## STRUCTURAL ANALYSIS REPORT

For

ME 5023 (LTE-3C)<br>BRADLEY'S CORNER<br>1050 Westbrook Street<br>Portland, ME 04102

## Antennas Mounted on Non-Penetrating Ballast Frames on Roof; Equipment Area at Ground Level



Prepared for:

## SAT

Dated: April 8, 2015

Prepared by:

Design Groupluc

## SCOPE OF WORK:

Hudson Design Group LLC (HDG) has been authorized by AT\&T to conduct a structural evaluation of the structure that will support the existing AT\&T equipment located in the areas depicted in the latest HDG's construction drawings.

This report represents this office's findings, conclusions and recommendations pertaining to the support of AT\&T's proposed equipment.

## CONCLUSION SUMMARY:

Building plans were not available and could not be obtained for our use. A limited visual survey of the structure was completed in or near the areas of the proposed work.

Based on our evaluation, we have determined that the existing structure IS CAPABLE of supporting the proposed equipment loading with the following modifications:

- Reconfigure ballast blocks as to meet the requirements shown below:

| MINIMUM BALLAST REQUIREMENTS (ALPHA/BETA) |  |  |
| :---: | :---: | :---: |
| SIDE | A (Back) | B (Front) |
| BALLAST <br> REQUIRED | 848 lbs. | 273 lbs. |
| TOTAL BALLAST <br> WEIGHT | 1121 lbs.$$ |  |


| MINIMUM BALLAST REQUIREMENTS (GAMMA) |  |  |
| :---: | :---: | :---: |
| SIDE | A (Back) | B (Front) |
| BALLAST <br> REQUIRED | 781 lbs. | 227 lbs. |
| TOTAL BALLAST <br> WEIGHT | 1008 lbs.$$ |  |

## APPURTENACE/EQUIPMENT CONFIGURATION:

(3) HPA-65R-BUU-H8 Antennas (92.4"x14.8"x7.4" Wt. = 73 lbs . /each) (One per sector)
(3) RRH (RRUS-12) (20.4" $\times 18.5$ " $\times 7.5$ " - Wt. $=58 \mathrm{lbs}$. /each) (One per sector)
(3) A2 Module ( 16.4 " $\times 15.2$ " $\times 3.4$ " - Wt. $=22$ lbs. /each) (One per sector)
(3) Surge Arrestors ( 24 " $\times 9.7$ " $\varnothing$ - Wt. $=32.8 \mathrm{lbs}$. /each) (One per sector)
(6) Powerwave 7770 Antennas ( 55 "xl 1 "x5" Wt. $=35 \mathrm{lbs}$. /each) (Two per sector)
(2) P65-17-XLH-RR Antennas ( $96 " \times 12$ "x6" Wt. $=70 \mathrm{lbs} . /$ each) (Alpha \& Beta sector)
(1) AM-X-CD-16-65-00T-RET Antenna ( 72 "xl $1.8 " \times 5.9$ " Wt. $=49$ lbs.) (Gamma sector)
(6) RRH (RRUS-11) (19.69"x16.97"x7.17" - Wt. = 50.7 lbs. /each) (Two per sector)

## DESIGN CRITERIA:

1. International Building Code (IBC) 2009, and ASCE 7-05 (Minimum Design Loads for Buildings and Other Structures).

Wind Analysis:

| Reference Wind Speed: | 100 mph | (FIG 6-1C; ASCE 7-05) |
| :--- | :--- | :--- |
| Category: | B | (Section 6.5.6.3; ASCE 7-05) |

Roof:

| Ground Snow, $\mathrm{Pg}:$ | 60 psf | (FIG 7-1; ASCE 7-05) |
| :--- | :--- | :--- |
| Importance Factor, I: | 1.0 | (Category II) |
| Exposure Factor, Ce: | 1.0 | (Exp. B- Partially Exposed) |
| Thermal Factor, Ct: | 1.0 | (Typical Structure) |
| Flat Roof Snow Load: | $\mathbf{4 2}$ psf | $\left(\mathrm{Pr}_{\mathrm{f}}=0 . \mathbf{7}^{*} \mathbf{C e}^{*} \mathbf{C l}^{*}{ }^{*}{ }^{*} \mathrm{P}_{\mathrm{g}}\right.$ ) |

2. EIA/TIA -222- G Structural Standards for Steel Antenna Towers and Antenna Supporting Structures

City/Town: Portland
County: Cumberland
Wind Load: $\quad 100 \mathrm{mph}$
Nominal Ice Thickness: 1 inch
3. Approximate height above grade to the center of the Antennas:

$$
74^{\prime}-0^{\prime \prime}+/-
$$

## ANTENNA/RRH/SURGE SUPPRESSOR SUPPORT RECOMMENDATIONS:

The new antennas, RRH's and surge suppressors are proposed to be secured to the existing non-penetrating ballast frames located on the roof of the existing building.

## Limitations and assumptions:

1. Reference the latest HDG construction drawings for all the equipment locations details.
2. Mount all equipment per manufacturer's specifications.
3. All structural members and their connections are assumed to be in good condition and are free from defects with no deterioration to its member capacities.
4. All antennas, coax cables and waveguide cables are assumed to be properly installed and supported as per the manufacturer requirements.
5. HDG is under the assumption that the ballast frames are located over structurally adequate roof supports (i.e. beams, columns, or bearing walls). HDG was not able to verify the roof structure and its components at the time of our visit.
6. HDG is not responsible for any modifications completed prior to and hereafter which HDG was not directly involved.
7. If field conditions differ from what is assumed in this report, then the engineer of record is to be notified as soon as possible.

## FIELD PHOTOS:



Photo 1: Sample photo illustrating the existing antennas.


Photo 2: Sample photo illustrating the existing antennas.


## Calculations

Date: 04/08/2015
Project Number: NH5023
Project Name: Bradley's Corner
Designed By: GH Checked By: MSC

### 2.6.5.2 Velocity Pressure Coeff:

| $\mathrm{K}_{\mathrm{z}}=2.01\left(\mathrm{z} / \mathrm{z}_{\mathrm{g}}\right)^{2 / \alpha}$ | $\mathrm{z}=$ | $74(\mathrm{ft})$ |  |
| ---: | ---: | ---: | ---: |
|  | $\mathrm{z}_{\mathrm{g}}=$ | $1200(\mathrm{ft})$ |  |
| $\mathrm{K}_{\mathrm{z}}=$ | 0.907 | $\alpha=$ | 7 |

## $K z m i n \leq K z \leq 2.01$

Table 2-4

| Exposure | $\mathbf{Z}_{\mathbf{g}}$ | $\boldsymbol{\alpha}$ | $\mathbf{K}_{\text {zmin }}$ | $\mathbf{K}_{\mathbf{e}}$ |
| :---: | :---: | :---: | :---: | :---: |
| B | 1200 ft | 7 | 0.70 | 0.90 |
| C | 900 ft | 9.5 | 0.85 | 1 |
| D | 700 ft | 11.5 | 1.03 | 1.10 |

### 2.6.6.4 Topographic Factor:

Table 2-5

| Topo. Category | $\mathbf{K}_{\mathbf{t}}$ |  |  | $\mathbf{f}$ |
| :---: | :---: | :---: | :---: | :---: |
| 2 | 0.43 | 1.25 |  |  |
| 3 | 0.53 | 2 |  |  |
| 4 | 0.72 | 1.5 |  |  |

$K_{z t}=\left[1+\left(K_{e} K_{t} / K_{h)}\right]^{2}\right.$

$$
\mathrm{K}_{\mathrm{zt}}=\text { \#DIV/0! }
$$

(If Cateqory 1 then $K_{z t}=1.0$ )
Category= $\quad 1$
$K_{h}=e^{\left(f^{*} z / H\right)}$

| $\mathrm{K}_{\mathrm{h}}=$ | \#DIV/0! |
| ---: | :--- |
| $\mathrm{K}_{\mathrm{e}}=$ | 0 (from Table 2-4) |
| $\mathrm{K}_{\mathrm{t}}=$ | 0 (from Table 2-5) |
| $\mathrm{f}=$ | 0 (from Table 2-5) |
| $\mathrm{z}=$ | 74 |
| $\mathrm{H}=$ | 0 (Ht. above surrounding terrain) |
| $\mathrm{K}_{\mathrm{zt}}=$ | 1.00 |

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### 2.6.7 Gust Effect Factors

### 2.6.7.1 Self Supporting Lattice Structures

$\mathrm{Gh}=1.0$ Latticed Structures $>600 \mathrm{ft}$

Gh $=0.85$ Latticed Structures 450 ft or less
$G h=0.85+0.15[h / 150-3.0] \quad h=h t$. of structure
$h=\quad 74$
$\mathrm{Gh}=\quad 0.474$
2.6.7.2 Guyed Masts
2.6.7.3 Pole Structures
$G h=\quad 1.1$

### 2.6.7.4 Structures Supported on Other Structures

(Cantilivered tubular or latticed spines, pole, structures on buildings (ht. : width ratio >5)
$\mathrm{Gh}=$
1.35
Gh=
1.35

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### 2.6.8 Design Ice Thickness:

$\mathrm{t}_{\mathrm{iz}}=2.0 * \mathrm{t}_{\mathrm{i}}{ }^{*}{ }^{*} \mathrm{~K}_{\mathrm{iz}} *\left(\mathrm{~K}_{\mathrm{zt}}\right)^{0.35}$

$$
t_{i z}=\quad 2.17
$$

| $\mathrm{t}_{\mathrm{i}}=$ | 1 |
| ---: | ---: |
| $\mathrm{l}=$ | 1 |
| $\mathrm{~K}_{\mathrm{iz}}=$ | 1.08 |
| $\mathrm{~K}_{\mathrm{zt}}=$ | 1 |

$\mathrm{K}_{\mathrm{i} 2}=[\mathrm{z} / 33]^{0.10} \leq 1.4$

$$
K_{i 2}=\quad 1.08
$$

Calculating the weight of ice, the cross-sectional area of ice shall be determined by:
$A_{i 2}=\pi^{*} t_{i z} *\left(D_{c}+t_{i z}\right)$
Dc=
96 (in) Largest Dim of Member

$$
A_{i z}=668.69
$$

### 2.6.9 Design Wind Load:

$$
\begin{aligned}
& \mathrm{F}=\mathbf{q z *} \mathbf{G h}^{*} \text { (EPA's) } \\
& \mathrm{q}_{\mathrm{z}}=0.00256 * \mathrm{~K}_{\mathrm{z}}{ }^{*} \mathrm{~K}_{\mathrm{zt}}{ }^{*} \mathrm{~K}_{\mathrm{d}} * \mathrm{~V}_{\text {max }}{ }^{2} \\
& q_{z}=\quad 22.05
\end{aligned}
$$

Table 2-2

| Structure Type | Wind Direction Probability Factor, Kd |
| :--- | :---: |
| Latticed structures with triangular, <br> square or rectangular cross sections | 0.85 |
| Tubular pole structures, latticed <br> structures with other cross sections, <br> appurtenances. | 0.95 |

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## Determine Cf:

If lattice Structure See Manual

If Tubular Pole Structure, Use Corrected Value from Table 2.7 Below

| C <br> mph.ft | Round | 18 Sided | 16 Sided | 12 Sided | 8 Sided |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $<32$ <br> (Subcritical) | 1.2 | 1.2 | 1.2 | 1.2 | 1.2 |
| 32 to 64 <br> (Transitional) | $38.4 / \mathrm{C}^{1.0}$ | $25.8 / \mathrm{C}^{0.885}$ | $12.6 / \mathrm{C}^{0.678}$ | $2.99 / \mathrm{C}^{0.263}$ | 1.2 |
| $>64$ <br> (Supercritical) | 0.6 | 0.65 | 0.75 | 1 | 1.2 |

$\mathrm{C}=\left(\mathrm{I}^{*} \mathrm{~K}_{\mathrm{zt}}{ }^{*} \mathrm{~K}_{\mathrm{z}}\right)^{0.5} \mathrm{~V}^{*} \mathrm{D}$

Dp = Outside Diameter or Out to Out: 0.25 feet

$$
C=\quad 23.81
$$

$\mathrm{Cf}=\quad 1.2$

| Appurtenances | Height | Width | Depth | Flat Area | Force Per Appurtenance |
| :---: | :---: | :---: | :---: | :---: | :---: |
| HPA-65R Antenna | 92.4 | 14.8 | 7.4 | 9.50 | 339.3 lbs |
| 7770 Antenna | 55 | 11 | 5 | 4.20 | 150.1 lbs |
| P65-17-XLH Antenna | 96 | 12 | 6 | 8.00 | 285.8 lbs |
| AM-X-CD Antenna | 72 | 11.8 | 5.9 | 5.90 | 210.8 lbs |
| RRUS-12 | 20.4 | 18.5 | 7.5 | 2.62 | 93.6 lbs |
| RRUS-11 | 19.7 | 17 | 7.2 | 2.33 | 83.1 lbs |
| Squid | 24 | 9.7 | 9.7 | 1.62 | 57.8 lbs |

Date:
Site Number: ME5023
Site Name: Bradley's Corner
Done by:

## Calculate Total Ballast Required for Ballast Mount (ALPHA \& BETA)

## WIND FORCES

## Fantenna $=$

Frrh $=$

Antenna Height =

RRH \& Surge Height =

## Overturning at Ballast

Moment $=$

## Hold Down Force =

## Wa Ballast

Equipment

> Frame =

150 lbs.
Total Ballast Required Wa=
847.14 lbs.

## Wb Ballast

Equipment

| Frame | 300 lbs. |
| :--- | :--- |
| Antennas | 213 lbs. |
| RRH's | 212 lbs. |
|  |  |
| Total $=$ | 725 lbs. |

272.14 lbs.

Total weight of Fully Loaded Ballast Frame = 1996 lbs
Footprint Area under Ballast Frame = Area Load under Ballast Frame =

926 lbs.

318 lbs.

5 ft

3 ft

6980 lbs.-ft
S.F.
1.25

7 ft
7 ft
997.14 lbs. Per Side

Date: 4/8/2015
Site Number: ME5023
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## Calculate Total Ballast Required for Ballast Mount (GAMMA)

## WIND FORCES

F antenna =

Frh =
Antenna Height =

RRH \& Surge Height =

## Overturning at Ballast

6511.25 lbs.-ft
930.18 lbs. Per Side

## Wa Ballast

Equipment


Total Ballast Required Wa=
780.18 lbs.

## Wb Ballast

Equipment

| Frame | 300 lbs. |
| :--- | :--- |
| Antennas | 192 lbs. |
| RRH's | 212 lbs. |
|  |  |
| Total $=$ | 704 lbs. |

Total Ballast Required Wb =
226.18 lbs.

Total weight of Fully Loaded Ballast Frame =
Footprint Area under Ballast Frame = Area Load under Ballast Frame =

1862 lbs
$62.5 \mathrm{ft}^{2}$
30 psf

