECTION TO EW ROOF BEAM



DETAIL F439 — PURLIN BRACE LAYOUT
(2) ROWS TOP – (0) ROWS BOTTOM



DETAIL F444 — PURLIN BRACE LAYOUT
(3) ROWS TOP – (2) ROWS BOTTOM



REV. DATE: 03/08/01 REV. NO. 08
DETAIL F440 — PURLIN BRACE LAYOUT
(2) ROWS TOP – (2) ROWS BOTTOM

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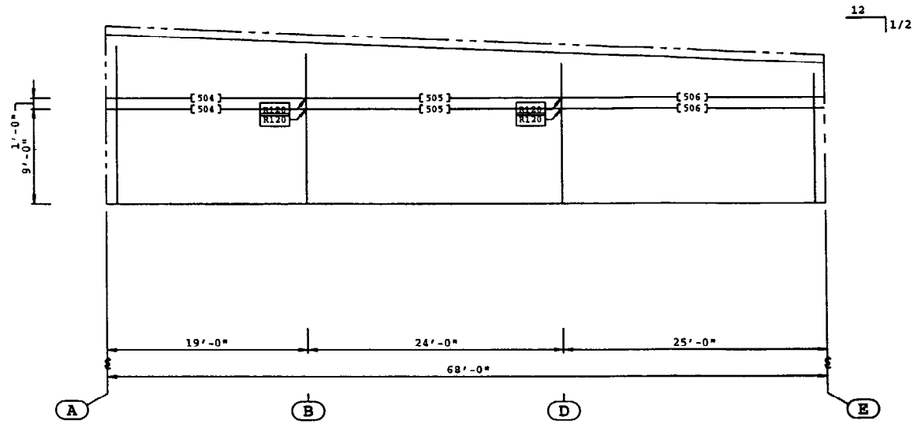
NOT FOR ERECTION

USERNAME: baconlon 06-23-2003 12:52:59 O:\Orders\104605\Enecdwg\104605-4a.dwg

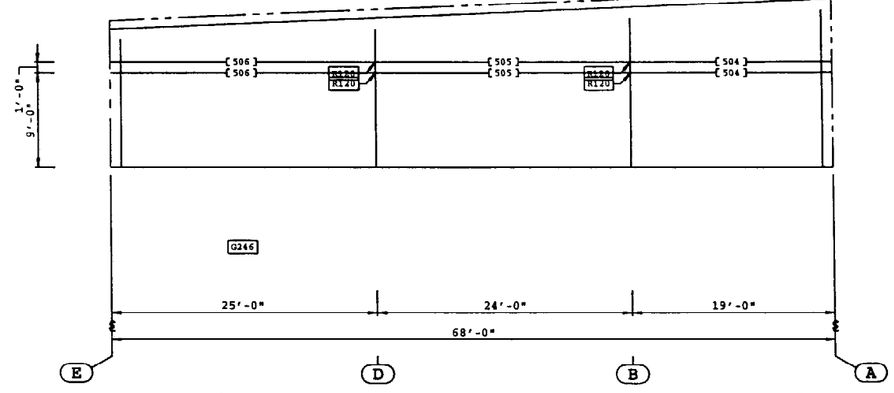
REVISION NO. 1		REVISION NO. 2								
DATE:		DATE:		BUTLER MANUFACTURING COMPANY GENERAL OFFICES-KANSAS CITY, MISSOURI	PRODUCT OF THE TRONO® ORDER ENGINEERING SYSTEM	BUILDER: M.H. SANBORN, INC. D/B/ ROWLEY, MA	PROJECT: TIME WARNER PROJECT– CUMBERLAND, MAINE	BUILDING ORDER DESCRIPTION: 68X110X14 MRSS 49# Roof SL + 5#CLL 90 MPH EXP B NBC 99	DRAWING TITLE: ROOF SECONDARY STRUCTURAL DETAIL SHEET	DRW: BASCANLAN ENG: BAS CHK: DATE: 06/23/03 DRAWING NUMBER: D03-104605-04A REV. 00
DRAWN BY:		DRAWN BY:								
CHECKED BY:		CHECKED BY:								

FOR GABLE ANGLE
INSTALLATION SEE
DRAWING 1080944



UNIT 1 FRONT EW STRUCTURAL ELEVATION AT GRID 1

FOR GABLE ANGLE
INSTALLATION SEE
DRAWING 1080944



UNIT 1 REAR EW STRUCTURAL ELEVATION AT GRID 6

PART SCHEDULE				
[ ]	PART NAME	PART NUMBER	PART LENGTH	FIELD WORK ▲
504	CEE GRT UP 8X.113	702272 227-4	18'-11 1/2"	
505	CEE GIRT 8 x .113	~~0800123~~ 287-4	23'-11 1/2"	
506	CEE GIRT 8 x .113	~~0800123~~ 299-4	24'-11 1/2"	

STANDARD NOTES:

WS0023 I M P O R T A N T
****
REFER TO PRONTO DETAIL G047 FOR PROPER GIRT ALIGNMENT.

WS0026 1/2" DIA. BOLTS AND NUTS ARE FURNISHED AS AN ASSEMBLY. THE DRAWINGS CALL OUT THESE BOLTS AND NUTS BY THEIR COMPONENT NO'S.

ASSEMBLY	CONTAINS	
0097461	1/2X1-1/4 BOLT (0095085) AND NUT (0095032)	
0097462	1/2X1-1/4 THN HD BOLT (0096636) AND NUT (0095032	1
0097463	1/2X1-1/2 BOLT (0095195) AND NUT (0095032)	
0097464	1/2X1-3/4 GALV BOLT (0095331) AND (0095032)	
0097465	1/2X2 A325T BOLT (0097280) AND (0095230)	

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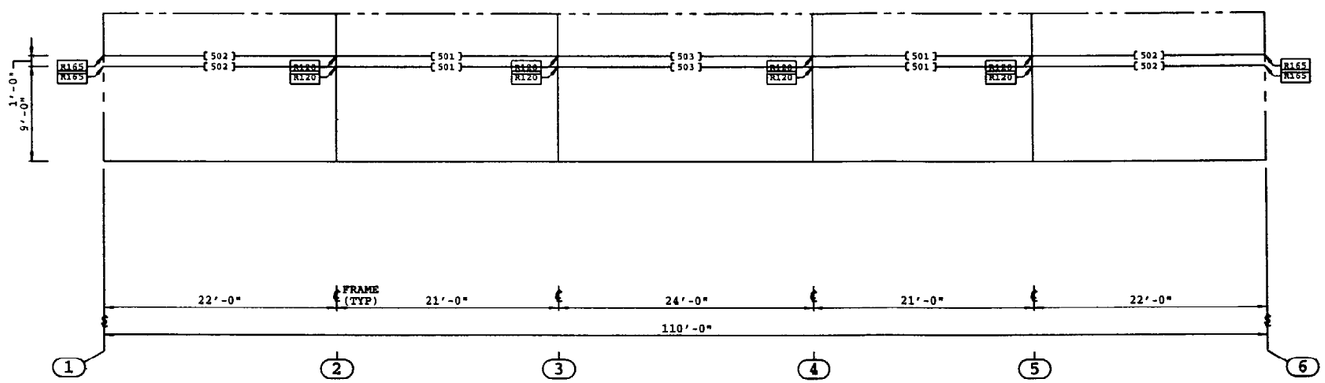
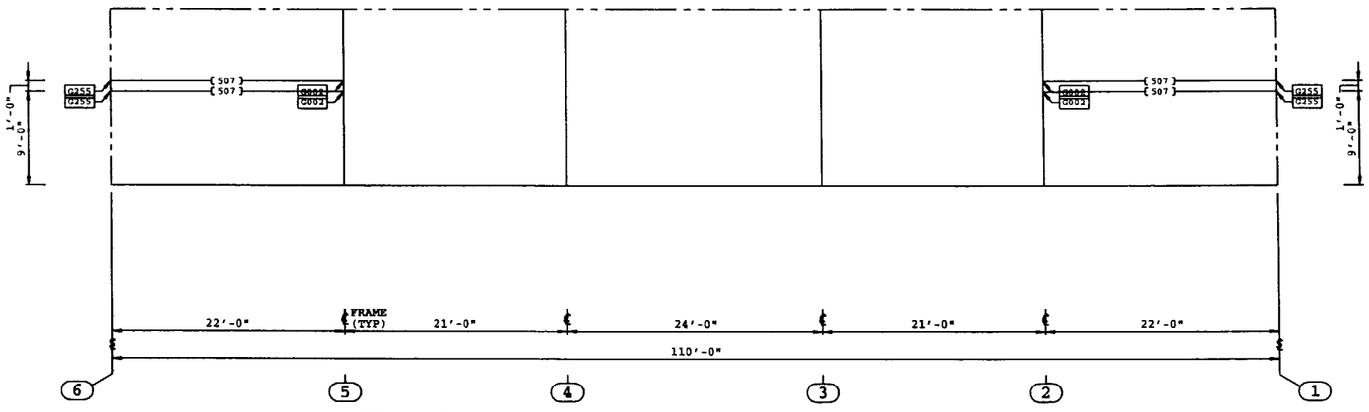
REVISION NO. 1	REVISION NO. 2							
DATE:	DATE:	BUTLER MANUFACTURING COMPANY GENERAL OFFICES-KANSAS CITY, MISSOURI	PRODUCT OF THE PRONTO® ORDER ENGINEERING SYSTEM	BUILDER:	PROJECT:	BUILDING ORDER DESCRIPTION:	DRAWING TITLE:	DRW: BASCANLAN / ENG: BAS
DRAWN BY:	DRAWN BY:			M.H. SANBORN, INC. D/B/ ROWLEY, MA	TIME WARNER PROJECT- CUMBERLAND, MAINE	68X110X14 MRSS 49# Roof SL + 5#CLL 90 MPH EXP B NBC 99	WALL SECONDARY STRUCTURAL ELEVATION	CHK: / DATE: 06/24/03
CHECKED BY:	CHECKED BY:						DRAWING NUMBER: D03-104605-05	REV. 00

PART SCHEDULE				
[ ]	PART NAME	PART NUMBER	PART LENGTH	FIELD WORK ▲
501	CEE GRT UP 8X.096	702271 251-4	20'-11 1/2"	
502	CEE GRT UP 8X.096	702271 263-4	21'-11 1/2"	
503	CEE GRT UP 8X.096	702271 287-4	23'-11 1/2"	
507	Z GIRT 8X113	GT80113 263-4	21'-11 1/2"	



UNIT 1 RIGHT SW STRUCTURAL ELEVATION AT GRID E



UNIT 1 LEFT SW STRUCTURAL ELEVATION AT GRID A

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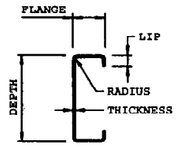
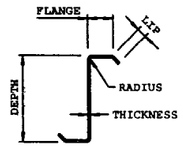
REVISION NO. 1		REVISION NO. 2		BUTLER MANUFACTURING COMPANY GENERAL OFFICES-KANSAS CITY, MISSOURI	PRODUCT OF THE PRO/TO® ORDER ENGINEERING SYSTEM	BUILDER:	PROJECT:	BUILDING ORDER DESCRIPTION:	DRAWING TITLE:	DRW: BASCANLAN	ENG: BAS
DATE:		DATE:				M.H. SANBORN, INC. D/B/ ROWLEY, MA	TIME WARNER PROJECT- CUMBERLAND, MAINE	68X110X14 MRSS 49# Roof SL + 5#CLL 90 MPH EXP B NBC 99	WALL SECONDARY STRUCTURAL ELEVATION	CHK:	DATE: 06/24/03
DRAWN BY:		DRAWN BY:								DRAWING NUMBER:	REV.
CHECKED BY:		CHECKED BY:								D03-104605-05A	00



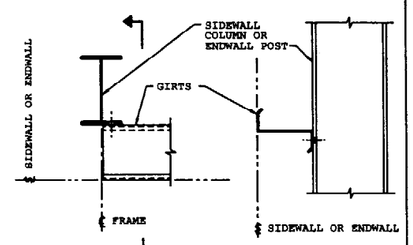
C - SECTION DESIGN INFORMATION

PART	DEPTH	FLANGE	LIP	THICKNESS	RADIUS
CEE GRT UP 8X.096 702271	8"	3 1/2"	1 3/8"	.096"	9/32"
CEE GRT UP 8X.113 702272	8"	3 1/2"	1 3/8"	.113"	9/32"
CEE GIRT CG80113	8"	3 1/2"	1 3/8"	.113"	9/32"

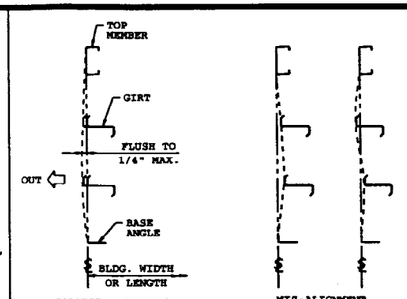


Z - SECTION DESIGN INFORMATION

PART	DEPTH	FLANGE	LIP	THICKNESS	RADIUS
GIRT GT80113	8"	2 5/8"	1 1/8"	.113"	1/4"

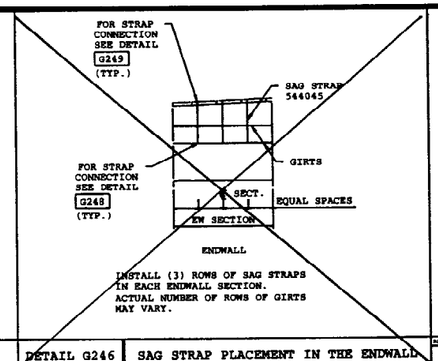


DETAIL G002 | SIMPLE SPAN GIRT CONNECTION (SINGLE)

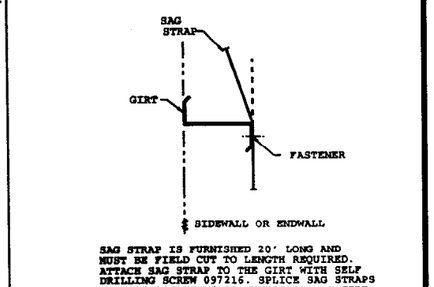


REV. DATE:02/07/03 REV. NO. 02

DETAIL G047 | SECONDARY STRUCTURAL ALIGNMENT

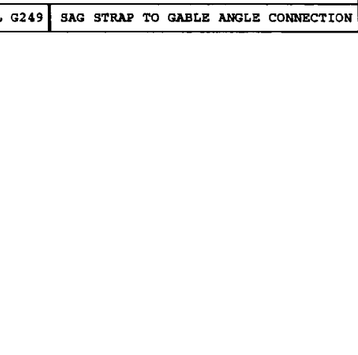


DETAIL G246 | SAG STRAP PLACEMENT IN THE ENDWALL

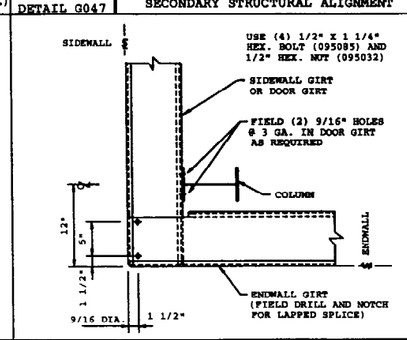


REV. DATE:02/07/03 REV. NO. 03

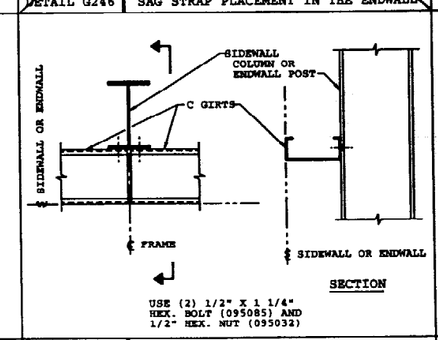
DETAIL G248 | SAG STRAP TO GIRT CONNECTION



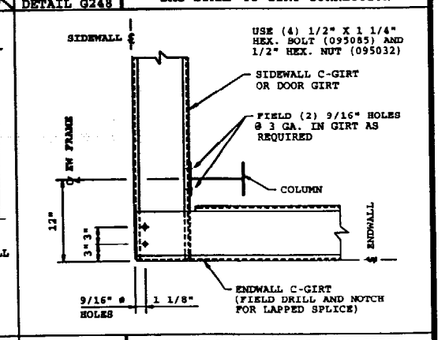
DETAIL G249 | SAG STRAP TO GABLE ANGLE CONNECTION



DETAIL G255 | 8" SW & 8" EW GIRT TO CORNER POST 12" SETBACK ENDWALL



DETAIL R120 | 8" C-GIRTS (DOUBLE) TO COLUMN INSET SW OR EW



DETAIL R165 | 8" SW C-GIRT TO 8" EW C-GIRT 12" SETBACK ENDWALL

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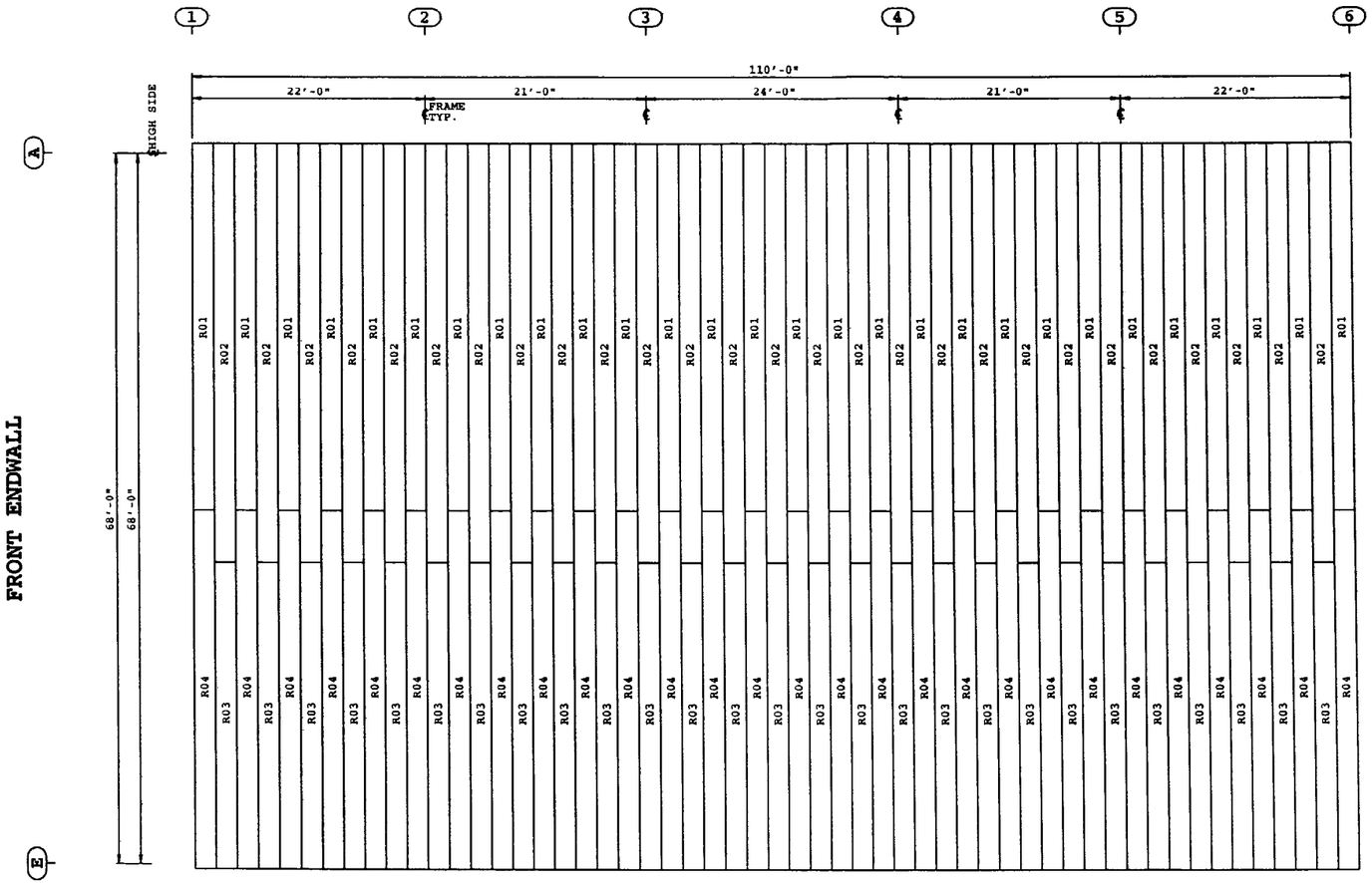
REVISION NO. 1	REVISION NO. 2							
DATE:	DATE:	BUTLER MANUFACTURING COMPANY GENERAL OFFICES-KANSAS CITY, MISSOURI	PRODUCT OF THE PRO/TD® ORDER ENGINEERING SYSTEM	BUILDER: M.H. SANBORN, INC. D/B/ ROWLEY, MA	PROJECT: TIME WARNER PROJECT- CUMBERLAND, MAINE	BUILDING ORDER DESCRIPTION: 68X110X14 MRSS 49# Roof SL + 5#CLL 90 MPH EXP B NBC 99	DRAWING TITLE: WALL SECONDARY STRUCTURAL ELEVATION DETAIL SHEET	DRW: BASCANLAN CHK: DRAWING NUMBER: D03-104605-05B	ENG: BAS DATE: 06/24/03 REV. 00
DRAWN BY:	DRAWN BY:								
CHECKED BY:	CHECKED BY:								



ROOF PANEL SCHEDULE					
ID	PART NUMBER	SUFF.	LENGTH	DESCRIPTION	HOLE TO HOLE
R01	560105	643	415-1	MR24 EAVE PANEL ALZN	33'-11 3/16"
R02	560105	643	475-2	MR24 EAVE PANEL ALZN	38'-11 1/4"
R03	560118	643	358-6	MR24 EAVE PANEL ALZN	28'-10 23/32"
R04	560118	643	418-6	MR24 EAVE PANEL ALZN	33'-10 25/32"

STANDARD NOTES:

RP0031 REFER TO GENERAL ROOF INDEX DRAWING 1081111 FOR ADDITIONAL ERECTION DRAWING REQUIREMENTS.

REFER TO DRAWING 1080876 WHEN FIELD WORK IS REQUIRED FOR VARIABLE-WIDTH ROOF PANELS.

RP0038 W A R N I N G
****
PANELS WITH PROTECTIVE OIL COATING ARE SLIPPERY. PROCEED WITH CAUTION. WIPE CLEAN IF NECESSARY.

RP0047 PANEL CLIP FASTENERS
****
USE SCRUBOLT 097196 (3/8 X 1) FOR PANEL CLIP TO PURLIN CONNECTIONS.

RP0055 DIRECTION OF ERECTION FOR MR-24 PANELS ON RIGHT SLOPE IS REAR TO FRONT OF ROOF SURFACE.

RP0062 ROOF PANELS MUST BE ATTACHED TO ALL ROOF STRUCTURALS TO INSURE THE STRUCTURAL INTEGRITY OF THE ROOF. THIS INCLUDES ALL PURLINS ADDED FOR SPECIFIC LOADING CONDITIONS, ETC.

ENOUGH PANEL-TO-STRUCTURAL FASTENERS, CLIPS (IF MR-24/CMR-24) HAVE BEEN FURNISHED FOR ALL ROOF STRUCTURALS.

RP0073 HOLE TO HOLE DIMENSION IN THE ROOF PANEL SCHEDULE IS NORMALLY THE DISTANCE BETWEEN THE STRUCTURAL ATTACHMENTS AT EACH END OF ROOF PANEL (SEE DWG 000001). THIS MAY NOT BE TRUE FOR CUSTOM PANEL CONDITIONS.

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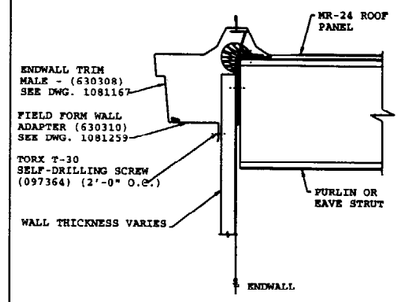
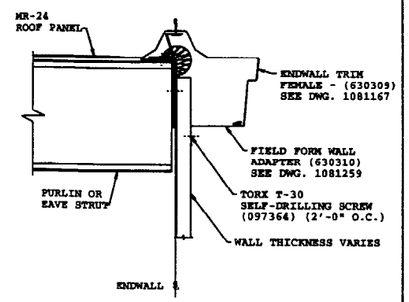
REVISION NO. 1	REVISION NO. 2							
DATE:	DATE:	BUTLER MANUFACTURING COMPANY GENERAL OFFICES-KANSAS CITY, MISSOURI	PRODUCT OF THE PRONTO® ORDER ENGINEERING SYSTEM	BUILDER: M.H. SANBORN, INC. D/B/ ROWLEY, MA	PROJECT: TIME WARNER PROJECT- CUMBERLAND, MAINE	BUILDING ORDER DESCRIPTION: 68X110X14 MRSS 49# Roof SL + 5#CLL 90 MPH EXP B NBC 99	DRAWING TITLE: ROOF PANEL DRAWING	DRW: BASCANLAN CHK: DRAWING NUMBER: D03-104605-07
DRAWN BY:	DRAWN BY:							ENG: BAS DATE: 06/24/03 REV. 00
CHECKED BY:	CHECKED BY:							



DETAIL K391 — GABLE TRIM LEFT WITH MR-24 ROOF VARIABLE THICKNESS WALL ADAPTER



DETAIL K392 — GABLE TRIM RIGHT WITH MR-24 ROOF VARIABLE THICKNESS WALL ADAPTER

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REVISION NO. 1	REVISION NO. 2							
DATE:	DATE:	BUTLER MANUFACTURING COMPANY GENERAL OFFICES-KANSAS CITY, MISSOURI	PRODUCT OF THE PRONTO® ORDER ENGINEERING SYSTEM	BUILDER: M.H. SANBORN, INC. D/B/ ROWLEY, MA	PROJECT: TIME WARNER PROJECT- CUMBERLAND, MAINE	BUILDING ORDER DESCRIPTION: 68X110X14 MRSS 49# Roof SL + 5#CLL 90 MPH EXP B NBC 99	DRAWING TITLE: EDGE OF ROOF DETAIL SHEET	DRW: BASCANLAN ENG: RAS CHK: DATE: 06/24/03 DRAWING NUMBER: D03-104605-07A REV. 00
DRAWN BY:	DRAWN BY:							
CHECKED BY:	CHECKED BY:							