Department of Planning and Urban Development
Inspection Services Division
Approved with Conditions
Date: 06/07/13

1. BUILDING CODE:

A. INTERNATIONAL BUILDING CODE - 2009 EDITION

B. ASCE 7-05 MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES

2. MINIMUM LOADING REQUIREMENTS:

A. ROOF SNOW LOADS: LOADS: (EXCEPT AT DRIFTING SNOW LOCATIONS AND THOSE LISTED BELOW)
- a. GROUND SNOW LOAD: Pg = 60.0 PSF
  - i. IMPORTANCE FACTOR: I = 1.0
  - ii. COLD ROOF SLOPE FACTOR: Cs = 1.0
  - iii. THERMAL FACTOR: Ct = 1.1
  - iv. EXPOSURE FACTOR: Ce= 1.0
  - v. TERRAIN CATEGORY: C
- b. FLAT ROOF SNOW LOAD: Pf = 46.2 PSF
- c. DRIFT - AS INDICATED ON THE DRAWINGS.

B. ROOF DEAD LOAD: 25.0 PSF.

C. ROOF LIVE LOAD:
- a. STANDARD ROOF LIVE LOAD: 20 PSF

D. FLOOR LIVE LOADS:

	UNIFORM	CONCENTRATED	PARTITION
a. OFFICE BUILDINGS			
i. LOBBIES AND 1ST FLOOR CORRIDORS	100 PSF	2,000#	
ii. OFFICES	50 PSF	2,000#	15 PSF
iii. CORRIDORS ABOVE 1ST FLOOR	80 PSF	2,000#	
b. STORAGE			
i. LIGHT	125 PSF		

E. WIND:

- a. FACTORS:
  - i. BASIC WIND SPEED: 100 MPH
  - ii. EXPOSURE CATEGORY: "C"
  - iii. IMPORTANCE FACTOR: 1.0
  - iv. BUILDING HEIGHT: <30'
- b. WIND DESIGN PRESSURE
  - i. MWFRS
    - 1. END ZONE WIDTH: 5 FEET
    - 2. TRANSVERSE
      - a. INTERIOR ZONE 15 PSF
      - b. END ZONE 22 PSF
    - 3. LONGITUDINAL
      - a. INTERIOR ZONE 15 PSF
      - b. END ZONE 22 PSF
  - ii. COMPONENTS AND CLADDING
    - 1. END ZONE WIDTH: 5 FEET
    - 2. WALLS
      - a. FIELD 21 PSF
      - b. END ZONES 21 PSF
  - iii. ROOF UPLIFT (IBC 2009)
    - 1. FIELD 23 PSF
    - 2. PERIMETER 29 PSF
    - 3. CORNERS 13 PSF
    - 4. STRIP WIDTH 5 FEET

F. SEISMIC
- a. COEFFICIENTS:
  - i. RESPONSE SPECTRAL ACC. (0.2 sec.) Ss = 0.352g
  - ii. RESPONSE SPECTRAL ACC. (1.0 sec.) S1 = 0.106g
  - iii. SOIL CLASSIFIATION: D
  - iv. SITE COEFFICIENTS: Fa = 1.52; Fv = 2.40
  - v. MAX. CONSIDERED EARTHQUAKE ACC @ 5% DAMPED DESIGN: SDS = 0.357; SD1 = 0.168
  - vi. SEISMIC DESIGN CATEGORY FOR 0.1 AND 1.0 SECONDS: C
  - vii. FUNDAMENTAL PERIOD Ta = 0.403 SEC
  - viii. SEISMIC RESPONSE COEFFICIENT Cs = 0.102
- b. DESIGN COEFFICIENTS AND FACTORS FOR SEISMIC FORCE RESISTING SYSTEMS
  - i. BEARING WALL SYSTEMS
    - 1. ORDINARY REINFORCED MASONRY SHEAR WALLS
      - a. RESPONSE MODIFICATION R = 2
      - b. SYSTEM OVERSTRENGTH FACTOR Ω0 = 2 ½
      - c. DEFLECTION AMPLIFICATION FACTOR CD = 1 ¾
  - ii. BUILDING FRAME SYSTEM
    - 1. ORDINARY MASONRY REINFORCED SHEAR WALLS
      - a. RESPONSE MODIFICATION R = 2
      - b. SYSTEM OVERSTRENGTH FACTOR Ω0 = 2 ½
      - c. DEFLECTION AMPLIFICATION FACTOR CD = 2

3. SEE ARCHITECTURAL WALL SECTIONS AND DETAILS FOR MISCELLANEOUS STEEL.

4. STRUCTURAL WOOD TO CONFORM TO THE LATEST NDS STANDARDS. PLYWOOD TO CONFORM TO THE LATEST PDS STANDARDS.

5. INSPECTION REPORTS SHALL BE FURNISHED TO THE OWNER, BUILDING OFFICIAL, ARCHITECT AND SER. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE CONTRACTOR AND IF NOT CORRECTED, SHALL BE REPORTED TO THE OWNER, BUILDING OFFICIAL, ARCHITECT AND SER. REPORTS SHALL BE OBTAINED FOR MASONRY, SOILS, CONCRETE AND WOOD CONSTRUCTION ELEMENTS.

A1 STRUCTURAL NOTES
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1. PROVIDE AND INSTALL MASONRY LINTELS FOR MASONRY WALL OPENINGS UNLESS INDICATED OTHERWISE ON DRAWINGS. PROVIDE MASONRY LINTELS OF SIZE AND REINFORCEMENT AS FOLLOWS:

   A - OPENINGS UP TO 3'-11" (UNLESS NOTED OTHERWISE): PROVIDE 8-INCH HIGH C.M.U. LINTEL W/ (2) #4 BARS IN 6-INCH WIDE UNITS (2) #4 BARS IN 8-INCH WIDE UNITS (3) #4 BARS IN 12-INCH WIDE
   B - OPENINGS 4'-0" TO 8'-0" (UNLESS OTHERWISE NOTED): PROVIDE 16-INCH HIGH C.M.U. LINTEL W/ (2) #6 BARS IN 6-INCH WIDE UNITS (2) #6 BARS IN 8-INCH WIDE UNITS (3) #6 BARS IN 12-INCH WIDE UNITS

2. CONCRETE MASONRY LINTELS SHALL HAVE 8-INCH (MIN) END BEARING UNLESS OTHERWISE NOTED.

3. CONCRETE MASONRY BLOCK WALLS WITH VERTICAL REINFORCING SHALL HAVE CORES FILLED WITH 3000 PSI CONCRETE. INSTALLATION OF REINFORCEMENT SHALL BE CONTINUOUS AND RUN UNOBSTRUCTED BY BAR JOIST SEAT/BEARING PLATE ARRANGEMENTS.

4. OMIT REBAR/GROUTING IN MASONRY CELLS WHICH SHALL RECEIVE ROOF DRAIN LEADERS, CONDUITS, ETC. REQUIRED REINFORCEMENT SHALL BE INSTALLED IN THE ADJACENT CELL AND SHALL BE GROUTED SOLID.

5. HOLLOW CONCRETE BLOCK UNITS: GRADE N, 1000 PSI, MINIMUM COMPRESSIVE STRENGTH. WALL DESIGN STRENGTH, F'M = 1500 psi.

6. LAY UNITS IN RUNNING BOND - CORNERS SHALL HAVE A STANDARD BOND BY OVERLAPPING UNITS.

7. MORTAR: TYPE S.

8. GROUT: (3000) PSI MINIMUM 28 DAY COMPRESSIVE STRENGTH. ROD GROUT IMMEDIATELY AFTER POURING AND AGAIN APPROX. 5 MINUTES LATER.

9. MAXIMUM GROUT LIFT WITHOUT CLEANOUTS SHALL NOT EXCEED 4'-0" IN BLOCK WALLS.

10. TIE VERTICAL REINFORCING AT EACH END AND AT 8'-0" MAXIMUM VERTICAL SPACING USING SINGLE WIRE AND LOOP TYPE TIES AS MANUFACTURED BY A.A. WIRE PRODUCTS COMPANY OR APPROVED EQUAL.

11. IN 8-INCH WALLS, PROVIDE VERTICAL REINFORCING IN CENTER OF GROUT, AT CENTER OF WALL, CONTINUOUS FULL HEIGHT OF WALL AS FOLLOWS:
    A - (1) #4 VERTICAL AT CORNERS, INTERSECTIONS, WALL ENDS, JAMBS AND EACH SIDE OF EXPANSION OR CONTROL JOINTS.
    B - (1) #4 VERTICAL AT 32-INCHES ON-CENTER TYPICAL. (UNLESS NOTED ON PLAN)
    C - (1) #4 VERTICAL IN EACH CORE WITHIN 12-INCHES OF WALL CORNERS.

12. PLACE BOND BEAM REINFORCING CONTINUOUS THROUGH EXPANSION AND CONTROL JOINTS, WRAPPING BARS WITH 1/8-INCH THICK BOND BREAKING TAPE 24-INCHES BOTH SIDES OF JOINT. DO NOT SPLICE BOND BEAM REINFORCING WITHIN 6'-0" OF AN EXPANSION OR CONTROL JOINT.

13. PROVIDE CONTINUOUS WIRE LATHE GROUT BARRIERS AS REQUIRED UNDER FIRST COURSE OF GROUTED (3000 PSI CONC.) CELLS.

14. WET MASONRY WALLS THOROUGHLY FOR (3) CONSECUTIVE DAYS IMMEDIATELY AFTER PLACEMENT IF TEMPERATURES ARE/WILL BE ABOVE 80°F DURING THE DAY.

15. NO EXPANSION BOLTS SHALL BE ALLOWED IN MASONRY WALLS. (CHEMICAL ANCHORS ONLY)

16. MASONRY LAID IN OUTSIDE AIR TEMPERATURES BELOW 40°F SHALL BE PROTECTED IN ACCORDANCE WITH THE PROVISIONS OF THE "IMIAWC RECOMMENDED PRACTICES AND GUIDE SPECIFICATIONS FOR COLD WEATHER MASONRY".

E4 MASONRY NOTES
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1. GENERAL:
   - A. ALL TIMBER FRAMING SHALL BE IN ACCORDANCE WITH THE AITC TIMBER CONSTRUCTION MANUAL – LATEST EDITION, AND THE AF & PA NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION (NDS) LATEST EDITION.
   - B. EACH PIECE OF LUMBER SHALL BE "S-DRY" AND BEAR THE GRADE STAMP OF A GRADING RULES AGENCY APPROVED BY THE AMERICAN LUMBER STANDARDS COMMITTEE.
   - C. DO NOT NOTCH OR DRILL JOISTS, BEAMS OR LOAD BEARING STUDS WITHOUT APPROVAL.
2. CONNECTIONS:
   - A. FOLLOW TABLE 2304.9.1 OF THE 2009 INTERNATIONAL BUILDING CODE FOR NAILING REQUIREMENTS NOT INDICATED ON THESE DRAWINGS.
   - B. NAIL ROOF PLYWOOD WITH 10D COMMON NAILS AT 6" O.C. AT ALL EDGES AND BOUNDARY MEMBERS AND 10" O.C. AT INTERMEDIATE SUPPORTS.
   - C. PROVIDE METAL HANGERS BY SIMPSON STRONGTIE AT ALL FLUSH FRAMING CONDITIONS.
3. STRUCTURAL SAWN LUMBER:
   - A. 2 X 6 THRU 2 X 14 JOISTS: SPRUCE PINE FIR NO. 1 WITH FB (REPETITIVE) = 1200 P.S.I.
4. PLYWOOD:
   - A. ROOF SHEATHING: C-D INT-APA (PS 1-95) WITH EXTERIOR GLUE; 5/8" WITH IDENTIFICATION INDEX 24/0. LAY UP WITH FACE GRAIN PERPENDICULAR TO SUPPORTS. STAGGER JOINTS.
   - B. EACH PLYWOOD PIECE TO BE CONTINUOUS OVER A MINIMUM OF TWO SPANS WITH A MINIMUM WIDTH OF 1'-0" UNLESS BLOCKING IS PROVIDED AT ALL JOINTS.

C4 WOOD NOTES
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±	PLUS OR MINUS	EW	EACH WAY	NTS	NOT TO SCALE
AFF	ABOVE FINISH FLOOR	Vu	FACTORED SHEAR	#, No.	NUMBER
ALT	ALTERNATE	FFE	FINISH FLOOR ELEVATION	O.C.	ON CENTER
ACI	AMERICAN CONCRETE INSTITUTE	FLR	FLOOR (ing)	OPNG	OPENING
APA	AMERICAN PLYWOOD ASSOC.	FD	FLOOR DRAIN	PSF	POUNDS PER SQUARE FOOT
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS	FDN	FOUNDATION	PSI	POUNDS PER SQUARE INCH
		FOB	FACE OF BRICK	PL	PLATE
AWS	AMERICAN WELDING SOCIETY	FOC	FACE OF CONCRETE	PWD	PLYWOOD
AB	ANCHOR BOLT	FOM	FACE OF MASONRY	PLF	POUNDS PER LINEAL FOOT
ARCH	ARCHITECTURAL	FOS	FACE OF STUDS	P.T.	PRESSURE-TREATED
@	AT	Ft	FOOT	REINF	REINFORCING
BPL	BEARING PLATE	FTG	FOOTING	REV	REVISION (s) (ed)
Fb	BENDING STRESS	FV	FIELD VERIFY	RD	ROOF DRAIN
BOT	BOTTOM	GALV	GALVANIZED	Fv	SHEAR STRESS
BOF	BOTTOM OF FOOTING	GA	GAUGE	S.B.	SHORT BAR
BOW	BOTTOM OF WALL	GC	GENERAL CONTRACTOR	SIM	SIMILAR
BLDG	BUILDING	HVAC	HEATING-VENTILATING-AIR-CONDITIONING	SP	SPACER (es)
CLR	CLEAR (ance)			SPEC	SPECIFICATION (s)
CO	CLEAN OUT	HGT	HEIGHT	SQ	SQUARE
COL	COLUMN	HORIZ	HORIZONTAL	STD	STANDARD
CONC	CONCRETE	IN	INCH (es)	STL	STEEL
F'c	CONCRETE COMPRESSIVE STRENGTH	INSUL	INSULATION	SJI	STEEL JOIST INSTITUTE
		INT	INTERIOR	THRU	THROUGH
CMU	CONCRETE MASONRY UNIT	L.B.	LONG BAR	T&G	TONGUE AND GROOVE
CONN	CONNECTION	LLH	LONG LEG HORIZONTAL	TOM	TOP OF MASONRY
CONST	CONSTRUCTION	LLV	LONG LEG VERTICAL	TOP	TOP OF PLATE
CONT	CONTINUOUS	MAS	MASONRY	TOSL	TOP OF SLAB
CJ	CONTROL JOINT	F'm	MASONRY COMPRESSIVE STRENGTH	TOS	TOP OF STEEL
CFT	CUBIC FOOT			TOW	TOP OF WALL
CYD	CUBIC YARD	MO	MASONRY OPENING	TYP	TYPICAL
Ø, DIA	DIAMETER	MAX	MAXIMUM	VIF	VERIFY IN FIELD
DIAG	DIAGONAL	MDO	MEDIUM DENSITY OVERLAY	VERT	VERTICAL
DIM	DIMENSION	MTL	METAL	WWM	WELDED WIRE MESH
DWG	DRAWING	MIN	MINIMUM	WO	WITHOUT
EA	EACH	MPH	MILES PER HOUR	w /	WITH
ELEV	ELEVATION	MISC	MISCELLANEOUS	WD	WOOD
EQ	EQUAL	E	MODULUS OF ELASTICITY	Fy	YIELD STRESS
(E)	EXISTING	NOM	NOMINAL		
EXT	EXT	NIC	NOT IN CONTRACT		

A4 ABBREVIATIONS
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1. ALL CONTRACTORS SHALL CONFORM TO SAFETY REQUIREMENTS OF THE OWNER, OSHA SAFETY AND HEALTH STANDARDS, AND OTHER LOCAL AUTHORITIES IN CONNECTION WITH THE PERFORMANCE OF THIS PROJECT.

2. ALL REFERENCED STANDARDS OR PUBLICATIONS SHALL PERTAIN TO THE MOST CURRENT DATA, STANDARD OR PUBLICATION, UNLESS NOTED OTHERWISE.

3. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE SPECIFICATIONS, AS WELL AS THE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL AND CIVIL DRAWINGS. ALL DIMENSIONS AND ELEVATIONS SHOWN ON STRUCTURAL DRAWINGS, WITH THE EXCEPTION OF STRUCTURAL MEMBER SIZES, ARE GENERATED BY OTHER DISCIPLINES. ANY DIMENSIONS OR ELEVATIONS OMITTED OR NOT SHOWN ON THE STRUCTURAL DRAWINGS SHOULD BE OBTAINED FROM THE DRAWINGS OF THE OTHER DISCIPLINES. ANY INCONSISTENCIES WITH THE DRAWINGS AND SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER PRIOR TO PROCEEDING WITH THE AFFECTED PORTIONS OF THE WORK.

4. THE CONTRACTOR SHALL VISIT THE SITE AT A DESIGNATED TIME APPROVED BY THE OWNER, TO VERIFY EXISTING CONDITIONS, DIMENSIONS, LOCATION OF EXISTING UTILITIES ETC. THE CONTRACTOR SHALL NOTIFY ENGINEER OF ANY DISCREPANCIES WITHOUT EXCEPTION.

5. THE STRUCTURE HAS BEEN DESIGNED AS A SELF-SUPPORTING SYSTEM ONCE ALL WORK CONTAINED ON THESE DRAWINGS HAS BEEN COMPLETED. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR ERECTION PROCEDURES AND SEQUENCE OF INSTALLATION TO ENSURE SAFETY OF THE BUILDING AND ITS OCCUPANTS DURING CONSTRUCTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MEANS AND METHODS AND TEMPORARY SHORING, PRECAUTIONS DURING BUILDING OPERATIONS, PROTECTION OF PUBLIC AND WORKERS, REMOVAL OF WASTE MATERIAL, PROTECTION OF ADJACENT PROPERTY, PROTECTION OF HAZARDOUS OPENINGS, SAFETY PRECAUTIONS, AND SANITARY PROVISIONS OF EMPLOYEES AND SUBCONTRACTORS AS REQUIRED FOR THE DURATION OF THE CONTRACT.

6. WORK SHALL BE DONE IN AN ORDERLY AND PROFESSIONAL MANNER. THE GENERAL CONTRACTOR IS RESPONSIBLE FOR COORDINATING ALL WORK TO BE DONE BY SUBCONTRACTORS, LOCAL AUTHORITIES, STATE AGENCIES AND/OR UTILITY COMPANIES WHICH MAY HAVE JURISDICTION OVER THIS PROJECT.

7. THE CONTRACTOR IS RESPONSIBLE FOR REPLACING ANY EXISTING ITEMS DAMAGED BY NEW CONSTRUCTION, AND FOR ANY INCIDENTAL REPAIRS OF EXISTING FINISHED SURFACES DISTURBED BY NEW CONSTRUCTION; SUCH REPAIRS SHALL MATCH EXISTING TO THE OWNER'S SATISFACTION.

8. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING, HANDLING, AND STORAGE OF ITEMS/MATERIALS TO REMAIN THE PROPERTY OF THE OWNER WITH THE OWNER'S REPRESENTATIVE.

9. SPECIAL INSPECTIONS AS REQUIRED BY IBC 2009 SECTION 1704 SHALL BE PERFORMED BY AN INSPECTION AGENCY CONTRACTED BY THE OWNER FOR ALL WOOD, MASONRY, CONCRETE AND SOIL ACTIVITIES. SPECIFIC REQUIREMENTS FOR STEEL TESTING ARE OFFERED IN THE STRUCTURAL NOTES SECTION OF S-000.

E8 GENERAL NOTES
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FOUNDATIONS:

1. FOUNDATION SYSTEMS HAVE BEEN DESIGNED WITH AN ASSUMED BEARING CAPACITY OF 2 PSF. THE ALLOWABLE BEARING PRESSURE SHALL BE VERIFIED BY THE OWNER'S TESTING AGENCY PRIOR TO PLACING FOOTING CONCRETE.

2. ALL FOOTING BOTTOMS SHALL BE INSPECTED AND APPROVED, IN WRITING, BY A REGISTERED SOILS ENGINEER PRIOR TO PLACING CONCRETE. WRITTEN APPROVAL SHALL SPECIFY THAT THE SOIL HAS THE CAPACITY TO SUPPORT THE DESIGNED BEARING PRESSURE.

3. EXCAVATIONS FOR BUILDING FOUNDATIONS AND STRUCTURES SHALL BE IN ACCORDANCE WITH OSHA REQUIREMENTS. BRACED EXCAVATIONS SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE PROJECT STATE. DO NOT UNDERMINE EXISTING ADJACENT FOUNDATIONS.

4. INTERSECTING CONCRETE WALLS SHALL BE TIED WITH #4 L-BARS 3'-0" LONG (BENT 18-INCHES – 18-INCHES), SPACED AT 12-INCHES ON-CENTER, OUTSIDE FACE ONLY.

5. IN NO CASE SHALL HEAVY EQUIPMENT BE PERMITTED CLOSER THAN 6'-0" FROM ANY FOUNDATION/BASEMENT WALL. IF THE CONTRACTOR DEEMS IT NECESSARY TO OPERATE SUCH EQUIPMENT CLOSER THAN 8'-0", THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE AND, AT HIS OWN EXPENSE, PROVIDE ADEQUATE SUPPORTS OR WALL BRACES TO WITHSTAND THE ADDITIONAL LOADS SUPERIMPOSED FROM SUCH EQUIPMENT.

6. CONCRETE SHALL NOT BE PLACED ON FROZEN GROUND OR IN WATER.

CONCRETE:

1. CONCRETE WORK SHALL COMPLY WITH ACI 301 "SPECIFICATIONS FOR STRUCTURAL CONCRETE BUILDINGS"; ACI 318 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE"; AND ACI 315 "ACI DETAIL MANUAL", AND CRSI "MANUAL OF STANDARD PRACTICE".

2. CONTRACTOR SHALL PROVIDE TIES AND BRACING WHERE NECESSARY DURING CONSTRUCTION, TO REMAIN IN PLACE UNTIL THE STRUCTURES ARE COMPLETE.

3. CONCRETE SHALL BE:
   - a. FOOTING AND FOUNDATION WALLS: 3500 PSI AT (28) DAYS WITH AIR TO 6%. SLUMP SHALL NOT EXCEED 4-INCHES. W/C = 0.52.
   - b. INTERIOR SLABS-ON-GRADE: 4,000 PSI CONCRETE AT (28) DAYS WITH AIR TO 6%. SLUMP SHALL NOT EXCEED 4-INCHES. W/C = 0.49.

4. ALL CONSTRUCTION JOINTS FOR SLABS SHALL BE KEY JOINTED AT MID-SPAN WITH REINFORCING DISCONTINUOUS AT JOINT.

5. FLOOR SLAB CONTROL JOINTS SHALL BE PLACED AS SHOWN ON THE FOUNDATION PLAN OR AS DIRECTED BY THE ENGINEER. UNLESS OTHERWISE NOTED, CONTROL JOINTS WILL BE SPACED NOT TO EXCEED 13'-0" ON-CENTER IN BOTH DIRECTIONS AND SHALL BE FILLED WITH SEALANT AT THE COMPLETION OF THE PROJECT.

6. CONTRACTOR WILL CHECK WITH EACH TRADE TO ASSURE CORRECT LOCATION, SIZE, LINE AND ELEVATION OF SLEEVES, BOND-OUTS, ETC. REQUIRED IN CONCRETE FLOORS AND WALLS.

7. WELDING OF REINFORCEMENT IS NOT PERMITTED.

8. ADMIXTURES CONTAINING CALCIUM CHLORIDE SHALL NOT BE USED. CONCRETE SHALL NOT BE IN DIRECT CONTACT WITH ALUMINUM.

9. PROVIDE IN SLABS ON GRADE (2) BARS 4'-0" LONG AT EACH REENTRANT CORNER AND BOTH SIDES OF DOOR OPENING.

10. REFER TO ACI 318 (LATEST EDITION) FOR MINIMUM CONCRETE COVER FOR REINFORCING STEEL.

11. UNLESS OTHERWISE NOTED, REINFORCING LAP SPLICES SHALL BE ACI CLASS B SPLICES USING THE FOLLOWING LAP LENGTHS:

BAR SIZE	3	4	5	6	7	8	9	10	11
LAP IN.)	22	29	36	43	63	72	80	89	98

12. FOOTINGS SHALL BEAR ON VIRGIN SOIL OR STRUCTURAL BACKFILL COMPACTED TO ACHIEVE 95 PERCENT RELATIVE COMPACTION AS DETERMINED BY THE MODIFIED PROCTOR TEST (ASTM D1557)

A8 FOUNDATION NOTES
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Project:

EMBASSY SUITES

1050 Westbrook Street
Portland, Maine 04102

Project Owner:

KTB Hospitality, LLC

1050 Westbrook St.
Portland, Maine 04102

BMA
Architectural Group
Professional Corporation
Architects ■ Designers ■ Planners
12 Middle Street
Amherst ■ New Hampshire ■ 03031
TEL 603 673 1991 FAX 603 672 1385
www.bmaworld.com

Seal:

STATE OF MAINE
WILLIAM P. FAUCHER
No. 7133
LICENSED
PROFESSIONAL ENGINEER

Allied Engineering
Structural Mechanical Electrical Commissioning
160 Veranda Street
Portland, Maine 04103
T: 207.221.2260
F: 207.221.2266
Web: www.allied-eng.com

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GENERAL NOTES AND ABBREVIATIONS

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DATE: 24 MAY 2013

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