

GENERAL NOTES:

1. DESIGN CRITERIA

- A. CODE: 2009 INTERNATIONAL BUILDING CODE
- B. DESIGN LOADS:
- GRAVITY**
- ROOF LOADS..... (SEE WOOD TRUSS NOTES BELOW)
FLOOR LOADS..... (SEE WOOD TRUSS NOTES BELOW)
MECH/ELEC AREA LIVE LOAD..... 50 PSF
- SNOW**
- GROUND SNOW LOAD (Pg)..... 60 PSF
ROOF SLOPE FACTOR (Cs)..... 1.0
EXPOSURE FACTOR (Ce)..... 1.0
THERMAL FACTOR (ct)..... 1.0
IMPORTANCE FACTOR..... 1.0
- WIND**
- BASIC WIND SPEED (3 SEC GUST)..... 100 MPH
IMPORTANCE FACTOR..... 1.0
EXPOSURE..... C
BUILDING CLASSIFICATION – ENCLOSED
INTERNAL PRESSURE COEFFICIENT, GCPI..... ±0.18
- SEISMIC**
- IMPORTANCE FACTOR..... 1.0
SHORT PERIOD RESPONSE COEF (Ss)..... 0.314
1 SEC PERIOD RESPONSE COEF (S1)..... 0.077
SITE CLASS..... E
SHORT PERIOD RESPONSE COEF (Sds)..... 0.481
1 SEC PERIOD RESPONSE COEF (Sd1)..... 0.179
SEISMIC DESIGN CATEGORY..... C
SEISMIC-FORCE-RESISTING SYSTEM:
LIGHT FRAME WALLS WITH SHEAR PANELS
DESIGN BASE SHEAR
LONGITUDINAL..... 7.1 PSF
TRANSVERSE..... 7.5 PSF
SEISMIC RESPONSE COEF (Cs)..... 0.074
RESPONSE MODIFICATION FACTOR (R)..... 6.5
ANALYSIS PROCEDURE..... E.L.F.P.

2. GENERAL

- A. THE FOLLOWING SPECIFICATIONS ARE A SUPPLEMENT TO ALL OTHER REQUIREMENTS. WHERE CONFLICTS EXIST BETWEEN THESE SPECIFICATIONS AND LOCAL CODE REQUIREMENTS ARE IN EXCESS OF THOSE CONTAINED HEREIN, THE STRICTEST REQUIREMENT SHALL GOVERN.
- B. IT IS THE CONTRACTOR'S RESPONSIBILITY TO REVIEW AND SUBMIT ALL SHOP DRAWINGS AND REPORT ALL DISCREPANCIES TO THE ARCHITECT PRIOR TO FABRICATION OR ERECTION.
- C. ALL DIMENSIONS TO TAKE PRECEDENCE OVER SCALE SHOWN ON PLANS, SECTIONS AND DETAILS.
- D. SPECIFIC NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS.
- E. WHERE A SECTION IS CUT ON THE DRAWINGS, IT SHALL APPLY AT ALL LIKE OR SIMILAR CONDITIONS UNO.
- F. SEE ARCHITECTURAL DRAWINGS FOR THE FOLLOWING:
1. SIZE & LOCATION OF ALL DOOR & WINDOW OPENINGS
 2. SIZE & LOCATION OF ALL ROOF OPENINGS.
 3. FLOOR AND ROOF FINISHES.
 4. DETAILS OF VENEER ATTACHMENT.
 5. LOC'N & EXTENT OF INSULATION.
- G. SEE MECHANICAL, PLUMBING, ELECTRICAL AND CIVIL DRAWINGS FOR THE FOLLOWING INFORMATION:
1. PIPE RUNS, SLEEVES, HANGERS, TRENCHES, WALL AND SLAB OPENINGS, ETC.
 2. ELECTRICAL CONDUIT RUNS, BOXES, OUTLETS IN WALLS AND SLABS.
 3. CONCRETE INSERTS FOR ELECTRICAL, MECHANICAL OR PLUMBING FIXTURES.
 4. UNDERGROUND CONCRETE DUCTS, TRENCHES, PITS OR MANHOLES.
 5. CONCRETE AND ASPHALT PAVEMENT
- H. THE STRUCTURAL DRAWINGS REPRESENT THE FINISHED STRUCTURE, UNLESS OTHERWISE INDICATED. THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL ASSUME SOLE RESPONSIBILITY FOR ALL MEANS AND METHODS OF CONSTRUCTION AND SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE, WORKMEN OR OTHER PERSONS DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO: BRACING, SHORING FOR CONSTRUCTION EQUIPMENT, SHORING FOR THE BUILDING, SHORING FOR EARTH BANKS, FORMS, SCAFFOLDING, PLANKING, SAFETY NETS, SUPPORT AND BRACING FOR CRANES, ETC.
- J. ALL CONNECTOR TYPES REFER TO SIMPSON STRONG-TIE SPECIFICATIONS. ANY CHANGE, MODIFICATION OR SUBSTITUTION MUST BE APPROVED BY THE ENGINEER OF RECORD PRIOR TO CONSTRUCTION.

3. FOUNDATION

- A. DESIGN IS BASED ON THE REPORT TITLED "REPORT OF A SUBSURFACE EXPLORATION AND GEOTECHNICAL ENGINEERING EVALUATION PROPOSED HILTON GARDEN INN ADDITION, PORTLAND, MAINE" PROJECT NO. JPSEP #210213, DATED FEBRUARY 25, 2012 BY JOHN P. STOPEN ENGINEERING PARTNERSHIP IN SYRACUSE, NY.
- B. DESIGN IS BASED ON AN ALLOWABLE BEARING PRESSURE OF 3,000 PSF. ACTUAL ALLOWABLE BEARING PRESSURES SHALL BE VERIFIED BY THE SOILS ENGINEER PRIOR TO FOUNDATION CONSTRUCTION.
- C. THE SOILS ENGINEER OF RECORD SHALL CERTIFY IN WRITING THAT ALL FOUNDATIONS WERE PLACED AND COMPLETED AS SPECIFIED.
- E. CONTRACTOR TO PROVIDE FOR DE-WATERING IN EXCAVATIONS FROM EITHER SURFACE WATER, GROUND WATER, OR SEEPAGE.
- F. CONTRACTOR SHALL PROVIDE AND INSTALL ALL CRIBBING, SHEATHING, AND SHORING REQUIRED TO SAFELY RETAIN THE EARTH BANKS.
- G. CONTRACTOR SHALL PROTECT ALL UTILITY LINES, ETC. ENCOUNTERED DURING EXCAVATION AND BACKFILLING.
- H. ALL EXCAVATIONS SHALL BE PROPERLY BACKFILLED, BUT NOT BEFORE CONCRETE HAS ATTAINED FULL DESIGN STRENGTH.
- I. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO COMPLY WITH THE REQUIREMENTS AND RECOMMENDATIONS MADE IN THE ABOVE REFERENCED GEOTECHNICAL REPORT IN ORDER TO ACHIEVE THE ALLOWABLE BEARING PRESSURE NOTED ABOVE.
- H. WATER DRAINAGE SYSTEMS, ON BACK FILL SIDE OF RETAINING WALLS, ARE NOT SHOWN ON THE STRUCTURAL DRAWINGS. WALLS HAVE NOT BEEN DESIGNED TO SUPPORT HYDROSTATIC PRESSURES. DRAINAGE SYSTEMS SHALL BE PROVIDED IN ACCORDANCE WITH THE GEOTECHNICAL ENGINEER'S RECOMMENDATIONS.
- J. EARTH MOVERS AND OTHER HEAVY EQUIPMENT SHOULD NOT OPERATE WITHIN 10 FEET OF BELOW GRADE WALLS.
- K. BOTTOM OF EXTERIOR FOOTING DEPTH SHALL BE PLACED MINIMUM OF 48" BELOW ADJACENT FINISHED GRADE ELEVATION FOR FROST DEPTH.

4. CONCRETE

(REFERENCE STANDARDS: ACI 318-05)

- A. CONCRETE PROTECTION FOR REINFORCEMENT OF POURED-IN-PLACE MEMBERS: (SEE SECTION 7.7 OF ACI 318).
- B. PORTLAND CEMENT SHALL CONFORM TO ASTM C150, TYPE I.
- C. REFER TO ARCHITECTURAL DRAWINGS FOR CLIPS, GROOVES, GROUNDS, ETC., TO BE CAST IN CONCRETE AND CONCRETE FINISHES.
- D. ALL REINFORCING BARS, ANCHOR BOLTS AND OTHER CONCRETE INSERTS SHALL BE SECURED IN POSITION PRIOR TO PLACING CONCRETE.
- E. SLEEVE PLUMBING OPENINGS IN SLABS BEFORE PLACING CONCRETE AND BEND REINFORCING AROUND SLEEVES. CORING NOT PERMITTED IN FLOOR SLABS, UNLESS APPROVED BY STRUCTURAL ENGINEER.
- F. ULTIMATE COMPRESSIVE STRENGTH AT 28 DAYS SHALL BE AS FOLLOWS:
- | | |
|---|----------|
| FOOTINGS / GRADE BEAMS AND WALL FOOTINGS..... | 3000 PSI |
| SLAB ON GRADE..... | 3000 PSI |
| WALLS..... | 4000 PSI |
| TOPPING SLAB..... | 2500 PSI |
- G. CONCRETE SLUMP SHALL BE 3" TO 5" AT TIME OF PLACEMENT.
- H. CONCRETE MIX DESIGNS SHALL BE ESTABLISHED BY THE SUPPLIER IN ACCORDANCE WITH THE ABOVE REFERENCED STANDARDS. MIX DESIGNS SHALL BE SUBMITTED WITH BACK-UP DATA PER ACI 318 TO THE ARCHITECT FOR REVIEW PRIOR TO CONCRETE PLACEMENT.
- J. ALL CONCRETE EXPOSED TO THE WEATHER SHALL CONTAIN 5% TO 7% ENTRAINED AIR.
- K. ALL CONCRETE CONSTRUCTION SHALL COMPLY W/ THE ABOVE REFERENCED STANDARDS AND CONCRETE TEST REPORTS SHALL BE AVAILABLE AT JOB SITE.

5. REINFORCING STEEL

(REFERENCE STANDARDS: ACI 117-90, 315-99, 318-05)

- A. REINFORCING BARS SHALL BE DEFORMED BARS CONFORMING TO ASTM A-615, GRADE 60.
- B. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185.
- C. CLEAR COVERAGE OF CONCRETE OVER OUTER REINFORCING BARS SHALL BE IN ACCORDANCE WITH SECTION 7.7 OF ACI 318, UNLESS SPECIFICALLY DETAILED OTHERWISE ON THE DWGS.
- D. ALL REINFORCING BAR BENDS ARE TO BE MADE COLD.
- E. CONTRACTORS SHALL NOT PLACE ANY REINFORCING UNTIL APPROVED SHOP DRAWINGS ARE RECEIVED ON THE JOB.
- F. BARS SHALL BE IN CONTACT WHEN FORMING A LAP SPICE, UNLESS NOTED OTHERWISE.
- G. PROVIDE CORNER BARS @ ALL TURN-DOWN SLAB CORNERS AND C.I.P. CONCRETE WALL CORNERS. PROVIDE 30" LAP BETWEEN CORNER BARS AND MAIN REINFORCING.
- H. REINFORCING STEEL MARKED "CONTINUOUS" SHALL BE LAPPED W/ CLASS "B" LAP SPICE UNLESS SPECIFICALLY DETAILED OTHERWISE. LAP WELDED WIRE MESH ONE FULL MESH AT SIDE AND END LAPS.

6. POST INSTALLED ANCHORS

- A. POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE DRAWINGS.
- B. CONTRACTOR SHALL OBTAIN APPROVAL FROM ENGINEER OF RECORD PRIOR TO USING POST-INSTALLED ANCHORS FOR MISSING OR MISPLACED CAST-IN-PLACE ANCHORS.
- C. CARE SHALL BE GIVEN TO AVOID CONFLICTS WITH EXISTING REBAR. HOLES SHALL BE DRILLED AND CLEANED PER THE MANUFACTURER'S INSTRUCTIONS. ANCHORS SHALL BE INSTALLED PER THE MANUFACTURER'S INSTALLATION INSTRUCTIONS AT NOT LESS THAN MINIMUM EDGE DISTANCES AND/OR SPACING INDICATED IN THE MANUFACTURER'S LITERATURE.
- D. UNLESS SPECIFIED OTHERWISE, ANCHORS SHALL BE EMBEDDED IN THE APPROPRIATE SUBSTRATE WITH A MINIMUM EMBEDMENT OF 8 TIMES THE NOMINAL ANCHOR DIAMETER OR THE EMBEDMENT REQUIRED TO SUPPORT THE INTENDED LOAD.
- E. SUBSTITUTION REQUESTS, FOR PRODUCTS OTHER THAN THOSE LISTED BELOW, SHALL BE SUBMITTED TO THE ENGINEER WITH CALCULATIONS THAT ARE PREPARED & SEALED BY A REGISTERED PROFESSIONAL ENGINEER SHOWING THAT THE SUBSTITUTED PRODUCT WILL ACHIEVE AN EQUIVALENT CAPACITY USING THE APPROPRIATE DESIGN PROCEDURE REQUIRED BY THE BUILDING CODE.
- F. ACCEPTABLE PRODUCTS
- | | |
|--------------------|--|
| EXPANSION ANCHORS: | |
| 1. | "STRONG-BOLT" BY SIMPSON STRONG-TIE |
| 2. | "KWIK BOLT III" BY HILTI |
| ADHESIVE ANCHORS: | |
| 1. | "SET-XP EPOXY" W/ "XP ANCHOR" RODS BY SIMPSON STRONG-TIE |
| 2. | "HIT RE 500" W/ "HAS SUPER" ANCHOR RODS BY HILTI |
| SCREW ANCHORS: | |
| 1. | "TITEN HD" BY SIMPSON STRONG-TIE |

7. SOLID SAWN & LAMINATED LUMBER

(REFERENCE STANDARD: AF&PA 2005-NATIONAL DESIGN SPECIFICATION)

- A. ALL LUMBER SHALL BE VISUALLY GRADED, DIMENSION LUMBER, SEASONED AND WITH 19% MAX MOISTURE CONTENT, UNO, AND IN ACCORDANCE WITH THE FOLLOWING MINIMUM GRADE REQUIREMENTS:
- | | |
|---------------------|---------------------------|
| STUDS | SEE STUD SCHEDULE ON S4.0 |
| JOISTS | SPRUCE PINE FIR NO. 2 |
| BEAMS (2"-4" THICK) | SPRUCE PINE FIR NO. 2 |
| POSTS | SPRUCE PINE FIR NO. 2 |
| PLATE STOCK | SPRUCE PINE FIR NO. 2 |
- B. GRADES SHALL BE DETERMINED IN ACCORDANCE WITH SPIB GRADING RULES AGENCY.
- C. BRACE STUD WALLS UNTIL ALL WOOD DECKING, ROOF TRUSSES, AND SHEAR PANELS ARE IN PLACE.
- D. USE WATER-BORNE PRESSURE TREATED WOOD (CBA-A, ACQ-C, ACQ-D, CA-B, SBX-DOT) FOR ALL LUMBER EXPOSED TO WEATHER OR POOL MOISTURE AND SILL PLATES IN CONTACT WITH MASONRY OR CONCRETE. IN ADDITION, FOR LUMBER EXPOSED TO WEATHER, A POLYMER SEALER/BINDER IS ALSO RECOMMENDED.

- E. THE FOLLOWING CHART SHALL BE USED IN DETERMINING FINISHES FOR HARDWARE IN CONTACT W/ PRESSURE TREATED WOOD:

TREATMENT TYPE	SBX (DOT)	ACQ-C ACQ-D	CBA-A CA-B
GALVANIZED FINISH	X	-	-
G90	X	-	-
G185	X	X	X
POST HOT DIP GALVANIZED	X	X	X
STAINLESS STL (TYPES 304 & 316)	X	X	X

NOTE: FOR WOOD WITH ACTUAL RETENTION LEVELS GREATER THAN 0.40 PCF FOR ACQ, 0.41 PCF FOR CBA-A OR 0.21 FOR CA-B, STAINLESS STEEL CONNECTORS AND FASTENERS ARE RECOMMENDED. VERIFY ACTUAL RETENTION LEVEL WITH THE WOOD SUPPLIER/TREATER. WHEN USING STAINLESS STL CONNECTORS, USE STAINLESS STL FASTENERS. WHEN USING GALVANIZED CONNECTORS, USE GALVANIZED FASTENERS.

- F. ALL FASTENERS IN CONTACT W/ PRESSURE TREATED LUMBER SHALL BE BATCH/POST HOT-DIP GALVANIZED (PER ASTM A153) OR MECHANICALLY GALVANIZED (PER ASTM B695, CLASS 55 OR GREATER).
- EXCEPTION: BOLTS & LAG SCREWS W/ SHANK DIAMETERS GREATER THAN 1/2" IN DIAMETER AND PROTECTED FROM THE WEATHER ARE NOT REQUIRED TO BE GALVANIZED.
- G. AS A MINIMUM, ALL CONNECTORS EXPOSED TO WEATHER SHALL HAVE A G185 (SIMPSON ZMAX) GALVANIZED FINISH.
- H. ALL FOUNDATION PLATES OR SILLS SHALL BE ANCHORED TO THE FOUNDATION WITH NOT LESS THAN 1/2" DIA STEEL BOLTS OR SIMPSON MAS ANCHORS @ 48" O.C. MAX. BOLTS SHALL BE EMBEDDED AT LEAST 7" INTO CONCRETE OR MASONRY. THERE SHALL BE A MIN OF 2 ANCHORS PER PIECE W/ ONE ANCHOR NOT MORE THAN 12" OR LESS THAN 4" FROM EA END OF EA PIECE.

EXCEPTIONS:

1. INTERIOR SILL PLATES MAY BE ANCHORED WITH HILTI X-CP 72 P8S23 POWDER ACTUATED FASTENERS @ 18" O.C. MAX. PROVIDE PINS @ 6" AND 10" FROM ENDS OF PLATE WITH 2 PINS MIN IN ANY PLT.
 2. SHEAR WALL SILL PLATE ANCHORAGE SHALL GOVERN WITH REGARD TO ANCHOR TYPE AND SPACING WHEN LESS THAN THAT REFERENCED ABOVE. SEE BRACING PLANS AND SHEAR WALL SILL PLATE ANCHORAGE SCHEDULE ON DRAWING 55.0 FOR ADD'L INFO.
- I. ALL HANDRAILS, GUARDRAILS AND STAIR STRINGERS INCLUDING COMPONENTS AND ASSOCIATED CONNECTIONS SHALL BE DESIGNED BY THE SUPPLIER IN ACCORDANCE WITH THE LOCAL BUILDING CODE.
- J. INSTALL BEAMS WITH CROWN UP.
- K. ALL LVL MEMBERS SHALL BE (MIN) : Fb=2600 PSI, Fv=285 PSI & E=19000000 PSI.
- L. ALL LSL MEMBERS SHALL BE (MIN) : Fb=2600 PSI, Fv=400 PSI & E=17000000 PSI.
- M. THE NUMBER OF WALL STUDS AT BEARING POINTS OF 2X MEMBER FLUSH BEAMS SHALL MATCH THE NUMBER OF MEMBERS IN THE BEAM UNLESS NOTED OTHERWISE. ALL LVL AND LSL FLUSH BEAMS SHALL HAVE A (3) STUD MIN BEARING UNLESS NOTED OTHERWISE. THE CENTERLINE OF THE BEAM SHALL BE THE CENTERLINE OF THE SUPPORTING WALL STUDS.
- N. ALL NAILS ARE COMMON WIRE NAILS UNLESS NOTED OTHERWISE. SEE TYPICAL FASTENING SCHEDULE FOOTNOTES ON S0.2 FOR MINIMUM COMMON WIRE NAIL SIZES. ANY CHANGE, MODIFICATION OR SUBSTITUTION MUST BE APPROVED BY THE ENGINEER OF RECORD PRIOR TO CONSTRUCTION.

8. LATERAL LOAD RESISTING SYSTEM

(REFERENCE STANDARD: IBC 2006 TABLES 2306.3.1 & 2306.4.1)

- A. ROOF DECK AND SUBFLOORS ARE DESIGNED AS UNBLOCKED DIAPHRAGMS.
1. ROOF SHEATHING SHALL BE MIN 15/32" THICK EXPOSURE 1 RATED O.S.B. WITH A 32/16 PANEL SPAN INDEX (U.S.) AND BEAR THE TRADEMARK STAMP OF APA - THE ENGINEERED WOOD ASSOCIATION. PANELS SHALL BE NAILED WITH 8d NAILS @ 6" o.c. AT ALL PANEL EDGES AND 12" o.c. AT ALL INTERIOR SUPPORTS UNLESS NOTED OTHERWISE ON PLAN.
 2. FLOOR SHEATHING SHALL BE 23/32" THICK T & G EXPOSURE 1 RATED O.S.B. WITH A 48/24 PANEL SPAN INDEX (U.S.) AND BEAR THE TRADEMARK STAMP OF APA - THE ENGINEERED WOOD ASSOCIATION. PANELS SHALL BE NAILED WITH 10d NAILS @ 6" o.c. AT ALL PANEL EDGES AND 12" o.c. AT ALL INTERIOR SUPPORTS UNLESS NOTED OTHERWISE ON PLAN.
- B. FOR SHEAR WALL BRACING & ANCHORAGE SEE BRACING PLANS.
- C. FRAMING DETAILS INCORPORATE MINIMUM REQUIREMENTS FOR LATERAL LOAD TRANSFER. ANY CHANGE, MODIFICATION OR SUBSTITUTION OF MATERIALS (INCLUDING GRADE OR SPECIES) OR FASTENERS MUST BE APPROVED BY THE ENGINEER OF RECORD PRIOR TO INSTALLATION.
- D. TEMPORARY BRACING OF THE BLDGS IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR AND MUST REMAIN IN PLACE UNTIL ALL FRAMING DETAILS, ROOF AND FLOOR SHEATHING AND SHEAR WALL CONSTRUCTION IS COMPLETE.
- E. TEMPORARY STACKING OF WOOD SHEATHING ON ELEVATED WOOD FRAMING SHALL BE LIMITED TO 10 SHEETS LAID FLAT W/ LONG DIMENSION PERPENDICULAR TO JOIST SPAN.
- F. TEMPORARY STACKING OF GYPSUM SHEATHING ON ELEVATED WOOD FRAMING SHALL BE LIMITED TO 15 SHEETS LAID FLAT OR ON EDGE W/ LONG DIMENSION PERPENDICULAR TO JOIST SPAN.

9. PRE-ENGINEERED WOOD TRUSSES

(REFERENCE STANDARD: TRUSS PLATE INSTITUTE ANSI/TP1 1-2002)

- A. TRUSS DESIGN LOADS ARE AS FOLLOWS:

	TOP CHORD	BOTTOM CHORD
ROOF DL:	10 psf	10 psf
ROOF LL:	20 psf	0 psf
FLOOR DL:	20 psf	5 psf
FLOOR LL:	40 psf	0 psf
STAIRWELL DL:	45 psf	5 psf
STAIRWELL LL:	100 psf	0 psf

- B. ALL TRUSS CONNECTOR PLATES SHALL BE MANUFACTURED FROM ASTM A445-72 GRADE A GALVANIZED STEEL OF NO LESS THAN 20 GAGE THICKNESS WITH A MINIMUM YIELD OF 33,000 PSI AND AN ULTIMATE TENSILE STRENGTH OF 45,000 PSI. CONNECTOR PLATE GAUGES SHALL BE AS REQUIRED BY MANUFACTURERS DESIGN CALCULATIONS.
- C. TRUSS SHOP DRAWINGS SHALL BE SUBMITTED FOR THE ARCHITECTS REVIEW PRIOR TO FABRICATION AND SHALL INCLUDE THE FOLLOWING:
1. SLOPE OR DEPTH, SPAN AND SPACING
 2. LOCATION OF ALL JOINTS
 3. DESIGN LOADS AS APPLICABLE:
 - a. TOP CHORD LIVE LOAD (INCLUDING SNOW LOADS)
 - b. TOP CHORD DEAD LOAD
 - c. BOTTOM CHORD LIVE LOAD
 - d. BOTTOM CHORD DEAD LOAD
 - e. CONCENTRATED LOADS AND THEIR POINTS OF APPLICATION
 - f. CONTROLLING WIND AND EARTHQUAKE LOADS
 4. ADJUSTMENTS TO LUMBER AND METAL CONNECTOR PLATE DESIGN VALUES FOR CONDITIONS OF USE
 5. EACH REACTION FORCE AND DIRECTION
 6. METAL CONNECTOR PLATE TYPE, SIZE, THICKNESS OR GAGE, AND DIMENSIONED LOCATION OF EACH METAL CONNECTOR PLATE EXCEPT WHERE SYMMETRICALLY LOCATED RELATIVE TO THE JOINT INTERFACE
 7. LUMBER SIZE, SPECIES, AND GRADE FOR EACH MEMBER
 8. CONNECTION REQUIREMENTS FOR:
 - a. TRUSS TO TRUSS GIRDER
 - b. TRUSS PLY TO FLY
 - c. OVERBUILD TRUSS TO SUPPORTING TRUSS
 - d. FIELD SPLICES
 9. CALCULATED DEFLECTION RATIO AND/OR MAXIMUM DEFLECTION FOR LIVE AND TOTAL LOAD (MAX TOTAL DEFLECTION LIMITS SHALL BE L/240 FOR ROOF TRUSSES & L/360 FOR FLOOR TRUSSES U.N.O.)
 10. MAXIMUM AXIAL COMPRESSION FORCES IN THE TRUSS MEMBERS
 11. REQUIRED PERMANENT TRUSS MEMBER BRACING LOCATION
 12. LUMBER SPECIES AND GRADES OF TRUSS MEMBERS
 13. SEAL AND SIGNATURE OF TRUSS DESIGN ENGINEER RESPONSIBLE FOR ALL TRUSS ENGINEERED DOCUMENTS AND/OR DRAWINGS
 14. UNIFORM, LATERAL AND CONCENTRATED LOAD REQUIREMENTS AS NOTED ON PLANS AND/OR CORRESPONDING DETAILS

- D. FIELD REPAIR OF DAMAGED TRUSSES MUST BE APPROVED IN WRITING BY THE TRUSS ENGINEER AND ENGINEER OF RECORD.
- E. ALL ROOF TRUSS BEARING WALLS SHALL HAVE METAL FASTENERS TO RESIST CODE PRESCRIBED UPLIFT FORCES.
- F. TRUSS SUPPLIER IS TO PROVIDE PLAN AND PROCEDURES FOR INSTALLING, SECURING AND BRACING OF ALL TRUSSES.
- G. TRUSS SUPPLIER SHALL PROVIDE TRUSS BLOCKS CAPABLE OF TRANSFERRING LATERAL LOADS AS NOTED ON PLANS AND/OR DETAILS.
- H. APPROVED TRUSS PLANS SHALL BE AVAILABLE ON JOB SITE DURING TIMES OF INSPECTION.
- I. TRUSS MANUFACTURER TO PROVIDE OR ALIGN TRUSS ABOVE ALL SHEAR WALLS AS SHOWN ON THE PLANS.
- J. TRUSS DESIGNER SHALL PROVIDE VIERENDEEL TRUSS OPENINGS WHERE POSSIBLE TO ACCOMMODATE MECHANICAL, PLUMBING AND ELECTRICAL RUNS.
- K. TRUSS DESIGNER SHALL DESIGN TRUSSES TO SUPPORT ALL MECHANICAL AND FIRE SPRINKLER EQUIPMENT AS NOTED/SHOWN ON THE MECHANICAL/PLUMBING DWGS. SEE MECHANICAL/PLUMBING DWGS FOR EQUIPMENT LOADS & ADD'L INFO.

10. WOOD SCREWS

- A. UNLESS SPECIFIED OTHERWISE, SCREWS SHALL BE EMBEDDED IN THE APPROPRIATE SUBSTRATE WITH A MINIMUM EMBEDMENT OF 7 TIMES THE SHANK DIAMETER.
- B. WHERE MINIMUM EMBEDMENT DEPTHS ARE NOTED, SCREWS SHALL PROVIDE AN EMBEDMENT INTO THE MAIN MEMBER EQUAL TO OR GREATER THAN THE MINIMUM REQUIRED.
- C. SUBSTITUTION REQUESTS, FOR PRODUCTS OTHER THAN THOSE LISTED BELOW, SHALL BE SUBMITTED TO THE ENGINEER WITH CALCULATIONS THAT ARE PREPARED & SEALED BY A REGISTERED PROFESSIONAL ENGINEER SHOWING THAT THE SUBSTITUTED PRODUCT WILL ACHIEVE AN EQUIVALENT CAPACITY USING THE APPROPRIATE DESIGN PROCEDURE REQUIRED BY THE BUILDING CODE.
- D. ACCEPTABLE PRODUCTS
- | | |
|---------------------|---|
| SIMPSON STRONG-TIE: | |
| 1. | "SDS WOOD SCREW" (CORROSION RESISTANT FASTENER) |
| FASTENMASTER: | |
| 1. | "LEDGERLOK" (CORROSION RESISTANT FASTENER) |
| 2. | "TIMBERLOK" (HEAVY DUTY CORROSION RESISTANT FASTENER) |
| 3. | "TRUSSLOK" (MULTI-PLY ENGINEERED WOOD FASTENER) |

PROJECT

PORTLAND
HILTON GARDEN
INN ADDITION

LOCATION

PORTLAND

145 JETPORT BLVD
PORTLAND, MAINE 04102

ARCHITECT

hc
hogan campis architecture

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STAMP



ISSUED FOR: DATE:

REVIEW

PRICING JULY 20, 2012

PERMITTING JAN 30, 2013

CONSTRUCTION

REVISIONS

JOB NO.

0479.000

DRAWING TITLE

GENERAL NOTES

SHEET NO.

S0.1