FRAMING PLAN

Scale: 3/16"=1'-0"

FRONT ELEVATION

Scale: 3/16"=1'-0"

G2 E1 SECONDARY DRAIN OVERFLOW DETAIL

ALL FASTENERS - #10x3/4" TEKS @ 24" O.C. AND/OR AT EA. OUTRIGGER UNLESS NOTED OTHERWISE.

1 E1 FASCIA SECTION

G9 E1 ADJUSTABLE DRAIN DETAIL
NO SCALE

ERECTOR NOTE:
PULL DRAIN ELBOW END THRU HOLE AND GLUE 4"ϕ NIPPLE INTO ELBOW AND GLUE 4"ϕ COUPLING INTO NIPPLE.

DRAIN ASSEMBLY DETAIL

STEEL NOTES
1. DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO THE LATEST AISC SPECIFICATIONS. DESIGN, FABRICATION AND ERECTION OF COLD FORMED STEEL SECTIONS SHALL CONFORM TO THE LATEST AISI SPECIFICATIONS.
2. STRUCTURAL MATERIALS:
WIDE FLANGE SECTIONS - ASTM A992 OR A572 GRADE 50 (Fy = 50 KSI)
ANGLES / CHANNELS - ASTM A36 (Fy = 36 KSI)
HOLLOW STRUCTURAL SECTIONS (TUBE) - ASTM A500 GRADE B (Fy = 46 KSI)
PIPE SECTIONS - ASTM A53, GRADE B (Fy = 35 KSI)
PLATE - ASTM A36 (Fy = 36 KSI)
ROOF DECK - ASTM A653, GRADE 50 (Fy = 50 KSI), GALVANIZED (G60) WITH BAKED ENAMEL FINISH
STEEL OUTRIGGERS - ASTM A653 GR. CS (Fy = 25 KSI), GALVANIZED (G90) PER ASTM 924
STRUCTURAL BOLTS - ASTM A325
ANCHOR BOLTS - ASTM F1554 GR. 36; ASTM A36; OR ASTM A307 MATERIAL (Fy = 36 KSI)
3. WELDING OF STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH LATEST ANSI / AWS D1.1
4. FIELD CONNECTIONS SHALL BE BOLTED CONNECTIONS UNLESS SPECIFIED ON DRAWING.
5. ALL STRUCTURAL BOLTED CONNECTIONS SHALL USE ASTM A325 BOLTS. BOLTED JOINTS SHALL BE TIGHTENED TO SNUG TIGHT PER LATEST RCSC SPECIFICATION.
6. STRUCTURAL STEEL SHALL BE SHOP COATED WITH A RED-OXIDE RUST INHIBITIVE PRIMER. FIELD TOUCH-UP, FINISH PAINTING, AND MAINTENANCE SHALL BE THE RESPONSIBILITY OF THE OWNER (UNLESS OTHERWISE SPECIFIED).
7. DESIGN LOADS PER LOCAL BUILDING CODE REQUIREMENTS:

ROOF LIVE LOAD = 20 PSF

FLAT ROOF SNOW LOAD = 50 PSF
BASED ON GROUND SNOW LOAD= 50 PSF

WIND LOADS:
LATERAL = 25 PSF
UPLIFT = 20 PSF
BASED ON 100 MPH 3 SECOND GUST WIND SPEED PER ASCE 7-05
IMPORTANCE FACTOR = 1.0

SEISMIC LOADS:
SEISMIC USE GROUP I, SITE CLASS "D" ASSUMED, SEISMIC DESIGN CATEGORY "B"
Sds = 0.32G, Sd1 = 0.100G
SEISMIC FORCE RESISTING SYSTEM IS INVERTED PENDULUM - CANTILEVERED COLUMN, R = 2.0
Cs = 0.16, DESIGN BASE SHEAR = CsxW = 2.63 K/COL USING EQUIVALENT LATERAL FORCE PROCEDURE

DEAD LOADS:
DECK / GUTTER / LIGHTS - 5 PSF
FASCIA - 5 TO 15 PLF (PER DESIGN)
STRUCTURAL STEEL - SELF WT
CONCRETE - 145 PCF

S2 E1 CROSSBEAM TO COLUMN CONNECTION

S3 E1 CROSSBEAM TO PURLIN CONNECTION

ISSUE	DESCRIPTION	BY	CHK.	DATE
0	APPROVAL / PERMIT	FCS	DES	9/13/2013

REVISIONS

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LICENSE# 9549

STATE OF MAINE DAVID E. SCHATTE No. 9549 LICENSED PROFESSIONAL ENGINEER
09/16/13
Job # 408

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SITE:
HERTZ
1049 WESTBROOK ST.
PORTLAND, ME

SIZE: 28'-0"X42'-0" 2-COLUMN CANOPY

JOB NO. 40806

TITLE: FRAMING PLAN AND DETAILS

DRAWING NO. E1 OF 1

CITY OF PORTLAND, MAINE RESURGAM
Reviewed for Code Compliance
Inspections Division
Approved with Conditions
Date: 10/31/13