R. W. Gillespie & Associates, Inc.

86 Industrial Park Road, Suite 4, Saco, ME 04072 207-286-8008

200 International Drive, Suite 170, Portsmouth, NH 03801 603-427-0244

LETTER OF TRANSMITTAL

Date:	Project No.:
July 20, 2010	557-14
Attention: Mr. Cuyler Feagles (cmf@portlandmaine.gov)	
Re: Concrete Testing Terminal Enhancement, Portland Int. Jetport Portland, Maine	

City of Portland, Portand Int. Jetport

1001 Westbrook Street

Portland, Maine 04102

We are sending you attached concrete cylinder test results.

Cylinder No. (s)	Age (Days)
65935	28
65936	28
65939	28
65940	28
65943	28
65944	28

Remarks:

Copy To:
Roy Williams: rsw@portlandmaine.gov
Jim Stanislaski: jim_stanislaski@gensler.com
Cliff Takara: clifford_takara@gensler.com
Lacey Fogg: Lacey.Fogg@amec.com
Mike Fusco: mfusco@tcco.com
Shaun Winner: swinner@tcco.com
Phil Coleman: pcoleman@tcco.com
Elizabeth O'Toole: eotoole@tcco.com
TMM@portlandmaine.gov
ldobson@portlandmaine.gov
rdixon@tcco.com
gemitchell@tcco.com

Signed: Bertha Dawn

If enclosures are not as noted, kindly notify us at once.

R. W. GILLESPIE & ASSOCIATES, INC.

86 Industrial Park Road, Suite 4, Saco, ME 04072 207-286-8008
200 International Drive, Suite 170, Portsmouth, NH 03801 603-427-0244

CONCRETE TEST/PLACEMENT REPORT

Project Name: Terminal Enhancement, Portland Int. Jetport
Project No: 557-14
Weather Conditions: Sunny
Method of Placement: Pump
Admixtures: Micro Air, Pozzolith, Glenium 7500
Placement Location: Footings & Foundation Walls - Zone 4 & 5
Test Cylinder Location: Footing XF/Z4

Date Cylinders Cast: 22-Jun-10
Concrete Supplier: Auburn
General Contractor: Turner
Design Strength: 4,000
Max Agg. Size: 3/4

Date Report Issued: JUL 2 1 2010

4x8 Cylinders	4	Cast by	Rodney R. Collard	Time	
Load No.	1	Slump (in) ASTM C 143	5.0	Batched @	12:01
Ticket No.	173033	Air (°F)	80	Arrived @	--
Truck No.	94	Concrete (°F) ASTM C 1064	74	Total Time	39
Cubic Yds.	10	Air Content (%) ASTM C 231	5.9		

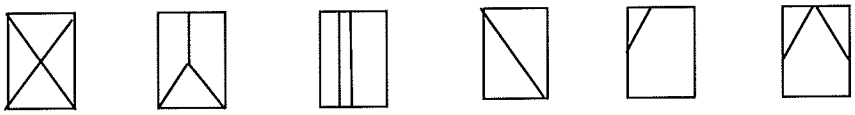
*Concrete sampled by ASTM C 172

Specimen Storage ASTM C 31: Field cure days: 1
Date received 23-Jun-10
Condition of Cylinders: Good

Lab No.	Test Date	Avg Dia (in)	Area (in²)	Age (Days)	Load (lbs)	Compressive Strength (psi)	Break type
65934	29-Jun-10	4.018	12.68	7	48,160	3800	6
65935	20-Jul-10	4.017	12.67	28	69,040	5450	2
65936	20-Jul-10	4.017	12.67	28	67,540	5330	2
65937	HOLD			HOLD			

*Concrete compressive strength by ASTM C 39

Types of Breaks



Cone 1 | Cone & Split 2 | Columnar 3 | Shear 4 | Side Fracture 5 | Double Side Fracture 6

Load	Ticket Number	Truck Number	Cubic Yds	Slump (inches)	Air Temp (°F)	Conc Temp (°F)	(%) Air Content	Time (min.)
2	173034	99	10	--	--	--	--	33
3	173035	101	10	--	--	--	--	28
4	173036	82	10	--	--	--	--	33
5	173037	97	10	--	--	--	--	--

Remarks: Total loads = 13
Curing Temperatures: Max = 88°, Min = 65°

Checked by: Matthew T. Grady, Manager of MTS

R. W. GILLESPIE & ASSOCIATES, INC.

86 Industrial Park Road, Suite 4, Saco, ME 04072 207-286-8008
200 International Drive, Suite 170, Portsmouth, NH 03801 603-427-0244

CONCRETE TEST/PLACEMENT REPORT

Project Name: Terminal Enhancement, Portland Int. Jetport
Project No: 557-14
Weather Conditions: Sunny
Method of Placement: Pump
Admixtures: Micro Air, Pozzolith, Glenium 7500
Placement Location: Footings & Foundation Walls - Zone 4 & 5
Test Cylinder Location: Footing XF/Z6

Date Cylinders Cast: 22-Jun-10
Concrete Supplier: Auburn
General Contractor: Turner
Design Strength: 4,000
Max Agg. Size: 3/4

Date Report Issued: JUL 2 1 2010

4x8 Cylinders	4	Cast by	Rodney R. Collard	Time	
Load No.	6	Slump (in) ASTM C 143	5.0	Batched @	1:13
Ticket No.	173038	Air (°F)	80	Arrived @	1:30
Truck No.	--	Concrete (°F) ASTM C 1064	75	Total Time	32
Cubic Yds.	10	Air Content (%) ASTM C 231	5.0		

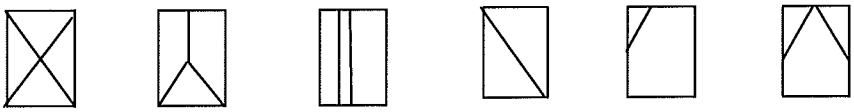
*Concrete sampled by ASTM C 172

Specimen Storage ASTM C 31: Field cure days: 1
Date received 23-Jun-10
Condition of Cylinders: Good

Lab No.	Test Date	Avg Dia (in)	Area (in²)	Age (Days)	Load (lbs)	Compressive Strength (psi)	Break type
65938	29-Jun-10	4.018	12.68	7	49,920	3940	2
65939	20-Jul-10	4.017	12.67	28	71,820	5670	2
65940	20-Jul-10	4.017	12.67	28	71,860	5670	2
65941	HOLD			HOLD			

*Concrete compressive strength by ASTM C 39

Types of Breaks



Cone 1 | Cone & Split 2 | Columnar 3 | Shear 4 | Side Fracture 5 | Double Side Fracture 6

Load	Ticket Number	Truck Number	Cubic Yds	Slump (inches)	Air Temp (°F)	Conc Temp (°F)	(%) Air Content	Time (min.)
7	173039	116	10	--	--	--	--	34
8	173040	84	10	--	--	--	--	35
9	173041	82	10	--	--	--	--	--

Remarks: Total loads = 13
Curing Temperatures: Max = 88°, Min = 65°

Checked by: _____
Matthew T. Grady, Manager of MTS

R. W. GILLESPIE & ASSOCIATES, INC.

86 Industrial Park Road, Suite 4, Saco, ME 04072 207-286-8008
200 International Drive, Suite 170, Portsmouth, NH 03801 603-427-0244

CONCRETE TEST/PLACEMENT REPORT

Project Name: Terminal Enhancement, Portland Int. Jetport
Project No: 557-14
Weather Conditions: Sunny
Method of Placement: Pump
Admixtures: Micro Air, Pozzolith, Glenium 7500
Placement Location: Footings & Foundation Walls - Zone 4 & 5
Test Cylinder Location: Walls F.5/1ZB.4

Date Cylinders Cast: 22-Jun-10
Concrete Supplier: Auburn
General Contractor: Turner
Design Strength: 4,000
Max Agg. Size: 3/4

Date Report Issued: JUL 2 1 2010

4x8 Cylinders	4	Cast by	Rodney R. Collard	Time	
Load No.	10	Slump (in) ASTM C 143	5.0	Batched @	2:32
Ticket No.	173043	Air (°F)	80	Arrived @	2:50
Truck No.	98	Concrete (°F) ASTM C 1064	74	Total Time	43
Cubic Yds.	10	Air Content (%) ASTM C 231	4.6		

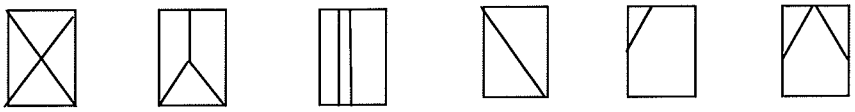
*Concrete sampled by ASTM C 172

Specimen Storage ASTM C 31: Field cure days: 1
Date received 23-Jun-10
Condition of Cylinders: Good

Lab No.	Test Date	Avg Dia (in)	Area (in²)	Age (Days)	Load (lbs)	Compressive Strength (psi)	Break type
65942	29-Jun-10	4.018	12.68	7	47,540	3750	2
65943	20-Jul-10	4.017	12.67	28	70,200	5540	5
65944	20-Jul-10	4.017	12.67	28	71,180	5620	2
65945	HOLD			HOLD			

*Concrete compressive strength by ASTM C 39

Types of Breaks



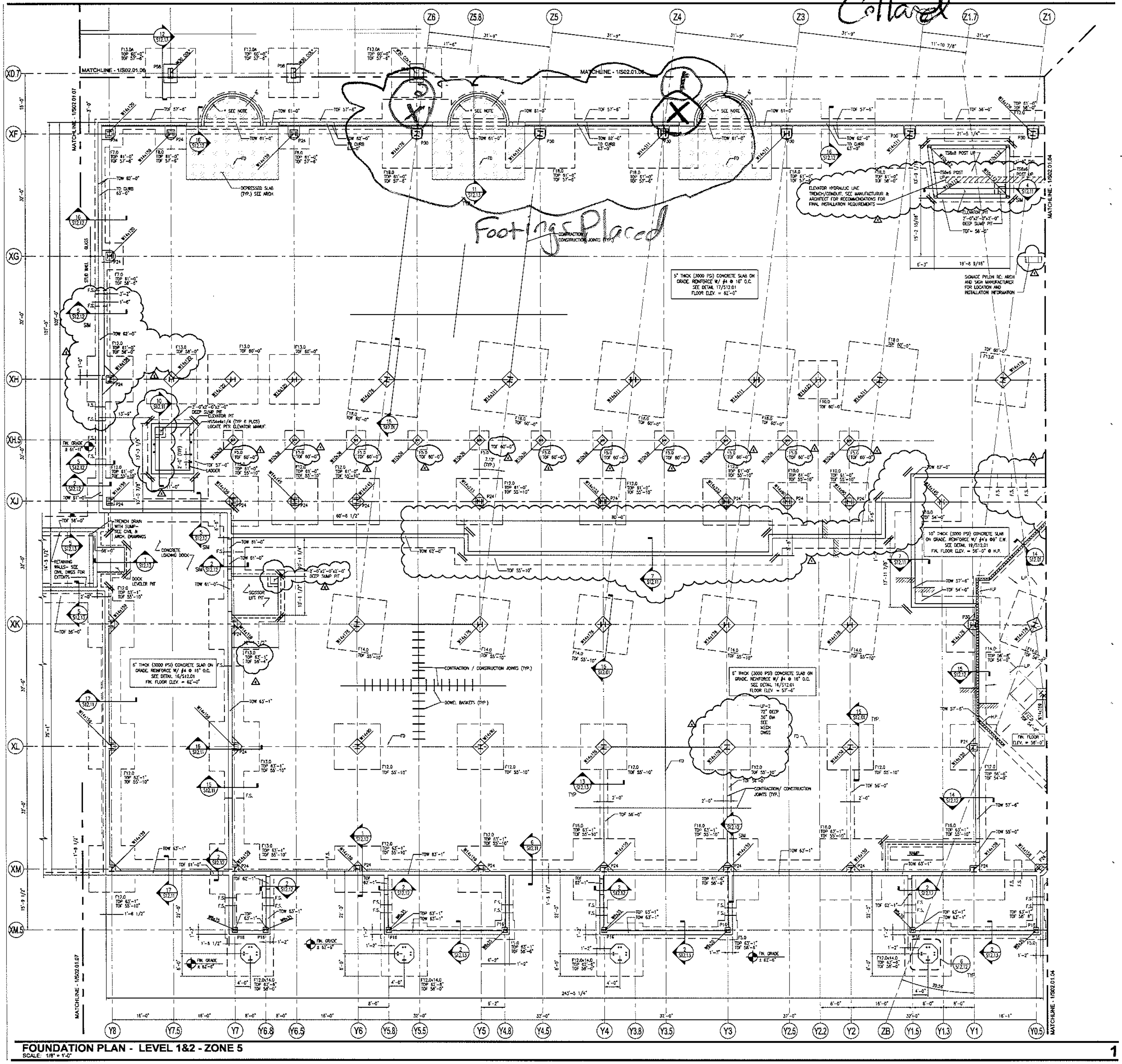
Cone 1 | Cone & Split 2 | Columnar 3 | Shear 4 | Side Fracture 5 | Double Side Fracture 6

Load	Ticket Number	Truck Number	Cubic Yds	Slump (inches)	Air Temp (°F)	Conc Temp (°F)	(%) Air Content	Time (min.)
11	173044	116	10	--	--	--	--	48
12	173047	101	6.5	--	--	--	--	--
13	173048	97	6.5	--	--	--	--	--

Remarks: Total loads = 13
Curing Temperatures: Max = 88°, Min = 65°

Checked by: Matthew T. Grady, Manager of MTS

Rodney Collard RRC 557-14 6/22/10



FOUNDATION PLAN - LEVEL 1&2 - ZONE 5
SCALE: 1/8" = 1'-0"

1

SHEET NOTES

F.S.	INDICATES FOOTING STEP, SEE DETAIL ON SHEET S12.01
TOF X'-X"	(EXAMPLE) INDICATES TOP OF FOOTING ELEVATION. G.C. VERIFY/COORDINATE WITH FINAL GRADES. ALL BOTTOM OF FOOTING ELEVATIONS ARE (-) 4'-0" (MIN.) BELOW FINISHED GRADE.
TOW X'-X"	(EXAMPLE) INDICATES TOP OF WALL. TOW VARIES SEE DETAILS & COORDINATE W/ ARCH DWGS.
TOP X'-X"	(EXAMPLE) INDICATES TOP OF PIER. TOP TO BE 0'-8" (U.N.O.)
PXX	INDICATES PIER TYPE, SEE SCHEDULE ON SHEET S02.01.04
FXX	INDICATES FOOTING TYPE, SEE SCHEDULE ON SHEET S02.01.04
(E)	INDICATES EXISTING.
*	COORDINATE DIMENSION W/ REVOLVING DOOR MANUFACTURER, VERIFY W/ ARCHITECT.
——	INDICATES SLAB PITCH
o	INDICATES FLOOR DRAIN, SEE ARCH. & MECH DWGS.

GENERAL NOTES

1. ALL EXISTING CONDITIONS SHALL BE VERIFIED IN THE FIELD PRIOR TO BEGINNING OF ANY WORK. IF EXISTING FIELD CONDITIONS DO NOT PERMIT THE INSTALLATION OF THE WORK IN ACCORDANCE WITH THE DETAILS AS SHOWN, THE CONTRACTOR SHALL NOTIFY THE ARCHITECT IMMEDIATELY AND PROVIDE A SKETCH OF THE CONDITION WITH PROPOSED MODIFICATION FOR REVIEW BY ARCHITECT.
2. CONTRACTOR TO CONFIRM ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO START OF WORK.
3. SEE SHEET S00.00 FOR GENERAL STRUCTURAL NOTES AND DESIGN LOADS
4. SEE SHEET S12.01 FOR TYPICAL FOUNDATION DETAILS
5. CONTRACTOR SHALL COORDINATE ALL FLOOR OPENINGS WITH MECHANICAL DRAWINGS.
6. AT ALL LOCATIONS WHERE PIPING RUNS BENEATH CMU OR CONCRETE WALLS, DEEPEN FOOTINGS AS REQUIRED TO ALLOW 6" (MIN.) CLEARANCE BETWEEN PIPE AND FOOTING.
7. CONTRACTOR SHALL SUBMIT LAY-OUT FOR SLAB ON GRADE CONSTRUCTION AND CONTRACTION JOINTS TO THE STRUCTURAL ENGINEER FOR APPROVAL PRIOR TO PLACING CONCRETE.
8. SEE ARCH DWGS FOR DEPRESSED SLAB LOCATIONS AND DEPTHS.
9. PROVIDE (2) #3 x 4'-0" LONG REINFORCING BARS AT ALL RE-ENTRANT SLAB CORNERS, SEE DETAILS ON SHEET S12.01
10. ALL FOOTINGS TO BE CENTERED ON COL. GRID LINE (U.N.O.)
11. FLOOR SLABS AT THIS LEVEL SHALL BE INSTALLED WITH RADIANT HEAT. COORDINATE W/ ARCH & MECH DWGS.
12. SEE ARCH DWGS FOR WATERPROOFING AND INSULATING REQUIREMENTS AND LOCATIONS.
13. PER GEOTECHNICAL REPORT BY HALEY & ALDRICH (H&A FILE NO. 35024-001 DATED 11 NOVEMBER 2008) FOUNDATIONS HAVE BEEN DESIGNED FOR A MAXIMUM ALLOWABLE BEARING PRESSURE OF 3000PSF. CONTRACTOR SHALL ALLOW OWNER'S GEOTECHNICAL ENGINEER TO OBSERVE SUBGRADE CONDITIONS TO CONFIRM BEARING CAPACITY PRIOR TO PLACEMENT OF FORMS OR REINFORCING STEEL.
14. SEE SHEET S00.00 FOR GENERAL STRUCTURAL NOTES AND DESIGN LOADS.
15. REFERENCE S02.01.00 AND SPECIFICATION SECTION 334600 FOR UNDERSLAB DRAINAGE REQUIREMENTS.
16. OVER-EXCAVATION/REPLACEMENT OF UNSUITABLE SOILS MAY BE REQUIRED BELOW FOUNDATION BEARING LEVELS. SEE SHEET S00.00 AND SPECIFICATION SECTION 312000 FOR ADDITIONAL INFORMATION.

KEY PLAN

7 6 5 4 3 2 1

Portland International Jetport
1001 Westbrook Street
Portland, Maine 04102

Gensler
2020 K Street, NW
Suite 200
Washington, DC 20006
Telephone 202.721.5200
Facsimile 202.872.8587

oest ASSOCIATES, INC.
engineers · architects · surveyors · construction managers

Issue	Date & Issue Description	By	Check
01	07/11/08 SCHEMATIC DESIGN	JDE	--
02	09/22/08 DESIGN DEVELOPMENT	JDE	--
03	12/03/08 75% CONSTRUCTION DOCUMENTS	JDE	--
04	01/23/09 95% CONSTRUCTION DOCUMENTS	JDE	--
05	10/26/09 ISSUED FOR PERMIT	JDE	--
06	11/12/09 ADDENDUM #2	JDE	--
07	01/12/10 BULLETIN #1	JDE	--

Seal/Signature
STATE OF MAINE ... LICENSED PROFESSIONAL ENGINEER 1.12.10

Project Name
PWM Terminal Enhancement

Project Number
09.6396.000

CAD File Name
T:\5330101\SHEETS\S02.01.05.DWG

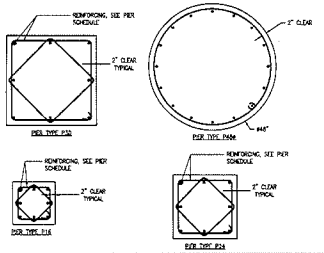
Description
FOUNDATION PLAN - LEVEL 1&2 - ZONE 5

Scale
1/8" = 1'-0"

Ref. North

S02.01.05

557-14
TERMINAL ENHANCEMENT
4/22/10
RRC

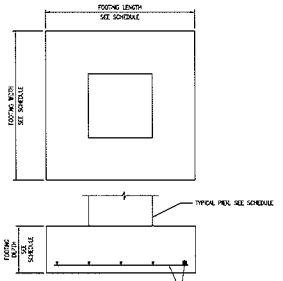


PIER SCHEDULE			
PIER TYPE	PIER DIMENSIONS	VERTICAL REINFORCING	TIES
P24	18" x 18"	(8) #6 VERTICAL BARS	#3 TIES @ 9" O.C.
P24a	24" x 24"	(8) #6 VERTICAL BARS	#3 TIES @ 9" O.C.
P32	30" x 30"	(8) #10 VERTICAL BARS	#3 TIES @ 6" O.C.
P48	42" x 24"	(16) #10 VERTICAL BARS	#3 TIES @ 6" O.C.
P48a	48ø	(14) #14 VERTICAL BARS	#3 TIES @ 6" O.C.
P36	36" x 42"	(20) #8 VERTICAL BARS	#3 TIES @ 6" O.C.

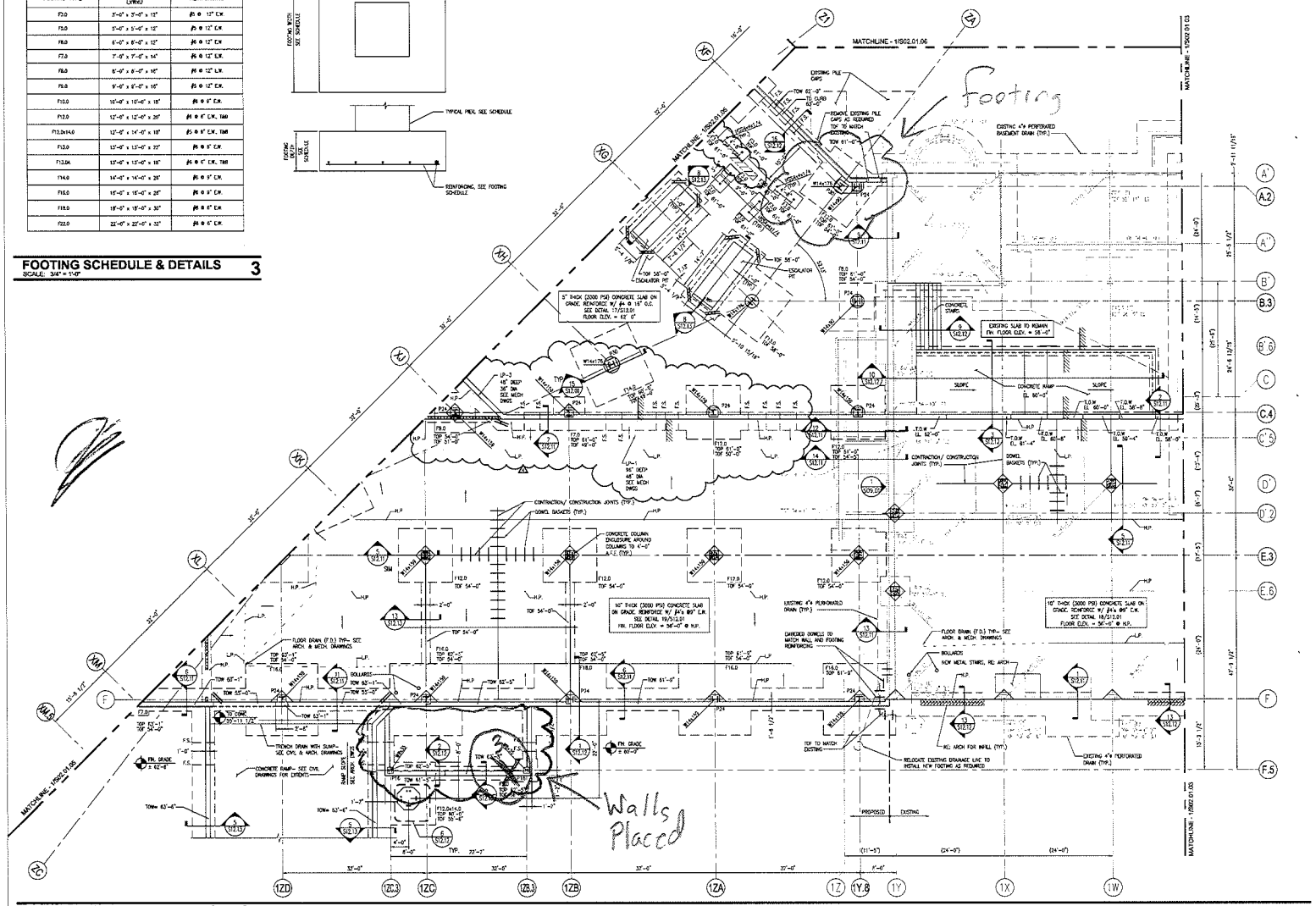
1. VERTICAL PIER REINFORCING TO BE TIED WITH #3 REBAR AS PER ACI-318 LATEST EDITION AND AS SHOWN.
2. USE (2) #3 TIES WITHIN TOP 5" AT THE TOP OF EACH PIER AND FIRST TIE ABOVE THE FOOTING SHALL BE ≤ OR < 4" FROM THE TOP OF FOOTING.
3. DOWELS TO BE SAME SIZE AND SPACING AS VERTICAL BARS (PROVIDE TENSION LAP SPLICE)
4. REFER TO ACI-318 FOR COVER REQUIREMENTS.
5. REFER TO SECTIONS FOR WALL REINFORCING.
6. f'c CONCRETE 4000 PSI @ 28 DAYS (NORMAL WEIGHT 150 PCF). fy = 60 KSI
7. REFER TO PLAN FOR TOP OF PIER ELEVATION.

PIER SCHEDULE & DETAILS
SCALE: 3/4" = 1'-0" **2**

FOOTING SCHEDULE		
FOOTING TYPE	FOOTING DIMENSIONS LxWxD	REINFORCING
F2.0	5'-0" x 5'-0" x 12"	#5 @ 12" E.W.
F3.0	5'-0" x 5'-0" x 12"	#5 @ 12" E.W.
F8.0	6'-0" x 6'-0" x 12"	#6 @ 12" E.W.
F7.0	7'-0" x 7'-0" x 14"	#6 @ 12" E.W.
F8.0	8'-0" x 8'-0" x 16"	#6 @ 12" E.W.
F9.0	9'-0" x 9'-0" x 16"	#5 @ 12" E.W.
F10.0	10'-0" x 10'-0" x 18"	#6 @ 9" E.W.
F12.0	12'-0" x 12'-0" x 20"	#6 @ 6" E.W., TBB
F12.0x14.0	12'-0" x 14'-0" x 18"	#5 @ 9" E.W., TBB
F13.0	13'-0" x 13'-0" x 22"	#6 @ 9" E.W.
F13.0A	13'-0" x 13'-0" x 18"	#6 @ 6" E.W., TBB
F14.0	14'-0" x 14'-0" x 28"	#6 @ 9" E.W.
F16.0	16'-0" x 16'-0" x 28"	#6 @ 9" E.W.
F18.0	18'-0" x 18'-0" x 30"	#6 @ 6" E.W.
F22.0	22'-0" x 22'-0" x 32"	#6 @ 6" E.W.



FOOTING SCHEDULE & DETAILS
SCALE: 3/4" = 1'-0" **3**



FOUNDATION PLAN - LEVEL 1&2- ZONE 4
SCALE: 1/8" = 1'-0" **1**

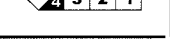
SHEET NOTES

F.S.	INDICATES FOOTING STEP, SET DETAIL ON SHEET S12.01
TOF X'-X"	[EXAMPLE] INDICATES TOP OF FOOTING ELEVATION. G.C. VERIFY/COORDINATE WITH FINAL GRADES. ALL BOTTOM OF FOOTING ELEVATIONS ARE (-) 4'-0" [MIN.] BELOW FINISHED GRADE.
TOW X'-X"	[EXAMPLE] INDICATES TOP OF WALL. TOW VARIES SEE DETAILS & COORDINATE W/ ARCH DWGS.
TOP X'-X"	[EXAMPLE] INDICATES TOP OF PIER. TOP TO BE 6'-0" [U.N.O.]
PXX	INDICATES PIER TYPE, SEE SCHEDULE ON SHEET S02.01.04
FXX	INDICATES FOOTING TYPE, SEE SCHEDULE ON SHEET S02.01.04
(E)	INDICATES EXISTING
▪	COORDINATE DIMENSION W/ OVERLYING DOOR MANUFACTURER, VERIFY W/ ARCHITECT
———	INDICATES SLAB PITCH
D	INDICATES FLOOR DRAIN, SEE ARCH. & MECH DWGS.
H.P.	INDICATES HIGH POINT OF FLOOR SLAB, SEE ARCH. & MEP DRAWINGS FOR REQUIREMENTS
L.P.	INDICATES LOW POINT OF FLOOR SLAB, SEE ARCH. & MEP DRAWINGS FOR REQUIREMENTS.

GENERAL NOTES

1. ALL EXISTING CONDITIONS SHALL BE VERIFIED IN THE FIELD PRIOR TO BEGINNING OF ANY WORK. IF EXISTING FIELD CONDITIONS DO NOT PERMIT THE INSTALLATION OF THE WORK IN ACCORDANCE WITH THE DETAILS AS SHOWN, THE CONTRACTOR SHALL NOTIFY THE ARCHITECT IMMEDIATELY AND PROVIDE A SKETCH OF THE CONDITION WITH PROPOSED MODIFICATION FOR REVIEW BY ARCHITECT.
2. CONTRACTOR TO CONFIRM ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO START OF WORK.
3. SEE SHEET S00.00 FOR GENERAL STRUCTURAL NOTES AND DESIGN LOADS.
4. SEE SHEET S12.01 FOR TYPICAL FOUNDATION DETAILS
5. CONTRACTOR SHALL COORDINATE ALL FLOOR OPENINGS WITH MECHANICAL DRAWINGS.
6. AT ALL LOCATIONS WHERE PIPING RUNS BENEATH CMU OR CONCRETE WALLS, DEEPEN FOOTINGS AS REQUIRED TO ALLOW 6" [MIN.] CLEARANCE BETWEEN PIPE AND FOOTING.
7. CONTRACTOR SHALL SUBMIT LAY-OUT FOR SLAB ON GRADE CONSTRUCTION AND CONTRACTION JOINTS TO THE STRUCTURAL ENGINEER FOR APPROVAL PRIOR TO PLACING CONCRETE.
8. SEE ARCH DWGS FOR DEPRESSED SLAB LOCATIONS AND DEPTHS.
9. PROVIDE (2) #3 x 4'-0" LONG REINFORCING BARS AT ALL RE-ENTRANT SLAB CORNERS, SEE DETAILS ON SHEET S12.01.
10. ALL FOOTINGS TO BE CENTERED ON COL. GRID LINE (U.N.O.).
11. FLOOR SLABS AT THIS LEVEL SHALL BE INSTALLED WITH RADIANT HEAT. COORDINATE W/ ARCH & MECH DWGS.
12. SEE ARCH DWGS FOR WATERPROOFING AND INSULATING REQUIREMENTS AND LOCATIONS.
13. PDX GEOTECHNICAL REPORT BY HALEY & ALDRICH (HAA FILE NO. 30674-001 DATED 11 NOVEMBER 2005) FOUNDATIONS HAVE BEEN DESIGNED FOR A MAXIMUM ALLOWABLE BEARING PRESSURE OF 3000 PSF. CONTRACTOR SHALL ALLOW OWNER'S GEOTECHNICAL ENGINEER TO OBSERVE SUBGRADE CONDITIONS TO CONFIRM BEARING CAPACITY PRIOR TO PLACEMENT OF FORMS OR REINFORCING STEEL.
14. DIMENSIONS IN PARENTHESIS FOR REFERENCE ONLY.
15. EXISTING BUILDING INFORMATION USED TO PREPARE PLANS OF THIS AREA WERE TAKEN FROM AS-BUILT DRAWINGS PREPARED BY PLISA DATED SEPTEMBER 2006.
16. REFERENCE S20.01.00, AND SPECIFICATION SECTION 334600 FOR UNDERSLAB DRAINAGE REQUIREMENTS.
17. OVER-EXCAVATION/REPLACEMENT OF UNSUITABLE SOILS MAY BE REQUIRED BELOW FOUNDATION BEARING LEVELS. SEE SHEET S00.00 AND SPECIFICATION SECTION 312000 FOR ADDITIONAL INFORMATION.

KEY PLAN



Portland International Jetport
1001 Westbrook Street
Portland, Maine 04102

Gensler
2020 K Street, NW
Suite 200
Washington, DC 20006
Telephone: 202.721.5200
Facsimile: 202.872.8587

oest ASSOCIATES, INC.
engineers · architects · surveyors · construction managers

△	Issue	Date & Issue Description	By	Check
--	01	07/11/08 SCHEMATIC DESIGN	JDE	--
--	02	09/22/08 DESIGN DEVELOPMENT	JDE	--
--	03	12/03/08 75% CONSTRUCTION DOCUMENTS	JDE	--
--	04	01/23/09 95% CONSTRUCTION DOCUMENTS	JDE	--
--	05	10/26/09 ISSUED FOR PERMIT	JDE	--
△	06	01/12/10 BULLETIN #1	JDE	--

Seal/Signature

Project Name: PWM Terminal Enhancement

Project Number: 08.6360.000

CAD File Name: T:\SS01001\SHEETS\S02.01.04.DWG

Description: FOUNDATION PLAN - LEVEL 1&2 - ZONE 4

Scale: 1/8" = 1'-0"

Ref. North

S02.01.04

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