193-E-1 11 Pomeroy Street Single Family and Street Construction Chabad Lubavitch of Maine, Inc.

#2012-471



Planning Division

Scott Hanson, Preservation Compliance Coordinator

Osale of atparcel

Buno sik lighting?

Dlandscaping buffer

majoring sure built according to plan

389 Congress Street, 4th floor . Portland, ME . (207) 756-8023 . Fx 756-8258 Email: sth@portlandmaine.gov



Planning Division Scott Hanson, Preservation Compliance Coordinator

4/29/13 a stow tree Ine + what

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Planning Division

Scott Hanson, Preservation Compliance Coordinator

2) définitions, simple family home v. multi-

we were less merous process-

how is rooming have permitted? Cohede-place of assembly is Cu. of B?

Dask relighting-@ will there be blacking?

Dheight restrictions?



Scott Hanson, Preservation Compliance Coordinator

. Will it be a St houx? . Water issue - barement flooded 1/2 the time.

6/6/13. JEAN GILPATPICY-.

389 Congress Street,

detention pand? *swate wan't do it.

sond new glans. 6/6/13

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PORTIAND MAINE

Planning Division

Scott Hanson, Preservation Compliance Coordinator

10/10

marino 04102 @ yahoo.

Well-

Blease call Dick Marino

899-4154 (Pives in Redland Park)

-cesting about status of Borning St + SF hse

- parlang mea + lighting - wone.

- soprave fortage?

- permitted to have services in home? exact #

- city standards

drainage arbs

- generators? permitting? - (an yourun on bossis

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Planning Division

Scott Hanson, Preservation Compliance Coordinator

Dwg Bablark - [doug. bablark-@maine.

- land truct -

- email - most recent plans.

add to interested list.

TAMMY KDIEDER - Dava Buskir - 210-6419 95 Redfor park Pd. 100 ce proporal 712 5140

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Scott Hanson, Preservation Compliance Coordinator

Zach Barchard 617 304 6970 Il Pameray Street 103 wolcott-

zachary. boucharde . com.

lots abots ald city lot - paper theet?

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Strengthening a Remarkable City, Building a Community for Life • www.portlandmaine.gov

Planning & Urban Development Department Gregory A. Mitchell, Acting Director

Planning Division Alexander Jaegerman, Director February 3, 2012

William Thompson Andrew Morrell BH2M 28 State Street Gorham, ME 04038 Dear Mr. Thompson and Mr. Morrell:

application is incomplete. In order to process the application, you will need to submit the materials The Planning Division received a Level I Minor Residential application on January 30, 2012. for a building permit and pay the application fee. Secondly, the proposal includes the extension of Pomeroy Street which requires a Level II Site Plan Review. Please submit a Level II application along with all required information and fees. The material you have submitted is on hold at the Planning Division. I recommend that you pick up the applications and complete them for re-submittal.

Sincerely,

Berbara Back

Barbara Barhydt

Development Review Program Manager

cc: Alexander Jaegerman, Planning Division Director Tammy Munson, Inspections Division Director Danielle West-Chuhta, Corporation Counsel Marge Schmuckal, Zoning Administrator

Agr. 7 2 6,2012 Andy morell-* Single family 1 Administrative \$400 KR Real Estato King weistein Hold - until Day check or status

- Cover letter (1) Amended Plan - not amended - New application 2007 - Conditional Use - Expired -- Lamber - maximum extension Dyes 3) Need NRPA Overnit - for wetland - 6,773 55. Stormwater - Permit 5. Lot - out lot will be reviewed contrated assessment - show building triologe / and of gotontal 6. Building Permit Submittal Reg. - full but assircation & submit items 7. Evidence of General Congresty don't see it in the cacket 420 = 210 X 42 = 8400 -7,500 - Subnit - a level IT Site glar for the constranction @ Food way --and - wo D35 - put advise - a function dis req -5 com/ = - Level I - 90hy Dinning



LESTER S. BERRY
WILLIAM A. THOMPSON
ROBERT C. LIBBY, Jr.
WALTER E. PELKEY
ANDREW S. MORRELL

January 24, 2012 (Revised February 3, 2012)

City of Portland Planning Division 389 Congress Street Portland, ME 04103

RE:

Amended Level I-Minor Residential Development Review Site Plan

Single Family Lot 11 Pomeroy Street

To whom it may concern:

On behalf of the applicant, Chabad Lubavitch of Maine Inc., our office is submitting an amended Level 1-Minor Residential Development Review Site Plan for the above referenced project. Please find attached the following information in support of this submission:

- Level 1-Minor Residential Development Review Application
- Subject Parcel Deed (Book 21417 Page 198)
- Financial Capacity Information
- Fees (\$300 Application Fee and \$100 Inspection Fee Total \$400)
- Tax Map 193 (Block E, Lot 1)
- Site Plans (Sheets 1 through 5)
- BH2M Technical Capacity Information
- U.S.G.S. Map
- Medium Intensity Soils Map
- DEP storm water Permit-by-Rule
- NRPA Wetland Permit (Tier 1)
- Building Elevations

The parcel is located on the Southern side of the undeveloped paper right of way known as Pomeroy Street North of Capisic Street and is known as Tax Map 193, Block E, Lot 1. This parcel currently exists as an undeveloped wood lot owned by Chabad Lubavitch of Maine Inc. The total parcel consists of 73,821 s.f. and is zoned R-3 Residential (see Existing Conditions Plan-Sheet 2 for more information). This parcel was previously approved for a Conditional Use Permit back in 2007 by the City of Portland for a single family residence with attached synagogue. The applicant would like to amend this Permit to exclude the synagogue portion of the project, thus the current proposal is for a single family residence only along with an outsale lot as shown on the attached revised

Site Plans. The proposed Lot will be served by public sewer, water and underground utilities from Bancroft Street. The development of this parcel will involve the construction of approximately 200 feet of Pomeroy Street. The site design is very similar to the previously approved site, with the major differences being the reduction in the proposed building footprint for the removal of the synagogue portion of the building. The current design does not require any storm water permits, therefore, the proposed vegetated under drained soil filter field has been removed from the design. All appropriate erosion control techniques have been utilized to assure no adverse impacts are created to any abutters as a result of this project (see Details Sheet for Erosion Control Details and Notes).

The prior project required the following permits:

Conditional Use Permit-City of Portland

The reserved are in the conditions of the conditions of

The current project appears eligible for an amended Level 1 Site Plan subject to City staff approval.

NRPA Wetlands Permit(Tier 1)-Maine DEP

The original permit was granted for the disturbance of 13,028 s.f. of wetlands. The current amended project requires the disturbance of only 6,773 s.f.

Storm water Permit by Rule-Maine DEP

The current project does not require any state storm water permits since the disturbed area and impervious area has been reduced below current permitting thresholds.

The following is a list of the Fire Department Items required as part of this submission:

Applicant:

Chabad Lubavitch of Maine Inc.

11 Pomeroy Street Portland, Maine 04102

Architect:

Foreside Architects, LLC.

PO Box 66736

Falmouth, Maine 04105

(207) 781-3344

Proposed Use:

Single Family Residence

• Building Footprint:

777 s.f. (2-Story)

• Fire Protection:

New House will contain sprinkler system meeting City

Ordinance

• Hydrant location:

Hydrant proposed as part of Pomeroy Street construction located on the Northern Side of Pomeroy Street approximately 140 feet East of Bancroft Street and across from the proposed driveway of the proposed single family residence.

A copy of this application and supporting information has been submitted to the Fire Department.

We look forward to working with City Staff and the Fire Department on this project. Please contact our office if you have any questions or if you need additional information.

Sincerely

Andrew S. Morrell, E.I.

Cc: Frederick Lamontagne, City of Portland Fire Chief

Level I Site Plan Revised Submission-2-3-2012

1. Name of Applicant:	Chabad Lube Maine, Inc		5. Name of Agent: (if applicable)		Huff McDonald
2. Applicant's Mailing Address:	101 Craigid	ME 04101	6. Agent's Mailing Address:	NACON 92.31 - 900 (0.10 20 20 20 3	e Street ME 04038
3, Applicant's Daytime Phone #: 4. Applicant's Fax #:	207-871-894	47	7. Agent's Daytime Phone #:	207-839	-2771
(if available) 9. Location of Project:	N/A		3. Agent's Fax # and email address: 10. Town:	207-839	-8250
(Road, Street, Rt.#)	Pomeroy Str	eet	I. County:	Portlan	
12. Is this PBR for ren	ewal of an individua		and the same of the last	Cumber1 27 and signature p	
				Control of the Contro	☑ No
13. Type of Direct Watershed; (Check all that apply)	☐ Lake not most a ☐ Lake most at ris	sk	 Amount of Develop Area: 		f <u>1.59</u> acres OR
(Сисск ин шат арргу)	River, stream or	k, severely blooming	5. Amount of	☐ Total # of ☐ ☐ Total # of	
	Urban impaired	stream	Impervious Area:	10tal # 01	O.68 acres
	☐ Freshwater wetl ☐ Coastal wetland		2 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	☐ Total # of	square feet
	☐ Wellhead of pul	1823			
16. Creating a common development or sale?		7. Name of waterbody site drains:	(ies) to which the pro	ject Fores	sted wetlands
18. Brief Project Descri	ption: Constru	uction of access	driveway for c	onstruction of	a single-
	city s	tandards.	I and construct	10H 43U Teet (of Pomeroy St. to
19. Size of Lot or Parcel	Total of	square feet OR	20. UTM Locations:(known)	if UTM Northing:	N/A
	₩ Total of		KHOWII)	UTM Easting:	N/A
21. Deed Reference Num	bers: Book#: 625	8 Page#: 53 22	. Map and Lot Numb		#:193 Lot #: 19
 Project started prior application? 	to ☐ Yes→	If yes, Completed?	☐ Yes 24. Resub	mission lication?	☐ Yes ☑ No
25. Written Notice of Violation?		name of DEP enforcem	S. 100 L. A. 100	N/A	
26. Detailed Directions to					
(Attach separate sheet	if necessary)		to Bancroft Str e to Pomeroy St		
7. SUBMISSIONS V	Asia a second		10	28 300 1000	on Tight.
This form (signed and					idual Stormwater
Fee Fee				it only:	4 7 43
	Approva (if in Esse		- 1	nis form (signed and opy of original storm	V 00000
	(11 III E350)	iniai Haoitat) — 5.	□ Fe		karımı
CERTIF	ICATIONS	AND SIGNAT	TURES LOCA	ATED ON P	AGE 2
OFFICE USE ONLY	Ck.#		Staff	Staff	
PBR #	FP	Date	Acc. Date	Def. Date	After
					Photos

Stormwater Application

Page 2

03/06

CERTIFICATIONS / SIGNATURES

Applicant's Statement:

If the applying for a Stormwater PRK and have attached the required PBK submissions. I have read the requirements herein and l'arthur dire any project satisfies the applicable submissions in magement smadards. I authorize stability State and 'Rederm agencies having 'Juristicion' over this activity, to access the project site for the purpose of iteraming sompliance with the rules.

Signed:

Bate:

With this Stormwater PBR notification form and my signature below, I am filing notice of my with Maine Goastfuction

General Permit. I have read and will comply with all of the MCGP standards. In addition, I will file a Notice of Termination (NOT) within 20 days of project completion.

If this form is not being signed by the landowner or lessee of the property, attach documentation showing authorization to sign.

Date: 7/12/07

ORIGINAL TO STATE OF MAINE PERSON RECEIPT FOR DEPARTMENTAL COLLECTIONS MAKING Huff meand Millison PAYMENT THIS IS YOUR OFFICIAL RECEIPT. DOLLARS PLEASE RETAIN IT FOR FUTURE MONEY ORDER REFERENCE. NAME OF DEPARTMENT OR INST. MAKING COLLECTION

17 State House Station Augusta, ME 04333

OL-18-2887 13-133 THAINE D

IN THE MATTER OF

CHABAD LUBAVITCH OF MAINE INC Portland, Cumberland County HOUSE AND CHAPEL ACCESS DRIVE L-22414-TB-A-N (approval)) NATURAL RESOURCES PROTECTION ACT) FRESHWATER WETLAND ALTERATION) WATER QUALITY CERTIFICATION) FINDINGS OF FACT AND ORDER

Project Description: The applicant proposes to alter 13,028 square feet of forested wetland to construct a project located off Pomeroy Street in the City of Portland. The project consists of a residence with attached chapel, access drive and stormwater detention pond. On-site impacts include 4,713 square feet of wetland fill associated with construction of portions of the access drive and detention pond. The parcel is situated off the westerly side of Pomeroy Street, a road previously approved by the City but never constructed.

According to a letter dated August 10, 2005, from City Planner Ethan Boxer-Macomber, the City of Portland's Land Use Code requires any developer of a lot located on an unimproved City street to construct that street to City standards along the entirety of their site's frontage. As such, the applicant is required to impact an additional 8,315 square feet of forested wetland to construct Pomery Street to City standards. Total proposed impacts for the project are 13,028 square feet. The project is shown on a plan entitled, "Site Plan – Land of Chabad Lubavitch of Maine Inc." drawn by BH2M and dated March 2004. Wetland impacts were reduced to the current total after site visit by Department staff and subsequent discussions with the applicant's agent. Details of the revised plans are outlined in a memo from BH2M dated June 30, 2005.

Permit for:	X Tier 1
DEP Decision:	X Approved Denied (see attached letter)
CORPS Action:	The Corps has been notified of your application. The following are subject to Federal screening: (1) projects with previously authorized or unauthorized work, in combination with a Tier 1 permit for a single and complete project, which total more than 15,000 square feet of altered area; (2) projects with multiple state permits and/or state exemptions which apply to a single and complete project that total more than 15,000 square feet of altered area; and (3) projects that may impact a vernal pool, as determined by the State of Maine or the Corps. If your activity is listed above, Corps approval is required for your project. For information regarding the status of your application contact the Corps' Maine Project Office at 623-8367.
tandard Conditions:	7 2000 0200 02000

1)

- 1) If construction or operation of the activity is not begun within two (2) years from the date signed, this permit shall lapse and the applicant shall reapply to the Department for a new permit. This permit is transferable only with prior approval from the Department. If the activity is associated with a larger project, starting any aspect of that project constitutes start of construction.
- The project shall be completed according to the plans in the application. Any change in the project plans must be reviewed and approved by the Department.
- 3) Properly installed erosion control measures shall be installed prior to beginning the project, and all disturbed soil should be stabilized immediately upon project completion.
- 4) A copy of this approval will be sent to the City of Portland. Department approval of your activity does not supersede or substitute the need for any necessary local approvals.

Please note the attached sheet for guidance on appeal procedures.

THIS APPROVAL DOES NOT CONSTITUTE OR SUBSTITUTE FOR ANY OTHER REQUIRED STATE, FEDERAL OR LOCAL APPROVALS NOR DOES IT VERIFY COMPLIANCE WITH ANY APPLICABLE SHORELAND ZONDE ORDINANCES.

DAWN R. GALLAGHER, COMMISSIONER

Date of initial application June 14, 2005

Date of initial application accepted for processing June 25, 2005

Date application accepted for processing June 25, 2005

Date of initial application accepted for processing June 25, 2005

Date Sied with Board of Environmental Protection

WB/ATS#55224/L22414AN



NATURAL RESOURCE PROTECTION ACT (NRPA) STANDARD CONDITIONS

THE FOLLOWING STANDARD CONDITIONS SHALL APPLY TO ALL PERMITS GRANTED UNDER THE NATURAL RESOURCE PROTECTION ACT, TITLE 38, M.R.S.A. SECTION 480-A ET.SEQ. UNLESS OTHERWISE SPECIFICALLY STATED IN THE PERMIT.

- A. <u>Approval of Variations From Plans.</u> The granting of this permit is dependent upon and limited to the proposals and plans contained in the application and supporting documents submitted and affirmed to by the applicant. Any variation from these plans, proposals, and supporting documents is subject to review and approval prior to implementation.
- B. <u>Compliance With All Applicable Laws.</u> The applicant shall secure and comply with all applicable federal, state, and local licenses, permits, authorizations, conditions, agreements, and orders prior to or during construction and operation, as appropriate.
- C. <u>Erosion Control.</u> The applicant shall take all necessary measures to ensure that his activities or those of his agents do not result in measurable erosion of soils on the site during the construction and operation of the project covered by this Approval.
- D. <u>Compliance With Conditions</u>. Should the project be found, at any time, not to be in compliance with any of the Conditions of this Approval, or should the applicant construct or operate this development in any way other the specified in the Application or Supporting Documents, as modified by the Conditions of this Approval, then the terms of this Approval shall be considered to have been violated.
- E. <u>Initiation of Activity Within Two Years</u>. If construction or operation of the activity is not begun within two years, this permit shall lapse and the applicant shall reapply to the Board for a new permit. The applicant may not begin construction or operation of the activity until a new permit is granted. Reapplications for permits shall state the reasons why the applicant will be able to begin the activity within two years form the granting of a new permit, if so granted. Reapplications for permits may include information submitted in the initial application by reference.
- F. Reexamination After Five Years. If the approved activity is not completed within five years from the date of the granting of a permit, the Board may reexamine its permit approval and impose additional terms or conditions to respond to significant changes in circumstances which may have occurred during the five-year period.
- G. No Construction Equipment Below High Water. No construction equipment used in the undertaking of an approved activity is allowed below the mean high water line unless otherwise specified by this permit.
- H. Permit Included In Contract Bids. A copy of this permit must be included in or attached to all contract bid specifications for the approved activity.
- I. <u>Permit Shown To Contractor.</u> Work done by a contractor pursuant to this permit shall not begin before the contractor has been shown by the applicant a copy of this permit.

Revised (4/92) DEP LW0428

Post-It* Fax Note 7571	Date 7-10 - 07 pages > 2
TO MARSHALL TINKLE	From BILL BULLAND
	MDEP COLLEGE
Phone #	Phone # 822 - 6380
FAX# 874-6705	Fax #

Location/Address of Site:	PRINCE CO. STORY			
Pomeroy Street				
Total Square Footage of Proposed		quare feet):	Number of Stories:	
Structure/Area:	73,821 s.	.f.	2	
Tax Assessor's Chart, Block & Lot(s)	Fees Paid: (for C	Office Use Only)	Cost of Work:	
Chart# 193 Block # E Lot#001	Site Plan Building Permit Inspection		Work: \$ C of O Fee \$	
Current Legal Use: Number of residential Units	If vacant, previous	use?	Is property part of a subdivision? No	
. 0	wood lot		If yes, please name: N/A	
Proposed Use and Project Description	:	×		
Single Family Resid	dence			
Applicant – must be owner, Lessee or	Buyer		Applicant Contact Information	
Name: Chabad Lubavitch of M	Maine Inc.	Work# N/A		
Business Name, if applicable: N/A		Home# N/A		
Address: Bancroft Street		Cell# N/A		
City/State: Portland, Me Zip	Code: 04102	e-mail: N/A		
Owner - (if different from Applicant)		Owner Contact Info	ormation	
Name: Same as above		Work#	shows	
Address:		Home#	above	
City/State : Zip Code:		Cell#		
		e-mail:		
Agent/ Contractor		Agent/Contractor C	Contact Information	
		Work#	ontact information	
Name: William Thompson, BH2	M	(207) 83	39–2771	
Address: 28 State St.		Home# N/A		
City/State: Gorham, Me Zip C	Code: 04038	Cell# N/A		
		e-mail: wthompso	m@bh2m.com	
Billing Information	× × × × × × × × × × × × × × × × × × ×	W	ling Permit is Ready:	
Name: King Weinstein		Name: King Weinstein		
Address: 198 Saco Avenue		Address: 198 Sac	o Avenue	
City/State: 01d Orchard Beach Dip C	Code: 04064	City/State: 01d Or	chard Beach Zip Code: 04064	
Phone Number: N/A		Phone Number: N		

DEVELOPMENT REVIEW FEES:

Check all reviews that apply. Payment may be made in cash or check to the City of Portland.

		-
Level I Minor Residential Site Plan	Fees Paid (office use)	
X Application Fee (\$300.00 flat fee)		
Inspection Fee (\$100.00 flat fee) (for an inspection conducted by the Planning Division		
The City invoices separately for the following: Notices (\$.75 each)		
Performance Guarantee		Exempt except for those projects that complete construction in the winter and the site work is incomplete.
Building Permit Fee	\$30 for the first \$1,000 construction cost, \$10 per additional \$1,000 cost.	

Application Check List:

As of December 1, 2010, all site plans and written application materials must be uploaded to a website for review. At the time of application, instructions for uploading the plans will be provided to the applicant. One paper set of the plans, written materials and application fee must be submitted to the Inspections Division to start the review process.

Refer to the application checklist for a detailed list of submittal requirements.

Portland's development review process and requirements are outlined in the Land Use Code (Chapter 14), which includes the Subdivision Ordinance (Section 14-491) and the Site Plan Ordinance (Section 14-521). Portland's Land Use Code is on the City's web site: www.portlandmaine.gov Copies of the ordinances may be purchased through the Planning Division. All of the information on the checklist must be submitted for review. The applicant must check off the items contained in the application package to ensure the application is complete.

Property Taxes:

If you or the property owner owes real estate or personal property taxes or user charges on any property within the City, payment arrangements must be made before a permit of any kind is accepted.

Separate Permits:

Separate permits are required for internal and external plumbing, HVAC, and electrical installations.

I hereby certify that I am the Owner of record of the named property, or that the owner of record authorizes the proposed work and that I have been authorized by the owner to make this application as his/her authorized agent. I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in this application is issued, I certify that the Planning Authority and Code Enforcement's authorized representative shall have the authority to enter all areas covered by this permit at any reasonable hour to enforce the provisions of the codes applicable to this permit.

Signature of Applicant:	Date:
and - agent	1/24/12

This is not a permit; you may not commence any work until the permit is issued.

d #	General Submittal Requirements – Level I Minor Residential				
Applicant Checklist	Planner Checklist (internal)	Number of Copies	Submittal Requirement		
X		3 (1 paper copy as of Dec. 1)	Completed application form and check list.		
X		1	Application fees.		
X		3 (1 paper copy as of Dec. 1)	Evidence of right, title and interest.		
X		3 (1 paper copy as of Dec. 1)	Copies of required state and/or federal permits.		
X		3 (1 paper copy as of Dec. 1)	Written Description of existing and proposed easements or other burdens.		
\vartical 2		3 (1 paper copy as of Dec. 1)	Written requests for waivers from individual site plan and/or technical standards.		
₩		3 (1 paper copy as of Dec. 1)	Evidence of financial and technical capacity.		
₩		3 (1 paper copy as of Dec. 1)	Written summary of fire safety (referencing NFPA fire code and Section 3 of the City of Portland Technical Manual). Refer to Fire Department Checklist on page 6 of this application.		

HE SEE	Site Plans and	Boundary Surv	ey Requirements – Level I Minor Residential	
Applicant Checklist	Planner Checklist (internal)	Number of Copies	Submittal Requirement	
		Boundary survey meeting the requirements of section 13 of the City of Portland Technical Manual with the site plan informatio listed below shown on the plan, including a north arrow and a greater than or equal to 1"=20'. (Photocopies of the plat or har drawn building footprints will not be accepted.)		
K		overlay zon		
K		 Existing and proposed structures (including location of proposed piers, docks or wharves if in Shoreland Zone). 		
K		 Location and dimension of existing and proposed paved areas. 		
ĸ		 Proposed ground floor area of building. 		
K		Finish floor elevation (FEE) or sill elevation.		
×		Exterior building elevations (show all 4 sides).		
×		Existing and proposed utilities (or septic system, where applicable)		
X		Existing and proposed grading and contours.		
X		■ Proposed st	tormwater management and erosion controls.	

X	 Total area and limits of proposed land disturbance. 		
X	 Proposed protections to or alterations of watercourses. 		
(X	Proposed wetland protections or impacts.		
Dž.	 Existing vegetation to be preserved and proposed site landscaping and street trees (2 trees per unit for a single or two-family house). 		
□ x	Existing and proposed curb and sidewalk, except for a single family home.		
X	 Existing and proposed easements or public or private rights of way. 		
□ _{N/A}	Show foundation/perimeter drain and outlet.		
□ _{N/A}	 Additional requirements may apply for lots on unimproved streets. 		
X	Three sets of the reduced boundary survey/site plan is required if original is larger than 11'x17' as of Dec. 1)		

Building Permit Submittal Requirements –Level I: Minor Residential Development				
Applicant Checklist	Planner Checklist (internal)	Number of Copies	Submittal Requirement	
100		1	One (1) complete set of construction drawings must include:	
			 Cross section with framing details 	
			Floor plans and elevations to scale	
			 Stair details including dimensions of : rise/run, head room, guards/handrails, baluster space 	
			Window and door schedules	
			 Foundation plans w/required drainage and damp proofing, if applicable 	
		222	 Detail egress requirements and fire separation, if applicable 	
			 Insulation R-factors of walls, ceilings & floors & U-factors of windows per the IEEC 2003 	
			 Deck construction including: pier layout, framing, fastenings, guards, stair dimensions 	
			 As of September 16, 2010 all new construction of one and two family homes are required to be sprinkled in compliance with NFPA 13D. This is required by City Code. (NFPA 101 2009 ed.) 	
			 Reduced plans or electronic files in pdf format are also required if original plans are larger than 11X17" 	

Reminder: Separate permits are required for internal and external plumbing, HVAC, and electrical installations. Please submit all of the information outlined in this application checklist. If the application is incomplete, the application may be refused. The Planning and Urban Development Department may request additional information prior to the issuance of a permit.

Site Plan Standards for Review of Level I: Minor Residential

Level I: Minor Residential site plan applications are subject to the following site plan standards*, as contained in section 14-526 of Article V. Site Plan:

• 14-526 (a) Transportation Standards:

2.a.(i) and (ii);

2.c (if the site plan is a two-family or multi-

family building); 4.a.(i) and (iv)

• 14-526 (b) Environmental Quality Standards:

1 2.a.

2.b.(iii)

3.a., c. and d. and e.

• 14-526 (c) Public Infrastructure and Community Safety Standards: 1. and 3.a through e.

• 14-526 (d) Site Design Standards:

5. and 9.

*Except as provided in Article III of the City Code, or to conditions imposed under Section 14-526(e) only, or to those submission requirements set forth in section 14-527 as relate solely thereto.



PORTLAND FIRE DEPARTMENT SITE REVIEW FIRE DEPARTMENT CHECKLIST



A separate drawing[s] shall be provided to the Portland Fire Department for all site plan reviews.

- 1. Name, address, telephone number of applicant.
- Name address, telephone number of architect
- 3. Proposed uses of any structures [NFPA and IBC classification]
- 4. Square footage of all structures [total and per story]
- Elevation of all structures
- Proposed fire protection of all structures
 - As of September 16, 2010 all new construction of one and two family homes are required to be sprinkled in compliance with NFPA 13D. This is required by City Code. (NFPA 101 2009 ed.)
- 7. Hydrant locations

WARRANTY DEED

KNOW ALL PERSONS BY THESE PRESENTS, that MOSHE WILANSKY and CHANA WILANSKY of Portland, Maine, for consideration paid, GRANTS TO CHABAD LUBAVITCH OF MAINE, INC., a Maine non-profit corporation whose mailing address is 101 Craigie Street, Portland, ME 04102, with WARRANTY COVENANTS, the premises situated in the City of Portland, County of Cumberland and State of Maine, described as follows:

A certain lot or parcel of land located on the southwesterly sideline of Pomeroy Street, so-called, in the City of Portland, County of Cumberland and State of Maine; said parcel being more particularly described as follows:

Beginning at an V2" iron rod found on the southwesterly sideline of said Pomeroy Street at the southeasterly corner of land now or formerly of Kriston Briggs;

Thence South 46° 11° 30" East along the southwesterly sideline of said Pomeroy Street a distance of 269.44 feet to a capped iron rod found (PLS #1278) and the northerly sideline of Motley Street, so-called;

Thence South 71° 08' 30" West along the northerly sideline of said Motley Street a distance of 58.71 fect to a point;

Thence in a general southwesterly direction along the northerly sideline of said Motley Street and along a circular curve to the left, circumscribed by a radius of 218.81 feet, an arc length of 44.47 feet to a capped iron rod found (PLS #1278) and land now or formerly of Joshua James and Tamara Rainsford Krieger; said capped iron rod found being South 65° 19' 13" West a tie distance of 44.39 feet from said previous capped iron rod found;

Thence North 34° 26' 02" West along the land of said Krieger a distance of 86.47 feet to a capped 5/8" iron rod found (PLS #1172);

Thence South 38° 44' 24" West along the land of said Krieger, along land now or formerly of the Redlon Park Homeowner's Association, and along land now or formerly of Nancy A. Roy a distance of 350.00 feet to a point and remaining land of Stuart B. Herrick, Jr.;

Thence North 28° 05' 22" West along the remaining land of Stuart B. Herrick, Jr. a distance of 286.03 feet to a point and land now or formerly of Thomas and Melody Lussier;

Thence North 60° 18' 30" East along the land of said Lussier, along land now or formerly of Roger and Donna Gendron, along land now or formerly of Jean Gilpatrick and along land of said Briggs a distance of 350.00 feet to the point of beginning.

The above-described parcel contains \$7,924 square feet. All bearings refer to Magnetic North as observed in 1973.

Reference is made to an unrecorded plan entitled Plan of Property, Bancroft Street, Portland, Maine, dated July 31, 1986, by R.P. Titcomb Associates, Inc. (Job #8660), as revised by unrecorded plan entitled Standard Boundary Survey and existing Conditions Plan for Stuart B. Herrick, Jr., dated March 2004, by BH2M (Job #03217).

Being the same premises conveyed to the Grantors herein by Warranty Deed of Stuart B. Herrick, Jr. dated May 28, 2004 and recorded in the Cumberland County Registry of Deeds at Book 21341, Page 37.

IN WITNESS WHEREOF, the said Moshe Wilansky and Chana Wilansky have caused this instrument to be signed and sealed on June $\frac{15}{100}$, 2004.

Witness

Moshe Wilansky

Witness

Chana Wilansky

State of Maine Cumberland, ss.

June 15, 2004

Then personally appeared the above-named Moshe Wilansky and/or Chana Wilansky and acknowledged the foregoing instrument to be his/her/their free act and deed.

Before me,

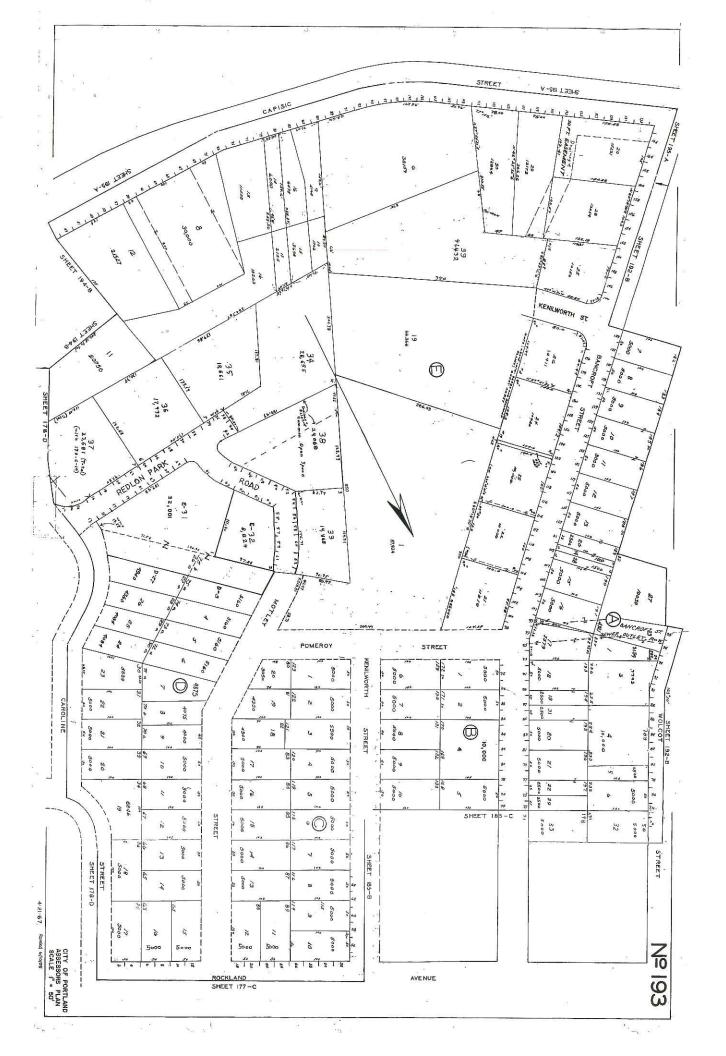
E.

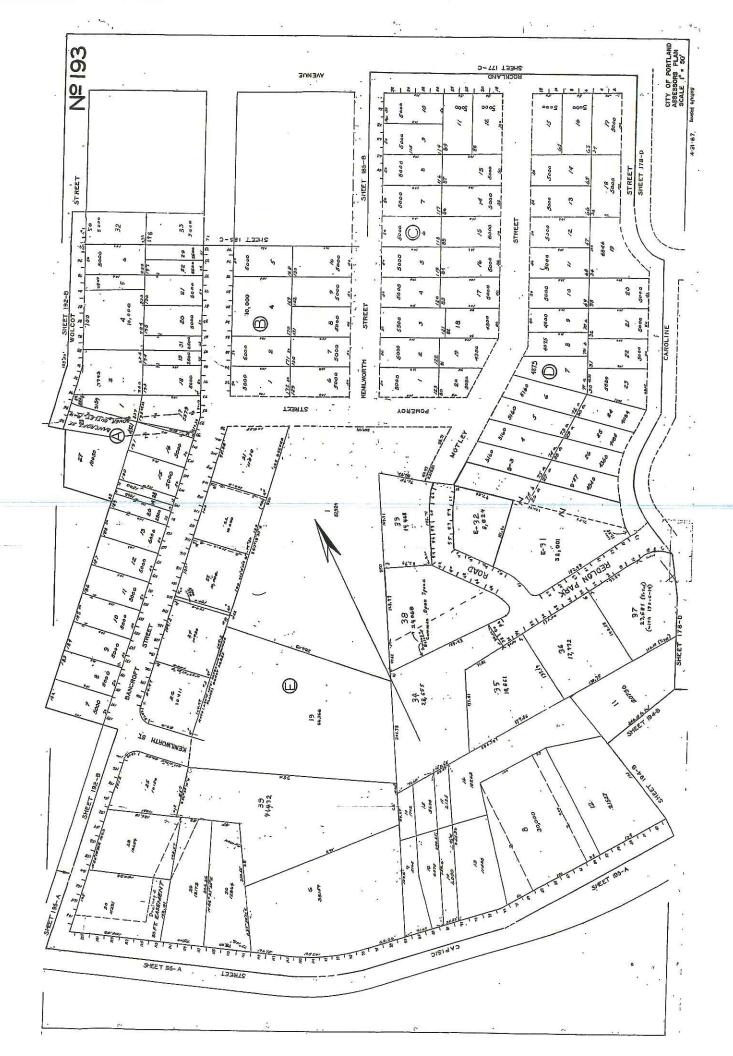
Print Name: Karen Belton

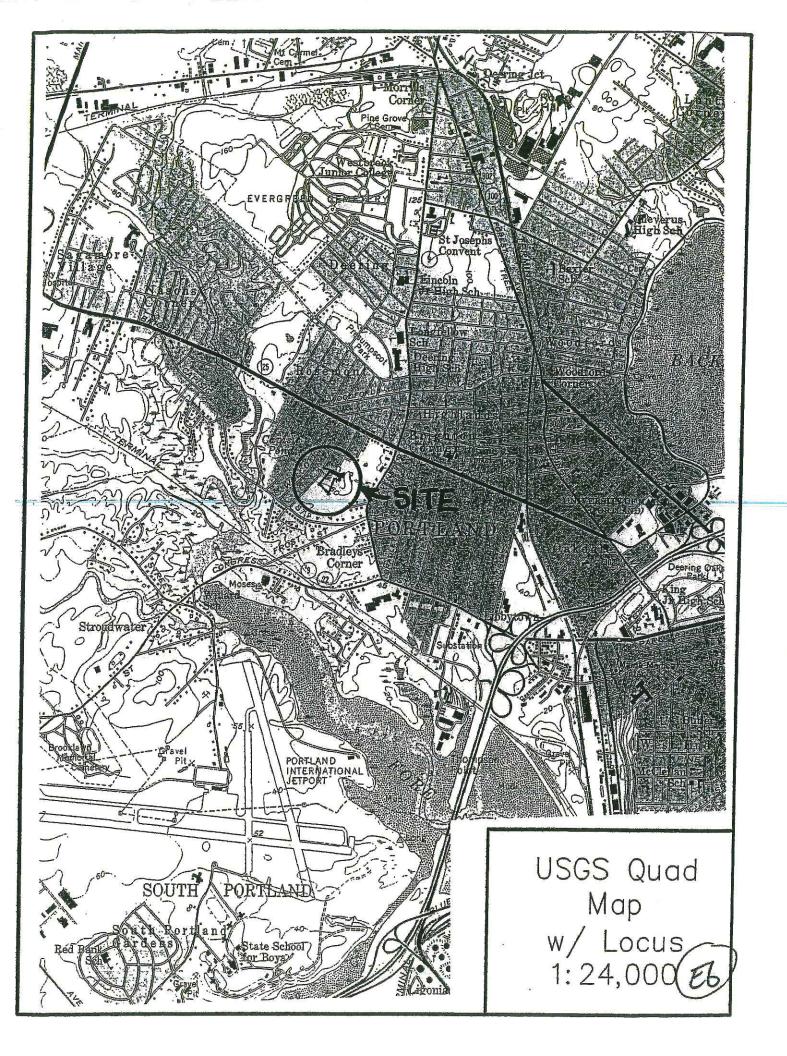
My commission expires 08/26/06

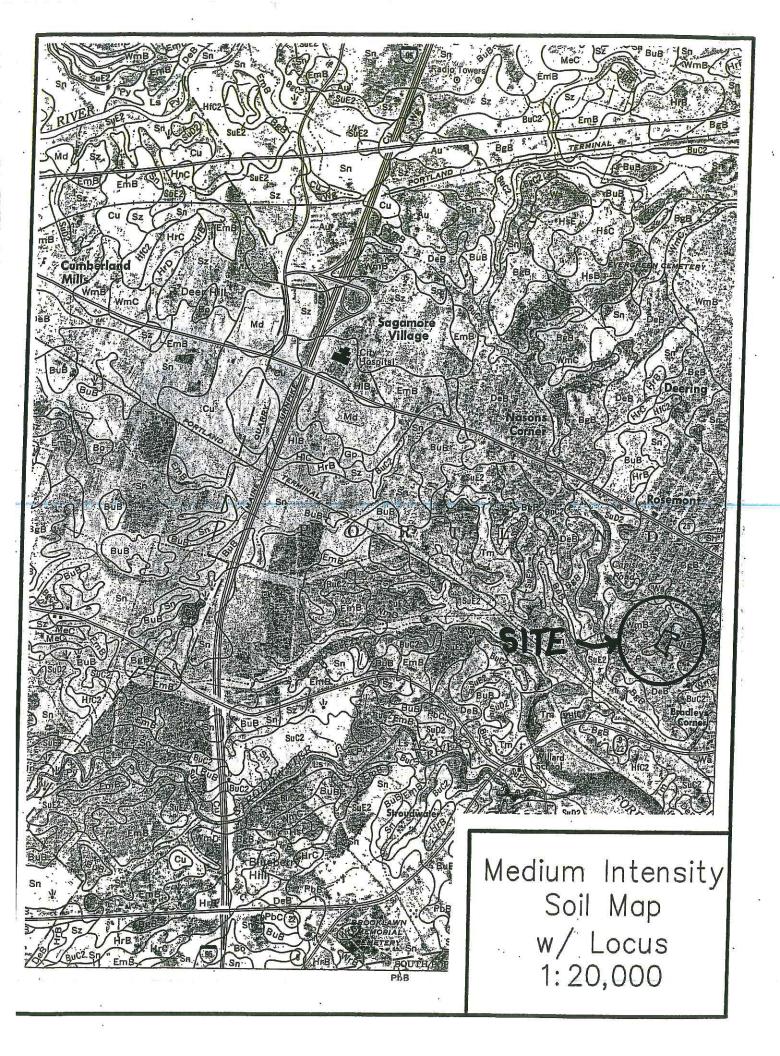
Received
Recorded Resister of Deeds
Jun 16-2004 11:22:22A
Cluburland Counts
John & Dirlan

Elbomelasscylourremtwifensky, mose & changlwarrenty deed to chahed doc











17 State House Station Augusta, ME 04333

IN THE MATTER OF

CHABAD LUBAVITCH OF MAINE INC Portland, Cumberland County HOUSE AND CHAPEL ACCESS DRIVE L-22414-TB-A-N (approval)

) NATURAL RESOURCES PROTECTION ACT) FRESHWATER WETLAND ALTERATION) WATER QUALITY CERTIFICATION) FINDINGS OF FACT AND ORDER

Project Description: The applicant proposes to alter 13,028 square feet of forested wetland to construct a project located off Pomeroy Street in the City of Portland. The project consists of a residence with attached chapel, access drive and stormwater detention pond. On-site impacts include 4,713 square feet of wetland fill associated with construction of portions of the access drive and detention pond. The parcel is situated off the westerly side of Pomeroy Street, a road previously approved by the City but never constructed.

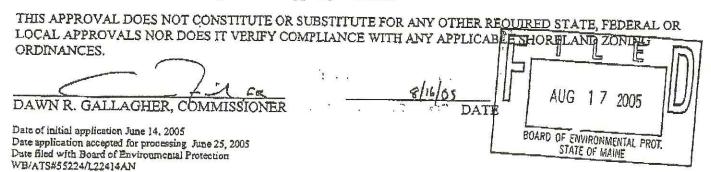
According to a letter dated August 10, 2005, from City Planner Ethan Boxer-Macomber, the City of Portland's Land Use Code requires any developer of a lot located on an unimproved City street to construct that street to City standards along the entirety of their site's frontage. As such, the applicant is required to impact an additional 8.315 square feet of forested wetland to construct Pomery Street to City standards. Total proposed impacts for the project are 13,028 square feet. The project is shown on a plan entitled, "Site Plan - Land of Chabad Lubavitch of Maine Inc." drawn by BH2M and dated March 2004. Wetland impacts were reduced to the current total after site visit by Department staff and subsequent discussions with the applicant's agent. Details of the revised plans are outlined in a memo from BH2M dated June 30, 2005.

Permit for:	X Tier l
DEP Decision:	X Approved Denied (see attached letter)
CORPS Action:	The Corps has been notified of your application. The following are subject to Federal screening: (1) projects with previously authorized or unauthorized work, in combination with a Tier 1 permit
	for a single and complete project, which total more than 15,000 square feet of altered area; (2) projects with multiple state permits and/or state exemptions which apply to a single and complete project that total more than 15,000 square feet of altered area; and (3) projects that may impact a vernal pool, as determined by the State of Maine or the Corps. If your activity is listed above, Corps approval is required for your project. For information regarding the status of your application contact the Corps' Maine Project Office at 623-8367.
Handland Camblelana	

Standard Conditions:

- 1) If construction or operation of the activity is not begun within two (2) years from the date signed, this permit shall lapse and the applicant shall reapply to the Department for a new permit. This permit is transferable only with prior approval from the Department. If the activity is associated with a larger project, starting any aspect of that project constitutes start of construction.
- 2) The project shall be completed according to the plans in the application. Any change in the project plans must be reviewed and approved by the Department.
- 3) Properly installed erosion control measures shall be installed prior to beginning the project, and all disturbed soil should be stabilized immediately upon project completion.
- 4) A copy of this approval will be sent to the City of Portland. Department approval of your activity does not supersede or substitute the need for any necessary local approvals.

Please note the attached sheet for guidance on appeal procedures.



STORMWATER MANAGEMENT REPORT FOR CHABAD LUBAVITCH OF MAINE INC. POMEROY STREET PORTLAND, MAINE

April 2005 Revised March 2012

Prepared By:

BH2M Engineers Engineers Surveyors Planners 28 State Street Gorham, ME 04038 207-839-2771 FAX 207-839-8250 lberry@bh2m.com

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Stormwater Quantity Control

- A. Narrative
- B. Maps
- C. Pre-development Site Plan
- D. Post-development Site Plan
- E. Runoff Analysis

Appendix A - Analysts' Qualifications

Appendix B – Maps

Appendix C – Stormwater Calculations (HydroCAD)

Appendix D - O & M Plan

STORMWATER MANAGEMENT REPORT

For: Chabad Lubavitch of Maine, Inc.

Portland, Maine

Introduction

Chabad Lubavitch of Maine, Inc. is proposing to construct a single-family residence on his parcel. This project also includes the construction of approximately 200 feet of Pomeroy Street. This site is a 1.69-acre parcel of land located off of Pomeroy Street in Portland. See attached USGS Location Map.

Pomeroy Street is an un-constructed public way. Part of the proposed development includes constructing Pomeroy Street to the City of Portland standards.

The project site will be served by public water and public sewer. Both utilities shall be extended to the site from Bancroft Street.

The total impervious area of the residence, driveway and Pomeroy Street equals 14,593 s.f. This is below the threshold for a DEP Stormwater Permit. Therefore, this report is for City of Portland Site Plan approval only.

A. Narrative

This site will house the residence of Rabbi Wilansky of Chabad Lubavitch of Maine, Inc. The building will be served by public water and sewer.

Construction of this building and related site improvements will not require a DEP Stormwater Permit. A similar project on this site was previously approved by the City of Portland for a single family home with attached assembly space (Site Plan and Conditional Use permits) back in August of 2005. These permits have since expired so the applicant is seeking to get new permits for the project with the removal of the assembly space. The original project received a DEP Tier I Wetland Alteration Permit (L22414-TB-A-N) for the filling of 13,028 s.f. of wetlands. An amendment to the Wetland Alteration Permit is required for this revised project since the wetland impacts have been reduced for the current proposed project (10,462 s.f.). This permit application will be filed by our office later in the approval process when the plans are further along.

B. Maps

See Appendix B.

C. Pre-development Site Plan

It is our understanding that abutters have drainage concerns about this project and continued concerns regarding a previous project called Redlon Park. We were able to obtain a digital file of the broader area that included the post-development drainage area for Redlon Park. As shown on the plan, the Redlon Park post-development basin generally passes to the south of the project site.

The project drainage basin consists of 6.10 acres of land that is tributary to an existing catchbasin on Bancroft Street. The project site (1.69 acres) makes up less than 1/3 of the drainage area. The basin is predominantly wooded except for the 5-6 existing homes. Although the site is moderately graded, the site appeared to be rather wet with sluggish drainage. The project will have no impact on the homes and land uphill (to the south).

D. Post-development Site Plan

The post-development plan is shown at a 1"=100 scale to provide an overall view. A second post-development plan is also included to provide a more detailed view. Key features include:

- 1. Pond 11 (South of access Drive) is a low spot that provides snow storage and also captures runoff on-site to protect the downstream abutters.
- 2. The driveway and single family house mostly drain back into the site (Ponds 15 & 11 or Existing Catch Basin in Bancroft Street).
- 3. A ditch along the northerly boundary should protect the Bancroft Street abutters from significant project site runoff.
- 4. All runoff eventually is discharged to the municipal combined sewer (Reach 8).

E. Runoff Analysis

The runoff from the site was calculated using HydroCAD computer software by Applied Microcomputer System, Chocura, New Hampshire. Both predevelopment and post-development calculations are in the same model (see calculations in Appendix C).

The analysis point is the combined sewer on Bancroft Street.

	Pre (SA-1)	Post (Reach 8)
2-Year Storm	3.13 c.f.s.	2.29 c.f.s.
10-Year Storm	7.57 c.f.s.	5.31 c.f.s.
25-Year Storm	9.86 c.f.s.	6.91 c.f.s.

The peak flow rates have been decreased for all storm events for this project. The project will also reduce drainage impacts to surrounding homes as the project is currently designed. No adverse impacts are anticipated to abutting homes or neighboring natural resources as a result of this project.

APPENDIX A

ANALYST'S QUALIFICATIONS



BH2M Providing Quality Civil-Site Engineering & Surveying for Over 33 Years.

SERVICES:

Site Development Design

Subdivision Design

- Stormwater Management Analysis & Design
- Utility Design

Roadway Design

Development Permitting

- Construction Administration & Oversight

- Full Service Survey Department

COMPANY OVERVIEW

Berry Huff McDonald Milligan Inc. (BH2M) was founded in 1978 in Gorham. Maine to provide quality civil-site engineering and surveying services. Over the past 33 years BH2M has worked on over 6,000 projects for our diverse client base, which consists of Municipal and Private Sector clients. BH2M has developed a reputation for a strong committment to excellence in all portions of a project. The staff structure at BH2M is unique in that all the engineers and project managers are partners within the company. This has been a successful formula that has resulted in many long standing relationships with our clients. Each project at BH2M is overseen by a senior principal within the company to assure the highest level of quality of work and performance.

EXPERIENCE

BH2M has provided Quality Civil-Site Engineering and Surveying on many projects within the City of Portland, Including:

Office & Commercial Developments

- Oakhurst Dairy
- WB Mason Headquarters
- Unum
- Walgreens Bayside Area

Hospitals

- Childrens Hospital
- Congress St. Medical Building

Roadway Improvements

- Oak Street

Major Industrial

- Brunswick Naval Air Station Hanger Project Brunswick
- Brunswick Naval Air Station Tower Project Brunswick
- Savage Intermodel Facility Auburn
- Pratt-Whitney North Berwick

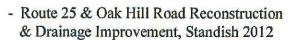
CURRENT PROJECTS

- Route 25 & Oak Hill Road Intersection Relocation and Sidewalk Project-Standish
- Summer Winds Condominiums Old Orchard Beach
- Black Point Park at Scarborough Beach Scarborough
- Village Square Sidewalk Restoration Gorham
- Fogg Brook Subdivision Buxton
- Sawyer Estates Subdivision Gorham

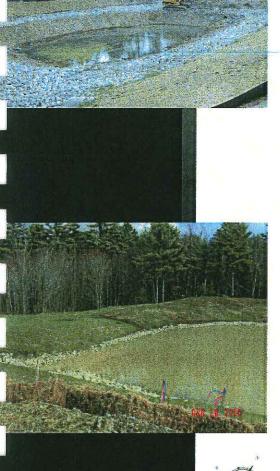


Selected Stormwater Management Project Experience





- Summer Winds Condominium, Old Orchard Beach 2011
- Village Square Sidewalk Reconstruction, Gorham 2011
- Black Point Park at Scarborough Beach, Scarborough 2011
- Fogg Brook Subdivision, Gorham 2011
- Sawyer Estates Subdivision, Gorham 2011
- Golden Ridge Subidvision, Cape Elizabeth 2011
- Juniper Knoll Subdivision, Saco 2010
- Meadowbrook Subdivision, Waterboro 2010
- Bradbury Ridge Subdivision, Hollis 2010
- Lands End Subdivision, Old Orchard Beach 2010
- Limington Salt Shed, Limington 2010
- Peterson Fields Subdivision, 2010
- The Trails Subdivision, Saco 2010
- Mitchell Hill Subdivision, Windham 2010
- Stonehill Subdivision, Biddeford 2010
- Kate's Homemade Butter Plant, Arundel 2010
- Carsons Point Subdivision, Saco 2010
- Pratts Brook Farm Subdivision, Yarmouth 2010
- Highland Glen Subdivision, Yarmouth 2010
- Tucker Road Culvert Improvements, Limington 2010
- Dunegrass Sections B & C, Old Orchard Beach 2010
- Skylark Commons Subdivision, Portland 2009
- Pleasant Ridge Subdivision, Buxton 2009
- Wholesale Distribution and Warehouse Facility, Brockton Ma 2009
- Willowdale Commons Condominium, Old Orchard Beach 2009
- Atlantic Park Condominium, Old Orchard Beach 2009
- Whispering Pines Subdivision, Buxton 2009
- Aceto Construction Facility, Buxton 2009
- D & E Enterprises Facility, Hollis 2009
- K & S Subdivision, Sanford 2009
- Wild Acres Campground Expansion, Old Orchard Beack 2009
- Hardware Store, Biddeford 2009
- Hid n Pines Campground, Old Orchard Beach 2009
- Green Court Acres Subdivision, Waterboro 2009





BH2M

Lester S. Berry, P.E.

EDUCATION:

B.S. Civil Engineering University of Maine

M.S. Civil Engineering University of Maine

PROFESSIONAL SOCIETIES:

American Society of Civil gineers

Maine Association of Planners

nstruction Specifications Institute

OFESSIONAL BACKGROUND:

e President BH2M 1978 - Present brham, Maine

Project Engineer lle E. Caruthers Company 75 - 1978 Gorham, Maine

gineer State of New Hampshire 71 - 1972 oncord, New Hampshire Lester S. Berry, P.E. Vice President & Senior Engineer

Les co-founded Berry Huff McDonald Milligan Inc. in 1978. He has 40 years of experience in both the public and private sector and has worked on projects in Maine and New Hampshire. His expertise includes a diversified range of all aspects of civil-site engineering, with a focus on site development and the design and implementation of state of the art Stormwater Management Systems.

The following is a list of recent projects worked on by Les:

- Route 25 & Oak Hill Road Intersection Relocation and Sidewalk Project, Standish 2012
- Summer Winds Condominium, Old Orchard Beach 2011
- Village Square Sidewalk Restoration, Gorham 2011
- Black Point Park at Scarborough Beach, Scarborough 2011
- Sawyer Estates Subdivision, Gorham 2011
- Limington Salt Shed, Limington 2011
- Kate's Homemade Butter Plant, Arundel 2011
- Peterson Fields Subdivision, Gorham 2010
- The Trails Subdivision, Gorham 2010
- Savage Intermodal Facility, Auburn 2010
- Childrens Hospital, Portland 2010
- Tucker Road Culvert Improvements, Limington 2010
- Mitchell Hill Subdivision, Windham 2010
- Stonehill Subdivision, Biddeford 2010
- Skylark Commons Subdivision, Portland 2010
- WB Mason Headquarters, Portland 2009
- Walgreens, Portland 2009
- Unum Site Improvements, Portland 2008
- Congress Street Medical Building, Portland 2008



ORMWATER EDUCATION: Hydraulic & related College course Erosion & Sediment Control Stormwater Management Water Conservation Districts, Phosphorus Management, Erosion Control Seminars by Maine DEP, HydroCAD & Advanced HydroCAD, BMP's for Stormwater and Erosion Control.



BH2M

Andrew S. Morrell, E.I.T.

EDUCATION:

B.S. Civil Engineering State University of New York uffalo, NY

PROFESSIONAL ACKGROUND:

Project Engineer H2M - Gorham, Me August 2001 - August 2007 April 2010 - Present

Project Engineer
PeLuca-Hoffman Associates
outh Portland, Me
August 2007 - March 2010

oject Engineer
Diversified Civil Engineering
Sestford, Ma
ay 1999 - August 2001

TORMWATER DUCATION:

ydraulics Review Class r Professional Engineering License Exam - ASCE 2009

ydrocad Seminar
Joint Environment Training
bordinating Committee 2002

STORMWATER KPERIENCE:

12 years experience rforming stormwater management design and calculations.

Andrew S. Morrell, E.I.T. Project Engineer

Andy has worked for BH2M for over 9 years and has over 12 years of experience in both the public and private sector and has worked on projects in Maine and Massachusetts. His expertise includes site development, subdivisions and the design of supporting Stormwater Management Systems.

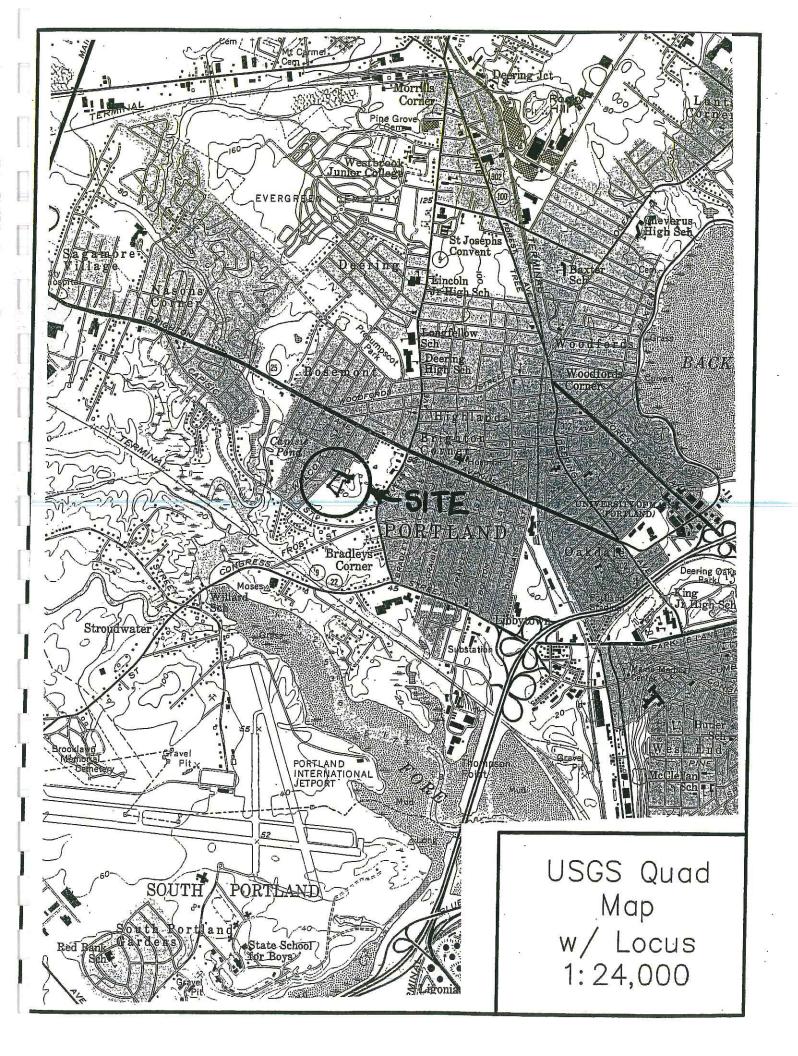
The following is a list of recent projects worked on by Andy:

- Route 25 & Oak Hill Road Intersection Relocation and Sidewalk Project, Standish 2012
- Childrens Hospital, Portland 2011
- Village Square Sidewalk Restoration, Gorham 2011
- Black Point Park at Scarborough Beach, Scarborough 2011
- Sawyer Estates Subdivision, Gorham 2011
- Juniper Knoll Subdivision, Saco 2010
- Limington Salt Shed, Limington 2010
- Kate's Homemade Butter Plant, Arundel 2010
- Tucker Road Culvert Improvements, Limington 2010
- Bradbury Ridge Subdivision, Buxton 2010
- Lands End Subdivision, Old Orchard Beach 2010
- Peterson Fields Subdivision, Gorham 2010
- The Trails Subdivision, Saco 2010
- Mitchell Hill Subdivision, Windham 2010
- Stonehill Subdivision, Biddeford 2010
- Carsons Point Subdivision, Saco 2010
- Pratts Brook Farm Subdivision, Yarmouth 2010
- Highland Glen Subdivision, Yarmouth 2010
- Skylark Commons Subdivision, Portland 2010
- Sunrise Ridge Subdivision, Buxton 2009
- Atlantic Park Condominium, Old Orchard Beach 2009
- Pleasant Ridge Subdivision, Buxton 2009
- Dunegrass Sections B & C, Old Orchard Beach 2009
- Meadowbrook Subdivision, Waterboro 2009
- Aceto Construction Facility, Buxton 2009
- Unum Site Improvements, Portland 2009



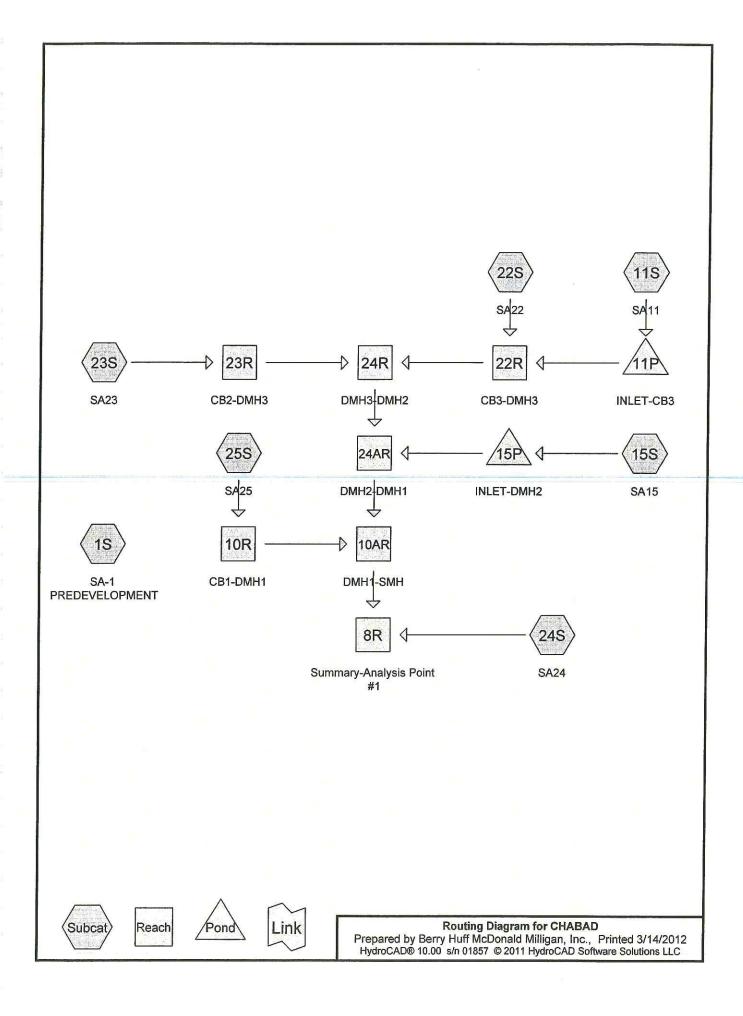
APPENDIX B

MAPS



APPENDIX C

STORMWATER CALCULATIONS



Area Listing (all nodes)

Are	ea CN	Description
(acre	es)	(subcatchment-numbers)
2.72	20 55	Woods, Good, HSG B (1S, 11S)
1.29	90 70	Woods, Good, HSG C (1S, 11S, 25S)
0.1	55 74	>75% Grass cover, Good, HSG C (11S, 15\$, 23S, 24S, 25S)
0.94	10 75	1/4 acre lots, 38% imp, HSG B (1S, 11S)
1.70	00 77	Woods, Good, HSG D (1S, 11S, 25S)
0.35	50 80	>75% Grass cover, Good, HSG D (11S, 15S, 24S)
2.12	20 83	1/4 acre lots, 38% imp, HSG C (1S, 24S)
2.55	50 87	1/4 acre lots, 38% imp, HSG D (1S, 11S, 24S)
0.04	10 98	Existing Impervious Area (25S)
0.33	35 98 ·	Proposed Impervious Area (11S, 15S, 22S, 23S, 24S, 25S)
12.20	00 75	TOTAL AREA

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S:SA-1 Runoff Area=6.100 ac 20.62% Impervious Runoff Depth>0.86"

Flow Length=1,000' Tc=41.4 min CN=75 Runoff=3.13 cfs 0.438 af

Subcatchment 11S:SA11

Runoff Area=4.160 ac 16.64% Impervious Runoff Depth>0.71"

Flow Length=816' Tc=59.2 min CN=72 Runoff=1.40 cfs 0.247 af

Subcatchment 15S:SA15

Runoff Area=0.160 ac 6.25% Impervious Runoff Depth>1.09"
Flow Length=257' Tc=18.4 min CN=79 Runoff=0.15 cfs 0.015 af

Subcatchment 22S:SA22 Runoff Area=0.010 ac 100.00% Impervious Runoff Depth>2.59" Flow Length=30' Slope=0.0300 '/' Tc=0.4 min CN=98 Runoff=0.03 cfs 0.002 af

Subcatchment 23S:SA23 Runoff Area=0.020 ac 75.00% Impervious Runoff Depth>2.04" Flow Length=30' Slope=0.0300 '/' Tc=0.4 min CN=92 Runoff=0.06 cfs 0.003 af

Subcatchment 24S:SA24

Runoff Area=0.990 ac 40.57% Impervious Runoff Depth>1.47"
Flow Length=401' Tc=19.3 min CN=85 Runoff=1.26 cfs 0.121 af

Subcatchment 25S:SA25 Runoff Area=0.760 ac 15.79% Impervious Runoff Depth>1.08" Flow Length=415' Tc=36.3 min CN=79 Runoff=0.53 cfs 0.068 af

Reach 8R: Summary-AnalysisPoint #1 Inflow=2.29 cfs 0.455 af Outflow=2.29 cfs 0.455 af

Reach 10R: CB1-DMH1 Avg. Flow Depth=0.23' Max Vel=3.37 fps Inflow=0.53 cfs 0.068 af 15.0" Round Pipe n=0.012 L=15.0' S=0.0100 '/' Capacity=7.00 cfs Outflow=0.53 cfs 0.068 af

Reach 22R: CB3-DMH3 Avg. Flow Depth=0.38' Max Vel=4.46 fps Inflow=1.40 cfs 0.248 af 15.0" Round Pipe n=0.012 L=10.0' S=0.0100 '/' Capacity=7.00 cfs Outflow=1.40 cfs 0.248 af

Reach 23R: CB2-DMH3 Avg. Flow Depth=0.08' Max Vel=1.77 fps Inflow=0.06 cfs 0.003 af 12.0" Round Pipe n=0.012 L=10.0' S=0.0100 '/' Capacity=3.86 cfs Outflow=0.06 cfs 0.003 af

Reach 24AR:DMH2-DMH1 Avg. Flow Depth=0.28' Max Vel=7.03 fps Inflow=1.45 cfs 0.265 af 15.0" Round Pipe n=0.012 L=124.0' S=0.0350 '/' Capacity=13.09 cfs Outflow=1.45 cfs 0.265 af

Reach 24R: DMH3-DMH2 Avg. Flow Depth=0.27' Max Vel=7.31 fps Inflow=1.41 cfs 0.251 af 15.0" Round Pipe n=0.012 L=41.0' S=0.0400 '/' Capacity=14.00 cfs Outflow=1.41 cfs 0.251 af

Pond 11P:INLET-CB3 Peak Elev=61.82' Storage=139 cf Inflow=1.40 cfs 0.247 af 15.0" Round Culvert n=0.012 L=10.0' S=0.0050 '/' Outflow=1.40 cfs 0.246 af

Pond 15P:INLET-DMH2

Peak Elev=59.86' Storage=26 cf Inflow=0.15 cfs 0.015 af 12.0" Round Culvert n=0.012 L=28.0' S=0.0200 '/' Outflow=0.15 cfs 0.014 af

Type III 24-hr 2-Year Storm Event Rainfall=3.00"

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Total Runoff Area = 12.200 ac Runoff Volume = 0.894 af Average Runoff Depth = 0.88" 79.45% Pervious = 9.693 ac 20.55% Impervious = 2.507 ac

41.4

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Summary for Subcatchment 1S: SA-1 PREDEVELOPMENT

Runoff = 3.13 cfs @ 12.62 hrs, Volume=

0.438 af, Depth> 0.86"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Storm Event Rainfall=3.00"

Area	(ac)	CN	Des	cription				
1.	050	87	1/4 8	acre lots, 3	88% imp, H	ISG D		
1.	050	77	Woo	ds, Good,	HSG D			
1.250 55 Woods, Good, HSG B								
0.760 75 1/4 acre lots, 38% imp, HSG B								
		83				ISG C		
0.	490	70	Woo	ds, Good,	HSG C			
6.	100	75	Weig	hted Aver	age			
			79.3	8% Pervio	us Area			
1.	258		20.6	2% Imperv	ious Area			
1 <u>2-2</u>	f2 924	12010	SY.	20 20 30				
				1000		Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
26.9	150	0.0	100	0.09		Sheet Flow, LAWN		
a consumption	Aver All trade that are a second	0	rumanum en	Legie amenggalastas ken		Grass: Dense n= 0.240 P2= 3.00"		
14.5	850	0.0	380	0.97		Shallow Concentrated Flow, LIGHT WOODS Woodland Kv= 5.0 fps		
	1. 1. 0. 1. 0. 6. 4. 1. Tc (min)	1.050 1.050 1.250 0.760 1.500 0.490 6.100 4.842 1.258 Tc Length (min) (feet) 26.9 150	1.050 87 1.050 77 1.050 77 1.250 55 0.760 75 1.500 83 0.490 70 6.100 75 4.842 1.258 Tc Length Si (min) (feet) (26.9 150 0.0	1.050 87 1/4 a 1.050 77 Woo 1.250 55 Woo 0.760 75 1/4 a 1.500 83 1/4 a 0.490 70 Woo 6.100 75 Weig 4.842 79.3 1.258 20.6 Tc Length Slope (min) (feet) (ft/ft) 26.9 150 0.0100	1.050 87 1/4 acre lots, 3 1.050 77 Woods, Good, 1.250 55 Woods, Good, 0.760 75 1/4 acre lots, 3 1.500 83 1/4 acre lots, 3 0.490 70 Woods, Good, 6.100 75 Weighted Aver 4.842 79.38% Pervio 1.258 20.62% Imperv Tc Length Slope Velocity (min) (feet) (ft/ft) (ft/sec) 26.9 150 0.0100 0.09	1.050 87 1/4 acre lots, 38% imp, H 1.050 77 Woods, Good, HSG D 1.250 55 Woods, Good, HSG B 0.760 75 1/4 acre lots, 38% imp, H 1.500 83 1/4 acre lots, 38% imp, H 0.490 70 Woods, Good, HSG C 6.100 75 Weighted Average 4.842 79.38% Pervious Area 1.258 20.62% Impervious Area Tc Length Slope Velocity Capacity (min) (feet) (ft/ft) (ft/sec) (cfs) 26.9 150 0.0100 0.09		

Summary for Subcatchment 11S: SA11

Runoff = 1.40 cfs @ 12.88 hrs, Volume=

1,000 Total

0.247 af, Depth> 0.71"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Storm Event Rainfall=3.00"

	Area (ac)	CN	Description
	0.670	70	Woods, Good, HSG C
	0.150	77	Woods, Good, HSG D
	1.300	87	1/4 acre lots, 38% imp, HSG D
*	0.130	98	Proposed Impervious Area
	0.190	80	>75% Grass cover, Good, HSG D
	0.070	74	>75% Grass cover, Good, HSG C
	0.180	75	1/4 acre lots, 38% imp, HSG B
	1.470	55	Woods, Good, HSG B
	4.160	72	Weighted Average
	3.468		83.36% Pervious Area
	0.692		16.64% Impervious Area

Type III 24-hr 2-Year Storm Event Rainfall=3.00"

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
43.5	150	0.0030	0.06	1000000	Sheet Flow,	
					Grass: Dense n= 0.240 P2= 3.00"	
15.7	666	0.0200	0.71		Shallow Concentrated Flow,	
					Woodland Kv= 5.0 fps	
59.2	816	Total				

Summary for Subcatchment 15S: SA15

Runoff = 0.15 cfs @ 12.27 hrs, Volume=

0.015 af, Depth> 1.09"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Storm Event Rainfall=3.00"

	Area (ac)	CN	Description
*	0.010	98	Proposed Impervious Area
	0.100	80	>75% Grass cover, Good, HSG D
	0.050	74	>75% Grass cover, Good, HSG C
0.	0.160	79	Weighted Average
	0.150		93.75% Pervious Area
	0.010		6.25% Impervious Area

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.5	16.3	150	0.0350	0.15	85 25	Sheet Flow,
						Grass: Dense n= 0.240 P2= 3.00"
	2.1	107	0.0150	0.86		Shallow Concentrated Flow,
1:2				W		Short Grass Pasture Kv= 7.0 fps
	18.4	257	Total		_	

Summary for Subcatchment 22S: SA22

Runoff = 0.03 cfs @ 12.00 hrs, Volume=

0.002 af, Depth> 2.59"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Storm Event Rainfall=3.00"

	Area	(ac) C	N Des	cription					
*	0.	010 9	98 Prop	osed Impe	ervious Are	a			
	0.	010	100	.00% Impe	rvious Area	a			
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
	0.4	30	0.0300	1.23		Sheet Flow, Smooth surfaces	n= 0.011	P2= 3.00"	-

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Summary for Subcatchment 23S: SA23

Runoff 0.06 cfs @ 12.01 hrs, Volume= 0.003 af, Depth> 2.04"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Storm Event Rainfall=3.00"

	Area	(ac)	CN	Des	cription					
*	0.	.015	98	Prop	osed Impe	ervious Are	a			
_	0.	005	74	>759	% Grass co	over, Good	, HSG C			
	0.	020 005 015	92	25.0	ghted Aver 0% Pervio 0% Imper					
2	Tc (min)	Lengt (feet		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
	0.4	3	0.	0300	1.23		Sheet Flow, Smooth surfaces	n= 0.011	P2= 3 00"	

Summary for Subcatchment 24S: SA24

1.26 cfs @ 12.27 hrs, Volume= 0.121 af, Depth> 1.47"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Storm Event Rainfall=3.00"

	Area	(ac) C	N Des	cription						
	0.	200	37 1/4 8	acre lots, 3	38% imp, H	SG D				
	0.	620			38% imp, H					
*	0.	090	98 Prop	osed Imp	ervious Are	ea				
	0.				over, Good	.t				
_	0.	020	74 >759	% Grass c	over, Good	, HSG C				
				ghted Ave						
0.588 59.43% Pervious Area										
	0.	402	40.5	7% Imper	vious Area					
	_	19 (4 (4 (4 (4 (4 (4 (4 (4 (4 (4 (4 (4 (4			120					
	Tc	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	16.7	150	0.0330	0.15		Sheet Flow,				
		-				Grass: Dense n= 0.240 P2= 3.00"				
	1.3	94	0.0300	1.21		Shallow Concentrated Flow,				
	2 (22)		12 12 122	227 (2752)		Short Grass Pasture Kv= 7.0 fps				
	1.3	157	0.0100	2.03		Shallow Concentrated Flow,				
						Paved Kv= 20.3 fps	4			
	100000									

Type III 24-hr 2-Year Storm Event Rainfall=3.00"

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Summary for Subcatchment 25S: SA25

Runoff = 0.53 cfs @ 12.53 hrs, Volume=

0.068 af, Depth> 1.08"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Storm Event Rainfall=3.00"

	Area	(ac)	CN De	escription		
*	0.	080	98 Pr	oposed Imp	ervious Are	ea
	0.	010		5% Grass of		
	0.	130	70 W	oods, Good	HSG C	######################################
	0.	500	77 W	oods, Good	, HSG D	
*	0.	040	98 Ex	isting Imper	vious Area	
	0.	760	79 W	eighted Ave	rage	
	0.	640	84	.21% Pervio	ous Area	
	0.	120	15	.79% Imper	vious Area	
	Тс	Length			Capacity	Description
	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)	
	30.7	150	0.020	0.08		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.00"
الدائيل المستعد	5.6	265	0.025	0.79	والمستوا المتاكرة الرازي ومعتدات	Shallow Concentrated Flow,
					1974	Woodland Kv= 5.0 fps
	36.3	415	Total			

Summary for Reach 8R: Summary-Analysis Point #1

Inflow Area = 6.100 ac, 20.48% Impervious, Inflow Depth > 0.90" for 2-Year Storm Event event

Inflow = 2.29 cfs @ 12.59 hrs, Volume= 0.455 af

Outflow = 2.29 cfs @ 12.59 hrs, Volume= 0.455 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Summary for Reach 10AR: DMH1-SMH

Inflow Area = 5.110 ac, 16.58% Impervious, Inflow Depth > 0.78" for 2-Year Storm Event event

Inflow = 1.81 cfs @ 12.81 hrs, Volume= 0.334 af

Outflow = 1.81 cfs @ 12.81 hrs, Volume= 0.334 af, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 10.02 fps, Min. Travel Time= 0.0 min Avg. Velocity = 4.30 fps, Avg. Travel Time= 0.1 min

Peak Storage= 3 cf @ 12.81 hrs

Average Depth at Peak Storage= 0.24'

Bank-Full Depth= 1.50' Flow Area= 1.8 sf, Capacity= 33.07 cfs

Type III 24-hr 2-Year Storm Event Rainfall=3.00"

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18.0" Round Pipe n= 0.012 Length= 18.0' Slope= 0.0844 '/' Inlet Invert= 54.55', Outlet Invert= 53.03'



Summary for Reach 10R: CB1-DMH1

Inflow Area = 0.760 ac, 15.79% Impervious, Inflow Depth > 1.08" for 2-Year Storm Event event

Inflow = 0.53 cfs @ 12.53 hrs, Volume= 0.068 af

Outflow = 0.53 cfs @ 12.53 hrs, Volume= 0.068 af, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.37 fps, Min. Travel Time= 0.1 min Avg. Velocity = 1.68 fps, Avg. Travel Time= 0.1 min

Peak Storage= 2 cf @ 12.53 hrs Average Depth at Peak Storage= 0.23' Bank-Full Depth= 1.25' Flow Area= 1.2 sf, Capacity= 7.00 cfs

15.0" Round Pipe n= 0.012 Length= 15.0' Slope= 0.0100 '/' Inlet Invert= 54.80', Outlet Invert= 54.65'



Summary for Reach 22R: CB3-DMH3

Inflow Area = 4.170 ac, 16.84% Impervious, Inflow Depth > 0.71" for 2-Year Storm Event event

Inflow = 1.40 cfs @ 12.90 hrs, Volume= 0.248 af

Outflow = 1.40 cfs @ 12.90 hrs, Volume= 0.248 af, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.46 fps, Min. Travel Time= 0.0 min Avg. Velocity = 1.78 fps, Avg. Travel Time= 0.1 min

Peak Storage= 3 cf @ 12.90 hrs Average Depth at Peak Storage= 0.38' Bank-Full Depth= 1.25' Flow Area= 1.2 sf, Capacity= 7.00 cfs

Type III 24-hr 2-Year Storm Event Rainfall=3.00"

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15.0" Round Pipe n = 0.012Length= 10.0' Slope= 0.0100 '/' Inlet Invert= 60.93', Outlet Invert= 60.83'



Summary for Reach 23R: CB2-DMH3

Inflow Area =

0.020 ac, 75.00% Impervious, Inflow Depth > 2.04" for 2-Year Storm Event event

Inflow

0.06 cfs @ 12.01 hrs, Volume= 0.003 af

Outflow

0.06 cfs @ 12.01 hrs, Volume=

0.003 af, Atten= 1%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.77 fps, Min, Travel Time= 0.1 min Avg. Velocity = 0.63 fps, Avg. Travel Time= 0.3 min

Peak Storage= 0 cf @ 12.01 hrs Average Depth at Peak Storage= 0.08' Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 3.86 cfs

12.0" Round Pipe n = 0.012Length= 10.0' Slope= 0.0100 '/' Inlet Invert= 60.93', Outlet Invert= 60.83'



Summary for Reach 24AR: DMH2-DMH1

Inflow Area =

4.350 ac, 16.72% Impervious, Inflow Depth > 0.73" for 2-Year Storm Event event

0.265 af

Inflow

1.45 cfs @ 12.89 hrs, Volume=

0.265 af, Atten= 0%, Lag= 0.5 min

Outflow

1.45 cfs @ 12.90 hrs, Volume=

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 7.03 fps, Min. Travel Time= 0.3 min Avg. Velocity = 2.92 fps, Avg. Travel Time= 0.7 min

Type III 24-hr 2-Year Storm Event Rainfall=3.00"

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Peak Storage= 26 cf @ 12.90 hrs Average Depth at Peak Storage= 0.28' Bank-Full Depth= 1.25' Flow Area= 1.2 sf, Capacity= 13.09 cfs

15.0" Round Pipe n= 0.012 Length= 124.0' Slope= 0.0350 '/' Inlet Invert= 58.99', Outlet Invert= 54.65'



Summary for Reach 24R: DMH3-DMH2

Inflow Area = 4.190 ac, 17.12% Impervious, Inflow Depth > 0.72" for 2-Year Storm Event event

Inflow = 1.41 cfs @ 12.90 hrs, Volume= 0.251 af

Outflow = 1.41 cfs @ 12.90 hrs, Volume= 0.251 af, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 7.31 fps, Min. Travel Time= 0.1 min Avg. Velocity = 3.01 fps, Avg. Travel Time= 0.2 min

Peak Storage= 8 cf @ 12.90 hrs Average Depth at Peak Storage= 0.27' Bank-Full Depth= 1.25' Flow Area= 1.2 sf, Capacity= 14.00 cfs

15.0" Round Pipe n= 0.012 Length= 41.0' Slope= 0.0400 '/' Inlet Invert= 60.73', Outlet Invert= 59.09'



Summary for Pond 11P: INLET-CB3

Inflow Area = 4.160 ac, 16.64% Impervious, Inflow Depth > 0.71" for 2-Year Storm Event event

Inflow = 1.40 cfs @ 12.88 hrs, Volume= 0.247 af

Outflow = 1.40 cfs @ 12.90 hrs, Volume= 0.246 af, Atten= 0%, Lag= 1.2 min

Primary = 1.40 cfs @ 12.90 hrs, Volume= 0.246 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Type III 24-hr 2-Year Storm Event Rainfall=3.00"

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Peak Elev= 61.82' @ 12.90 hrs Surf.Area= 295 sf Storage= 139 cf

Plug-Flow detention time= 2.8 min calculated for 0.246 af (100% of inflow)

Center-of-Mass det. time= 1.7 min (867.8 - 866.1)

Volume	Ir	vert	Avail.Sto	rage Storage	Description	
#1	6	1.08'	1,0	33 cf Custom	Stage Data (P	rismatic) Listed below
Elevatio		Su	rf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
61.0	08		10	0	0	
62.0	00		365	173	173	
63.0	00		1,356	861	1,033	
Device	Routin	g	Invert	Outlet Devices	3	
#1	Primar	У	61.08'	15.0" Round	Culvert	20
				Inlet / Outlet Ir		headwall, Ke= 0.900 31.03' S= 0.0050 '/' Cc= 0.900 f

Primary OutFlow Max=1.40 cfs @ 12.90 hrs HW=61.82' (Free Discharge)
—1=Culvert (Barrel Controls 1.40 cfs @ 2.67 fps)

Summary for Pond 15P: INLET-DMH2

Inflow Area = 0.160 ac, 6.25% Impervious, Inflow Depth > 1.09" for 2-Year Storm Event event

Inflow = 0.15 cfs @ 12.27 hrs, Volume= 0.015 af

Outflow = 0.15 cfs @ 12.28 hrs, Volume= 0.014 af, Atten= 0%, Lag= 0.8 min

Primary = 0.15 cfs @ 12.28 hrs, Volume= 0.014 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 59.86' @ 12.28 hrs Surf.Area= 80 sf Storage= 26 cf

Plug-Flow detention time= 14.4 min calculated for 0.014 af (97% of inflow)

Center-of-Mass det. time= 5.4 min (823.7 - 818.4)

Volume	Invert	Avail	.Storage	Storage	e Description			
#1	59.39'		1,925 cf	Custor	n Stage Data (Pris	<mark>matic)</mark> Lis	ted below	
Elevation (feet)	//E.OTOE.5	Area (sq-ft)		Store c-feet)	Cum.Store (cubic-feet)			
59.39		10		0	0			
60.00		100		34	34			
61.00		187		144	177			
62.00		784		486	663			
63.00	ä	1,740		1,262	1,925			

Device	Routing	Invert	Outlet Devices
#1	Primary	59.65'	12.0" Round Culvert L= 28.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 59.65' / 59.09' S= 0.0200 '/' Cc= 0.900

Type III 24-hr 2-Year Storm Event Rainfall=3.00"

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n= 0.012, Flow Area= 0.79 sf

Primary OutFlow Max=0.15 cfs @ 12.28 hrs HW=59.86' (Free Discharge)
—1=Culvert (Inlet Controls 0.15 cfs @ 1.23 fps)

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S:SA-1

Runoff Area=6.100 ac 20.62% Impervious Runoff Depth>2.02" Flow Length=1,000' Tc=41.4 min CN=75 Runoff=7.57 cfs 1.026 af

Subcatchment 11S:SA11

Runoff Area=4.160 ac 16.64% Impervious Runoff Depth>1.78" Flow Length=816' Tc=59.2 min CN=72 Runoff=3.73 cfs 0.616 af

Subcatchment 15S:SA15

Runoff Area=0.160 ac 6.25% Impervious Runoff Depth>2.36" Flow Length=257' Tc=18.4 min CN=79 Runoff=0.33 cfs 0.031 af

Subcatchment 22S:SA22

Runoff Area=0.010 ac 100.00% Impervious Runoff Depth>4.15" Flow Length=30' Slope=0.0300 '/' Tc=0.4 min CN=98 Runoff=0.05 cfs 0.003 af

Subcatchment 23S:SA23

Runoff Area=0.020 ac 75.00% Impervious Runoff Depth>3.59" Flow Length=30' Slope=0.0300 '/' Tc=0.4 min CN=92 Runoff=0.10 cfs 0.006 af

Subcatchment 24S:SA24

Runoff Area=0.990 ac 40.57% Impervious Runoff Depth>2.89" Flow Length=401' Tc=19.3 min CN=85 Runoff=2.44 cfs 0.239 af

Subcatchment 25S:SA25

Runoff Area=0.760 ac 15.79% Impervious Runoff Depth>2.35" Flow Length=415' Tc=36.3 min CN=79 Runoff=1.17 cfs 0.149 af

Reach 8R: Summary-AnalysisPoint #1

Inflow=5.31 cfs 1.042 af

Outflow=5.31 cfs 1.042 af

Reach 10AR: DMH1-SMH

Avg. Flow Depth=0.37' Max Vel=13.11 fps Inflow=4.52 cfs 0.803 af 18.0" Round Pipe n=0.012 L=18.0' S=0.0844 '/' Capacity=33.07 cfs Outflow=4.52 cfs 0.803 af

Reach 10R: CB1-DMH1

Avg. Flow Depth=0.35' Max Vel=4.23 fps Inflow=1.17 cfs 0.149 af

15.0" Round Pipe n=0.012 L=15.0' S=0.0100 '/' Capacity=7.00 cfs Outflow=1.17 cfs 0.149 af

Reach 22R: CB3-DMH3

Avg. Flow Depth=0.65' Max Vel=5.78 fps Inflow=3.70 cfs 0.618 af 15.0" Round Pipe n=0.012 L=10.0' S=0.0100 '/' Capacity=7.00 cfs Outflow=3.70 cfs 0.618 af

Reach 23R: CB2-DMH3

Avg. Flow Depth=0.11' Max Vel=2.08 fps Inflow=0.10 cfs 0.006 af

12.0" Round Pipe n=0.012 L=10.0' S=0.0100 '/' Capacity=3.86 cfs Outflow=0.10 cfs 0.006 af

Reach 24AR:DMH2-DMH1

Avg. Flow Depth=0.46' Max Vel=9.23 fps Inflow=3.78 cfs 0.655 af

15.0" Round Pipe n=0.012 L=124.0' S=0.0350 '/' Capacity=13.09 cfs Outflow=3.78 cfs 0.655 af

Reach 24R: DMH3-DMH2

Avg. Flow Depth=0.44' Max Vel=9.63 fps Inflow=3.70 cfs 0.624 af

15.0" Round Pipe n=0.012 L=41.0' S=0.0400 '/' Capacity=14.00 cfs Outflow=3.70 cfs 0.624 af

Pond 11P: INLET-CB3

Peak Elev=62.42' Storage=538 cf Inflow=3.73 cfs 0.616 af

15.0" Round Culvert n=0.012 L=10.0' S=0.0050 '/' Outflow=3.69 cfs 0.615 af

Pond 15P: INLET-DMH2

Peak Elev=59.97' Storage=32 cf Inflow=0.33 cfs 0.031 af

12.0" Round Culvert n=0.012 L=28.0' S=0.0200 '/' Outflow=0.33 cfs 0.031 af

Type III 24-hr 10-Year Storm Event Rainfall=4.70"

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Total Runoff Area = 12.200 ac Runoff Volume = 2.070 af Average Runoff Depth = 2.04" 79.45% Pervious = 9.693 ac 20.55% Impervious = 2.507 ac

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Summary for Subcatchment 1S: SA-1 PREDEVELOPMENT

Runoff = 7.57 cfs @ 12.58 hrs, Volume=

1.026 af, Depth> 2.02"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Storm Event Rainfall=4.70"

Area	(ac) (ON Des	cription						
1.	1.050 87 1/4 acre lots, 38% imp, HSG D								
1.	.050	77 Woo	ods, Good,	HSG D					
1.	.250	55 Woo	ods, Good,	HSG B					
0.	.760	75 1/4	acre lots, 3	38% imp, H	ISG B				
				38% imp, H	SG C				
0.	490	70 Woo	ods, Good,	HSG C					
6.	6.100 75 Weighted Average								
4.	4.842 79.38% Pervious Area								
1.	258	20.6	2% Imper	vious Area					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
26.9	150	0.0100	0.09		Sheet Flow, LAWN				
<u> </u>					Grass: Dense n= 0.240 P2= 3.00"				
14.5	850	0.0380	0.97		Shallow Concentrated Flow, LIGHT WOODS Woodland Kv= 5.0 fps				

41.4 1,000 Total

Summary for Subcatchment 11S: SA11

Runoff = 3.73 cfs @ 12.83 hrs, Volume=

0.616 af, Depth> 1.78"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Storm Event Rainfall=4.70"

	Area (ac)	CN	Description
	0.670	70	Woods, Good, HSG C
	0.150	77	Woods, Good, HSG D
	1.300	87	1/4 acre lots, 38% imp, HSG D
*	0.130	98	Proposed Impervious Area
	0.190	80	>75% Grass cover, Good, HSG D
	0.070	74	>75% Grass cover, Good, HSG C
	0.180	75	1/4 acre lots, 38% imp, HSG B
	1.470	55	Woods, Good, HSG B
	4.160	72	Weighted Average
	3.468		83.36% Pervious Area
	0.692		16.64% Impervious Area

Type III 24-hr 10-Year Storm Event Rainfall=4.70"

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	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
,	43.5	150	0.0030	0.06		Sheet Flow,
	15.7	666	0.0200	0.71		Grass: Dense n= 0.240 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps
26.	59.2	816	Total			

Summary for Subcatchment 15S: SA15

Runoff = 0.33 cfs @ 12.26 hrs, Volume=

0.031 af, Depth> 2.36"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Storm Event Rainfall=4.70"

_	Area (ac)	CN	Description
*	0.010	98	Proposed Impervious Area
	0.100	80	>75% Grass cover, Good, HSG D
	0.050	74	>75% Grass cover, Good, HSG C
	0.160	79	Weighted Average
	0.150		93.75% Pervious Area
	0.010		6.25% Impervious Area

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	16.3	150	0.0350	0.15		Sheet Flow,
						Grass: Dense n= 0.240 P2= 3.00"
	2.1	107	0.0150	0.86		Shallow Concentrated Flow,
7				100000000000000000000000000000000000000		Short Grass Pasture Kv= 7.0 fps
-	18.4	257	Total			The state of the s

Summary for Subcatchment 22S: SA22

Runoff = 0.05 cfs @ 12.00 hrs, Volume=

0.003 af, Depth> 4.15"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Storm Event Rainfall=4.70"

_	Area	(ac) C	N Des	cription								
*	0.	010	98 Pro	Proposed Impervious Area								
5	0.	010	100	.00% Impe	rvious Area	а						
400000	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description						
	0.4	30	0.0300	1.23	388	Sheet Flow, Smooth surfaces	n= 0.011	P2= 3.00"				

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Summary for Subcatchment 23S: SA23

Runoff

0.10 cfs @ 12.01 hrs, Volume=

0.006 af, Depth> 3.59"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Storm Event Rainfall=4.70"

7-2-2	Area	(ac)	CN	Des	cription								
*	0	.015	98	Prop	Proposed Impervious Area								
	0.	.005	74	>75	% Grass co	over, Good	, HSG C						
12	0.	.020	92	Weig	ghted Aver	rage							
	0.005 25.00% Pervious Area												
	0.015			75.0	0% Imper	vious Area							
	Тс	Lengtl	h S	Slope	Velocity	Capacity	Description						
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•						
	0.4	30	0.0	0300	1.23		Sheet Flow, Smooth surfaces	n= 0.011	P2= 3.00"				

Summary for Subcatchment 24S: SA24

2.44 cfs @ 12.26 hrs, Volume= 0.239 af, Depth> 2.89"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Storm Event Rainfall=4.70"

	Area	(ac) (CN Des	cription	-						
0.200 87 1/4 acre lots, 38% imp, HSG D 0.620 83 1/4 acre lots, 38% imp, HSG C											
									*	* 0.090 98 Proposed Impervious Area	
	0.060 80 >75% Grass cover, Good, HSG D										
-	0.020 74 >75% Grass cover, Good, HSG C										
	0.990 85 Weighted Average										
		588	59.4	3% Pervio	us Area						
	0.	402	40.5	7% Imper	vious Area						
	W-00-	191 199	(10 <u>00</u> 10)	NUMBER OF BRIDE							
	Tc	Length	Slope	Velocity	Capacity	Description					
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	16.7	150	0.0330	0.15		Sheet Flow,					
	50 TS					Grass: Dense n= 0.240 P2= 3.00"					
	1.3	94	0.0300	1.21		Shallow Concentrated Flow,	Wi				
						Short Grass Pasture Kv= 7.0 fps					
	1.3	157	0.0100	2.03		Shallow Concentrated Flow,					
						Paved Kv= 20.3 fps					
	19.3	401	Total								

Type III 24-hr 10-Year Storm Event Rainfall=4.70"

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Summary for Subcatchment 25S: SA25

Runoff = 1.17 cfs @ 12.51 hrs, Volume=

0.149 af, Depth> 2.35"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Storm Event Rainfall=4.70"

	Area	(ac)	CN D	escription									
*	0.	080	98 Pi	roposed Impervious Area									
	0.	010		75% Grass cover, Good, HSG C									
	0.	130		Voods, Good, HSG C									
	0.	500	77 W	Woods, Good, HSG D									
*	0.	040	98 Ex	kisting Imper	vious Area								
	0.760 79 Weighted Average												
	0.640 84.21% Pervious Area												
	0.	120	15	5.79% Imper	vious Area								
	Tc	Length			Capacity	Description							
	(min)	(feet)	(ft/f	t) (ft/sec)	(cfs)								
	30.7	150	0.020	0.08		Sheet Flow,							
						Woods: Light underbrush n= 0.400 P2= 3.00"							
	5.6	265	0.025	0 0.79		Shallow Concentrated Flow,							
-						Woodland Kv= 5.0 fps							
	36.3	415	Total										

Summary for Reach 8R: Summary-Analysis Point #1

Inflow Area = 6.100 ac, 20.48% Impervious, Inflow Depth > 2.05" for 10-Year Storm Event event

Inflow = 5.31 cfs @ 12.63 hrs, Volume= 1.042 af

Outflow = 5.31 cfs @ 12.63 hrs, Volume= 1.042 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Summary for Reach 10AR: DMH1-SMH

Inflow Area = 5.110 ac, 16.58% Impervious, Inflow Depth > 1.89" for 10-Year Storm Event event

Inflow = 4.52 cfs @ 12.81 hrs, Volume= 0.803 af

Outflow = 4.52 cfs @ 12.82 hrs, Volume= 0.803 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 13.11 fps, Min. Travel Time= 0.0 min

Avg. Velocity = 5.55 fps, Avg. Travel Time= 0.1 min

Peak Storage= 6 cf @ 12.81 hrs

Average Depth at Peak Storage= 0.37'

Bank-Full Depth= 1.50' Flow Area= 1.8 sf, Capacity= 33.07 cfs

Type III 24-hr 10-Year Storm Event Rainfall=4.70"

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18.0" Round Pipe n= 0.012 Length= 18.0' Slope= 0.0844 '/' Inlet Invert= 54.55', Outlet Invert= 53.03'



Summary for Reach 10R: CB1-DMH1

Inflow Area =

0.760 ac, 15.79% Impervious, Inflow Depth > 2.35" for 10-Year Storm Event event

Inflow =

1.17 cfs @ 12.51 hrs. Volume= 0.149 af

Outflow =

1.17 cfs @ 12.51 hrs, Volume=

0.149 af, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity = 4.23 fps, Min. Travel Time = 0.1 min Avg. Velocity = 1.97 fps, Avg. Travel Time = 0.1 min

Peak Storage= 4 cf @ 12.51 hrs Average Depth at Peak Storage= 0.35' Bank-Full Depth= 1.25' Flow Area= 1.2 sf, Capacity= 7.00 cfs

15.0" Round Pipe n= 0.012 Length= 15.0' Slope= 0.0100 '/' Inlet Invert= 54.80', Outlet Invert= 54.65'



Summary for Reach 22R: CB3-DMH3

Inflow Area =

4.170 ac, 16.84% Impervious, Inflow Depth > 1.78" for 10-Year Storm Event event

Inflow =

3.70 cfs @ 12.90 hrs, Volume=

0.618 af

Outflow =

3.70 cfs @ 12.90 hrs, Volume=

0.618 af, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 5.78 fps, Min. Travel Time= 0.0 min Avg. Velocity = 2.34 fps, Avg. Travel Time= 0.1 min

Peak Storage= 6 cf @ 12.90 hrs

Average Depth at Peak Storage= 0.65'

Bank-Full Depth= 1.25' Flow Area= 1.2 sf, Capacity= 7.00 cfs

Type III 24-hr 10-Year Storm Event Rainfall=4.70"

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15.0" Round Pipe n= 0.012 Length= 10.0' Slope= 0.0100 '/' Inlet Invert= 60.93', Outlet Invert= 60.83'



Summary for Reach 23R: CB2-DMH3

Inflow Area =

0.020 ac, 75.00% Impervious, Inflow Depth > 3.59" for 10-Year Storm Event event

Inflow =

0.10 cfs @ 12.01 hrs, Volume=

0.006 af

Outflow =

0.10 cfs @ 12.01 hrs, Volume=

0.006 af, Atten= 1%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.08 fps, Min. Travel Time= 0.1 min Avg. Velocity = 0.73 fps, Avg. Travel Time= 0.2 min

Peak Storage= 0 cf @ 12.01 hrs Average Depth at Peak Storage= 0.11' Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 3.86 cfs

12.0" Round Pipe n= 0.012 Length= 10.0' Slope= 0.0100 '/' Inlet Invert= 60.93', Outlet Invert= 60.83'



Summary for Reach 24AR: DMH2-DMH1

Inflow Area =

4.350 ac, 16.72% Impervious, Inflow Depth > 1.81" for 10-Year Storm Event event

Inflow =

3.78 cfs @ 12.89 hrs, Volume=

0.655 af

Outflow =

3.78 cfs @ 12.90 hrs, Volume=

0.655 af, Atten= 0%, Lag= 0.4 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity = 9.23 fps, Min. Travel Time = 0.2 min Avg. Velocity = 3.81 fps, Avg. Travel Time = 0.5 min

Type III 24-hr 10-Year Storm Event Rainfall=4.70"

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Peak Storage= 51 cf @ 12.90 hrs Average Depth at Peak Storage= 0.46' Bank-Full Depth= 1.25' Flow Area= 1.2 sf, Capacity= 13.09 cfs

15.0" Round Pipe n= 0.012 Length= 124.0' Slope= 0.0350 '/' Inlet Invert= 58.99', Outlet Invert= 54.65'



Summary for Reach 24R: DMH3-DMH2

Inflow Area = 4.190 ac, 17.12% Impervious, Inflow Depth > 1.79" for 10-Year Storm Event event

Inflow = 3.70 cfs @ 12.90 hrs, Volume= 0.624 af

Outflow = 3.70 cfs @ 12.90 hrs, Volume= 0.624 af, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 9.63 fps, Min. Travel Time= 0.1 min Avg. Velocity = 3.93 fps, Avg. Travel Time= 0.2 min

Peak Storage= 16 cf @ 12.90 hrs Average Depth at Peak Storage= 0.44' Bank-Full Depth= 1.25' Flow Area= 1.2 sf, Capacity= 14.00 cfs

15.0" Round Pipe n= 0.012 Length= 41.0' Slope= 0.0400 '/' Inlet Invert= 60.73', Outlet Invert= 59.09'



Summary for Pond 11P: INLET-CB3

Inflow Area = 4.160 ac, 16.64% Impervious, Inflow Depth > 1.78" for 10-Year Storm Event event

Inflow = 3.73 cfs @ 12.83 hrs, Volume= 0.616 af

Outflow = 3.69 cfs @ 12.90 hrs, Volume= 0.615 af, Atten= 1%, Lag= 3.8 min

Primary = 3.69 cfs @ 12.90 hrs, Volume= 0.615 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Type III 24-hr 10-Year Storm Event Rainfall=4.70"

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Peak Elev= 62.42' @ 12.90 hrs Surf.Area= 786 sf Storage= 538 cf

Plug-Flow detention time= 2.3 min calculated for 0.615 af (100% of inflow)

Center-of-Mass det. time= 1.7 min (848.6 - 847.0)

Volume	Inv	ert Avail.	Storage	Storage D	escription		
#1	61.0	08'	1,033 cf	Custom S	Stage Data (Pri	smatic) Listed below	
Elevatio		Surf.Area (sq-ft)		Store c-feet)	Cum.Store (cubic-feet)		
61.08		10		0	0		
62.00		365		173	173		
63.00	0	1,356		861	1,033		
Device	Routing	Inve	ert Outle	et Devices			
#1	Primary	61.0	L= 1		projecting, no h	neadwall, Ke= 0.900 .03' S= 0.0050 '/' Cc= 0.900	

Primary OutFlow Max=3.69 cfs @ 12.90 hrs HW=62.42' (Free Discharge)
1=Culvert (Barrel Controls 3.69 cfs @ 3.48 fps)

Summary for Pond 15P: INLET-DMH2

n= 0.012, Flow Area= 1.23 sf

Inflow Area = 0.160 ac, 6.25% Impervious, Inflow Depth > 2.36" for 10-Year Storm Event event Inflow = 0.33 cfs @ 12.26 hrs, Volume= 0.031 af Outflow = 0.33 cfs @ 12.27 hrs, Volume= 0.031 af, Atten= 0%, Lag= 0.5 min 0.031 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 59.97' @ 12.27 hrs Surf.Area= 96 sf Storage= 32 cf

Plug-Flow detention time= 8.5 min calculated for 0.031 af (98% of inflow) Center-of-Mass det. time= 3.9 min (805.1 - 801.2)

Volume	Inv	ert Avai	I.Storage	Storage	Description		
#1	59.3	39'	1,925 cf	Custom	Stage Data (Pr	ismatic) Listed be	low
Elevation (feet		Surf.Area (sq-ft)		:.Store c-feet)	Cum.Store (cubic-feet)		
59.3	9	10		0	0		
60.00	0	100		34	34		
61.00	0	187		144	177		
62.00	0	784		486	663		
63.00	0	1,740		1,262	1,925		
Device	Routing	lnv	vert Outle	et Device	s		

#1 Primary 59.65' **12.0" Round Culvert**L= 28.0' CPP, projecting, no headwall, Ke= 0.900
Inlet / Outlet Invert= 59.65' / 59.09' S= 0.0200 '/' Cc= 0.900

Type III 24-hr 10-Year Storm Event Rainfall=4.70"

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n= 0.012, Flow Area= 0.79 sf

Primary OutFlow Max=0.33 cfs @ 12.27 hrs HW=59.97' (Free Discharge)
1=Culvert (Inlet Controls 0.33 cfs @ 1.52 fps)

Pond 15P: INLET-DMH2

Type III 24-hr 25-Year Storm Event Rainfall=5.50"

Peak Elev=60.01' Storage=36 cf Inflow=0.42 cfs 0.040 af

12.0" Round Culvert n=0.012 L=28.0' S=0.0200 '/' Outflow=0.42 cfs 0.040 af

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

	g by Stor-Ind+Tra	ans method -	Pond routing b	y Stor-Ind me	ethod	
Subcatchment 1S:SA-1	Flow	Runoff Area=6 Length=1,000'	.100 ac 20.62% Tc=41.4 min			
Subcatchment 11S:SA11	Flo	Runoff Area=4. ow Length=816'	.160 ac 16.64% Tc=59.2 min			
Subcatchment 15S:SA15	Fic	Runoff Area=(ow Length=257'	0:160 ac			
Subcatchment 22S:SA22	l Flow Length=30'	Runoff Area=0.0 Slope=0.0300 '/				
Subcatchment 23S:SA23	Flow Length=30'		020 ac 75.00% " Tc=0.4 min			
Subcatchment 24S:SA24	Flo	Runoff Area=0. w Length=401'	990 ac 40.57% Tc=19.3 min (
Subcatchment 25S:SA25	Anna China da Cara da		760 ac 15.79%			
	Flo	w Length=415'	Tc=36.3 min (CN=79 Runo	ff=1.49 cfs ().190 af
Reach 8R: Summary-Analy	sisPoint#1				w=6.91 cfs ' w=6.91 cfs '	
				od.iio.	W-0.51 CIS	
Reach 10AR:DMH1-SMH 18.0" Round Pip	Avg. e n=0.012 L=18	Flow Depth=0.4 .0' S=0.0844 '/'		4.15 fps Inflo	w=5.91 cfs '	
18.0" Round Pip Reach 10R: CB1-DMH1	e n=0.012 L=18	.0' S=0.0844 '/' g. Flow Depth=0	' Capacity=33.0	4.15 fps Inflov 07 cfs Outflov 4.53 fps Inflov	w=5.91 cfs / w=5.91 cfs / w=1.49 cfs (1.054 af 0.190 af
18.0" Round Pip Reach 10R: CB1-DMH1 15.0" Round Pi Reach 22R: CB3-DMH3	e n=0.012 L=18 Avg pe n=0.012 L=1	.0' S=0.0844 '/' g. Flow Depth=0 5.0' S=0.0100 g. Flow Depth=0	' Capacity=33.0 .39' Max Vel=4 '/' Capacity=7.0 .77' Max Vel=6	4.15 fps Inflov 07 cfs Outflov 4.53 fps Inflov 00 cfs Outflov 5.17 fps Inflov	w=5.91 cfs of w=5.91 cfs of w=1.49 cfs (w=1.49 cfs (1.054 af 0.190 af 0.190 af 0.817 af
18.0" Round Pip Reach 10R: CB1-DMH1 15.0" Round Pi Reach 22R: CB3-DMH3 15.0" Round Pi Reach 23R: CB2-DMH3	Avg pe n=0.012 L=1 Avg pe n=0.012 L=1	g. Flow Depth=0 5.0' S=0.0100 g. Flow Depth=0 0.0' S=0.0100 g. Flow Depth=0	' Capacity=33.0 0.39' Max Vel=4 1/' Capacity=7.0 0.77' Max Vel=6 1/' Capacity=7.0 1.12' Max Vel=2	4.15 fps Inflov 07 cfs Outflov 4.53 fps Inflov 00 cfs Outflov 3.17 fps Inflov 00 cfs Outflov	w=5.91 cfs 2 w=5.91 cfs 2 w=1.49 cfs 0 w=1.49 cfs 0 w=4.89 cfs 0 w=4.90 cfs 0	1.054 af 0.190 af 0.190 af 0.817 af 0.817 af
18.0" Round Pip Reach 10R: CB1-DMH1 15.0" Round Pi Reach 22R: CB3-DMH3 15.0" Round Pi Reach 23R: CB2-DMH3	Avg pe n=0.012 L=18 Avg pe n=0.012 L=1 Avg pe n=0.012 L=1 Avg pe n=0.012 L=1	g. Flow Depth=0 5.0' S=0.0100 g. Flow Depth=0 0.0' S=0.0100 g. Flow Depth=0 g. Flow Depth=0 0.0' S=0.0100 '	' Capacity=33.0 0.39' Max Vel=4 1/' Capacity=7.0 0.77' Max Vel=6 1/' Capacity=7.0 1.12' Max Vel=2 1/' Capacity=3.8	4.15 fps Inflov 07 cfs Outflov 4.53 fps Inflov 00 cfs Outflov 3.17 fps Inflov 00 cfs Outflov 2.19 fps Inflov 86 cfs Outflov	w=5.91 cfs w=5.91 cfs w=1.49 cfs w=1.49 cfs w=4.89 cfs w=4.90 cfs w=0.12 cfs w=0.11 cfs v=5.00 cfs	1.054 af 0.190 af 0.190 af 0.817 af 0.817 af 0.007 af 0.007 af
18.0" Round Pip Reach 10R: CB1-DMH1 15.0" Round Pi Reach 22R: CB3-DMH3 15.0" Round Pi Reach 23R: CB2-DMH3 12.0" Round Pi Reach 24AR: DMH2-DMH1 15.0" Round Pipe Reach 24R: DMH3-DMH2	Avg pe n=0.012 L=18 Avg pe n=0.012 L=1 Avg pe n=0.012 L=1 Avg pe n=0.012 L=1 Avg n=0.012 L=124	g. Flow Depth=0 5.0' S=0.0100 g. Flow Depth=0 0.0' S=0.0100 g. Flow Depth=0 0.0' S=0.0100' g. Flow Depth=0 0.0' S=0.0350'/'	' Capacity=33.0 .39' Max Vel=4 '/' Capacity=7.0 .77' Max Vel=6 '/' Capacity=7.0 .12' Max Vel=2 '/' Capacity=3.8 .54' Max Vel=9 Capacity=13.0	4.15 fps Inflov 07 cfs Outflov 4.53 fps Inflov 00 cfs Outflov 3.17 fps Inflov 00 cfs Outflov 2.19 fps Inflov 36 cfs Outflov 0.95 fps Inflov 0.90 cfs Outflov	w=5.91 cfs of w=5.91 cfs of w=1.49 cfs of w=4.89 cfs of w=4.90 cfs of w=0.11 cfs of w=5.00 cfs of w=5.00 cfs of w=4.90 cfs of w=5.00 cfs of w=4.90 cfs of w=4.90 cfs of w=5.00 cfs of w=4.90 cfs of w=5.00 cfs of w=4.90 cfs of w=4.90 cfs of w=5.00 cfs of w=4.90 cfs of w=4.90 cfs of w=5.00 cfs of w=4.90 cfs of w=5.00 cfs of w=4.90 cfs of w=4.90 cfs of w=5.00 cfs of w=4.90 cfs of w=5.00 cfs of w=4.90 cfs of w=5.00 cfs of w=4.90 cfs of w=4.90 cfs of w=5.00 cfs of w=4.90 cfs of w=	1.054 af 0.190 af 0.190 af 0.817 af 0.817 af 0.007 af 0.007 af 0.864 af 0.864 af

Type III 24-hr 25-Year Storm Event Rainfall=5.50"

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Total Runoff Area = 12.200 ac Runoff Volume = 2.688 af Average Runoff Depth = 2.64" 79.45% Pervious = 9.693 ac 20.55% Impervious = 2.507 ac

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Summary for Subcatchment 1S: SA-1 PREDEVELOPMENT

Runoff

=

9.86 cfs @ 12.58 hrs, Volume=

1.335 af, Depth> 2.63"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Storm Event Rainfall=5.50"

	Area	(ac) (CN Des	cription			
	1	.050	87 1/4	acre lots, 3	38% imp, H	SG D	
	1	.050	77 Woo	ods, Good,	HSG D		
	1.	250	55 Woo	ods, Good,	HSG B		
	0.	760	75 1/4	acre lots, 3	38% imp, H	SG B	
	1.	.500	83 1/4	acre lots, 3	38% imp, H	SG C	
_	0.	490	70 Woo	ods, Good,	HSG C		
	6.	100	75 Wei	ghted Avei	rage		
	4.	842	79.3	88% Pervio	us Area		
	1.	258	20.6	32% Imper	vious Area		
	Tc	Length	Soldie Abstract	Velocity	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	26.9	150	0.0100	0.09		Sheet Flow, LAWN	
						Grass: Dense n= 0.240 P2= 3.00"	the second second
	14.5	850	0.0380	0.97		Shallow Concentrated Flow, LIGHT WOODS	

41.4 1,000 Total

Summary for Subcatchment 11S: SA11

Woodland Kv= 5.0 fps

Runoff

=

4.96 cfs @ 12.82 hrs, Volume=

0.815 af, Depth> 2.35"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Storm Event Rainfall=5.50"

Vienestin	Area (ac)	CN	Description
	0.670	70	Woods, Good, HSG C
	0.150	77	Woods, Good, HSG D
	1.300	87	1/4 acre lots, 38% imp, HSG D
*	0.130	98	Proposed Impervious Area
	0.190	80	>75% Grass cover, Good, HSG D
	0.070	74	>75% Grass cover, Good, HSG C
	0.180	75	1/4 acre lots, 38% imp, HSG B
10-	1.470	55	Woods, Good, HSG B
	4.160	72	Weighted Average
	3.468		83.36% Pervious Area
	0.692		16.64% Impervious Area

Type III 24-hr 25-Year Storm Event Rainfall=5.50"

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 Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
43.5	150	0.0030	0.06	5- W	Sheet Flow,
					Grass: Dense n= 0.240 P2= 3.00"
15.7	666	0.0200	0.71		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
59.2	816	Total			

Summary for Subcatchment 15S: SA15

Runoff = 0.42 cfs @ 12.25 hrs, Volume=

0.040 af, Depth> 3.01"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Storm Event Rainfall=5.50"

	Area (ac)	CN	Description
*	0.010	98	Proposed Impervious Area
	0.100	80	>75% Grass cover, Good, HSG D
-	0.050	74	>75% Grass cover, Good, HSG C
	0.160	79	Weighted Average
	0.150		93.75% Pervious Area
	0.010		6.25% Impervious Area

122	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	16.3	150	0.0350	0.15		Sheet Flow,
						Grass: Dense n= 0.240 P2= 3.00"
	2.1	107	0.0150	0.86		Shallow Concentrated Flow,
				Ottobri din in di		Short Grass Pasture Kv= 7.0 fps
	18.4	257	Total			

Summary for Subcatchment 22S: SA22

Runoff = 0.06 cfs @ 12.00 hrs, Volume=

0.004 af, Depth> 4.87"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Storm Event Rainfall=5.50"

	Area	(ac) (CN	Desc	cription					
*	0.	010	98	Prop	osed Impe	ervious Are	a			
	0.	010		100.	00% Impe	rvious Area	a			
	Tc (min)	Length (feet)		ope t/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	0.4	30	0.03	300	1.23		Sheet Flow,	n= 0.011	P2= 3 00"	

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Summary for Subcatchment 23S: SA23

Runoff

0.12 cfs @ 12.01 hrs, Volume=

0.007 af, Depth> 4.33"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Storm Event Rainfall=5.50"

,	Area	(ac)	CN	Des	cription					
*		.015	98 74		A STATE OF THE STA	ervious Are				300 300 000 000 000 000 000 000 000 000
	0.	020 § 005	0.005 74 0.020 92 0.005 0.015		>75% Grass cover, Good, HSG C Weighted Average 25.00% Pervious Area 75.00% Impervious Area					
	Tc (min)	Length (feet		lope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
	0.4	30	0.0	300	1.23		Sheet Flow, Smooth surfaces	n= 0.011	P2= 3.00"	

Summary for Subcatchment 24S: SA24

Runoff

3.01 cfs @ 12.26 hrs, Volume=

0.296 af, Depth> 3.59"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Storm Event Rainfall=5.50"

_	Area	(ac) C	N Des	cription			
	0.	200	87 1/4	acre lots, 3	88% imp, H	SG D	
	0.	620	B3 1/4 a	acre lots, 3	88% imp, H	SG C	
*	0.	.090	98 Prop	osed Imp	ervious Are	ea	
	0.	060	30 >75°	% Grass c	over, Good	I, HSG D	
	0.	020	74 >75°	% Grass c	over, Good	, HSG C	
				ghted Avei			3
		588		3% Pervio			
	0.	402	40.5	7% Imper	ious Area		
	_		٥.	474 4 42			
	Tc	Length	Slope	Velocity	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	100 100 100 100 100 100 100 100 100 100	
	16.7	150	0.0330	0.15		Sheet Flow,	
		4.0	1111			Grass: Dense n= 0.240 P2= 3.00"	
	1.3	94	0.0300	1.21		Shallow Concentrated Flow,	
	1.0					•	
						Short Grass Pasture Kv= 7.0 fps	
	1.3	157	0.0100	2.03		Short Grass Pasture Kv= 7.0 fps Shallow Concentrated Flow,	
						Short Grass Pasture Kv= 7.0 fps	

Type III 24-hr 25-Year Storm Event Rainfall=5.50"

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Summary for Subcatchment 25S: SA25

Runoff = 1.49 cfs @ 12.50 hrs, Volume=

0.190 af, Depth> 3.00"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Storm Event Rainfall=5.50"

	Area	(ac) (CN Des	scription			
*	0.	.080	98 Pro	posed Imp	ervious Are	ea	
	0.	010			over, Good		
	0.	130	70 Wo	ods, Good,	HSG C		
	0.	500	77 Wo	ods, Good,	HSG D		
*	0.	040	98 Exi	sting Imper	vious Area		
	0.	760		ighted Ave			
		640	84.2	21% Pervio	us Area		
	0.	120	15.7	79% Imper	vious Area		
	_						
	Tc	Length			Capacity	Description	
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	30.7	150	0.0200	0.08		Sheet Flow,	
						Woods: Light underbrush n= 0.400 P2= 3.00"	
Name of the last o	5.6	265	0.0250	0.79		Shallow Concentrated Flow,	
						Woodland Kv= 5.0 fps	
	36.3	415	Total				

Summary for Reach 8R: Summary-Analysis Point #1

Inflow Area = 6.100 ac, 20.48% Impervious, Inflow Depth > 2.66" for 25-Year Storm Event event

Inflow = 6.91 cfs @ 12.62 hrs, Volume= 1.350 af

Outflow = 6.91 cfs @ 12.62 hrs, Volume= 1.350 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Summary for Reach 10AR: DMH1-SMH

Inflow Area = 5.110 ac, 16.58% Impervious, Inflow Depth > 2.47" for 25-Year Storm Event event

Inflow = 5.91 cfs @ 12.85 hrs, Volume= 1.054 af

Outflow = 5.91 cfs @ 12.85 hrs, Volume= 1.054 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 14.15 fps, Min. Travel Time= 0.0 min Avg. Velocity = 6.07 fps, Avg. Travel Time= 0.0 min

Peak Storage= 8 cf @ 12.85 hrs Average Depth at Peak Storage= 0.43' Bank-Full Depth= 1.50' Flow Area= 1.8 sf, Capacity= 33.07 cfs

Type III 24-hr 25-Year Storm Event Rainfall=5.50"

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18.0" Round Pipe n= 0.012 Length= 18.0' Slope= 0.0844 '/' Inlet Invert= 54.55', Outlet Invert= 53.03'



Summary for Reach 10R: CB1-DMH1

Inflow Area =

0.760 ac, 15.79% Impervious, Inflow Depth > 3.00" for 25-Year Storm Event event

Inflow =

1.49 cfs @ 12.50 hrs, Volume= 0.190 af

Outflow =

1.49 cfs @ 12.50 hrs, Volume=

0.190 af, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.53 fps, Min. Travel Time= 0.1 min Avg. Velocity = 2.07 fps, Avg. Travel Time= 0.1 min

Peak Storage= 5 cf @ 12.50 hrs Average Depth at Peak Storage= 0.39' Bank-Full Depth= 1.25' Flow Area= 1.2 sf, Capacity= 7.00 cfs

15.0" Round Pipe n= 0.012 Length= 15.0' Slope= 0.0100 '/' Inlet Invert= 54.80', Outlet Invert= 54.65'



Summary for Reach 22R: CB3-DMH3

Inflow Area =

4.170 ac, 16.84% Impervious, Inflow Depth > 2.35" for 25-Year Storm Event event

Inflow =

Outflow

4.89 cfs @ 12.90 hrs, Volume= 4.90 cfs @ 12.90 hrs, Volume= 0.817 af 0.817 af, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 6.17 fps, Min. Travel Time= 0.0 min Avg. Velocity = 2.58 fps, Avg. Travel Time= 0.1 min

Peak Storage= 8 cf @ 12.90 hrs

Average Depth at Peak Storage= 0.77'

Bank-Full Depth= 1.25' Flow Area= 1.2 sf, Capacity= 7.00 cfs

Type III 24-hr 25-Year Storm Event Rainfall=5.50"

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15.0" Round Pipe n= 0.012 Length= 10.0' Slope= 0.0100 '/' Inlet Invert= 60.93', Outlet Invert= 60.83'



Summary for Reach 23R: CB2-DMH3

Inflow Area =

0.020 ac, 75.00% Impervious, Inflow Depth > 4.33" for 25-Year Storm Event event

Inflow =

0.12 cfs @ 12.01 hrs, Volume=

0.007 af

Outflow =

0.11 cfs @ 12.01 hrs, Volume=

0.007 af, Atten= 1%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.19 fps, Min. Travel Time= 0.1 min Avg. Velocity = 0.77 fps, Avg. Travel Time= 0.2 min

Peak Storage= 1 cf @ 12.01 hrs Average Depth at Peak Storage= 0.12' Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 3.86 cfs

12.0" Round Pipe n= 0.012 Length= 10.0' Slope= 0.0100 '/' Inlet Invert= 60.93', Outlet Invert= 60.83'



Summary for Reach 24AR: DMH2-DMH1

Inflow Area = 4.350 ac, 16.72% Impervious, Inflow Depth > 2.38" for 25-Year Storm Event event

Inflow = 5.00 cfs @ 12.89 hrs, Volume= 0.864 af

Outflow = 5.00 cfs @ 12.90 hrs, Volume= 0.864 af, Atten= 0%, Lag= 0.5 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 9.95 fps, Min. Travel Time= 0.2 min Avg. Velocity = 4.18 fps, Avg. Travel Time= 0.5 min CHABAD

Type III 24-hr 25-Year Storm Event Rainfall=5.50"

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Peak Storage= 62 cf @ 12.90 hrs Average Depth at Peak Storage= 0.54' Bank-Full Depth= 1.25' Flow Area= 1.2 sf, Capacity= 13.09 cfs

15.0" Round Pipe n= 0.012 Length= 124.0' Slope= 0.0350 '/' Inlet Invert= 58.99', Outlet Invert= 54.65'



Summary for Reach 24R: DMH3-DMH2

Inflow Area = 4.190 ac, 17.12% Impervious, Inflow Depth > 2.36" for 25-Year Storm Event event

Inflow = 4.90 cfs @ 12.90 hrs, Volume= 0.825 af

Outflow = 4.91 cfs @ 12.90 hrs, Volume= 0.824 af, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 10.40 fps, Min. Travel Time= 0.1 min Avg. Velocity = 4.31 fps, Avg. Travel Time= 0.2 min

Peak Storage= 19 cf @ 12.90 hrs Average Depth at Peak Storage= 0.51' Bank-Full Depth= 1.25' Flow Area= 1.2 sf, Capacity= 14.00 cfs

15.0" Round Pipe n= 0.012 Length= 41.0' Slope= 0.0400 '/' Inlet Invert= 60.73', Outlet Invert= 59.09'



Summary for Pond 11P: INLET-CB3

Inflow Area = 4.160 ac, 16.64% Impervious, Inflow Depth > 2.35" for 25-Year Storm Event event

Inflow = 4.96 cfs @ 12.82 hrs, Volume= 0.815 af

Outflow = 4.89 cfs @ 12.90 hrs, Volume= 0.813 af, Atten= 2%, Lag= 4.5 min

Primary = 4.89 cfs @ 12.90 hrs, Volume= 0.813 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

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Type III 24-hr 25-Year Storm Event Rainfall=5.50"

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Peak Elev= 62.80' @ 12.90 hrs Surf.Area= 1,162 sf Storage= 865 cf

Plug-Flow detention time= 2.4 min calculated for 0.813 af (100% of inflow)

Center-of-Mass det. time= 1.9 min (843.0 - 841.1)

Volume	Inv	ert Ava	il.Storage	Storage D	escription	
#1	61.0	08'	1,033 cf	Custom 9	Stage Data (P	rismatic) Listed below
Elevation (feet	205.0	Surf.Area (sq-ft)		Store c-feet)	Cum.Store (cubic-feet)	
61.08 62.00 63.00	0	10 365 1,356		0 173 861	0 173 1,033	
Device	Routing	<u>In</u>	vert Outle	et Devices		
#1	Primary	61	L= 1		projecting, no	headwall, Ke= 0.900 61.03' S= 0.0050'/' Cc= 0.900

Primary OutFlow Max=4.89 cfs @ 12.90 hrs HW=62.80' (Free Discharge) 1=Culvert (Inlet Controls 4.89 cfs @ 3.99 fps)

Summary for Pond 15P: INLET-DMH2

n= 0.012, Flow Area= 1.23 sf

Inflow Area = 0.160 ac, 6.25% Impervious, Inflow Depth > 3.01" for 25-Year Storm Event event Inflow = 0.42 cfs @ 12.25 hrs, Volume= 0.040 af

Outflow = 0.42 cfs @ 12.28 hrs, Volume= 0.040 af, Atten= 1%, Lag= 1.3 min

Primary = 0.42 cfs @ 12.28 hrs, Volume= 0.040 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 60.01' @ 12.28 hrs Surf.Area= 101 sf Storage= 36 cf

Plug-Flow detention time= 7.2 min calculated for 0.040 af (99% of inflow)

Center-of-Mass det. time= 3.5 min (799.2 - 795.6)

Volume	Inv	ert Avai	I.Storage	Storage	Description		
#1	59.3	39'	1,925 cf	Custom	Stage Data (Pr	rismatic) Listed below	
Elevatior (feet)	-	Surf.Area (sq-ft)		c.Store c-feet)	Cum.Store (cubic-feet)		
59.39	9	10		0	0		
60.00)	100		34	34		
61.00)	187		144	177		
62.00)	784		486	663		
63.00)	1,740		1,262	1,925		
Device I	Routing	Inv	vert Out	et Devices	3		

#1 Primary 59.65' **12.0" Round Culvert**L= 28.0' CPP, projecting, no headwall, Ke= 0.900
Inlet / Outlet Invert= 59.65' / 59.09' S= 0.0200 '/' Cc= 0.900

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Type III 24-hr 25-Year Storm Event Rainfall=5.50"

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n= 0.012, Flow Area= 0.79 sf

Primary OutFlow Max=0.41 cfs @ 12.28 hrs HW=60.01' (Free Discharge) 1=Culvert (Inlet Controls 0.41 cfs @ 1.62 fps)

APPENDIX D

OPERATION & MAINTENANCE PLAN

OPERATIONS & MAINTENANCE PLAN

For: Chabad Lubavitch Portland, Me

The applicant, Chabad Lubavitch of Maine LLC, will be responsible for all required maintenance until the roadway (Pomeroy Street) are offered to the City of Portland for consideration as a public street at which time the City will be responsible for all maintenance within the public Right of Way and Chabad Lubavitch of Maine LLC will remain responsible for the maintenance outside of the Public Right of way. The following is a summary of the required maintenance:

Roadways

- 1. On-site inspection of the roads on an annual schedule or after a significant period of rainfall.
 - a.) All low spots of pooling water shall be regraded to direct the water off the pavement.
 - b.) Areas of erosions shall be repaired immediately.
 - c.) Sweeping the roadway free of sand after the winter season should be completed annually.

Stormdrain Inlet & Outlet

- 1. On-site inspection of the rip-rap surrounding the stormdrain inlets and outlets on a monthly schedule or after a significant period of rainfall.
 - a.) Carefully inspect to determine if high flows have caused scour beneath the rip-rap or dislodged any of the stones. If repairs are needed, they should be accomplished immediately.

Vegetated Swale with Check Dams

- 1. On-site inspection of the vegetated ditches on a monthly schedule or after a significant period of rainfall.
 - a.) Ditches should be inspected to repair erosion problems, remove any accumulated debris and to check the condition and integrity of the check dams.

Storm Drain System Includes Catchbasins

- 1. Inspect catchbasin inlets, culvert entrances and field inlets on a monthly basis for debris or conditions which could inhibit flow entry. Remove debris.
- 2. Inspect all catchbasin structures on an annual basis.
 - a.) Check that rim elevations are properly set to optimize flow entry.
 - b.) Measure and record silt accumulation, if any.
- 3. Check pipelines on an annual basis to determine silt accumulation, if any.

- 4. Inspect swales, channels, and ditches on a semi-annual basis.
 - a.) Check for debris that may inhibit flow remove as warranted.
 - b.) Note and remove excessive vegetation mow monthly.
 - c.) Note any erosion or non-vegetated areas which could lead to erosion.
 - d.)

Housekeeping

- 1. Use attached "Inspection & Maintenance Log" and keep records in three-ring binder.
- 2. See attached Appendix B "Inspection and Maintenance" from MDEP Stormwater Regulations.
- 3. See attached "Appendix C" from MDEP Stormwater Regulations for performance standards.

CHABAD LUBAVITCH OF MAINE LLC STORMWATER MANAGEMENT INSPECTION & MAINTENANCE LOG

	INSPECTION & MAINTENANCE LOG										
FACILITY:		YEAR:									
LOCATION:		CONTRACTOR:									
FUNCTION:		DEP PROJ. MANAC	GER:								
DATE OF INSPECT	ΓΙΟN:	INSPECTOR:									
ITEM ID	DESCRIPTION	MAINTENANCE DATE OF									
	OF CONDITIONS	ACCOMPLISHED	MAINTENANCE								
ROADWAYS											
	_										
STORMDRAIN INLET & OUTLET											
VEGETATED SWALES											
STORMDRAIN SYSTEM											
CHECK DAMS											
CATCH BASINS & FIELD INLETS											

APPENDIX B. Inspection and maintenance

This appendix applies to all projects. A project that is only required to meet basic standards (stormwater PBR) must meet the standards in Section 1. All other projects must meet standards in Sections 1 through 5.

See Appendix D(5) for additional maintenance requirements related to infiltration of stormwater.

- 1. During construction. The following standards must be met during construction.
 - (a) Inspection and corrective action. Inspect disturbed and impervious areas, erosion control measures, materials storage areas that are exposed to precipitation, and locations where vehicles enter or exit the site. Inspect these areas at least once a week as well as before and after a storm event, and prior to completing permanent stabilization measures. A person with knowledge of erosion and stormwater control, including the standards and conditions in the permit, shall conduct the inspections.
 - (b) Maintenance. Maintain all measures in effective operating condition until areas are permanently stabilized. If best management practices (BMPs) need to be maintained or modified, additional BMPs are necessary, or other corrective action is needed, implementation must be completed within 7 calendar days and prior to any storm event (rainfall).
 - (c) Documentation. Keep a log (report) summarizing the inspections and any corrective action taken. The log must include the name(s) and qualifications of the person making the inspections, the date(s) of the inspections, and major observations about the operation and maintenance of erosion and sedimentation controls, materials storage areas, and vehicles access points to the parcel. Major observations must include BMPs that need maintenance, BMPs that failed to operate as designed or proved inadequate for a particular location, and location(s) where additional BMPs are needed. For each BMP requiring maintenance, BMP needing replacement, and location needing additional BMPs, note in the log the corrective action taken and when it was taken.

The log must be made accessible to department staff and a copy must be provided upon request. The permittee shall retain a copy of the log for a period of at least three years from the completion of permanent stabilization.

- 2. Post-construction. The following standards must be met after construction.
 - (a) Plan. Carry out an approved inspection and maintenance plan that is consistent with the minimum requirements of this section. The plan must address inspection and maintenance of the project's permanent erosion control measures and stormwater management system. This plan may be combined with the plan listed in Section 2(a) of this appendix. See Section 8(C)(2) for submission requirements.
 - (b) Inspection and corrective action. All measures must be maintained in effective operating condition. A person with knowledge of erosion and stormwater control, including the standards and conditions in the permit, shall conduct the inspections. The following areas, facilities, and measures must be inspected and identified deficiencies must be corrected. Areas, facilities, and measures other than those listed below may also require inspection on a specific site. Inspection

or maintenance tasks other than those discussed below must be included in the maintenance plan developed for a specific site.

NOTE: Expanded and more-detailed descriptions for specific maintenance tasks may be found in the Maine DEP's "Stormwater Management for Maine: Best Management Practices."

- (i) Inspect vegetated areas, particularly slopes and embankments, early in the growing season or after heavy rains to identify active or potential erosion problems. Replant bare areas or areas with sparse growth. Where rill erosion is evident, armor the area with an appropriate lining or divert the erosive flows to on-site areas able to withstand the concentrated flows. See permanent stabilization standards in Appendix A(5).
- (ii) Inspect ditches, swales and other open stormwater channels in the spring, in late fall, and after heavy rains to remove any obstructions to flow, remove accumulated sediments and debris, to control vegetated growth that could obstruct flow, and to repair any erosion of the ditch lining. Vegetated ditches must be mowed at least annually or otherwise maintained to control the growth of woody vegetation and maintain flow capacity. Any woody vegetation growing through riprap linings must also be removed. Repair any slumping side slopes as soon as practicable. If the ditch has a riprap lining, replace riprap on areas where any underlying filter fabric or underdrain gravel is showing through the stone or where stones have dislodged. The channel must receive adequate routine maintenance to maintain capacity and prevent or correct any erosion of the channel's bottom or sideslopes.
- (iii) Inspect culverts in the spring, in late fall, and after heavy rains to remove any obstructions to flow; remove accumulated sediments and debris at the inlet, at the outlet, and within the conduit; and to repair any erosion damage at the culvert's inlet and outlet.
- (iv) Inspect and, if required, clean-out catch basins at least once a year, preferably in early spring. Clean-out must include the removal and legal disposal of any accumulated sediments and debris at the bottom of the basin, at inlet any grates, at any inflow channels to the basin, and at any pipes between basins. If the basin outlet is designed to trap floatable materials, then remove the floating debris and any floating oils (using oil-absorptive pads).
- (v) Inspect resource and treatment buffers at least once a year for evidence of erosion, concentrating flow, and encroachment by development. If flows are concentrating within a buffer, site grading, level spreaders, or ditch turn-outs must be used to ensure a more even distribution of flow into a buffer. Check down slope of all spreaders and turn-outs for erosion. If erosion is present, adjust or modify the spreader's or turnout's lip to ensure a better distribution of flow into a buffer. Clean-out any accumulation of sediment within the spreader bays or turn-out pools.

(c) Regular maintenance

(i) Clear accumulations of winter sand in parking lots and along roadways at least once a year, preferably in the spring. Accumulations on pavement may be removed by pavement sweeping. Accumulations of sand along road shoulders may be removed by grading excess sand to the pavement edge and removing it manually or by a front-end loader. Grading of gravel roads, or grading of the gravel shoulders of gravel or paved roads, must be routinely performed to ensure that stormwater drains immediately off the road surface to adjacent buffer areas or stable ditches, and is not impeded by accumulations of graded material on the

road shoulder or by excavation of false ditches in the shoulder. If water bars or open-top culverts are used to divert runoff from road surfaces, clean-out any sediments within or at the outlet of these structures to restore their function.

(ii) Manage each buffer's vegetation consistently with the requirements in any deed restrictions for the buffer. Wooded buffers must remain fully wooded and have no disturbance to the duff layer. Vegetation in non-wooded buffers may not be cut more than three times per year, and may not be cut shorter than six inches.

NOTE: Contact the department's Division of Watershed Management (Maine DEP) for assistance developing inspection and maintenance requirements for other drainage control and runoff treatment measures installed on the site. The maintenance needs for most measures may be found in the Maine DEP's "Stormwater Management for Maine: Best Management Practices."

(d) Documentation. Keep a log (report) summarizing inspections, maintenance, and any corrective actions taken. The log must include the date on which each inspection or maintenance task was performed, a description of the inspection findings or maintenance completed, and the name of the inspector or maintenance personnel performing the task. If a maintenance task requires the clean-out of any sediments or debris, indicate where the sediment and debris was disposed after removal.

The log must be made accessible to department staff and a copy provided to the department upon request. The permittee shall retain a copy of the log for a period of at least three years from the completion of permanent stabilization.

- 3. Maintenance contract. Contract with a third-party or other qualified professional, as approved by the department, for the removal of accumulated sediments, oils, and debris within any proprietary devices and the replacement of any absorptive filters. The frequency of sediment clean-out and filter replacements must be consistent with the unit's storage capacity and the estimated pollutant load from the contributing drainage area. This clean-out frequency is usually established by the manufacturer of the proprietary system when sizing the device for the project.
- 4. Re-certification. Submit a certification of the following to the department within three months of the expiration of each five-year interval from the date of issuance of the permit.
 - (a) Identification and repair of erosion problems. All areas of the project site have been inspected for areas of erosion, and appropriate steps have been taken to permanently stabilize these areas.
 - (b) Inspection and repair of stormwater control system. All aspects of the stormwater control system have been inspected for damage, wear, and malfunction, and appropriate steps have been taken to repair or replace the system, or portions of the system.
 - (c) Maintenance. The erosion and stormwater maintenance plan for the site is being implemented as written, or modifications to the plan have been submitted to and approved by the department, and the maintenance log is being maintained.

Municipalities with separate storm sewer systems regulated under the Maine Pollutant Discharge Elimination System (MPDES) Program may report on all regulated systems under their control as part of their required annual reporting in lieu of separate certification of each system. Municipalities not regulated by MPDES, but that are responsible for maintenance of permitted stormwater systems, may report on multiple stormwater systems in one report.

- 5. Duration of maintenance. Perform maintenance as described and required in the permit unless and until the system is formally accepted by the municipality or quasi-municipal district, or is placed under the jurisdiction of a legally created association that will be responsible for the maintenance of the system. If a municipality or quasi-municipal district chooses to accept a stormwater management system, or a component of a stormwater system, it must provide a letter to the department stating that it assumes responsibility for the system. The letter must specify the components of the system for which the municipality or district will assume responsibility, and that the municipality or district agrees to maintain those components of the system in compliance with department standards. Upon such assumption of responsibility, and approval by the department, the municipality, quasi-municipal district, or association becomes a co-permittee for this purpose only and must comply with all terms and conditions of the permit.
- 6. Additional requirements. Additional requirements may be applied on a site-specific basis.

APPENDIX C. Housekeeping

These performance standards apply to all projects.

- 1. Spill prevention. Controls must be used to prevent pollutants from being discharged from materials on site, including storage practices to minimize exposure of the materials to stormwater, and appropriate spill prevention, containment, and response planning and implementation.
- 2. Groundwater protection. During construction, liquid petroleum products and other hazardous materials with the potential to contaminate groundwater may not be stored or handled in areas of the site draining to an infiltration area. An "infiltration area" is any area of the site that by design or as a result of soils, topography and other relevant factors accumulates runoff that infiltrates into the soil. Dikes, berms, sumps, and other forms of secondary containment that prevent discharge to groundwater may be used to isolate portions of the site for the purposes of storage and handling of these materials.

See Appendix D for license by rule standards for infiltration.

NOTE: Lack of appropriate pollutant removal best management practices (BMPs) may result in violations of the groundwater quality standard established by 38 M.R.S.A. §465-C(1).

3. Fugitive sediment and dust. Actions must be taken to ensure that activities do not result in noticeable erosion of soils or fugitive dust emissions during or after construction. Oil may not be used for dust control.

NOTE: An example of the use of BMPs to control fugitive sediment and dust is as follows. Operations during wet months that experience tracking of mud off the site onto public roads should provide for sweeping of road areas at least once a week and prior to significant storm events. Where chronic mud tracking occurs, a stabilized construction entrance should be provided. Operations during dry months, that experience fugitive dust problems, should wet down the access roads once a week or more frequently as needed.

NOTE: Dewatering a stream without a permit from the department violates state water quality standards and the Natural Resources Protection Act.

4. Debris and other materials. Litter, construction debris, and chemicals exposed to stormwater must be prevented from becoming a pollutant source.

NOTE: To prevent these materials from becoming a source of pollutants, construction and post-construction activities related to a project may be required to comply with applicable provision of rules related to solid, universal, and hazardous waste, including, but not limited to, the Maine solid waste and hazardous waste management rules; Maine hazardous waste management rules; Maine oil conveyance and storage rules; and Maine pesticide requirements.

5. Trench or foundation de-watering. Trench de-watering is the removal of water from trenches, foundations, coffer dams, ponds, and other areas within the construction area that retain water after excavation. In most cases the collected water is heavily silted and hinders correct and safe

06-096 DEPARTMENT OF ENVIRONMENTAL PROTECTION

construction practices. The collected water must be removed from the ponded area, either through gravity or pumping, and must be spread through natural wooded buffers or removed to areas that are specifically designed to collect the maximum amount of sediment possible, like a cofferdam sedimentation basin. Avoid allowing the water to flow over disturbed areas of the site. Equivalent measures may be taken if approved by the department.

NOTE: For guidance on de-watering controls, consult the Maine Erosion and Sediment Control BMPs", Maine Department of Environmental Protection."

- 6. Non-stormwater discharges. Identify and prevent contamination by non-stormwater discharges.
- 7. Additional requirements. Additional requirements may be applied on a site-specific basis.



NATURAL RESOURCE PROTECTION ACT (NRPA) STANDARD CONDITIONS

THE FOLLOWING STANDARD CONDITIONS SHALL APPLY TO ALL PERMITS GRANTED UNDER THE NATURAL RESOURCE PROTECTION ACT, TITLE 38, M.R.S.A. SECTION 480-A ET.SEQ. UNLESS OTHERWISE SPECIFICALLY STATED IN THE PERMIT.

- A. <u>Approval of Variations From Plans.</u> The granting of this permit is dependent upon and limited to the proposals and plans contained in the application and supporting documents submitted and affirmed to by the applicant. Any variation from these plans, proposals, and supporting documents is subject to review and approval prior to implementation.
- B. Compliance With All Applicable Laws. The applicant shall secure and comply with all applicable federal, state, and local licenses, permits, authorizations, conditions, agreements, and orders prior to or during construction and operation, as appropriate.
- C. <u>Erosion Control</u>. The applicant shall take all necessary measures to ensure that his activities or those of his agents do not result in measurable erosion of soils on the site during the construction and operation of the project covered by this Approval.
- D. <u>Compliance With Conditions</u>. Should the project be found, at any time, not to be in compliance with any of the Conditions of this Approval, or should the applicant construct or operate this development in any way other the specified in the Application or Supporting Documents, as modified by the Conditions of this Approval, then the terms of this Approval shall be considered to have been violated.
- E. <u>Initiation of Activity Within Two Years</u>. If construction or operation of the activity is not begun within two years, this permit shall lapse and the applicant shall reapply to the Board for a new permit. The applicant may not begin construction or operation of the activity until a new permit is granted. Reapplications for permits shall state the reasons why the applicant will be able to begin the activity within two years form the granting of a new permit, if so granted. Reapplications for permits may include information submitted in the initial application by reference.
- F. Reexamination After Five Years. If the approved activity is not completed within five years from the date of the granting of a permit, the Board may reexamine its permit approval and impose additional terms or conditions to respond to significant changes in circumstances which may have occurred during the five-year period.
- G. No Construction Equipment Below High Water. No construction equipment used in the undertaking of an approved activity is allowed below the mean high water line unless otherwise specified by this permit.
- H. <u>Permit Included In Contract Bids.</u> A copy of this permit must be included in or attached to all contract bid specifications for the approved activity.
- I. <u>Permit Shown To Contractor.</u> Work done by a contractor pursuant to this permit shall not begin before the contractor has been shown by the applicant a copy of this permit.

Revised (4/92) DEP LW0428

Post-It* Fax Note 7671	Date 7-16 -07 # of pages > 12
TO MARSHALL TINKLE	From BOLL BULLAND
Со/Dept.	CO.MDEP
Phone #	Phone # 822-6380
Fax# 874-6705	Fax#

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4. Applient's Daytime Phone#	207-871-89	47	7. Agenda Dayi 3. Phonests		207-839-27	71	201
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Source of divient to Comple . With this Stormwater PBR notification form and my signature below, I am filing notice of my Action of the Mark Construction of the property of the Mark Completion of the MCGP standards. In addition, I will file a

If this form is not being signed by the landowner or lessee of the property, attach documentation showing authorization to sign.

Date: 7/12/07

4 . 4.0 .

ORIGINAL TO PERSON MAKING PAYMENT

THIS IS YOUR OFFICIAL RECEIPT. PLEASE RETAIN IT FOR FUTURE REFERENCE

ORIGINAL TO STATE OF MAINE

RECEIPT FOR DEPARTMENTAL COLLECTIONS

Nº 792942 G

MONEY OFFICIAL MINISTER TO THE TOTAL TO

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NAME OF DEPARTMENT OR INST. MAKING COLLECTION

SIGNATURE OF PERSON MAKING COLLECTION

. FNGINEERS . SURVEYORS . PLANNERS

LESTER S. BERRY WILLIAM A. THOMPSON

ROBERT C. LIBBY, Jr. ANDREW S. MORRELL

July 12, 2007

Maine Dept. of Environmental Protection Southern Maine Regional Office 312 Canco Road Portland, ME 04101

RE:

Stormwater Permit-by-Rule Chabad Lubobitch of Maine, Inc. Pomeroy Street Portland

To Whom It May Concern:

Please find attached the following information for the above-referenced project:

- 1. Stormwater Permit-by-Rule Application
- 2. Required Fees (\$55)
- 3. Photographs of Site
- 4. U.S.G.S. Map
- 5. Site Plan (Sheet 1)
- 6. Erosion Control Plan (Sheet 6)

On behalf of the applicant, Chabad Lubobitch of Maine, Inc., we are submitting a Stormwater Permit-by-Rule. This project was approved by the City of Portland back in July 2005 and was not eligible for a stormwater permit at that time (according to DEP regulations). The project was never built and the applicant is looking to construct the project at this time. According to current DEP regulations, the project is eligible for a Stormwater Permit-by-Rule. The project does not fall within a watershed "most-at-risk" and results in the following areas:

Impervious Area = 0.68 acres

Developed Area = 1.59 acres

Please call if you have any questions or require additional information.

Sincerely,

Andrew S. Morrell, E.I.T.
Staff Engineer

Enclosure(s) cc: Richard Abrahams

ChabadDEPPBR



PORTLAND MAINE

Strengthening a Remarkable City, Building a Community for Life www.portlandmaine.gov

23 July 2007

Public Works Department Michael J. Bobinsky, Director

> Mr. William A. Thompson, Project Manager, Berry, Huff, McDonald, and Milligan, Inc., 28 State Street, Gorham, Maine 04038

> > RE: The Capacity to Handle Wastewater Flows, from the Proposed Residence and Assembly Area, for Chabad Lubobitch, at 123 Pomeroy Street.

Dear Mr. Thompson:

The existing eighteen-inch diameter vitrified clay sewer pipe located in Bancroft Street has adequate capacity to **transport**, while The Portland Water District sewage treatment facilities, located off Marginal Way, have adequate capacity to **treat** the total increase in wastewater flows of **1,044 GPD**, from the proposed building project.

Anticipated	Wastewater	Flows	from	the Pro	posed	Building	Project:
THEFT	11 000 00 11 000 02						

9 Proposed Bedrooms @ 90 GPD/Bedroom

= 990 GPD

27 Proposed Fixed Assembly Seats @ 2 GPD/Fixed Seat

= 54 GPD

Total Proposed Increase in Wastewater Flows for this Project

= 1.044 GPD

The City combined sewer overflow (C.S.O.) abatement consent agreement (with the U.S.E.P.A., and with the Maine D.E.P.) requires C.S.O. abatement, as well as storm water mitigation, in order to offset any increase in sanitary flows, from all projects. If the City can be of further assistance, please call 874-8832.

Sincerely,

CITY OF PORTLAND

Frank J Brancely, B.A., M.A.

Senior Engineering Technician

FJB

cc:

Alexander Q. Jaegerman, Director, Planning Division, Department of Planning, and Urban Development, City of Portland Barbara Barhydt, Development Review Services Manager, Department of Planning, and Urban Development, City of Portland Penny Littell, Associate Corporation Counsel, Corporation Counsel, City of Portland

Michael Farmer, P.E., Project Engineer, City of Portland

Bradley A. Roland, P.E., Environmental Projects Engineer, City of Portland

Stephen K. Harris, Assistant Engineer, City of Portland

Jane Ward, Administrative Assistant, City of Portland

William A. Thompson, Project Manager, Berry, Huff, McDonald, and Milligan, Incorporated.

Desk file



Portland Water District

FROM SEBAGO LAKE TO CASCO BAY

March 28, 2005

William A. Thompson BH2M 28 State St. Gorham, Me. 04038

Re: Pomeroy St.-Chabad Lubobitch

William:

This letter is to confirm there should be an adequate supply of clean and healthful water to serve the needs of the proposed building to be located off Pomroy St. in Portland after a main extension is done from Bancroft St. to the centerline of the lot in question.. Checking District records, I find the extension would need to be about 175 to 200 feet in length coming from Bancroft St.

The current data from the nearest hydrant indicates there should be adequate capacity of water to serve the needs of your proposed project.

Hydrant Location: Bancroft St. 650'NE of Capisic St.

Hydrant # 1432

Static pressure = 71 PSI

Flow = 1135 GPM

Last Tested = 8/3/93

If the District can be of further assistance in this matter, please let us know.

Sincerely,

Portland Water District

Jim Pandiscio

Means Coordinator

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BH2M Providing Quality Civil-Site Engineering & Surveying for Over 33 Years.

SERVICES:

- Site Development Design
- Subdivision Design
- Stormwater Management Analysis & Design
- Utility Design
- Roadway Design
- Development Permitting
- Construction Administration & Oversight
- Full Service Survey Department

COMPANY OVERVIEW

Berry Huff McDonald Milligan Inc. (BH2M) was founded in 1978 in Gorham, Maine to provide quality civil-site engineering and surveying services. Over the past 33 years BH2M has worked on over 6,000 projects for our diverse client base, which consists of Municipal and Private Sector clients. BH2M has developed a reputation for a strong committment to excellence in all portions of a project. The staff structure at BH2M is unique in that all the engineers and project managers are partners within the company. This has been a successful formula that has resulted in many long standing relationships with our clients. Each project at BH2M is overseen by a senior principal within the company to assure the highest level of quality of work and performance.

EXPERIENCE

BH2M has provided Quality Civil-Site Engineering and Surveying on many projects within the City of Portland, Including:

Office & Commercial Developments

- Oakhurst Dairy
- WB Mason Headquarters
- Unum
- Walgreens Bayside Area

Hospitals

- Childrens Hospital
- Congress St. Medical Building

Roadway Improvements

- Oak Street

Major Industrial

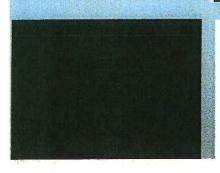
- Brunswick Naval Air Station Hanger Project Brunswick
- Brunswick Naval Air Station Tower Project Brunswick
- Savage Intermodel Facility Auburn
- Pratt-Whitney North Berwick

CURRENT PROJECTS

- Route 25 & Oak Hill Road Intersection Relocation and Sidewalk Project-Standish
- Summer Winds Condominiums Old Orchard Beach
- Black Point Park at Scarborough Beach Scarborough
- Village Square Sidewalk Restoration Gorham
- Fogg Brook Subdivision Buxton
- Sawyer Estates Subdivision Gorham



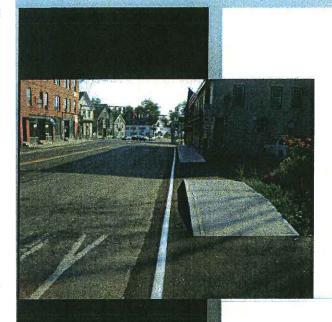
Selected Stormwater Management Project Experience



- Route 25 & Oak Hill Road Reconstruction
 & Drainage Improvement, Standish 2012
- Summer Winds Condominium, Old Orchard Beach 2011
- Village Square Sidewalk Reconstruction, Gorham 2011
- Black Point Park at Scarborough Beach, Scarborough 2011
- Fogg Brook Subdivision, Gorham 2011
- Sawyer Estates Subdivision, Gorham 2011
- Golden Ridge Subidvision, Cape Elizabeth 2011
- Juniper Knoll Subdivision, Saco 2010
- Meadowbrook Subdivision, Waterboro 2010
- Bradbury Ridge Subdivision, Hollis 2010
- Lands End Subdivision, Old Orchard Beach 2010
- Limington Salt Shed, Limington 2010
- Peterson Fields Subdivision, 2010
- The Trails Subdivision, Saco 2010
- Mitchell Hill Subdivision, Windham 2010
- Stonehill Subdivision, Biddeford 2010
- Kate's Homemade Butter Plant, Arundel 2010
- Carsons Point Subdivision, Saco 2010
- Pratts Brook Farm Subdivision, Yarmouth 2010
- Highland Glen Subdivision, Yarmouth 2010
- Tucker Road Culvert Improvements, Limington 2010
- Dunegrass Sections B & C, Old Orchard Beach 2010
- Skylark Commons Subdivision, Portland 2009
- Pleasant Ridge Subdivision, Buxton 2009
- Wholesale Distribution and Warehouse Facility, Brockton Ma 2009
- Willowdale Commons Condominium, Old Orchard Beach 2009
- Atlantic Park Condominium, Old Orchard Beach 2009
- Whispering Pines Subdivision, Buxton 2009
- Aceto Construction Facility, Buxton 2009
- D & E Enterprises Facility, Hollis 2009
- K & S Subdivision, Sanford 2009
- Wild Acres Campground Expansion, Old Orchard Beack 2009
- Hardware Store, Biddeford 2009
- Hid n Pines Campground, Old Orchard Beach 2009
- Green Court Acres Subdivision, Waterboro 2009



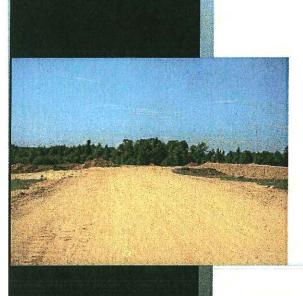
Selected Roadway Reconstruction and Drainage Improvement Project Experience



- Route 25 & Oak Hill Road Intersection Relocation and Sidewalk Project Standish, 2012
- Gorham Village Square Sidewalk Reconstruction Project Gorham, 2011
- Granite Street Extension Roadway Reconstruction and Drainage Improvement Project Biddeford, 2009
- Glendale Neighborhood Reconstruction and Drainage Improvement Project Auburn, 2004
- Carrier Development Reconstruction and Drainage Improvement Project Auburn, 2003
- Old Danville Road Reconstruction and Drainage Improvement Project Auburn, 1999
- Mill Street Reconstruction and Drainage Improvement Project Auburn, 1998
- CSO Neighborhood Improvement Project #2 Biddeford, 1998
- CSO Neighborhood Improvement Project #1 Biddeford, 1994
- Downtown Revitalization & Stormdrain Improvments Projects #1, #2 & #3
 Old Orchard Beach, 1991 - 1992

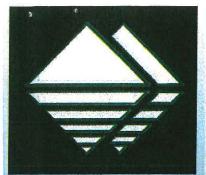


Selected Site Development Project Experience



- Black Point Park at Scarborough, Beach, Scarborough 2011
- Limington Salt Shed, Limington 2010
- Childrens Hospital, Portland 2010
- Kate's Homemade Butter Plant, Arundel 2010
- Savage Intermodal Facility, Auburn 2010
- Wholesale Distribution and Warehouse Facility, Brockton Ma 2009
- Aceto Construction Facility, Buxton 2009
- D & E Enterprises Facility, Hollis 2009
- Hardware Store, Biddeford 2009
- Oakhurst Dairy Facility, Portland 2009
- WB Mason Headquarters, Portland 2009
- Walgreens, Portland 2008
- Unum Site Improvements, Portland 2008
- Congress Street Medical Building, 2008
- Pratt Whitney Facility, North Berwick 2007
- Brunswick Naval Air Station Hanger Project, Brunswick 2007
- Morrison Center, Scarborough 2006
- Aubuchon Hardware, Limington 2006





Lester S. Berry, P.E.

EDUCATION:

B.S. Civil Engineering University of Maine

M.S. Civil Engineering University of Maine

PROFESSIONAL SOCIETIES:

American Society of Civil Engineers

Maine Association of Planners

Construction Specifications Institute

PROFESSIONAL BACKGROUND:

Vice President BH2M 1978 - Present Gorham, Maine

Project Engineer Dale E. Caruthers Company 1975 - 1978 Gorham, Maine

Engineer State of New Hampshire 1971 - 1972 Concord, New Hampshire Lester S. Berry, P.E. Vice President & Senior Engineer

Les co-founded Berry Huff McDonald Milligan Inc. in 1978. He has 40 years of experience in both the public and private sector and has worked on projects in Maine and New Hampshire. His expertise includes a diversified range of all aspects of civil-site engineering, with a focus on site development and the design and implementation of state of the art Stormwater Management Systems.

The following is a list of recent projects worked on by Les:

- Route 25 & Oak Hill Road Intersection Relocation and Sidewalk Project. Standish 2012
- Summer Winds Condominium, Old Orchard Beach 2011
- Village Square Sidewalk Restoration, Gorham 2011
- Black Point Park at Scarborough Beach, Scarborough 2011
- Sawyer Estates Subdivision, Gorham 2011
- Limington Salt Shed, Limington 2011
- Kate's Homemade Butter Plant, Arundel 2011
- Peterson Fields Subdivision, Gorham 2010
- The Trails Subdivision, Gorham 2010
- Savage Intermodal Facility, Auburn 2010
- Childrens Hospital, Portland 2010
- Tucker Road Culvert Improvements, Limington 2010
- Mitchell Hill Subdivision, Windham 2010
- Stonehill Subdivision, Biddeford 2010
- Skylark Commons Subdivision, Portland 2010
- WB Mason Headquarters, Portland 2009
- Walgreens, Portland 2009
- Unum Site Improvements, Portland 2008
- Congress Street Medical Building, Portland 2008



STORMWATER EDUCATION: Hydraulic & related College course Erosion & Sediment Control Stormwater Management Water Conservation Districts, Phosphorus Management, Erosion Control Seminars by Maine DEP, HydroCAD & Advanced HydroCAD, BMP's for Stormwater and Erosion Control.



William A. Thompson

EDUCATION:

A.A.S. Civil-Sanitary Tech. Blue Hills Regional Institute 1970

Northeastern University Boston, MA. 1971 - 1972

PROFESSIONAL BACKGROUND:

President/Project Manager BH2M January 1993 - Present Gorham, Maine

Vice President/ Project Manager BH2M January 1988 - January 1993 Gorham, Maine

Design/Drafting Deptartment Supervisor BH2M November 1978 -December 1987

Chief Draftsman Dale E. Caruthers Company June 1976 - November 1978 Gorham, Maine

Design Draftsman SEA Consultants Boston, MA. June 1970 - June 1976 William A. Thompson President & Project Manager

Bill has worked for BH2M for over 33 years and has 40 years of experience in both the public and private sector and has worked on projects in Maine and Massachusetts. His expertise includes site development, subdivisions and the design of supporting Stormwater Management Systems.

The following is a list of recent projects worked on by Bill:

- Sawyer Estates Subdivision, Gorham 2011
- Juniper Knoll Subdivision, Saco 2010
- Limington Salt Shed, Limington 2010
- Kate's Homemade Butter Plant, Arundel 2010
- Bradbury Ridge Subdivision, Buxton 2010
- Lands End Subdivision, Old Orchard Beach 2010
- Peterson Fields Subdivision, Gorham 2010
- The Trails Subdivision, Saco 2010
- Mitchell Hill Subdivision, Windham 2010
- Stonehill Subdivision, Biddeford 2010
- Carsons Point Subdivision, Saco 2010
- Skylark Commons Subdivision, Portland 2010
- Sunrise Ridge Subdivision, Buxton 2009
- Atlantic Park Condominium, Old Orchard Beach 2009
- Pleasant Ridge Subdivision, Buxton 2009
- Dunegrass Sections B & C, Old Orchard Beach 2009
- Meadowbrook Subdivision, Waterboro 2009
- Aceto Construction Facility, Buxton 2009





Andrew S. Morrell, E.I.T.

EDUCATION:

B.S. Civil Engineering State University of New York Buffalo, NY

PROFESSIONAL BACKGROUND:

Project Engineer BH2M - Gorham, Me August 2001 - August 2007 April 2010 - Present

Project Engineer DeLuca-Hoffman Associates South Portland, Me August 2007 - March 2010

Project Engineer Diversified Civil Engineering Westford, Ma May 1999 - August 2001

STORMWATER EDUCATION:

Hydraulics Review Class for Professional Engineering License Exam - ASCE 2009

Hydrocad Seminar Joint Environment Training Coordinating Committee 2002

STORMWATER EXPERIENCE:

12 years experience performing stormwater management design and calculations. Andrew S. Morrell, E.I.T. Project Engineer

Andy has worked for BH2M for over 9 years and has over 12 years of experience in both the public and private sector and has worked on projects in Maine and Massachusetts. His expertise includes site development, subdivisions and the design of supporting Stormwater Management Systems.

The following is a list of recent projects worked on by Andy:

- Route 25 & Oak Hill Road Intersection Relocation and Sidewalk Project, Standish 2012
- Childrens Hospital, Portland 2011
- Village Square Sidewalk Restoration, Gorham 2011
- Black Point Park at Scarborough Beach, Scarborough 2011
- Sawyer Estates Subdivision, Gorham 2011
- Juniper Knoll Subdivision, Saco 2010
- Limington Salt Shed, Limington 2010
- Kate's Homemade Butter Plant, Arundel 2010
- Tucker Road Culvert Improvements, Limington 2010
- Bradbury Ridge Subdivision, Buxton 2010
- Lands End Subdivision, Old Orchard Beach 2010
- Peterson Fields Subdivision, Gorham 2010
- The Trails Subdivision, Saco 2010
- Mitchell Hill Subdivision, Windham 2010
- Stonehill Subdivision, Biddeford 2010
- Carsons Point Subdivision, Saco 2010
- Pratts Brook Farm Subdivision, Yarmouth 2010
- Highland Glen Subdivision, Yarmouth 2010
- Skylark Commons Subdivision, Portland 2010
- Sunrise Ridge Subdivision, Buxton 2009
- Atlantic Park Condominium, Old Orchard Beach 2009
- Pleasant Ridge Subdivision, Buxton 2009
- Dunegrass Sections B & C, Old Orchard Beach 2009
- Meadowbrook Subdivision, Waterboro 2009
- Aceto Construction Facility, Buxton 2009
- Unum Site Improvements, Portland 2009





Robert C. Libby, Jr., PLS

REGISTRATION

Professional Land Surveyor Maine #2190 Licensed since August 1990

EDUCATION

B.S. Forestry Management/ Recreational Park Management University of Maine 1982

PROFESSIONAL SOCIETIES

Maine Society of Land Surveyors President 2009-2011

Narragansett Chapter Former President & Current Treasurer

PROFESSIONAL BACKGROUND

Survey Party/Chief Engineering Technician BH2M Gorham, Maine 1985 - 1993

Survey Department Head BH2M Gorham, Maine 1993 - Present Robert C. Libby, Jr. Professional Land Surveyor

Bob has worked for BH2M for over 26 years with experience in both the public and private sector working throughout York, Cumberland, Oxford & Androscoggin Counties. His experience includes Boundary Surveys, ALTA Surveys, Road Projects, Site Topography, As-Built Surveys and Construction Layout Surveys.

The following is a list of recent projects worked on by Bob:

- Route 25 & Oak Hill Road Intersection Relocation and Sidewalk Project, Standish 2012
- Town of Standish Public Works Site Topography, 2011
- Town of York Public Safety Building Topography & Boundary Survey, 2011
- York County Fish & Game Lyman Site Topography, 2011
- Jason Labonte Lot Division / Private Way, Saco, 2011
- Summer Winds Subdivision and Site Plan, Old Orchard Beach, 2011
- Town of Limington Salt Shed Topography & Boundary Survey, 2010
- Dunegrass Section B & C Subdivision, Old Orchard Beach, 2010
- Lakeside Community Church Waterboro, As-Built Survey, 2009
- Smith Elliott Smith & Garmey Project, Expert Witness, Hollis, 2008
- Maine Turnpike Authority Right of Way Maps, 2008



THE BANK OF MAINE

March 26, 2012

City of Portland Planning office. Congress St. Portland, ME 04101

RE: Chabad House

Dear Planning Department,

Windsor Construction has the financial and technical experience to construct this project per plans and approvals. If you need any additional information I can be reached at (207) 518-5792.

Sincerely,

Betty J. Olson

Senior Vice President

WARRANTY DEED

KNOW ALL PERSONS BY THESE PRESENTS, that MOSHE WILANSKY and CHANA WILANSKY of Portland, Maine, for consideration paid, GRANTS TO CHABAD LUBAVITCH OF MAINE, INC., a Maine non-profit corporation whose mailing address is 101 Craigie Street, Portland, ME 04102, with WARRANTY COVENANTS, the premises situated in the City of Portland, County of Cumberland and State of Maine, described as follows:

A certain lot or parcel of land located on the southwesterly sideline of Pomeroy Street, so-called, in the City of Portland, County of Cumberland and State of Maine; said parcel being more particularly described as follows:

Beginning at an V2" iron rod found on the southwesterly sideline of said Pomeroy Street at the southeasterly comer of land now or formerly of Kriston Briggs;

Thence South 46° 11° 30" East along the southwesterly sideline of said Pomeroy Street a distance of 269.44 feet to a capped iron rod found (PLS #1278) and the northerly sideline of Motley Street, so-called;

Thence South 71° 08' 30" West along the northerly sideline of said Motley Street a distance of 58.71 feet to a point;

Thence in a general southwesterly direction along the northerly sideline of said Motley Street and along a circular curve to the left, circumscribed by a radius of 218.81 feet, an arc length of 44.47 feet to a capped iron rod found (PLS #1278) and land now or formerly of Joshuz James and Tamara Rainsford Krieger; said capped iron rod found being South 65° 13° West a tie distance of 44.39 feet from said previous capped iron rod found;

Thence North 34° 26' 02" West along the land of said Krieger a distance of 86.47 feet to a capped 5/8" iron rod found (PLS #1172);

Thence South 38° 44' 24" West along the land of said Krieger, along land now or formerly of the Redion Park Homeowner's Association, and along land now or formerly of Nancy A. Roy a distance of 350.00 feet to a point and remaining land of Stuart B. Herrick, Jr.;

Thence North 28° 05' 22" West along the remaining land of Stuart B. Herrick, Jr. a distance of 286.03 feet to a point and land now or formerly of Thomas and Melody Lussier;

Thence North 60° 18° 30" East along the land of said Lussier, along land now or formerly of Roger and Donna Gendron, along land now or formerly of Jean Gilpatrick and along land of said Briggs a distance of 350.00 feet to the point of beginning.

The above-described parcel contains \$7,924 square feet. All bearings refer to Magnetic North as observed in 1973.

Reference is made to an unrecorded plan entitled Plan of Property, Bancroft Street, Portland, Maine, dated July 31, 1986, by R.P. Titcomb Associates, Inc. (Job #8660), as revised by unrecorded plan entitled Standard Boundary Survey and existing Conditions Plan for Stuart B. Herrick, Jr., dated March 2004, by BH2M (Job #03217).

Being the same premises conveyed to the Grantors herein by Warranty Deed of Stuart B. Herrick, Jr. dated May 28, 2004 and recorded in the Cumberland County Registry of Deeds at Book 21341, Page 37.

IN WITNESS WHEREOF, the said Moshe Wilansky and Chana Wilansky have caused this instrument to be signed and sealed on June $\frac{15}{1}$, 2004.

Witness Mines

Moshe Wilansky

Vitness

Chana Wilansky

State of Maine Cumberland, ss.

June 15, 2004

Then personally appeared the above-named Moshe Wilansky and/or Chana Wilansky and acknowledged the foregoing instrument to be his/her/their free act and deed.

Before me,

Notary Public, State of Maine

Print Name:

Karen Belton

My commission expires 08/25/06

Received
Recorded Resister of Deeds
Jun 16+2004 11+22:226
Cusberland Counts
John & Dirlen

Elbomelusacy/ourrent/wilansky, mose & changlwarranty deed to chahed doc