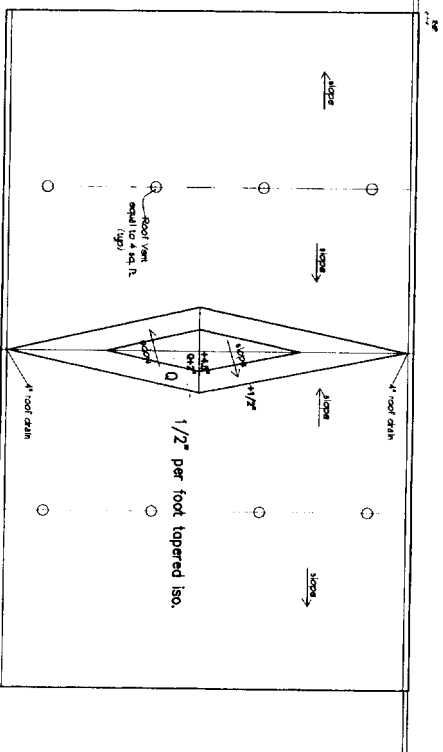
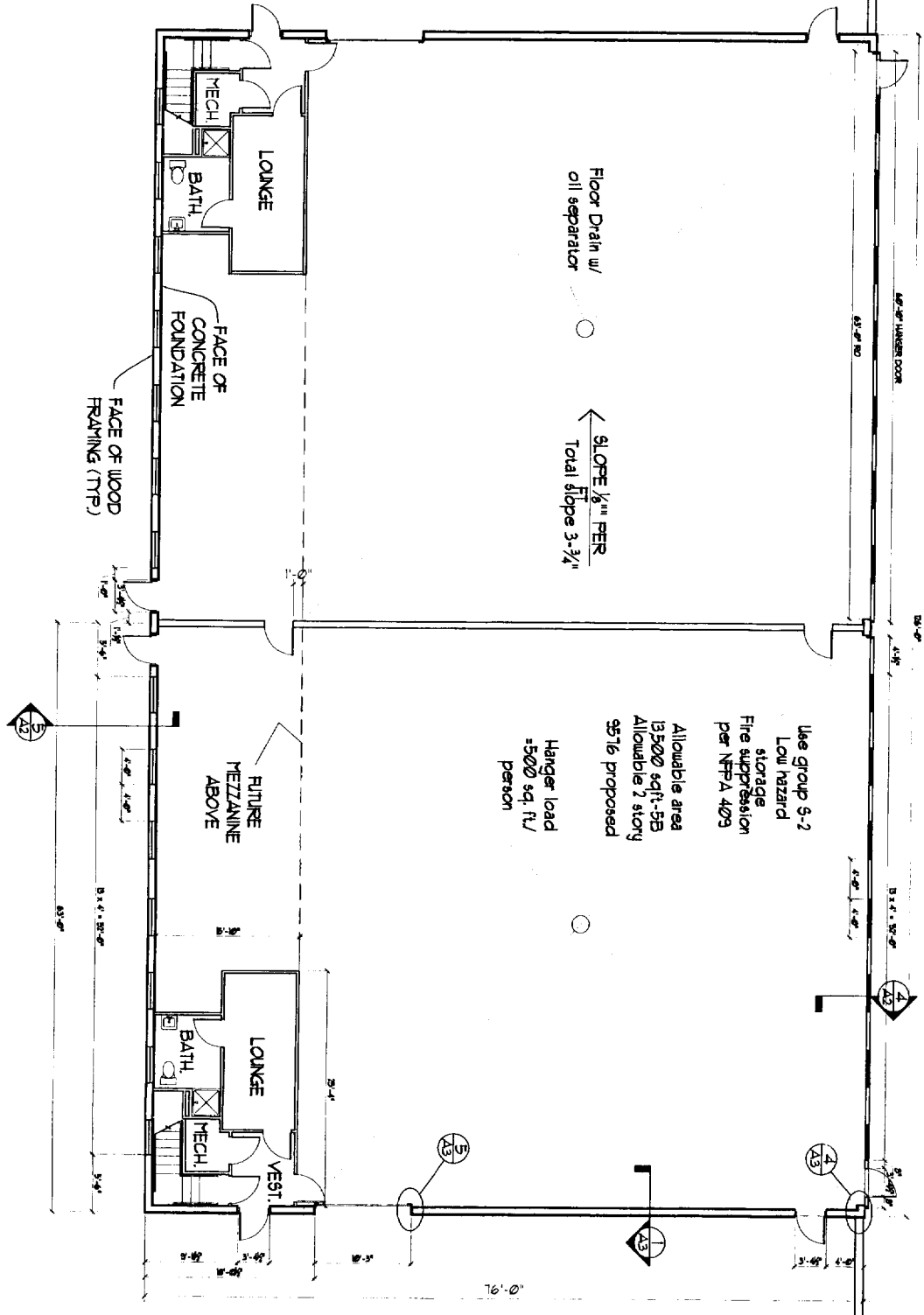


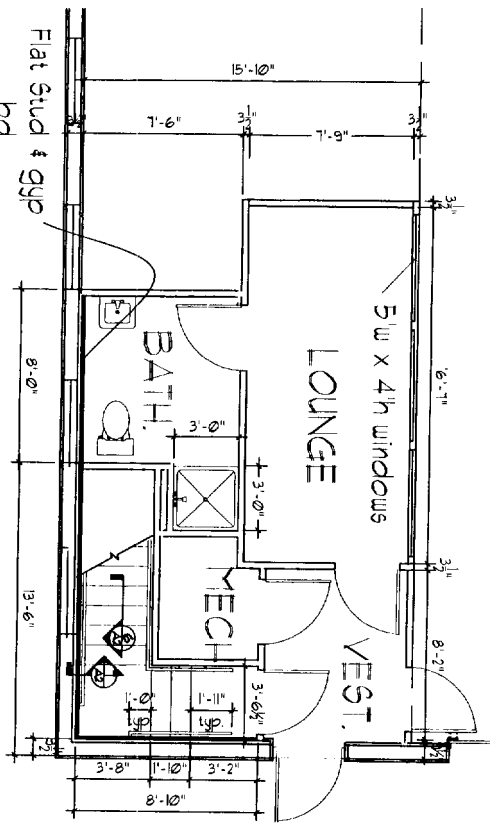
2 2nd FLOOR PLAN
SCALE: 1/8" = 1'-0"



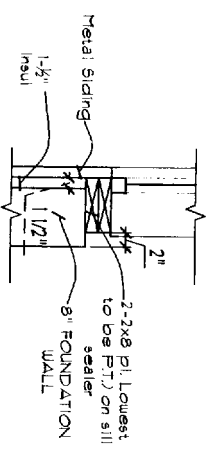
3 ROOF PLAN
SCALE: 1/16" = 1'-0"



1 1st FLOOR PLAN
SCALE: 1/8" = 1'-0"



4 STAIR
SCALE: 1/4" = 1'-0"



5 DETAIL
SCALE: 1" = 1'-0"

Note: All dimensions to interior mce of foundation wall

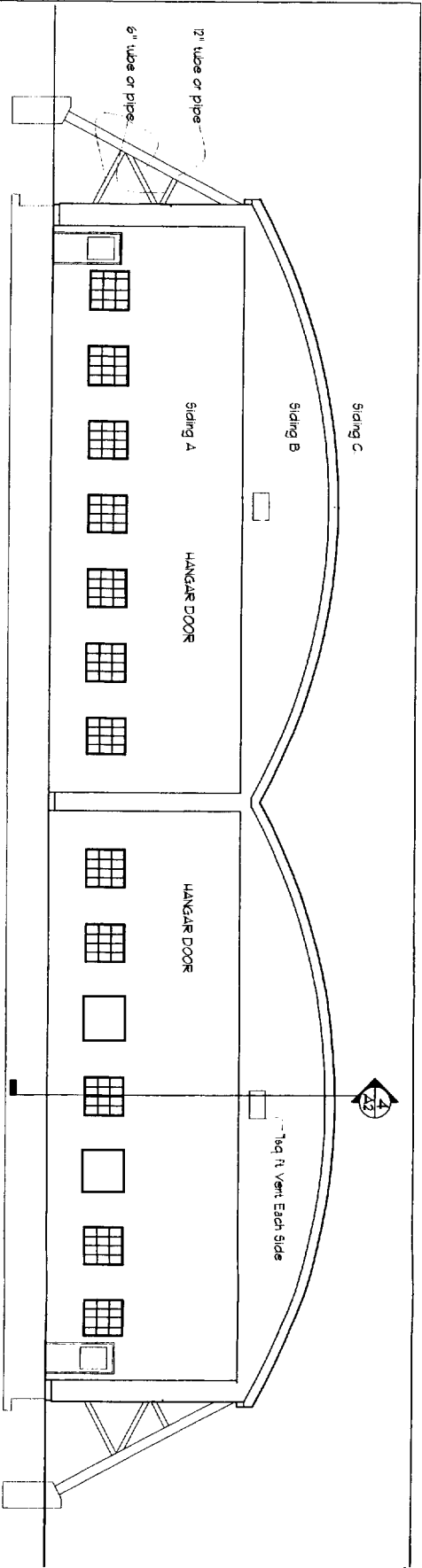
A01

PLANS	
Project:	AIRPLANE HANGER
WESTBROOK STREET PORTLAND, MAINE	

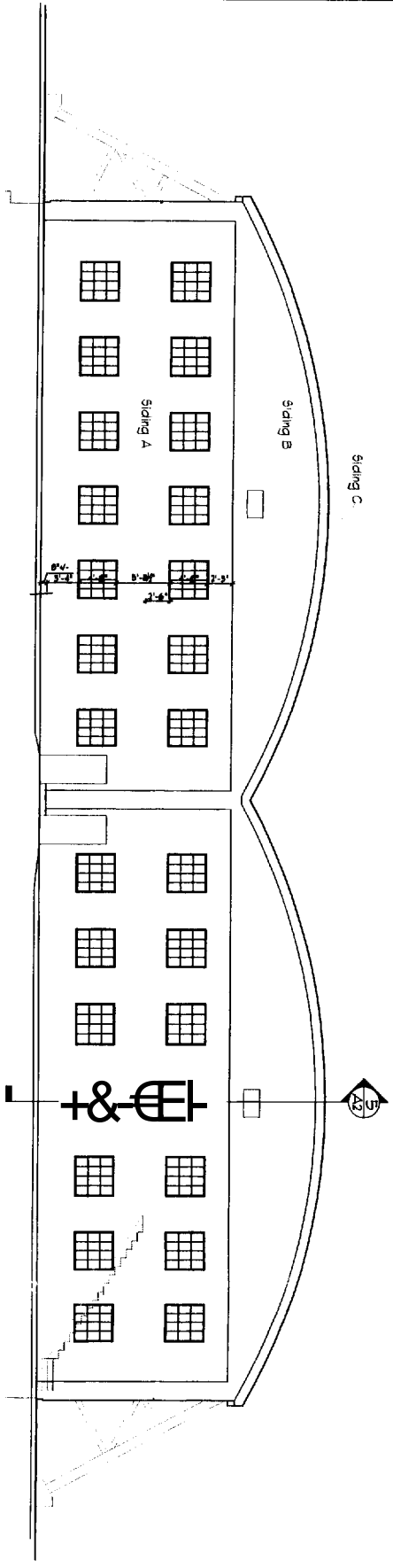
Date	NOV 11, 2005	Scale	AS NOTED
Revisions:		Checked By:	
1-11-06 Issued for Permit			

ARCHETYPE, P.A. ARCHITECTS	
48 Union Wharf Portland, Maine 04101 (207) 772-6022 Fax (207) 772-4056	

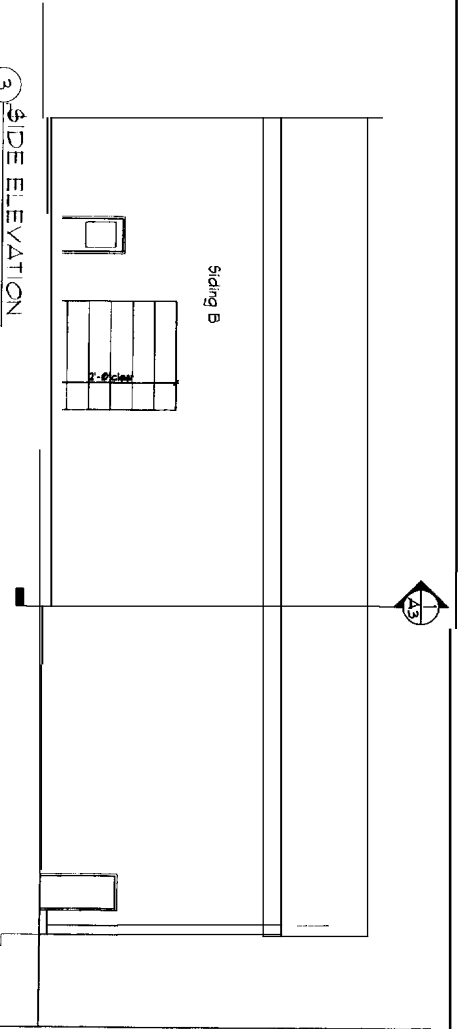
OWNER:	NEPTUNE PROPERTIES LLC
120 Exchange, Portland, Maine 04101 (207) 775-2100 Fax (207) 874-6988	



1 FRONT ELEVATION
SCALE: 1/4" = 1'-0"

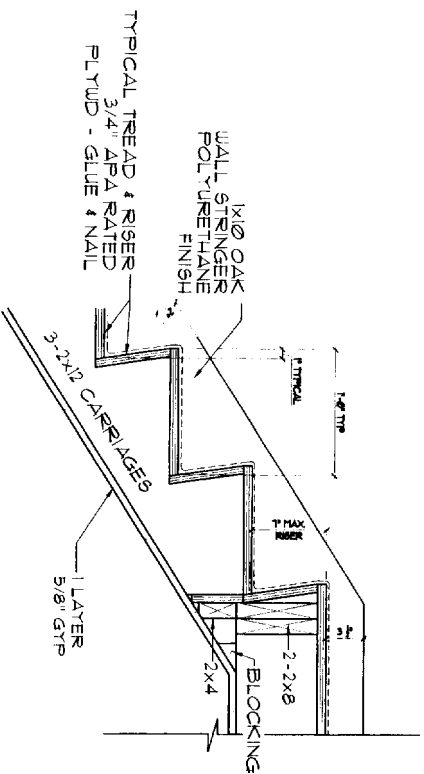


2 REAR ELEVATION
SCALE: 1/8" = 1'-0"

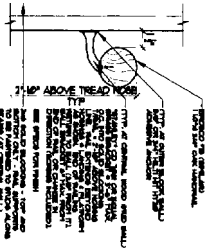


3 SIDE ELEVATION
SCALE: 1/8" = 1'-0"

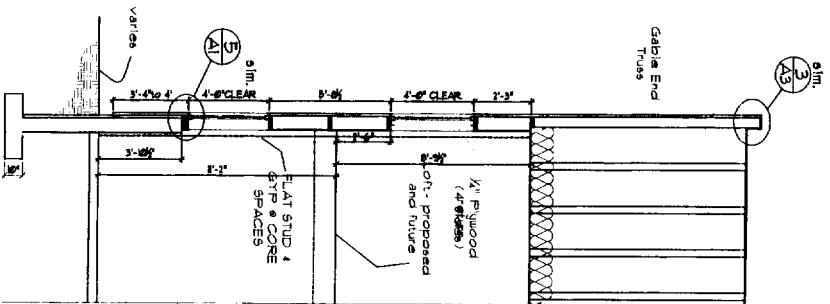
General Notes



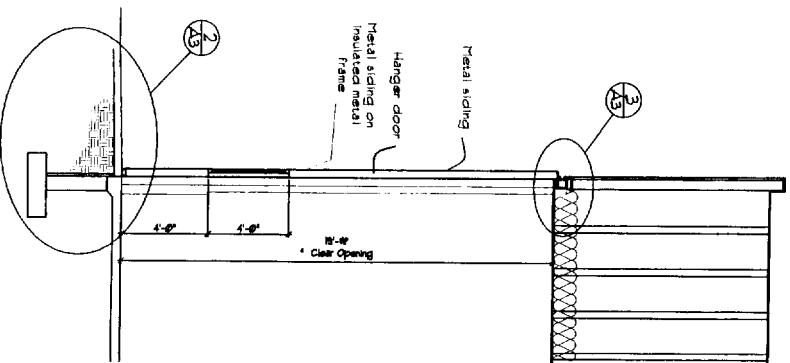
6 STAIR DETAIL
SCALE: 1/2" = 1'-0"



7 HANDRAIL DETAIL
SCALE: 6" = 1'-0"



4 WALL SECTION
SCALE: 1/4" = 1'-0"



5 WALL SECTION
SCALE: 1/4" = 1'-0"

A02

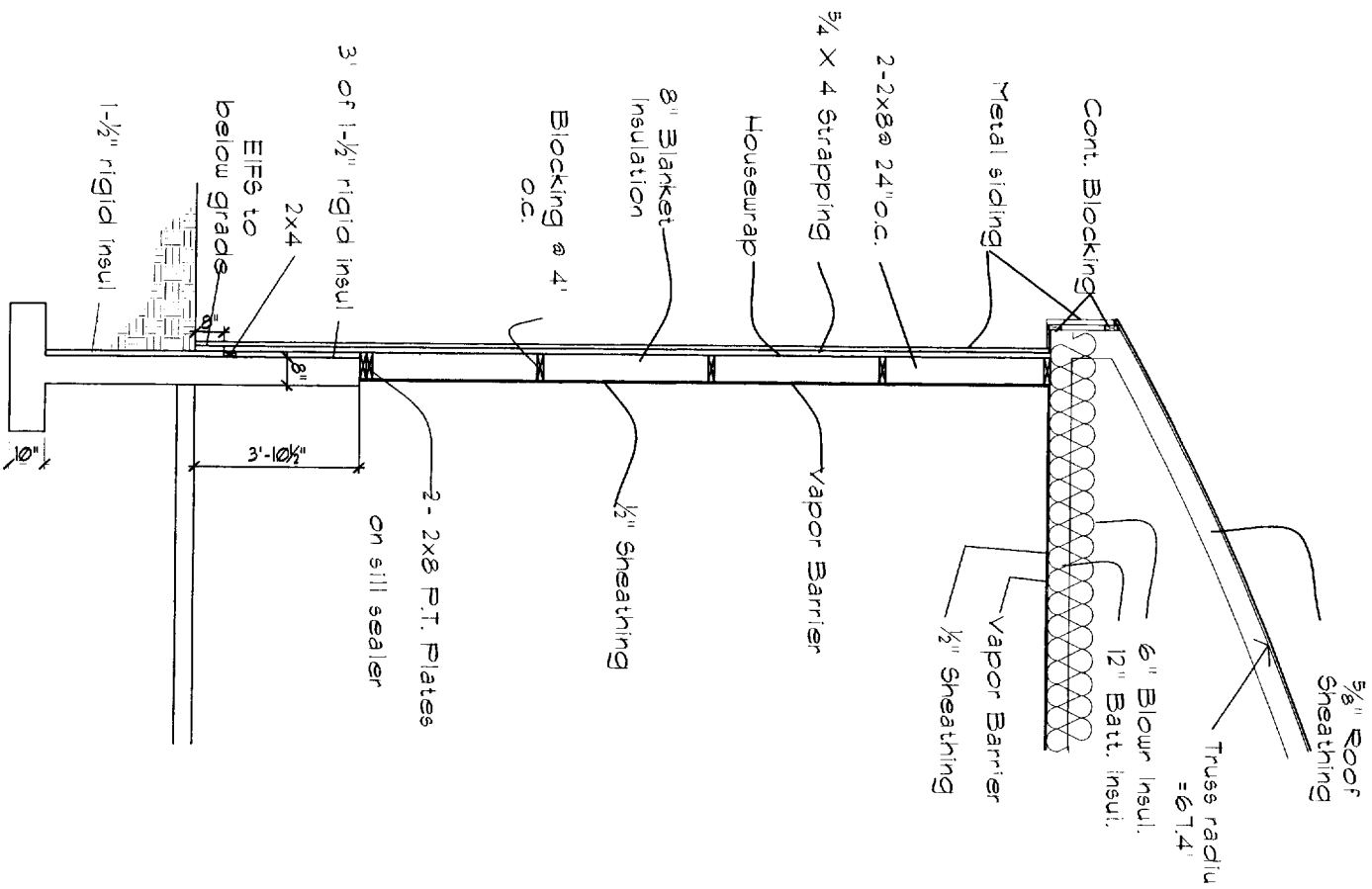
ELEVATIONS / WALL SECTIONS
Project: AIRPLANE HANGER
WESTBROOK STREET
PORTLAND, MAINE

Date: NOV 11, 2005
Revisions: 1-11-06 Issued for Permit
Scale: AS NOTED
Checked By:

ARCHETYPE, P.A.
ARCHITECTS

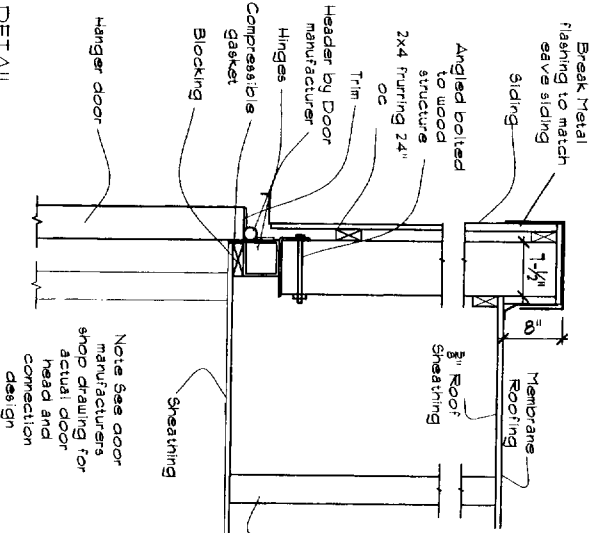
48 Union Wharf Portland, Maine 04101
(207) 772-6022 Fax (207) 772-4056

OWNER:
NEPTUNE PROPERTIES LLC
120 Exchange, Portland, Maine 04101
(207) 775-2100 Fax (207) 874-6988

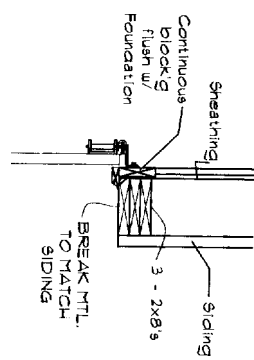


1 WALL SECTION
SCALE: 1/2" = 1'-0"

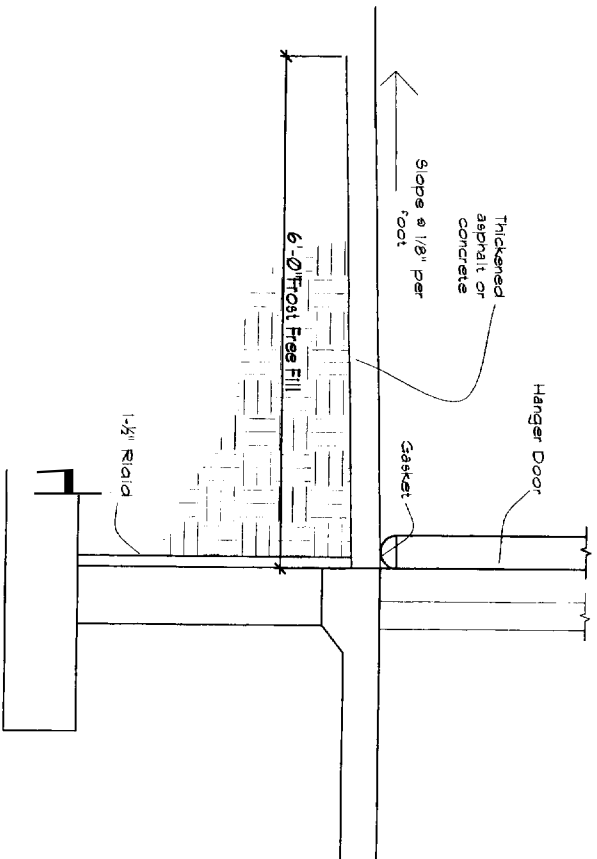
3 DETAIL
SCALE: 1" = 1'-0"



5 DETAIL OVERHEAD DOOR JAMB
SCALE: 1" = 1'-0"



2 DETAIL
SCALE: 1" = 1'-0"



A03	WALL SECTIONS & DETAILS		Date NOV 11, 2005	Scale AS NOTED	ARCHETYPE, P.A. ARCHITECTS 48 Union Wharf Portland, Maine 04101 (207) 772-6022 Fax (207) 772-4056	OWNER: NEPTUNE PROPERTIES LLC 120 Exchange, Portland, Maine 04101 (207) 775-2100 Fax (207) 874-6988
	Project: AIRPLANE HANGER WESTBROOK STREET PORTLAND, MAINE		Revisions:	Checked By:		
			1-11-06 Issued for Permit			

GENERAL NOTES:

- The notes on the drawings are not intended to replace specifications, see specifications for requirements in addition to these notes.
- Structural drawings shall be used in conjunction with job specifications and architectural, mechanical, electrical, plumbing and site drawings. Consult these drawings for locations and dimensions of openings, chases, inserts, relets, sleeves, depressions, and other details not shown on structural drawings.
- All dimensions and conditions must be verified in the field. Any discrepancy shall be reported immediately to the engineer before proceeding with the affected part of the work.
- Do not scale plans.
- Sections and details shown on any structural drawings shall be considered typical for similar conditions.
- All proprietary products shall be installed in accordance with the manufacturer's written instructions.
- The structure is designed to be self supporting and stable after the removal of temporary bracing, shoring and other equipment. It is the responsibility of the contractor to determine the necessity for the addition of necessary shoring, shoring, bracing, guys or tie downs. Such material shall remain the property of the contractor after completion of the project.
- All applicable federal, state and municipal regulations shall be followed, including the Federal Department of Labor Occupational Safety and Health Act.

USUAL LOADS:

- Building code: IBC International Building Code (2003).
- Design Live Loads: (Ground snow load = 60 PSF)
 Roof: 42 PSF + Drift
 Office Mezzanine: 50 psf & peritic
 Common areas: 100 PSF
 Stairs & exit ways: 100 PSF
 Hangar Floor/Storage areas: 125 PSF

- Design wind loads are based on exp e D q 10 m possic wind speed.
- Seismic design per IBC 2001 Code.

FOUNDATION NOTES:

- Foundations have been designed with a presumptive bearing capacity of 2000 PSF to be verified in the field by the general contractor.
- Interior spread footings and exterior strip footings shall be founded on native soil or compacted structural fill.
- Exterior strip and spread footings shall be founded on a minimum of 4'-0" below finished grade.
- Slabs on grade shall bear on a minimum of 12" of compacted structural fill. If loose or undesirable fills are encountered at the surface of the natural soil and replaced with structural fill. Refer to drawings and specifications for vapor barrier requirements.
- Structural fill shall be used at all locations below footings and slabs and adjacent to the foundation walls. Prior to placement of structural fill, remove all topsoil and other unsuitable material. Compacted structural fill shall consist of clean granular material free of organics, loam, trash, snow, ice, frozen soil or any other objectionable material. It shall be well graded within the following units:
 SCREEN OR SIEVE SIZE PERCENT FINER BY WEIGHT
 4 inch 100
 3 inch 90 to 100
 1/4 inch 25 to 90
 NO. 40 0 to 30
 NO. 200 0 to 5
- Structural fill beneath slabs shall be placed in layers not exceeding 6" to 12" in loose measure and compacted by self propelled compaction equipment at appropriate optimum moisture content to a dry density of at least 95% of the maximum in place dry density as determined by the modified proctor test (ASTM D-1557).
- Under drains shall be placed as shown on the site drawings. Under drains shall be installed to positively drain to a suitable discharge point away from the structure. Refer to the site drawings for additional information.
- Exterior concrete slabs on grade, shall be underlain by at least 4 feet of structural fill meeting gradation and compaction requirements noted above. Reinforce slabs with 6x6 - W21xW2.1 WWF.
- Backfill both sides of foundation walls simultaneously.

CONCRETE NOTES:

- All concrete work shall conform to ACI 318-Latest Edition.
- Concrete strength at 28 days shall be:
 a. 3000 PSI for footing, walls, and piers.
 b. 4000 PSI for all slabs on grade.
- All concrete shall be air entrained 4-6% per the specifications.
- Concrete shall not be placed in water or on frozen ground.
- Provide PVC sleeves where pipes pass through concrete walls or slabs.
- Reinforcing bars shall conform to ASTM A615 Grade 60 deformed bars and shall be deformed, fabricated and erected in accordance with ACI 315-Latest edition.
- Welded wire fabric shall be provided in flat slabs.
- Fiber reinforced concrete shall conform to ASTM C-1116.
- Complete shop drawings and schedules of all reinforcing steel shall be prepared by the contractor and submitted to the engineer for review prior to commencement of that portion of work. All accessories must be shown on the shop drawings. Submit (5) blue line prints and (1) reproducible (sepio) to the Architect.
- Splices of reinforcing bars shall be in accordance with ACI 318. Splices of WWF shall be 6" minimum.
- Concrete finishes: See Architectural drawings for additional information.
- Anchor bolts shall conform to ASTM A307 unless noted otherwise on plan.

- Provide control/contraction joints in foundation walls at a maximum spacing of 15 ft. from any corner or 30 ft. along length of wall. At control joints, discontinue every other horizontal bar. At construction joints all reinforcing shall be continuous through the joint.
- The general contractor shall be responsible for coordination of door bond out locations, slab depression and other required bond outs. Coordinate location of bond outs with Architectural, Mechanical & Plumbing, and Electrical drawings as necessary to properly install each specific item.
- Provide control joints in slab (1 1/4" deep x 1/2" wide) at 15' x 15' spacing (225 SF).

STRUCTURAL STEEL NOTES:

- Structural steel fabrication, erection, and connection design shall conform to AISC Specification for the design, fabrication, and erection of structural steel-Latest edition.
- Structural steel:
 a. Structural steel shall conform to ASTM A-36.
 b. Structural tubing shall conform to ASTM A-500 GRB.
 c. Structural pipe shall conform to ASTM A-53, TYPE E or S.
- Design connections for the reactions shown on the drawings or the maximum end reaction that can be produced by a laterally supported uniformly loaded beam for each given beam size and span.
- Field connections shall be bolted using 3/4" A325 high strength bolts except where field welding is indicated on the drawings. All welding shall conform to AWS D1.1-Latest edition. Welding electrodes shall be E70XX.

TIMBER FRAMING:

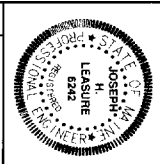
- All timber framing shall be in accordance with the AITC timber construction manual or the national design specifications (NDS) -latest edition.
- Individual timber framing members shall be visually graded, minimum grade #2 Spruce-Pine-Fir (SPF), kiln dried to 19% maximum moisture content.
- Pressure treated lumber shall be used where wood is in contact with ground, concrete or masonry. Timber shall be southern yellow pine treated with cca to 0.4 #/CF in accordance with AWPA C-18.
- Metal connectors shall be used at all timber to timber connections or as noted on the design drawings.
- Provide Simpson Hi hurricane anchors where timber framing and/or trusses bear on structural steel beams or bearing walls.
- Nailing not specified shall conform with BOCA 1993.
- Roof sheathing shall be 5/8" APA rated plywood w/ H-clips. Attach plywood to all supports using 10d nails spaced at 6" o.c. at panel edges and 12" o.c. at intermediate supports.
- Wall sheathing shall be 1/2" APA rated plywood. Attach plywood to all supports using 10d nails spaced at 4" o.c. at panel edges and 6" o.c. at intermediate supports. All panel edges shall be blocked to oil sheathing shall be 3/4" T&G APA rated "ADVANTEC". Attach ply o 4 to all supports using 10d nails spaced at 6" o.c. at panel edges and 12" o.c. at intermediate supports. All panel nailed and gued to the timber floor framing.
- Ceiling sheathing shall be 1/2" APA rated plywood. Attach plywood to all supports using 10d nails spaced at 6" o.c. at panel edges and 12" O.C. at intermediate supports.

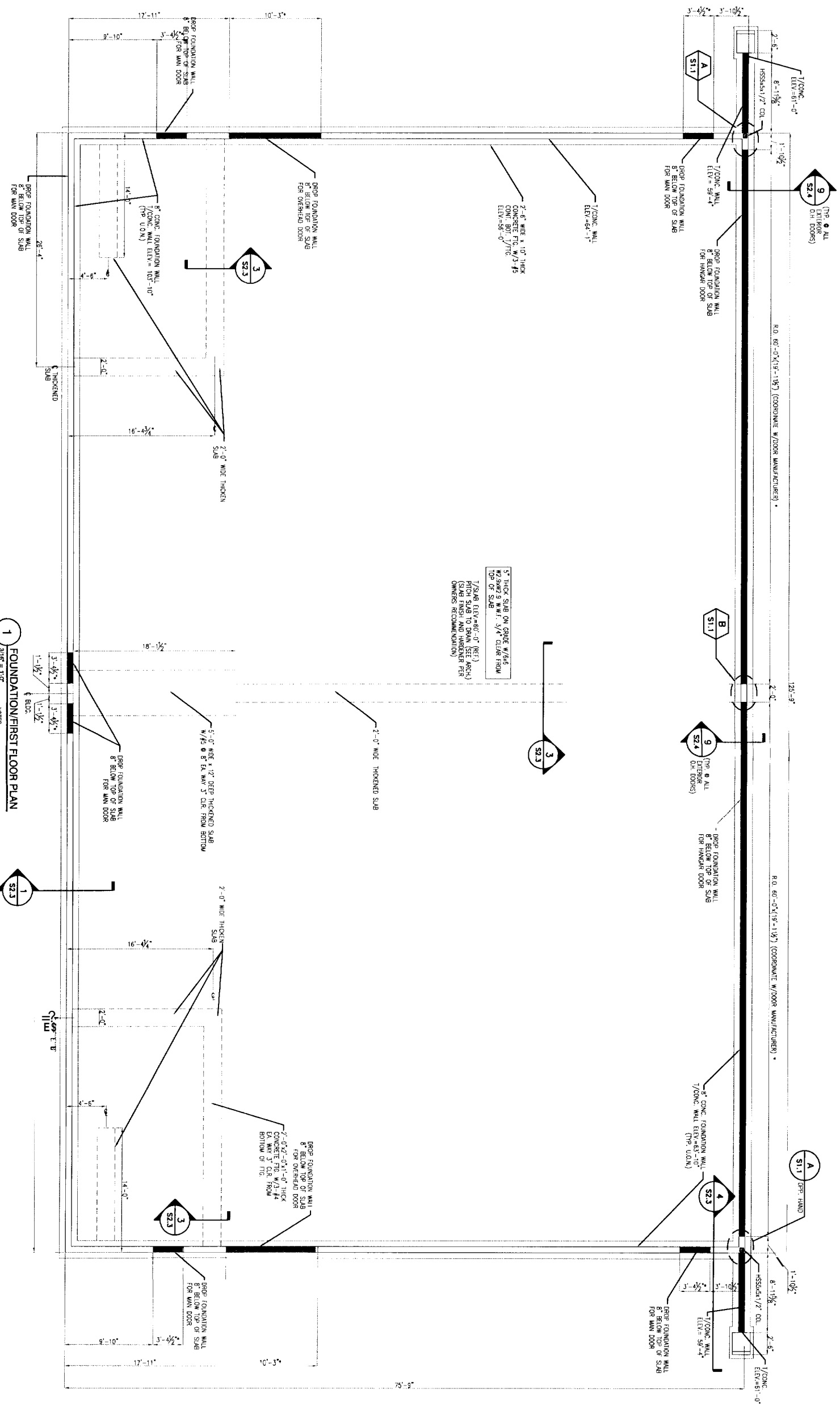
TIMBER TRUSS FRAMING:

- Material: Stress graded lumber, metal plate connectors. Minimum grade No. 2 M.S.R. Southern Pine, kiln dried, 15% maximum M.C., or approved alternate.
- Applicable specifications:
 a. National Design Specification for stress graded lumber and its fastening (NDS).
 b. Design specifications for light metal plate connected wood trusses (TPI-Latest edition).
- Bracing: The truss manufacturer shall specify all bracing required both for temporary construction loading and for permanent lateral support of compression members.
- Submittals:
 a. Submit design calculations, shop drawings and erection procedures all affixed with the seal of a professional structural engineer registered in the State of Maine.
 b. Shop drawings shall show stress grade and size of members, size and location of plate connectors, size and location of bracing and shall be approved by the truss designer.
- All fabricated trusses shall be inspected by the fabrication plant and approved trusses shall receive the TPI mark of approval in accordance with the truss plate institute in-plant inspection license agreement.
- Connector plates shall be galvanized.
- Timber trusses shall be designed in accordance with BOCA and ASCE 7-98.
- Provide temporary and permanent bottom chord and web bracing in accordance with the truss plate institute (TPI-latest edition).
- Trusses shall be designed for all potential load combinations of live loads (snow) and wind loads including unbalanced snow loads, drift loads and wind loads in accordance with BOCA 1993.

PRECAST CONCRETE

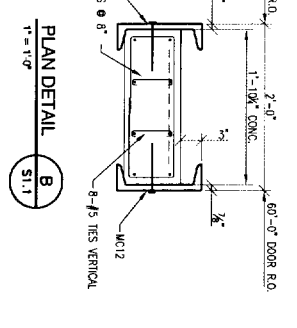
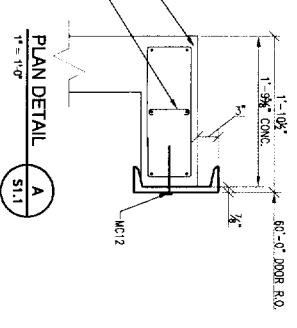
- Structural precast concrete shall conform to the Pre-stressed Concrete Institute's manual for quality control, publication WNL 116.
- Extent of structural precast concrete work is shown on drawings and in schedules.
- Structural precast concrete includes all 8" hollow core panels.
- Reinforcing Bars: ASTM A 615, Grade 60.
- Tendons shall be 7 wire stress relieved strand complying with ASTM A 416. Use 270 ksi 7 wire strand.
- Install tendons to produce units having a UL fire rating of 1 hour.
- Steel wire shall meet ASTM A 82, plain, cold-drawn, steel.
- Welded Wire Fabric: ASTM A 185.
- Deformed Welded Wire Fabric: ASTM A 497.
- Portland Cement: ASTM C 150, Type I or Type III.
- Aggregates: ASTM C 33. Provide aggregates from a single source for exposed concrete.
- Water shall be potable and free from foreign materials in amounts harmful to concrete and embedded steel.
- Air-Entraining Admixture: ASTM C 260
- Water-Reducing Admixture: ASTM C 494, Type A. Types B, C, D or E may be used, subject to the Architect's approval.
- Accessories: Provide clips, hangers, and other accessories of standard construction or finishes.
- Compressive strength: 5000 psi minimum at 28 days.
- Release strength for pre-stressed units:(fc'): 3500 psi.
- Use air-entraining admixture in concrete, unless otherwise indicated.
- Bearing Pads (1/8"x3"): Provide bearing pads for precast hollow slab units: KORO-LATH or approved alternate.
- Install flexible bearing pads where indicated, as precast units are being erected. Set pads on level, uniform bearing surfaces and maintain in correct position until precast units are placed.
- Powder-Actuated Fasteners are not permitted for surface attachment of precast units. Fasteners shall be installed by a professional engineer or architect and engineer of record.
- Grouting Connections and Joints: After precast concrete units have been placed and secured, grout open spaces at connection and joints where shown on the plans.
- Provide precast lintel were shown on the architectural drawings in brick veneer system. Lintels shall have 2#4 bars T&B typ.

	<p style="text-align: center;">L & L STRUCTURAL ENGINEERING SERVICES, INC.</p> <p style="text-align: center;">SIX Q STREET SOUTH PORTLAND, MAINE 04106</p> <p style="text-align: center;">PHONE: (207) 767-4830 FAX: (207) 799-5432</p>	<table border="1" style="width: 100%; border-collapse: collapse;"> <tr><td>designed by: JHL</td></tr> <tr><td>drawn by: LEM</td></tr> <tr><td>checked by: JHL</td></tr> <tr><td>scale: 3/16" = 1'-0"</td></tr> <tr><td>date: OCT. 04, 2005</td></tr> <tr><td>plot date: OCT 04, 2005</td></tr> <tr><td>project #:</td></tr> </table>	designed by: JHL	drawn by: LEM	checked by: JHL	scale: 3/16" = 1'-0"	date: OCT. 04, 2005	plot date: OCT 04, 2005	project #:	<table border="1" style="width: 100%; border-collapse: collapse;"> <tr><td>rev</td><td>date</td><td>description</td></tr> <tr><td>1</td><td>10-19-05</td><td>GENERAL REVISIONS</td></tr> <tr><td>2</td><td>01-11-06</td><td>MULTIPLE CHANGES TO DWGS</td></tr> </table>	rev	date	description	1	10-19-05	GENERAL REVISIONS	2	01-11-06	MULTIPLE CHANGES TO DWGS
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<h1 style="font-size: 2em; margin: 0;">SO.1</h1>	<p>AIPLANE HANGAR WESTBROOK STREET PORTLAND, MAINE</p> <p>PRECAST PLANK & MASONRY NOTES</p>																		



1 FOUNDATION/FIRST FLOOR PLAN
 1/8" = 1'-0"

- NOTES:
 1) SEE GENERAL NOTES ON DWG. S0.1
 2) TOP OF SLAB ELEVATION = 60'-0" (REFERENCE)
 3) * C.C. SHALL CONDUIT DOOR ROUGH OPENINGS W/CONCRETE BOND COLE



S1.1

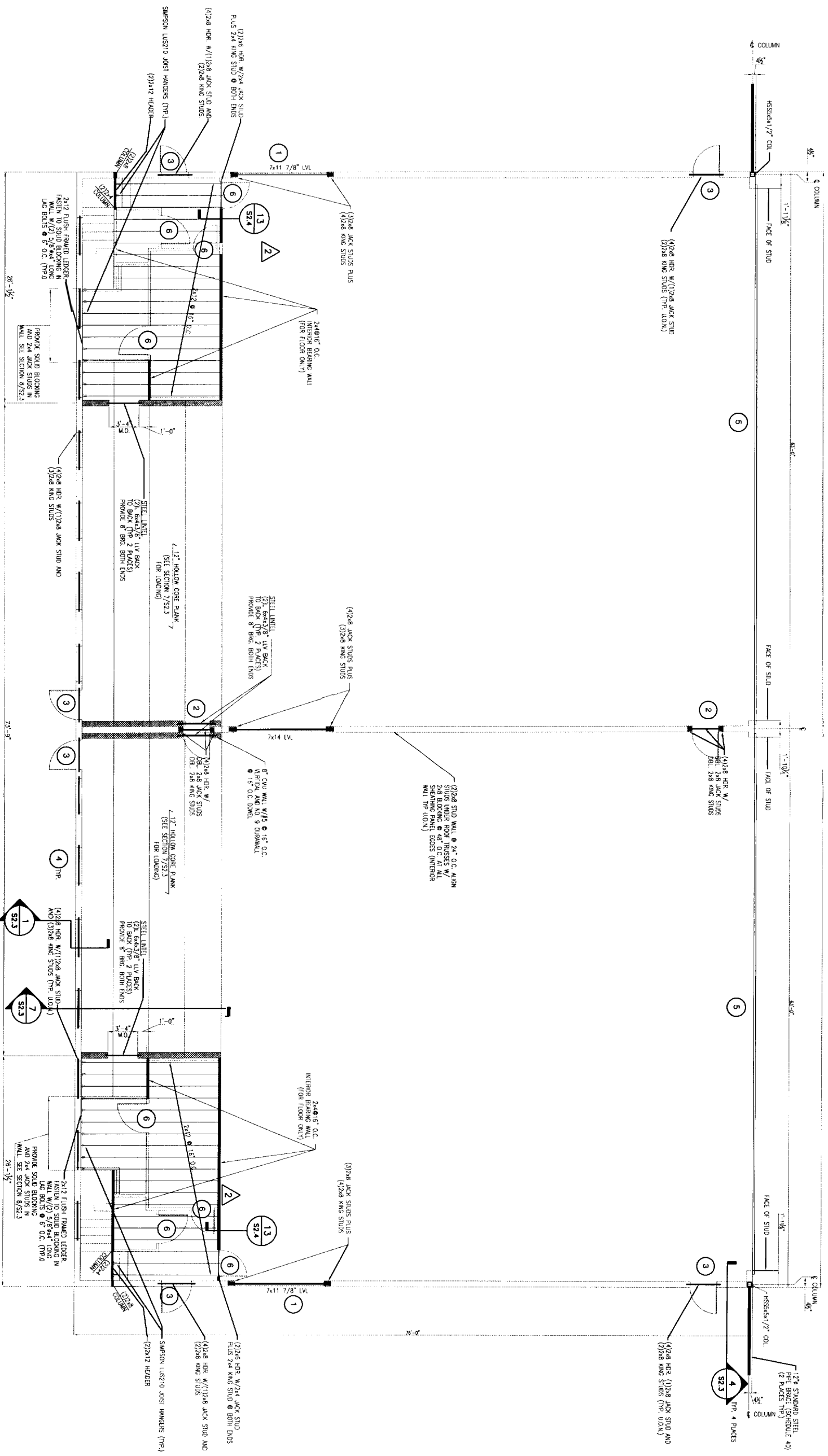
AIRPLANE HANGAR
 WESTBROOK STREET
 PORTLAND, MAINE
 FOUNDATION PLAN

rev	date	description	app'd
1	10-19-05	GENERAL REVISIONS	
2	01-11-06	MULTIPLE CHANGES TO DWGS.	

designed by: JHL
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**L & L STRUCTURAL
 ENGINEERING SERVICES, INC.**
 SIX O STREET
 SOUTH PORTLAND, MAINE 04106
 PHONE: (207) 767-4830
 FAX: (207) 799-5432



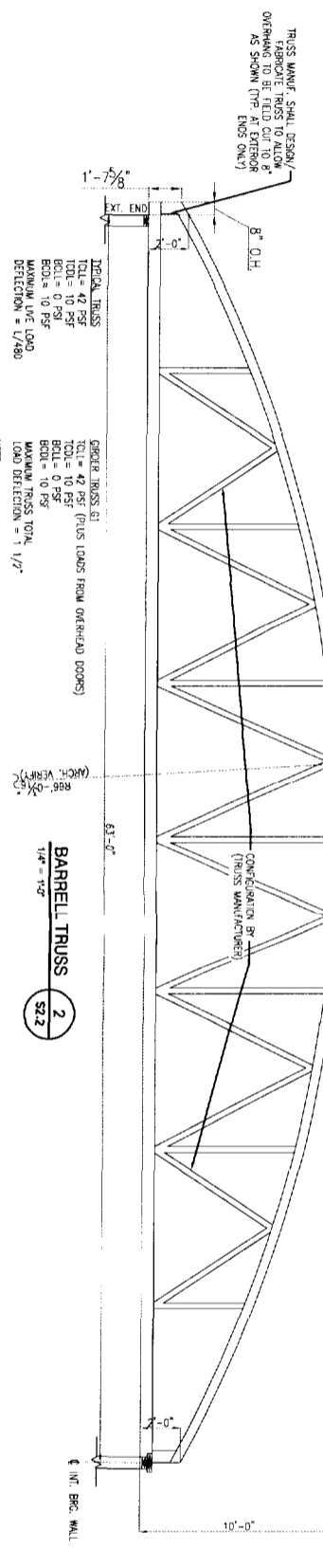
DOOR/WINDOW R.O. SCHEDULE

NO.	R.O.	HEADER HEIGHT	SILL HEIGHT
1	10'-3 1/2" - 1 1/2"	12'-1 1/2" A.F.F.	-----
2	5'-4 1/2" - 2 1/2"	7'-3 1/2" A.F.F.	-----
3	5'-4 1/2" - 2 1/2"	7'-3 1/2" A.F.F.	-----
4	4'-0" - 0"	B-1 1/2" R. 17'-8" A.F.F.	4'-1 1/2" R. 13'-8" A.F.F.
5	80'-0" - 9'-11 1/2"	19'-11 1/2" A.F.F.	-----
6	5'-1 1/2" - 1"	7'-1" A.F.F.	-----

MEZZANINE FLOOR FRAMING PLAN
3/16" = 1'-0"

- NOTES:**
- SEE GENERAL NOTES ON DWG. S01.
 - FOR HEADER NOT INDICATED SEE FOOT FRAMING PLAN ON DWG. S22.
 - ALL JOISTS LAMINATED VENEER LUMBER (LVL) SHALL BE INSTALLED BY BRIDGE ORSCORP OR APPROVED EQUAL.
 - EXTERIOR WALL SHALL BE (2)X8 @ 24" O.C. (ALONG ROOF TRUSSES W/ WALL STUDS) WALL STUDS ABOVE AND BELOW DOORS AND WINDOWS SHALL BE 2X8 @ 24" O.C. ALONG W/ TRUSSES.
 - INTERIOR 2X8 WALL SHALL BE (2)X8 @ 24" O.C. (ALONG ROOF TRUSSES W/ WALL STUDS).
 - DIAGONAL BRACK SHALL BE 4" X 4" STD. STEEL PIPE (SCHED. 40) AT INTERMEDIATE BRACKS.
 - MINIMUM SIZES - OWNER TO VERIFY SIZES. ENGINEER SHALL VERIFY CONNECTIONS DURING SHOP DWG. REVIEW.

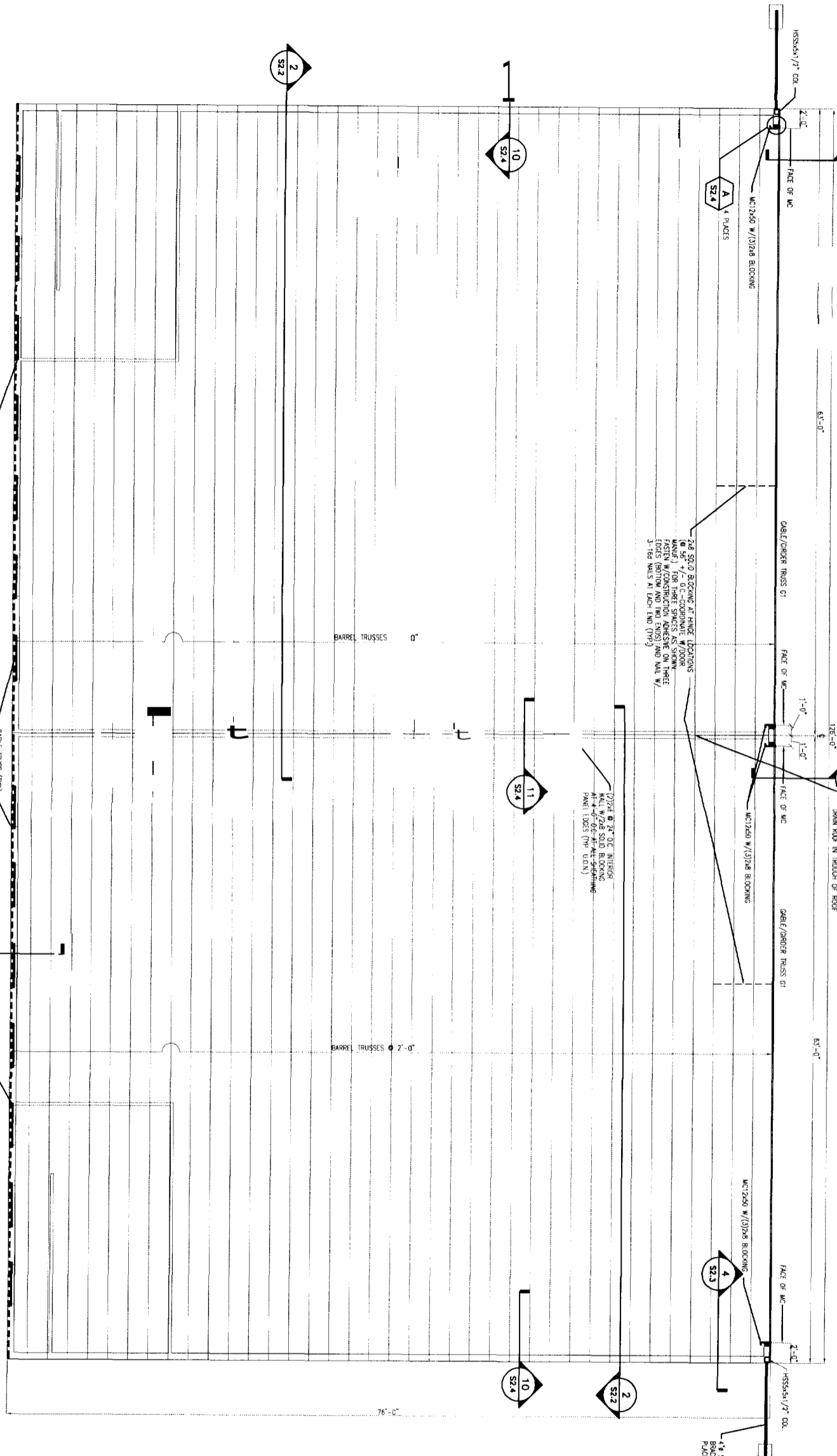
	AIRPLANE HANGAR WESTBROOK STREET PORTLAND, MAINE MEZZANINE FLOOR FRAMING PLAN	designed by: JHL drawn by: LEM checked by: JHL scale: 3/16" = 1'-0" date: SEPT 28, 2005 plot date: SEPT 28, 2005 project #	<table border="1"> <thead> <tr> <th>rev</th> <th>date</th> <th>description</th> </tr> </thead> <tbody> <tr> <td>1</td> <td>10-19-05</td> <td>GENERAL REVISIONS</td> </tr> <tr> <td>2</td> <td>01-11-06</td> <td>MULTIPLE CHANGES TO DWGS</td> </tr> </tbody> </table>	rev	date	description	1	10-19-05	GENERAL REVISIONS	2	01-11-06	MULTIPLE CHANGES TO DWGS		L & L STRUCTURAL ENGINEERING SERVICES, INC. SIX O STREET SOUTH PORTLAND, MAINE 04106 PHONE: (207) 767-4830 FAX: (207) 799-5432
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		APPROVED: _____ DATE: _____												



GENERAL TRUSS E1
 TOL = 42 PSF (PLUS LOADS FROM OVERHEAD DOORS)
 DEAD = 0 PSF
 BOLL = 0 PSF
 BOLL = 10 PSF
 MAXIMUM LIVE LOAD
 DEFLECTION = L/740

NOTE:
 TRUSS MANUF. TO CORRODATE
 WITH MANUF. OF MAIN MEMBERS
 (PANEL LOADS INDICATED FOR MANUF.)
 425# VERTICAL
 1070# HORIZONTAL

BARREL TRUSS 2
 1/4\"/>



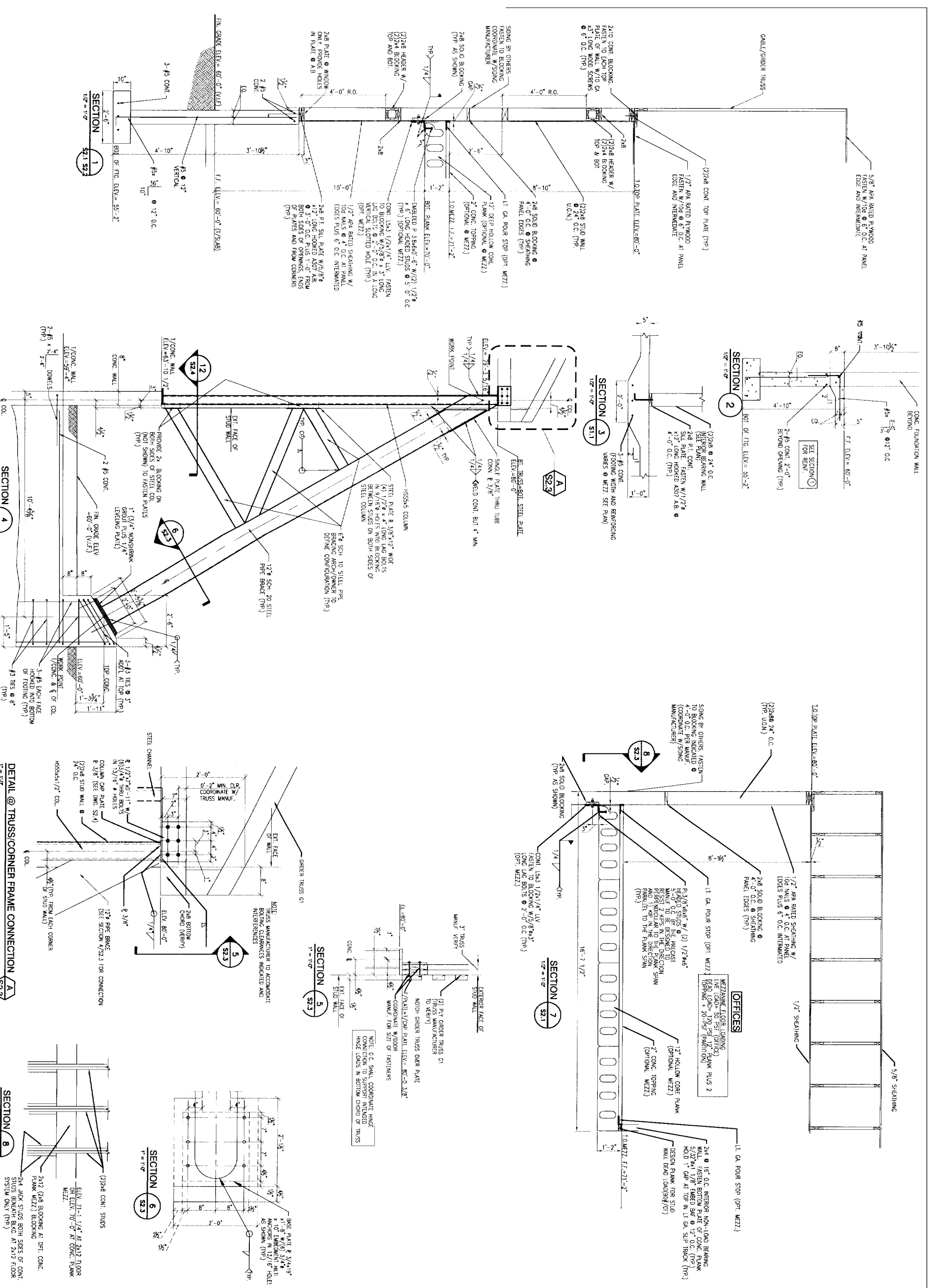
ROOF FRAMING PLAN
 3/16\"/>


(4) 2x8 HBR SINGLE 2x8 JACK
 (3) 2x8 KINGS (TR. U.O.N.)

NOTE:
 1) SEE GENERAL NOTES ON DWG S2.1
 2) FOR HEADERS NOT INDICATED SEE
 MEZZANINE FRAMING PLAN ON DWG S2.1

S2.2	AIRPLANE HANGAR WESTBROOK STREET PORTLAND, MAINE	<table border="1"> <thead> <tr> <th>rev</th> <th>date</th> <th>description</th> <th>appr'd</th> </tr> </thead> <tbody> <tr> <td>1</td> <td>10-19-05</td> <td>GENERAL REVISIONS</td> <td></td> </tr> <tr> <td>2</td> <td>01-11-06</td> <td>MULTIPLE CHANGES TO DWGS.</td> <td></td> </tr> </tbody> </table>	rev	date	description	appr'd	1	10-19-05	GENERAL REVISIONS		2	01-11-06	MULTIPLE CHANGES TO DWGS.	
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2	01-11-06	MULTIPLE CHANGES TO DWGS.												
ROOF FRAMING PLAN														

L & L STRUCTURAL
 ENGINEERING SERVICES, INC.
 516 O STREET
 SOUTH PORTLAND, MAINE 04106
 PHONE: (207) 767-4830
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<h1 style="margin: 0;">S2.3</h1>	<p>AIRPLANE HANGAR WESTBROOK STREET PORTLAND, MAINE</p> <p>DETAILS & SECTIONS</p>	<table border="1" style="width: 100%; border-collapse: collapse; font-size: 8px;"> <tr><th>designed by</th><td>JH</td></tr> <tr><th>drawn by</th><td>LEM</td></tr> <tr><th>checked by</th><td>JH</td></tr> <tr><th>scale</th><td>AS NOTED</td></tr> <tr><th>date</th><td>OCT. 04, 2005</td></tr> <tr><th>plot date</th><td>OCT. 04, 2005</td></tr> <tr><th>project #</th><td></td></tr> </table>	designed by	JH	drawn by	LEM	checked by	JH	scale	AS NOTED	date	OCT. 04, 2005	plot date	OCT. 04, 2005	project #		<table border="1" style="width: 100%; border-collapse: collapse; font-size: 8px;"> <tr><th>rev</th><th>date</th><th>description</th></tr> <tr><td>1</td><td>10-19-05</td><td>GENERAL REVISIONS</td></tr> <tr><td>1</td><td>01-11-06</td><td>MULTIPLE CHANGES TO DWGS</td></tr> </table>	rev	date	description	1	10-19-05	GENERAL REVISIONS	1	01-11-06	MULTIPLE CHANGES TO DWGS	<div style="text-align: center;">  <p>JOSEPH LEASURE LICENSED PROFESSIONAL ENGINEER STATE OF MAINE 6242</p> </div>	<p style="text-align: center;">L & L STRUCTURAL ENGINEERING SERVICES, INC. SIX C STREET SOUTH PORTLAND, MAINE 04106</p> <p style="text-align: center;">PHONE: (207) 767-4830 FAX: (207) 799-5432</p>
designed by	JH																											
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14
S2.4

10.00' PLATE ELEV.=80'-0"
1/2" O.P. PANEL

(4) 2x8 JACKS EA. SIDE OF OVERHEAD DOOR

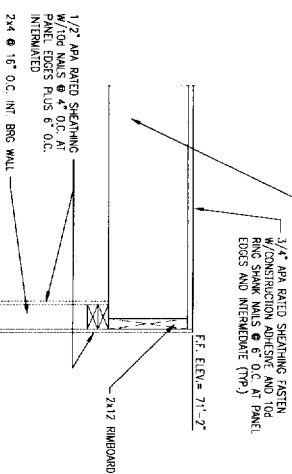
NOTE: OVERHEAD DOOR MANUFACTURER IS RESPONSIBLE FOR CONNECTIONS OF DOOR TO W/2.

2x8 P.1 SILL PLATE W/2/8" x 1x2" LONG HOOKED A307 A.B. @ 3'-0" O.C.

(3) 3/4"x6 5/8" EMBED HIT MIX ANCHORS AND CENTER OF 8" CONC. WALL O.C. SHALL NOT INTERFERE W/DOOR

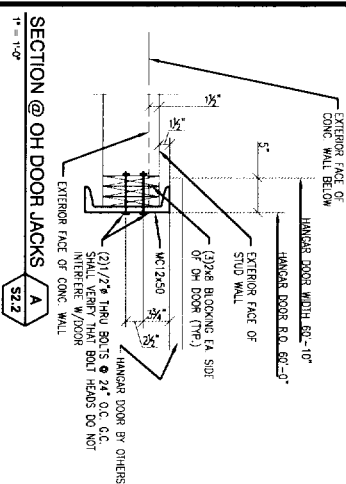
BOT. OF TRG. ELEV. = 55'-2"

SECTION 9
1/2" = 1'-0" S2.1

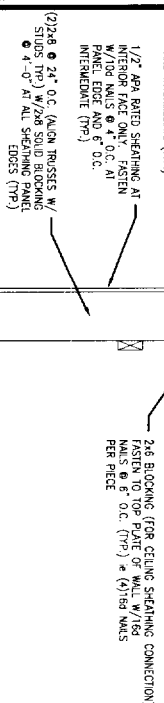


INTERIOR BEARING WALL DETAIL S2.1
1" = 1'-0"

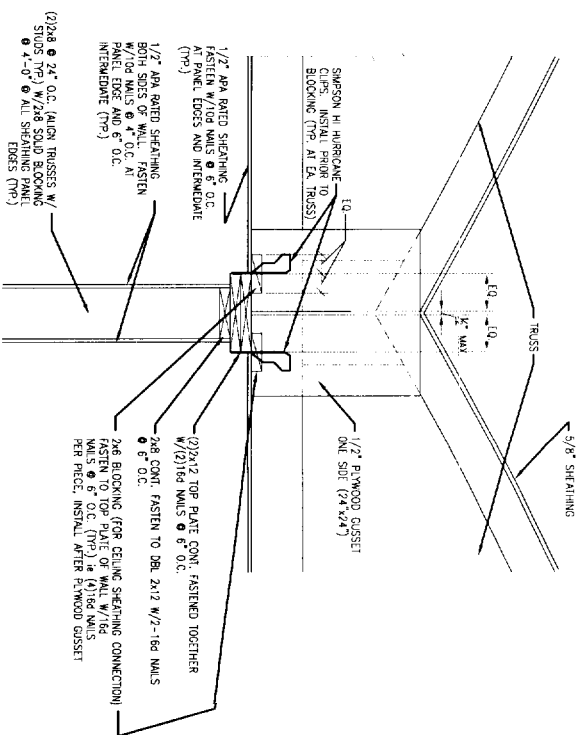
13
S2.4



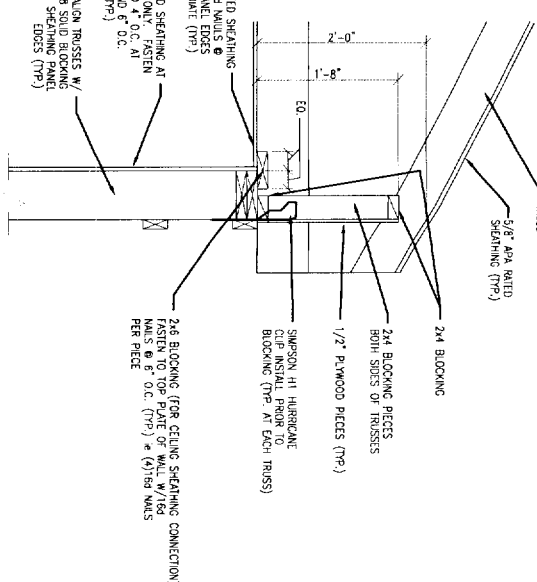
SECTION @ OH DOOR JACKS S2.2
1" = 1'-0"



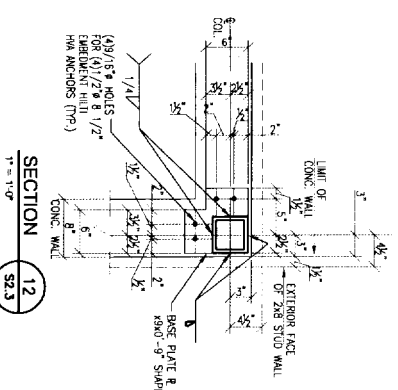
DETAIL @ TRUSS TO EXT. WALL CONNECTION S2.2
1" = 1'-0"



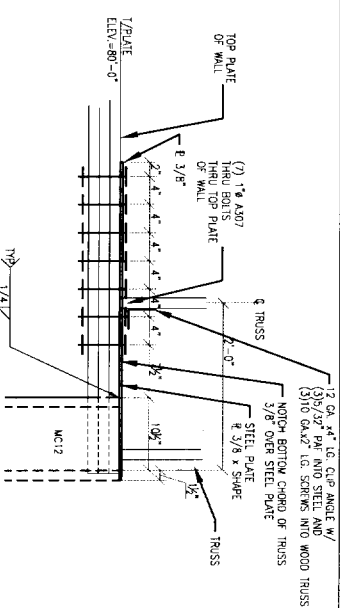
DETAIL @ TRUSS TO EXT. WALL CONNECTION S2.2
0-1" = 1'-0"



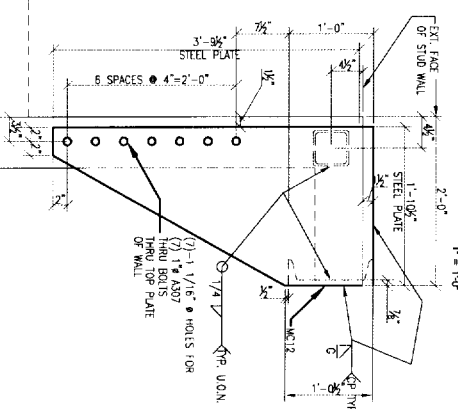
DETAIL @ TRUSS TO EXT. WALL CONNECTION S2.2
1" = 1'-0"



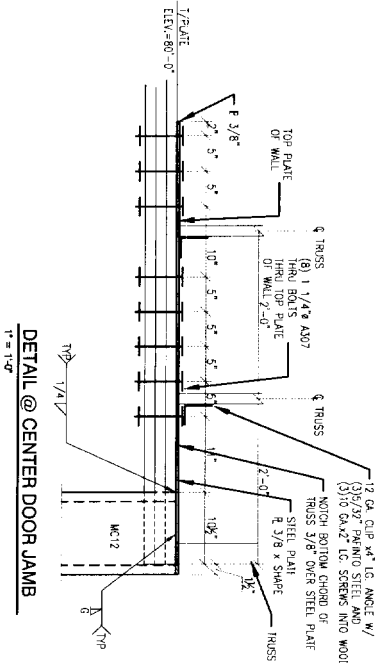
SECTION 12
1" = 1'-0" S2.3



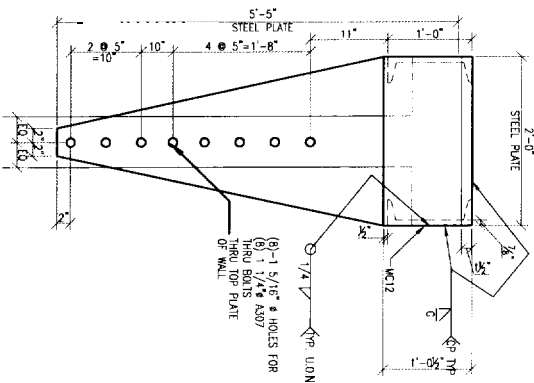
DETAIL @ DOOR JAMB
1" = 1'-0"



PLAN VIEW @ CORNER DOOR JAMB
1" = 1'-0"



DETAIL @ CENTER DOOR JAMB
1" = 1'-0"



PLAN VIEW @ CENTER DOOR JAMB
1" = 1'-0"



L L STRUCTURAL ENGINEERING SERVICES, INC.

SIX O STREET
SOUTH PORTLAND, MAINE 04106

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AIRPLANE HANGAR
WESTBROOK STREET
PORTLAND, MAINE

DETAILS & SECTIONS

S2.4