



**ADDENDUM 2**  
To Contract Documents for  
**EYECARE MEDICAL GROUP**  
53 SEWALL STREET  
PORTLAND, ME 04102

**E.M.G. - Phase 2 Addition & Renovation**

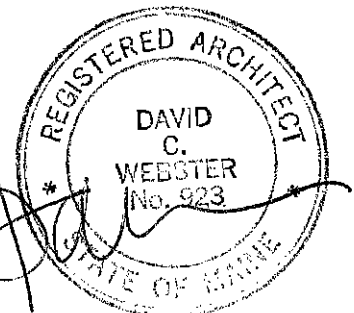
This Addendum modifies, amends and supplements designated parts of the Contract Documents, Project Manual and Drawings for **E.M.G. - Phase 2 - Addition and Renovation** and is hereby made a part thereof by reference and shall be as binding as though inserted in its entirety in the locations specified herein. It shall be the responsibility of the Contractor to notify all Subcontractors and Suppliers he proposes to use for the various parts of the work of any changes or modifications contained in this Addendum.



Architecture ■ Interior Design ■ Planning

49 Dartmouth Street  
Portland, Maine 04101  
207-775-1059 ■

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EYECARE MEDICAL GROUP  
PHASE 2 ADDITION AND RENOVATION  
ADDENDUM NO. 2  
July 15, 2013

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This Addendum modifies, amends and supplements designated parts of the Contract Documents, Project Manual and Drawings for  
E.M.G. - Phase 2 - Addition and Renovation  
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| <b>PART V</b>   | Addendum for Electrical Specifications and Drawings    |

GENERAL INFORMATION

- I. None

PART I- ADDENDUM FOR CIVIL SPECIFICATIONS AND DRAWINGS:

- I. REPLACE the Draft Geotechnical Report issued in Volume I, Div. 2, with the attached final Geotechnical Report.

PART II- ADDENDUM FOR STRUCTURAL SPECIFICATIONS AND DRAWINGS:

1. ADD ORN04 light support framing plan SKS-05.
2. ADD OR N03 light support framing plan SKS-06.
3. ADD typical light support detail SKS-07.
4. ADD typical light support detail SKS-08.
5. ADD typical light support details SKS-09.
6. CHANGE partial roof framing plan as indicated on SKS-10.
7. ADD detail at parapet along line-8 as indicated on SKS-II.

EYECARE MEDICAL GROUP  
PHASE 2 ADDITION AND RENOVATION  
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**PART III- ADDENDUM FOR ARCHITECTURAL PROJECT MANUALS AND DRAWINGS:**

- 1.** ADD soffits in Autoclave B119 as indicated on SKA-1.
- 2.** ADD soffits in Nurse B108 as indicated on SKA-2.
- 3.** REPLACE elevation A5/A201 with SKA-3, clarifying wall lighting layout.

**PART IV- ADDENDUM FOR MECHANICAL SPECIFICATIONS AND DRAWINGS:**

- 1.** CHANGE under slab domestic water and sprinkler entrance as indicated on SKP-1.
- 2.** DELETE under slab domestic water and sprinkler entrance indicated on SKP-2.

**PART V- ADDENDUM FOR ELECTRICAL SPECIFICATIONS AND DRAWINGS:**

- 1.** None

**END OF ADDENDUM**

# REPORT

June 27, 2013  
12-0392 S

## Geotechnical Engineering Services

Proposed Building Addition and Parking Lot  
53 Sewall Street  
Portland, Maine

**PREPARED FOR:**

Eyecare Medical Group, P.A.  
Attention: Clement Berry  
53 Sewall Street  
Portland, Maine 04102

**PREPARED BY:**

S.W.COLE ENGINEERING, INC.  
286 Portland Road  
Gray, Maine 04039  
207-657-2866



**S.W.COLE**  
ENGINEERING, INC.

- *Geotechnical Engineering*
- *Construction Materials Testing*
- *GeoEnvironmental Services*
- *Ecological Services*

[www.swcole.com](http://www.swcole.com)

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13-0392 S

June 27, 2013

Eyecare Medical Group, P.A.  
Attn: Clement Berry  
53 Sewall Street  
Portland, Maine 04102

Subject: Explorations and Geotechnical Engineering Services  
Proposed Building Addition and Parking Lot  
53 Sewall Street  
Portland, Maine

Dear Mr. Berry:

In accordance with our Proposal, dated May 9, 2013, we have performed subsurface explorations for the subject project in Portland, Maine. This report summarizes our findings and geotechnical recommendations and its contents are subject to the limitations set forth in Attachment A.

## **1.0 INTRODUCTION**

### **1.1 Scope and Purpose**

The purpose of our services was to obtain subsurface information at the site in order to develop geotechnical recommendations relative to foundations and earthwork associated with the proposed construction. Our scope of services included the making of five test boring explorations, soils laboratory testing, a geotechnical analysis of the subsurface findings and preparation of this report.

### **1.2 Proposed Construction**

The site is located at your existing facility on Sewall Street in Portland, Maine. Based on information provided by PDT Architects (project architect), we understand development plans call for construction of an approximate 45-foot by 125-foot addition off the west side of the existing building and a 20-space parking lot in the southwest corner of the site. We understand the building addition will be one-story with foundations and columns sized for a future second story addition.

Based on information provided by Allied Engineering (project structural engineer), we understand column loads will approach 126-kips, wall loads 1.6-kips/lf and floor loads 40 psf. Based on foundation plans for the original building, we understand the existing building is supported on spread footing foundations with on-grade floor slabs. We understand the proposed finished floor elevation will match the existing building at elevation 37.5 feet (project datum).

## **2.0 EXPLORATION AND TESTING**

### **2.1 Explorations**

Five test borings (B-101 through B-105) were made at the site on May 24, 2013 by Northern Test Boring, Inc. of Gorham, Maine working under subcontract to S.W. COLE ENGINEERING, INC. (SWCE). The boring locations were selected and established in the field by SWCE using taped measurements from existing site features. The approximate exploration locations are shown on the "Exploration Location Plan" attached as Sheet 1. Logs of the explorations are attached as Sheets 2 through 7. A key to the notes and symbols used on the logs is attached as Sheet 8.

### **2.2 Testing**

The borings were performed using a combination of solid stem auger, cased wash-boring and rod probing techniques. The soils were sampled at 2 to 5 foot samples using Standard Penetration Testing (SPT) techniques. Shelby tube sampling and in-situ Vane Shear Testing (VST) was performed in softer silty clay soils. SPT blow counts and VST results are shown on the logs.

Soil samples obtained from the explorations were returned to our laboratory for further classification and testing. Atterberg Limits and moisture content test results are noted on the logs. The results of a one-dimensional laboratory consolidation test are attached as Sheet 9.

## **3.0 SITE AND SUBSURFACE CONDITIONS**

### **3.1 Surficial Conditions**

The site is currently developed with a single-story medical office building and associated paved areas. The site is relatively flat and level. The proposed building addition is

situated over existing paved and landscape area. The proposed parking area is situated over a undeveloped wooded area. Proposed and existing site features are shown on the "Exploration Location Plan" attached as Sheet 1.

### **3.2 Subsurface Conditions**

Test borings B-101 and B-102 were made in the area of the proposed building addition. These test borings encountered a subsurface profile generally consisting of approximately 3 inches of asphalt pavement overlying 2 to 3 feet of compacted granular fill overlying 11 to 13 feet of hard to stiff brown silty clay overlying 8 to 20 feet of medium to soft gray silty clay overlying sand and gravel overlying refusal surfaces (probable bedrock) at depths of 34 to 36 feet below the ground surface. The gray silty clay is soft and compressible with approximately 1,800 psf of over-consolidation. Vane shear testing performed in the softer gray silty clay indicates undrained shear strengths on the order of 590 to 890 psf.

Test boring B-103 was made in an area of a future building addition contemplated as an elevated single story building with on-grade parking below connecting to the proposed west building addition. This test boring encountered a subsurface profile similar to B-101 and B-102 and was terminated on a refusal surface (probable bedrock) at a depth of about 45 feet.

Test borings B-104 and B-105 were made in the area of proposed parking. These test borings encountered dramatically different subsurface conditions. B-104 encountered 4 feet of fill overlying native stiff brown silty clay; whereas B-105 encountered 15 feet of fill with ash, clinker and brick overlying relic bay mud.

Refer to the attached logs for more detailed descriptions of the subsurface findings.

### **3.3 Groundwater Conditions**

The soils were generally wet at depths of 2 to 3 feet. Infiltrated precipitation likely becomes perched on the relatively impervious native clay encountered at the test borings. Long term groundwater information is not available. It should be anticipated that seasonal groundwater levels will fluctuate, especially during periods of snowmelt and precipitation. Groundwater may be tidally influenced considering the proximity of the Fore River Bay.



### **3.4 Seismic and Frost Considerations**

The 25-year Air Freezing Index for the Portland, Maine area is about 1,290-Fahrenheit degree-days, which corresponds to a frost penetration depth on the order of 4.5 feet. Based on the findings at the explorations, we interpret the site soils to correspond to Seismic Soil Site Class D according to 2009 IBC.

## **4.0 EVALUATION AND RECOMMENDATIONS**

### **4.1 General Findings**

Based on the subsurface findings, the proposed construction appears feasible from a geotechnical standpoint. The principle geotechnical considerations are as follows:

- **Proposed Building Addition:** Spread footing foundations and a slab-on-grade floors bearing on properly prepared subgrades appear suitable for the proposed building addition. All existing pavement, structures, utilities, fill and loose, disturbed soils must be completely removed beneath the proposed building addition footprint. Footings should bear on at least 9-inches of compacted Crushed Stone wrapped in geotextile fabric overlying undisturbed native soils.
- **Future Building Addition with On-Grade Parking Below:** Test boring B-103, made for the future building addition, encountered a layer of silty sand with organics extending to a depth of about 5 feet below the ground surface. Footings for the future addition will need to penetrate this layer of soil and bear on undisturbed native hard brown silty clay. Additionally, the existing pavement, gravel and silty sand with organics may need to be removed and replaced with non-frost susceptible sand and gravel in order to mitigate frost action that could adversely affect low-overhead clearance for on-grade parking below the future building addition.
- **Proposed Parking Area:** The test borings made in the area of the proposed parking area encountered 4 to 15 feet of uncontrolled fill. The fill composition varied and contained ash, cinders and brick and may be environmentally impacted with premium handling and disposal costs. The existing fills may also require some overexcavation and replacement in order to support pavement loads.

#### **4.2 Site and Subgrade Preparation**

We recommend that site preparation begin with the construction of an erosion control system to protect adjacent drainage ways and areas outside the construction limits. As much vegetation as possible should remain outside the construction areas to lessen the potential for erosion and site disturbance.

We recommend that excavation to subgrade be completed with a smooth-edged bucket to help lessen disturbance of bearing soils. We recommend at least 9 inches of compacted Crushed Stone be provided below all footings. The Crushed Stone should be fully enveloped in non-woven geotextile, such as Mirafi 160N or equivalent.

All existing pavement, structures, utilities, disturbed soils and fills must be completely removed beneath the proposed building addition footprint until undisturbed native hard to very stiff brown silty clay soils are encountered. Overexcavation of unsuitable material should extend 1-foot laterally outward from edge of perimeter footings for every 1-foot of vertical excavation depth (1H:1V bearing splay). Excavations must not undermine existing foundations. Overexcavations should be backfilled to footing subgrade elevation with additional thickness of geotextile wrapped Crushed Stone or to slab-on-grade subgrade elevation with compacted Structural Fill.

#### **4.3 Excavation and Dewatering**

Excavation work will generally encounter pavement, sandy and clayey fill materials, and native silty clays. Care must be exercised during construction to minimize disturbance of the bearing soils. Final cuts to subgrade elevation should be performed with a smooth-edged bucket to help minimize soil disturbance.

Sumping and pumping dewatering techniques should be adequate to control groundwater in excavations. Controlling the water levels to at least one foot below planned excavation depths will help stabilize subgrades during construction.

Excavations must be properly shored and/or sloped to prevent sloughing and caving of the sidewalls during construction. Temporary, unsupported soil excavations should be sloped in accordance with the OSHA trenching regulations. Care must be taken to preclude undermining adjacent structures and utilities.

#### **4.4 Foundations**

We recommend the proposed building additions be supported on spread footings founded on at least 9-inches of crushed stone wrapped in geotextile fabric bearing on hard to very stiff, undisturbed native brown silty clay.

For foundations bearing on properly prepared subgrades, we recommend the following geotechnical parameters for design consideration:

- Design Frost Depth = 4.5 feet
- Allowable Soil Bearing Pressure = 3.0 ksf or less
- Seismic Soil Site Class = D (IBC 2009)
- Base Friction Factor = 0.40
- Total Unit Weight of Backfill = 130 pcf (compacted Structural Fill)
- Passive Lateral Earth Pressure Coefficient = 3.0 (compacted Structural Fill)
- At-Rest Lateral Earth Pressure Coefficient = 0.5 (compacted Structural Fill)
- Internal Friction Angle of Backfill = 30° (compacted Structural Fill)

Based on structural loading information, laboratory consolidation testing and anticipated grades, we estimate 1/2-inch or less of total post-construction settlement with differential settlement approaching 1/2-inch or less.

#### **4.5 Foundation Drainage**

We recommend an underdrain system be installed within the 9-inches of Crushed Stone wrapped in geotextile filter fabric recommended below the perimeter spread footings. The underdrain pipe should consist of 4-inch diameter, perforated SDR-35 foundation drain pipe enveloped in 9-inches of Crushed Stone wrapped in filter fabric, such as Mirafi 160N. The underdrain pipe must be connected to a positive gravity outlet protected from freezing, clogging and backflow.

Exterior foundation backfill should be sealed with a surficial layer of clayey or loamy soil in areas that are not paved or occupied by entrance slabs. This is to reduce direct surface water infiltration into the backfill. Surface grades should be sloped away from the building for positive surface water drainage. General underdrain details are provided on Sheet 10.

#### **4.6 Slab-On-Grade**

On-grade floor slabs in heated areas may be designed using a subgrade reaction modulus of 150 pci (pounds per cubic inch) provided the slab is underlain by at least 12-inches of compacted Structural Fill placed over properly prepared subgrades. The structural engineer or concrete consultant must design steel reinforcing and joint spacing appropriate to slab thickness and function.

We recommend a sub-slab vapor retarder particularly in areas of the building where the concrete slab will be covered with an impermeable surface treatment or floor covering that may be sensitive to moisture vapors. The vapor retarder must have a permeance that is less than the floor cover or surface treatment that is applied to the slab. The vapor retarder must have sufficient durability to withstand direct contact with the sub-slab base material and construction activity. The vapor retarder material shall be placed according to the manufacturer's recommended method, including the taping and lapping of all joints and wall connections. The architect and/or flooring consultant should select the vapor retarder products compatible with flooring and adhesive materials.

The floor slab should be appropriately cured using moisture retention methods after casting. Typical floor slab curing methods should be used for at least 7 days. The architect or flooring consultant should assign curing methods consistent with current applicable American Concrete Institute (ACI) procedures with consideration of curing method compatibility to proposed surface treatments, flooring and adhesive materials.

#### **4.7 Entrance Slabs and Sidewalks**

Entrance slabs and sidewalks adjacent to buildings must be designed to reduce the effects of differential frost action between adjacent pavement, doorways, and entrances. We recommend that clean, non-frost susceptible Structural Fill be provided to a depth of at least 4.5 feet below the top of entrance slabs. This thickness of Structural Fill should extend the full width of the entrance slabs and outward at least 4.5 feet, thereafter transitioning up to the bottom of the adjacent sidewalk or pavement subbase gravel at a 3H:1V or flatter slope. General details of this frost transition zone are attached as Sheet 10.

#### **4.8 Backfill and Compaction**

The on-site soils are unsuitable for use in building and paved areas, but may be reused in landscape areas. For building and paved areas, we recommend the following fill and backfill materials:

Structural Fill: Fill to raise site grades and backfill for foundations should be clean, non-frost susceptible sand and gravel meeting the gradation requirements for Structural Fill as given below.

| <b>Structural Fill</b> |                                |
|------------------------|--------------------------------|
| <b>Sieve Size</b>      | <b>Percent Finer by Weight</b> |
| 4 inch                 | 100                            |
| 3 inch                 | 90 to 100                      |
| ¼ inch                 | 25 to 90                       |
| #40                    | 0 to 30                        |
| #200                   | 0 to 5                         |

Structural Fill is recommended for use as:

- Fill and backfill to raise grades in building areas
- Backfill for overexcavations
- Backfill against foundations
- Backfill within frost transition zones below entrances and sidewalks
- Minimum 12-inch thick layer below slab-on-grade

Crushed Stone: Crushed Stone, used beneath foundations and for underdrain aggregate, should meet the gradation requirements of MDOT Standard Specifications 703.22 "Underdrain Backfill Type C".

Placement and Compaction: Fill should be placed in horizontal lifts and compacted such that the desired density is achieved throughout the lift thickness with 3 to 5 passes of the compaction equipment. Loose lift thicknesses for grading, fill and backfill activities should not exceed 12 inches. We recommend that fill and backfill in building areas be compacted to at least 95 percent of its maximum dry density as determined by ASTM D-1557. Crushed Stone should be compacted with 3 to 5 passes of a vibratory plate compactor having a static weight of at least 500 pounds.

#### **4.9 Weather Considerations**

Construction activity should be limited during wet weather and the site soils may require drying before construction activities may continue. The contractor should anticipate the need for water to temper fills in order to facilitate compaction during dry weather. If construction takes place during cold weather, subgrades, foundations and floor slabs must be protected during freezing conditions. Concrete and fill must not be placed on frozen soil; and once placed, the concrete and soil beneath the structure must be protected from freezing.

#### **4.10 Design Review and Construction Testing**

S.W.COLE ENGINEERING, INC. should be retained to review the final design and specifications to determine that our earthwork and foundation recommendations have been properly interpreted and implemented.

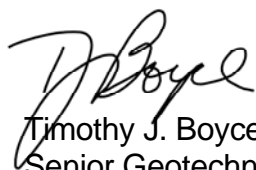
A soils and concrete testing program should also be implemented during construction to observe compliance with the design concepts, plans, and specifications. S.W.COLE ENGINEERING, INC. is available to provide subgrade observations for foundations as well as testing services for soils, concrete, asphalt, steel and spray-applied fireproofing construction materials.

#### **5.0 CLOSURE**

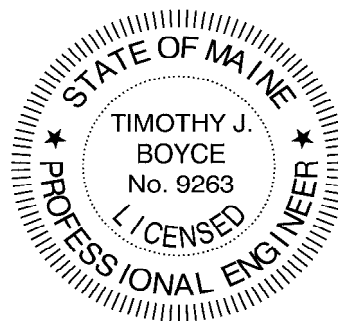
It has been a pleasure to be of assistance to you with this phase of your project. We look forward to working with you during the construction phase of the project.

Sincerely,

**S.W.COLE ENGINEERING, INC.**



Timothy J. Boyce, P.E.  
Senior Geotechnical Engineer



TJB:pfk

## **Attachment A Limitations**

This report has been prepared for the exclusive use of Eyecare Medical Group, P.A. for specific application to the proposed Building Addition and Parking Lot at 53 Sewall Street, Portland, Maine. S.W.COLE ENGINEERING, INC. has endeavored to conduct the work in accordance with generally accepted soil and foundation engineering practices. No warranty, expressed or implied, is made.

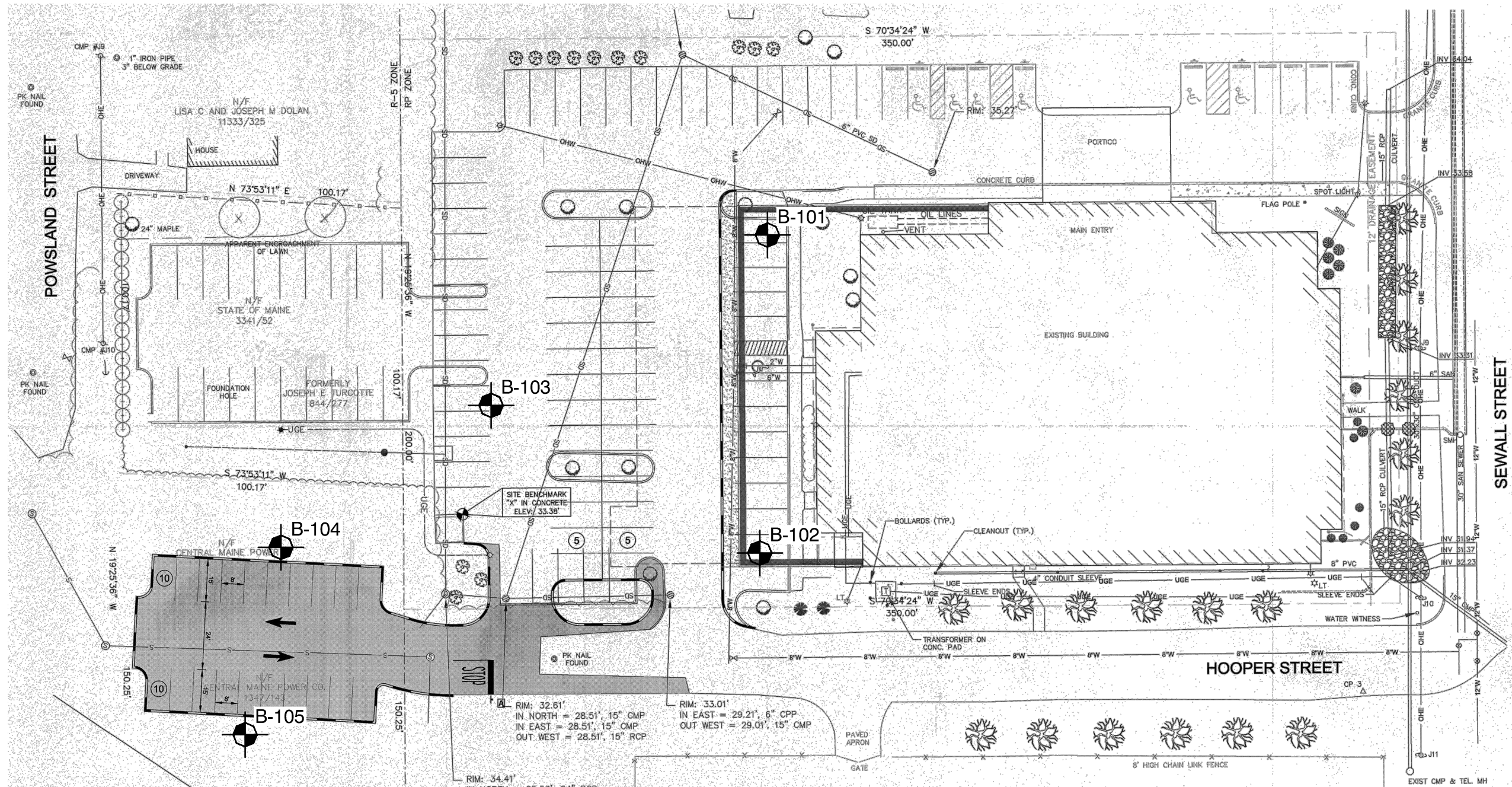
The soil profiles described in the report are intended to convey general trends in subsurface conditions. The boundaries between strata are approximate and are based upon interpretation of exploration data and samples.

The analyses performed during this investigation and recommendations presented in this report are based in part upon the data obtained from subsurface explorations made at the site. Variations in subsurface conditions may occur between explorations and may not become evident until construction. If variations in subsurface conditions become evident after submission of this report, it will be necessary to evaluate their nature and to review the recommendations of this report.

Observations have been made during exploration work to assess site groundwater levels. Fluctuations in water levels will occur due to variations in rainfall, temperature, and other factors.

S.W.COLE ENGINEERING, INC.'s scope of work has not included the investigation, detection, or prevention of any Biological Pollutants at the project site or in any existing or proposed structure at the site. The term "Biological Pollutants" includes, but is not limited to, molds, fungi, spores, bacteria, and viruses, and the byproducts of any such biological organisms.

Recommendations contained in this report are based substantially upon information provided by others regarding the proposed project. In the event that any changes are made in the design, nature, or location of the proposed project, S.W.COLE ENGINEERING, INC. should review such changes as they relate to analyses associated with this report. Recommendations contained in this report shall not be considered valid unless the changes are reviewed by S.W.COLE ENGINEERING, INC.



**LEGEND:**

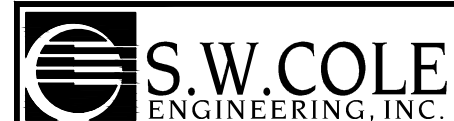
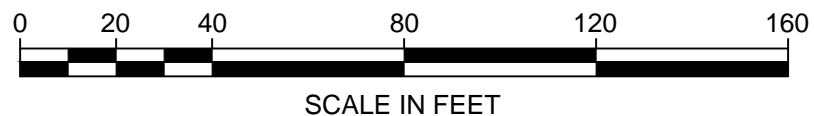


APPROXIMATE BORING LOCATION

**NOTES:**

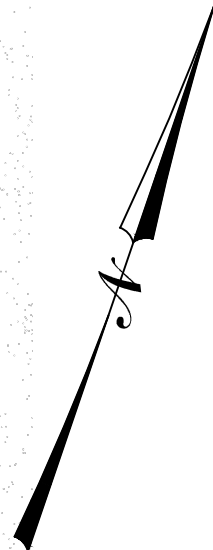
1. EXPLORATION LOCATION PLAN WAS PREPARED FROM A 1"=20' SCALE PLAN OF THE SITE ENTITLED "SITE LAYOUT PLAN," PREPARED BY DELUCA-HOFFMAN ASSOCIATES, INC., DATED MAY 2013, AND PROVIDED AS A PORTABLE DOCUMENT FORMAT (PDF).
2. THE BORINGS WERE LOCATED IN THE FIELD BY TAPED MEASUREMENTS FROM EXISTING SITE FEATURES.

3. THIS PLAN SHOULD BE USED IN CONJUNCTION WITH THE ASSOCIATED S.W. COLE ENGINEERING, INC. GEOTECHNICAL REPORT.
4. THE PURPOSE OF THIS PLAN IS ONLY TO DEPICT THE LOCATION OF THE EXPLORATIONS IN RELATION TO THE EXISTING CONDITIONS AND PROPOSED CONSTRUCTION AND IS NOT TO BE USED FOR CONSTRUCTION.



EYE CARE MEDICAL GROUP  
**EXPLORATION LOCATION PLAN**  
 PROPOSED BUILDING ADDITION AND PARKING LOT  
 53 SEWALL STREET  
 PORTLAND, MAINE

Job No.: 13-0392      Scale: 1" = 40'  
 Date: 06/03/2013      Sheet: 1







# BORING LOG

BORING NO.: **B-101**  
 SHEET: OF  
 PROJECT NO.: 13-0392  
 DATE START: 5/24/2013  
 DATE FINISH: 5/24/2013  
 ELEVATION: NOT AVAILABLE  
 SWC REP.: PJO

PROJECT / CLIENT: PROPOSED BUILDING ADDITION AND PARKING LOT / EYE CARE MEDICAL GROUP  
 LOCATION: 53 SEWALL STREET / PORTLAND MAINE  
 DRILLING CO.: NORTHERN TEST BORINGS, INC. DRILLER: MIKE NADEAU

|              | TYPE   | SIZE I.D. | HAMMER WT. | HAMMER FALL |
|--------------|--------|-----------|------------|-------------|
| CASING:      | SSA/HW | 4"        | 140 LBS.   | 30"         |
| SAMPLER:     | SS     | 1 3/8"    | 140 LBS.   | 30"         |
| CORE BARREL: |        |           |            |             |

WATER LEVEL INFORMATION  
 SOILS SATURATED BELOW 14' +/-

| CASING BLOWS PER FOOT | SAMPLE |               |       |             | SAMPLER BLOWS PER 6" |      |       |       | DEPTH | STRATA & TEST DATA   |   |
|-----------------------|--------|---------------|-------|-------------|----------------------|------|-------|-------|-------|--|---|
|                       | NO.    | PEN.          | REC.  | DEPTH @ BOT | 0-6                  | 6-12 | 12-18 | 18-24 |       |  |   |
|                       | 1D     | 24"           | 10"   | 2.3'        | 5                    | 4    | 4     | 3     | 3.0'  | 3 1/2" ASPHALT<br>BROWN GRAVELLY SAND SOME SILT (FILL)<br>~LOOSE~  |   |
|                       | 2D     | 24"           | 22"   | 7.0'        | 3                    | 6    | 7     | 6     | 14.0' | BROWN MOTTLED DARK BROWN SILTY CLAY<br>w = 29.3% $q_p = 8.0-9.0$ KSF<br>~HARD BECOMING...<br><br>WITH OCCASIONAL FINE SAND SEAMS<br>...STIFF~ $q_p = 5.0$ KSF<br>w = 36.0% $q_p = 3.0$ KSF |   |
|                       | 3D     | 24"           | 22"   | 12.0'       | 3                    | 4    | 3     | 4     |       |  |   |
|                       | 1V     | 3.5 X 7" VANE | 15.6' |             |                      |      |       |       |       | 34.7'  | $S_v = 0.70$ KSF / 0.11 KSF<br>$S_v = 0.78$ KSF / 0.12 KSF<br><br>~MEDIUM~<br><br>GRAY SILTY CLAY<br><br>$W_L = 36$ $W_p = 19$ w = 38.9%  |
|                       | 1V'    | 3.5 X 7" VANE | 16.2' |             |                      |      |       |       | 35.9' |  | $S_v = 0.59$ KSF / 0.09 KSF<br>$S_v = 0.64$ KSF / 0.12 KSF<br><br>$S_v = 1.1$ KSF<br>PROBABLE SAND SEAM<br><br>HYDRAULIC PUSH ROD PROBE FROM 25.6' TO 34.7'<br><br>PROBABLE SAND SEAM |
|                       | 1S     | 24"           | 18"   | 22.0'       | HYDRAULIC PUSH       |      |       |       |       |  |   |
|                       | 2V     | 3.5 X 7" VANE | 22.6' |             |                      |      |       |       |       | REFUSAL AT 35.9'<br>(PROBABLE BEDROCK)   |   |
|                       | 2V'    | 3.5 X 7" VANE | 23.2' |             |                      |      |       |       |       |  |   |
|                       | 3V     | 3.5 X 7" VANE | 25.6' |             |                      |      |       |       |       |  |   |

SAMPLES:  
 D = SPLIT SPOON  
 C = 2" SHELBY TUBE  
 S = 3" SHELBY TUBE  
 U = 3.5" SHELBY TUBE

SOIL CLASSIFIED BY:  
 DRILLER - VISUALLY  
 SOIL TECH. - VISUALLY  
 LABORATORY TEST

REMARKS:  
 STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY BETWEEN SOIL TYPES AND THE TRANSITION MAY BE GRADUAL.

2

BORING NO.: **B-101**



# BORING LOG

BORING NO.: **B-102**  
 SHEET: 1 OF 1  
 PROJECT NO.: 13-0392  
 DATE START: 5/24/2013  
 DATE FINISH: 5/24/2013  
 ELEVATION: NOT AVAILABLE  
 SWC REP.: PJO

PROJECT / CLIENT: PROPOSED BUILDING ADDITION AND PARKING LOT / EYE CARE MEDICAL GROUP  
 LOCATION: 53 SEWALL STREET / PORTLAND MAINE  
 DRILLING CO.: NORTHERN TEST BORINGS, INC. DRILLER: MIKE NADEAU

|              | TYPE     | SIZE I.D. | HAMMER WT. | HAMMER FALL |
|--------------|----------|-----------|------------|-------------|
| CASING:      | SSA / HW | 4"        | 140 LBS.   | 30"         |
| SAMPLER:     | SS       | 1 3/8"    | 140 LBS.   | 30"         |
| CORE BARREL: |          |           |            |             |

WATER LEVEL INFORMATION  
 SOILS SATURATED BELOW 15' +/-

| CASING BLOWS PER FOOT | SAMPLE |               |      |             | SAMPLER BLOWS PER 6" |      |       |       | DEPTH | STRATA & TEST DATA  |  |
|-----------------------|--------|---------------|------|-------------|----------------------|------|-------|-------|-------|---|--|
|                       | NO.    | PEN.          | REC. | DEPTH @ BOT | 0-6                  | 6-12 | 12-18 | 18-24 |       |   |  |
|                       |        |               |      |             |                      |      |       |       | 2.1'  | 2 3/4" ASPHALT<br>BROWN GRAVELLY SILTY SAND WITH CLAY LAYERS BELOW 1.7'   |  |
|                       | 1D     | 24"           | 14"  | 2.3'        | 6                    | 6    | 5     | 4     |       |   |  |
|                       | 2D     | 24"           | 12"  | 4.3'        | 3                    | 5    | 6     | 8     |       | ~HARD BECOMING... q <sub>p</sub> = 8.0 KSF  |  |
|                       | 3D     | 24"           | 22"  | 7.0'        | 4                    | 5    | 7     | 8     |       | BROWN MOTTLED SILTY CLAY q <sub>p</sub> = 9.0 KSF<br>WITH OCCASIONAL GRAY SILT SEAMS                                  |  |
|                       | 4D     | 24"           | 15"  | 9.0'        | 5                    | 6    | 9     | 8     |       | q <sub>p</sub> = 8.5 KSF  |  |
|                       | 5D     | 24"           | 24"  | 12.0'       | 3                    | 4    | 4     | 5     |       | ...STIFF~ q <sub>p</sub> = 4.0 KSF<br>q <sub>p</sub> = 3.0 KSF  |  |
|                       |        |               |      |             |                      |      |       |       | 15.0' |   |  |
|                       | 1V     | 2.5 X 5" VANE |      | 15.4'       |                      |      |       |       | 15.8' | S <sub>v</sub> = 1.12 KSF / 0.11 KSF GRAYISH-BROWN SILTY CLAY   |  |
|                       | 1V'    | 2.5 X 5" VANE |      | 15.8'       |                      |      |       |       |       | S <sub>v</sub> = 0.90 KSF / 0.08 KSF ~STIFF TO MEDIUM~  |  |
|                       | 6D     | 24"           | 20"  | 17.8'       | HYDRAULIC PUSH       |      |       |       |       |   |  |
|                       | 1S     | 24"           | 20"  | 20.0'       | HYDRAULIC PUSH       |      |       |       |       |   |  |
|                       | 2V     | 3.5 X 7" VANE |      | 20.6'       |                      |      |       |       | 23.5' | S <sub>v</sub> = 0.83 KSF / 0.13 KSF GRAY SILTY CLAY<br>W <sub>L</sub> = 39 W <sub>P</sub> = 19 w = 37.0%<br>~MEDIUM~ |  |
|                       | 2V'    | 3.5 X 7" VANE |      | 21.2'       |                      |      |       |       |       | S <sub>v</sub> = 0.89 KSF / 0.15 KSF  |  |
|                       |        |               |      |             |                      |      |       |       | 23.5' |   |  |
|                       | 7D     | 24"           | 15"  | 26.0'       | 24                   | 32   | 24    | 23    |       | BROWN SAND AND GRAVEL SOME SILT<br>~VERY DENSE~<br>ADVANCED BY ROLLER CONE FROM 26' TO 34'                            |  |
|                       |        |               |      |             |                      |      |       |       | 34.0' |   |  |
|                       |        |               |      |             |                      |      |       |       |       | REFUSAL AT 34.0'<br>(PROBABLE BEDROCK)  |  |

SAMPLES:  
 D = SPLIT SPOON  
 C = 2" SHELBY TUBE  
 S = 3" SHELBY TUBE  
 U = 3.5" SHELBY TUBE

SOIL CLASSIFIED BY:  
 DRILLER - VISUALLY  
 SOIL TECH. - VISUALLY  
 LABORATORY TEST

REMARKS:  
 STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY BETWEEN SOIL TYPES AND THE TRANSITION MAY BE GRADUAL.



# BORING LOG

BORING NO.: **B-103**  
 SHEET: 1 OF 2  
 PROJECT NO.: 13-0392  
 DATE START: 5/24/2013  
 DATE FINISH: 5/24/2013  
 ELEVATION: NOT AVAILABLE  
 SWC REP.: PJO

PROJECT / CLIENT: PROPOSED BUILDING ADDITION AND PARKING LOT / EYE CARE MEDICAL GROUP  
 LOCATION: 53 SEWALL STREET / PORTLAND MAINE  
 DRILLING CO.: NORTHERN TEST BORINGS, INC. DRILLER: MIKE NADEAU

|              | TYPE     | SIZE I.D. | HAMMER WT. | HAMMER FALL |
|--------------|----------|-----------|------------|-------------|
| CASING:      | SSA / HW | 4"        | 140 LBS.   | 30"         |
| SAMPLER:     | SS       | 1 3/8"    | 140 LBS.   | 30"         |
| CORE BARREL: |          |           |            |             |

WATER LEVEL INFORMATION  
 SOILS WET AT 2.2' +/-  
 SOILS SATURATED BELOW 15' +/-

| CASING BLOWS PER FOOT | SAMPLE |               |       |             | SAMPLER BLOWS PER 6" |      |       |       | DEPTH  | STRATA & TEST DATA  |
|-----------------------|--------|---------------|-------|-------------|----------------------|------|-------|-------|--|---|
|                       | NO.    | PEN.          | REC.  | DEPTH @ BOT | 0-6                  | 6-12 | 12-18 | 18-24 |  |   |
|                       |        |               |       |             |                      |      |       |       | 2.2'   | 2 1/2" ASPHALT<br>BROWN GRAVELLY SAND SOME SILT (FILL) ~MEDIUM DENSE~   |
|                       | 1D     | 24"           | 12"   | 2.2'        | 5                    | 9    | 10    | 9     | 5.0'   | GRAYISH-BROWN SILTY SAND<br>WITH ORGANIC SILT AND DARK GRAY CLAY LAYERS (DISTURBED)<br>~LOOSE~                              |
|                       | 2D     | 24"           | 16"   | 4.2'        | 5                    | 2    | 3     | 2     |  |   |
|                       | 3D     | 24"           | 20"   | 7.0'        | 3                    | 3    | 4     | 4     | 7.0'   | GRAYISH-BROWN SILTY CLAY WITH<br>FREQUENT FINE SAND SEAMS ~VERY STIFF~ q <sub>p</sub> = 7.0 KSF                             |
|                       | 4D     | 24"           | 14"   | 9.0'        | 3                    | 5    | 7     | 7     | 14.5'  | BROWN MOTTLED DARK BROWN SILTY CLAY q <sub>p</sub> = 8.0 KSF<br>~HARD BECOMING...<br><br>...STIFF~ q <sub>p</sub> = 7.0 KSF |
|                       | 5D     | 24"           | 22"   | 12.0'       | 4                    | 5    | 6     | 8     |  |   |
|                       | 6D     | 24"           | 24"   | 17.0'       | 2                    | 2    | 2     | 2     |  | GRAY SILTY CLAY q <sub>p</sub> = ≤ 0.5 KSF  |
|                       | 1V     | 3.5 X 7" VANE | 20.6' |             |                      |      |       |       | S <sub>v</sub> = 0.75 KSF / 0.13 KSF<br>S <sub>v</sub> = 0.67 KSF / 0.12 KSF | ~MEDIUM~  |
|                       | 1V'    | 3.5 X 7" VANE | 21.2' |             |                      |      |       |       |  |   |
|                       | 2V     | 3.5 X 7" VANE | 25.6' |             |                      |      |       |       | S <sub>v</sub> = 0.68 KSF / 0.13 KSF<br>S <sub>v</sub> = 0.78 KSF / 0.13 KSF |   |
|                       | 2V'    | 3.5 X 7" VANE | 26.2' |             |                      |      |       |       |  |   |
|                       |        |               |       |             |                      |      |       |       |  | HYDRAULIC PUSH ROD PROBE 26.2' TO 43.4'   |

SAMPLES:  
 D = SPLIT SPOON  
 C = 2" SHELBY TUBE  
 S = 3" SHELBY TUBE  
 U = 3.5" SHELBY TUBE

SOIL CLASSIFIED BY:  
 DRILLER - VISUALLY  
 SOIL TECH. - VISUALLY  
 LABORATORY TEST

REMARKS:  
 STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY BETWEEN SOIL TYPES AND THE TRANSITION MAY BE GRADUAL.

4

BORING NO.: **B-103**





# BORING LOG

BORING NO.: **B-104**  
 SHEET: 1 OF 1  
 PROJECT NO.: 13-0392  
 DATE START: 5/24/2013  
 DATE FINISH: 5/24/2013  
 ELEVATION: NOT AVAILABLE  
 SWC REP.: PJO

PROJECT / CLIENT: PROPOSED BUILDING ADDITION AND PARKING LOT / EYE CARE MEDICAL GROUP  
 LOCATION: 53 SEWALL STREET / PORTLAND MAINE  
 DRILLING CO.: NORTHERN TEST BORINGS, INC. DRILLER: MIKE NADEAU

|              | TYPE | SIZE I.D. | HAMMER WT. | HAMMER FALL |
|--------------|------|-----------|------------|-------------|
| CASING:      | SSA  | 4" O.D.   | 140 LBS.   | 30"         |
| SAMPLER:     | SS   | 1 3/8"    | 140 LBS.   | 30"         |
| CORE BARREL: |      |           |            |             |

WATER LEVEL INFORMATION  
 FILL SOILS WET AT 2.0' +/-

| CASING BLOWS PER FOOT | SAMPLE |      |      |             | SAMPLER BLOWS PER 6" |      |       |       | DEPTH | STRATA & TEST DATA   |
|-----------------------|--------|------|------|-------------|----------------------|------|-------|-------|-------|--|
|                       | NO.    | PEN. | REC. | DEPTH @ BOT | 0-6                  | 6-12 | 12-18 | 18-24 |       |  |
|                       | 1D     | 24"  | 18"  | 2.0'        | 2                    | 3    | 3     | 4     | 4.0'  | GRAYISH-BROWN CLAYEY SANDY SILT WITH ORGANICS (REWORKED FILL) ~LOOSE~        |
|                       | 2D     | 24"  | 12"  | 4.0'        | 4                    | 3    | 2     | 2     |       |  |
|                       | 3D     | 24"  | 18"  | 6.0'        | 6                    | 5    | 6     | 7     | 6.0'  | BROWN MOTTLED DARK BROWN SILTY CLAY ~VERY STIFF~<br>q <sub>p</sub> = 6.0 KSF |
|                       |        |      |      |             |                      |      |       |       |       | BOTTOM OF EXPLORATION AT 6.0'  |
|                       |        |      |      |             |                      |      |       |       |       |  |
|                       |        |      |      |             |                      |      |       |       |       |  |
|                       |        |      |      |             |                      |      |       |       |       |  |
|                       |        |      |      |             |                      |      |       |       |       |  |
|                       |        |      |      |             |                      |      |       |       |       |  |
|                       |        |      |      |             |                      |      |       |       |       |  |
|                       |        |      |      |             |                      |      |       |       |       |  |
|                       |        |      |      |             |                      |      |       |       |       |  |
|                       |        |      |      |             |                      |      |       |       |       |  |
|                       |        |      |      |             |                      |      |       |       |       |  |
|                       |        |      |      |             |                      |      |       |       |       |  |
|                       |        |      |      |             |                      |      |       |       |       |  |
|                       |        |      |      |             |                      |      |       |       |       |  |
|                       |        |      |      |             |                      |      |       |       |       |  |

SAMPLES:  
 D = SPLIT SPOON  
 C = 2" SHELBY TUBE  
 S = 3" SHELBY TUBE  
 U = 3.5" SHELBY TUBE

SOIL CLASSIFIED BY:  
 DRILLER - VISUALLY  
 SOIL TECH. - VISUALLY  
 LABORATORY TEST

REMARKS:  
 STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY BETWEEN SOIL TYPES AND THE TRANSITION MAY BE GRADUAL.



# BORING LOG

BORING NO.: **B-105**  
 SHEET: 1 OF 1  
 PROJECT NO.: 13-0392  
 DATE START: 5/24/2013  
 DATE FINISH: 5/24/2013  
 ELEVATION: NOT AVAILABLE  
 SWC REP.: PJO

PROJECT / CLIENT: PROPOSED BUILDING ADDITION AND PARKING LOT / EYE CARE MEDICAL GROUP  
 LOCATION: 53 SEWALL STREET / PORTLAND MAINE  
 DRILLING CO.: NORTHERN TEST BORINGS, INC. DRILLER: MIKE NADEAU

|              | TYPE | SIZE I.D. | HAMMER WT. | HAMMER FALL |
|--------------|------|-----------|------------|-------------|
| CASING:      | SSA  | 4" O.D.   | 140 LBS.   | 30"         |
| SAMPLER:     | SS   | 1 3/8"    | 140 LBS.   | 30"         |
| CORE BARREL: |      |           |            |             |

WATER LEVEL INFORMATION  
 FILL SOILS WET AT 2' +/-

| CASING BLOWS PER FOOT | SAMPLE |      |      |             | SAMPLER BLOWS PER 6" |      |       |       | DEPTH | STRATA & TEST DATA  |
|-----------------------|--------|------|------|-------------|----------------------|------|-------|-------|-------|---|
|                       | NO.    | PEN. | REC. | DEPTH @ BOT | 0-6                  | 6-12 | 12-18 | 18-24 |       |   |
|                       | 1D     | 24"  | 15"  | 2.0'        | 2                    | 2    | 3     | 4     | 4.0'  | BROWN SAND AND SILT SOME GRAVEL WITH TRACE AMOUNTS OF CLINKER, CHARCOAL, BRICK AND ASH (FILL)<br>~LOOSE~      |
|                       | 2D     | 24"  | 14"  | 4.0'        | 5                    | 5    | 5     | 5     |       |   |
|                       | 3D     | 24"  | 15"  | 7.0'        | 3                    | 4    | 6     | 3     | 15.0' | GRAY-BROWN CLAYEY SILT TRACE GRAVEL WITH ORGANICS (FILL)<br>~LOOSE~<br><br>~LOOSE~<br>PUSHING PIECE OF GRAVEL |
|                       | 4D     | 24"  | 7"   | 12.0'       | 4                    | 4    | 5     | 5     |       |   |
|                       | 5D     | 5"   | 5"   | 12.4'       | 35/5"                |      |       |       |       |   |
|                       | 6D     | 24"  | 20"  | 17.0'       | 4                    | 4    | 5     | 5     | 16.4' | BROWN MEDIUM TO COARSE SAND ~LOOSE~   |
|                       |        |      |      |             |                      |      |       |       | 17.0' | GRAY SANDY SILT SOME CLAY WITH ORGANICS (BAY DEPOSIT)   |
|                       |        |      |      |             |                      |      |       |       |       | BOTTOM OF EXPLORATION AT 17.0'  |

|   |   |   |   |
|---|---|---|---|
| SAMPLES:<br>D = SPLIT SPOON<br>C = 2" SHELBY TUBE<br>S = 3" SHELBY TUBE<br>U = 3.5" SHELBY TUBE | SOIL CLASSIFIED BY:<br><input type="checkbox"/> DRILLER - VISUALLY<br><input checked="" type="checkbox"/> SOIL TECH. - VISUALLY<br><input type="checkbox"/> LABORATORY TEST | REMARKS:<br>STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY BETWEEN SOIL TYPES AND THE TRANSITION MAY BE GRADUAL. | <div style="border: 1px solid black; border-radius: 50%; width: 30px; height: 30px; display: flex; align-items: center; justify-content: center; margin: 0 auto;">7</div> |
|   |   |   | BORING NO.: <b>B-105</b>  |



## **KEY TO THE NOTES & SYMBOLS**

### **Test Boring and Test Pit Explorations**

All stratification lines represent the approximate boundary between soil types and the transition may be gradual.

#### **Key to Symbols Used:**

|                |   |  |
|----------------|---|--|
| w              | - | water content, percent (dry weight basis)  |
| q <sub>u</sub> | - | unconfined compressive strength, kips/sq. ft. - based on laboratory unconfined compressive test                |
| S <sub>v</sub> | - | field vane shear strength, kips/sq. ft.  |
| L <sub>v</sub> | - | lab vane shear strength, kips/sq. ft.  |
| q <sub>p</sub> | - | unconfined compressive strength, kips/sq. ft. based on pocket penetrometer test                                |
| O              | - | organic content, percent (dry weight basis)  |
| W <sub>L</sub> | - | liquid limit - Atterberg test  |
| W <sub>P</sub> | - | plastic limit - Atterberg test   |
| WOH            | - | advance by weight of hammer  |
| WOM            | - | advance by weight of man   |
| WOR            | - | advance by weight of rods  |
| HYD            | - | advance by force of hydraulic piston on drill  |
| RQD            | - | Rock Quality Designator - an index of the quality of a rock mass. RQD is computed from recovered core samples. |
| γ <sub>T</sub> | - | total soil weight  |
| γ <sub>B</sub> | - | buoyant soil weight  |
| f              | - | finer content (percent by weight passing U.S. No. 200 Sieve)   |

#### **Description of Proportions:**

0 to 5% TRACE  
5 to 12% SOME  
12 to 35% "Y"  
35+% AND

**REFUSAL: Test Boring Explorations** - Refusal depth indicates that depth at which, in the drill foreman's opinion, sufficient resistance to the advance of the casing, auger, probe rod or sampler was encountered to render further advance impossible or impracticable by the procedures and equipment being used.

**REFUSAL: Test Pit Explorations** - Refusal depth indicates that depth at which sufficient resistance to the advance of the backhoe bucket was encountered to render further advance impossible or impracticable by the procedures and equipment being used.

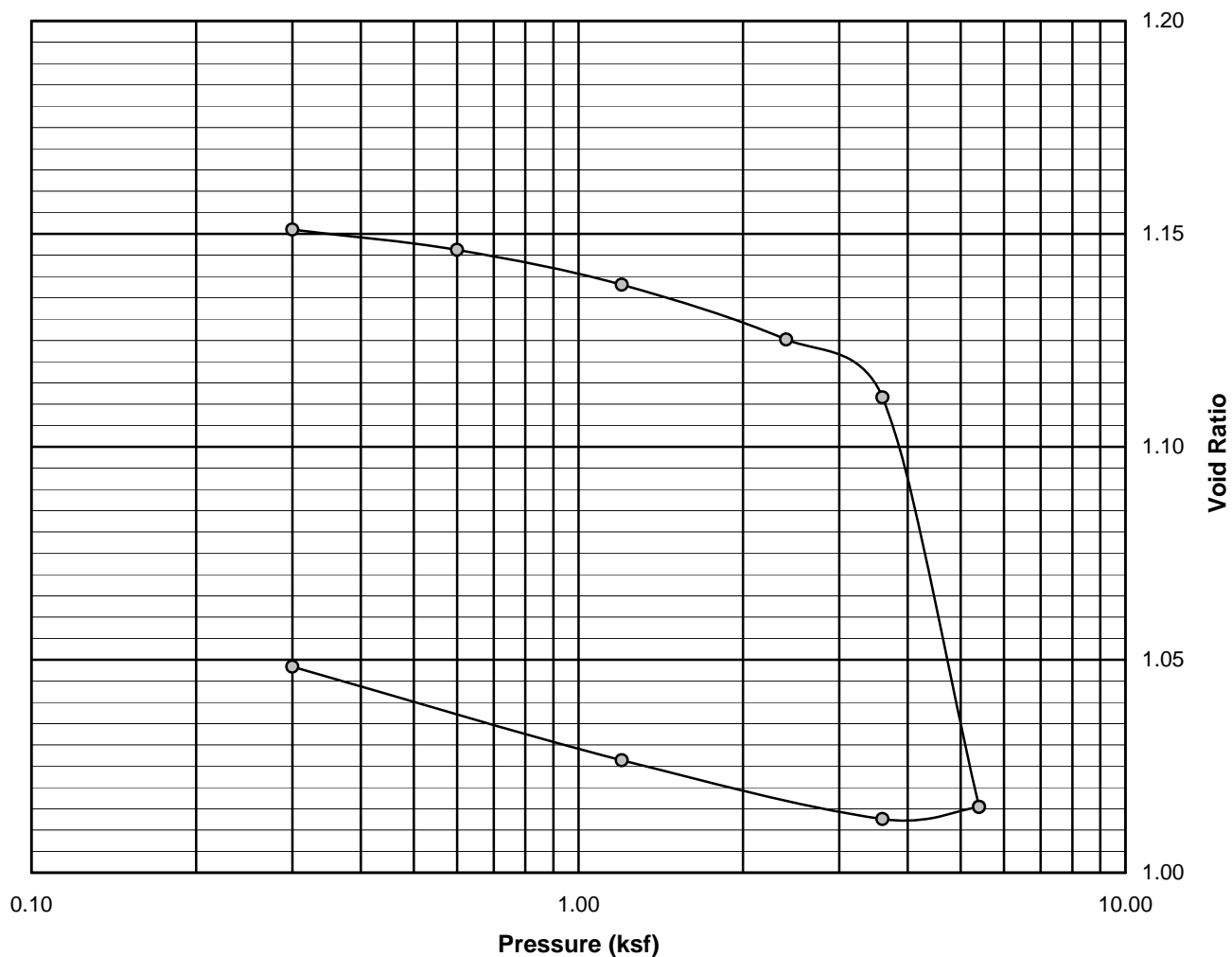
Although refusal may indicate the encountering of the bedrock surface, it may indicate the striking of large cobbles, boulders, very dense or cemented soil, or other buried natural or man-made objects or it may indicate the encountering of a harder zone after penetrating a considerable depth through a weathered or disintegrated zone of the bedrock.

Project Name Portland - Building Addition and Parking  
Client Eyecare Medical Group, P.A.

Project Number 13-0392  
Lab ID 16142B  
Date 6/5/2013  
Date Complete 6/24/2013

Boring B-101  
Sample 1S  
Depth 20'-22'

|                             |
|-----------------------------|
| $P_C = 3.5 \text{ KSF +/-}$ |
| $C_C = 0.04$                |
| $C_R = 0.55$                |
| $w = 38.9\%$                |
| $W_L = 36$                  |
| $W_P = 19$                  |



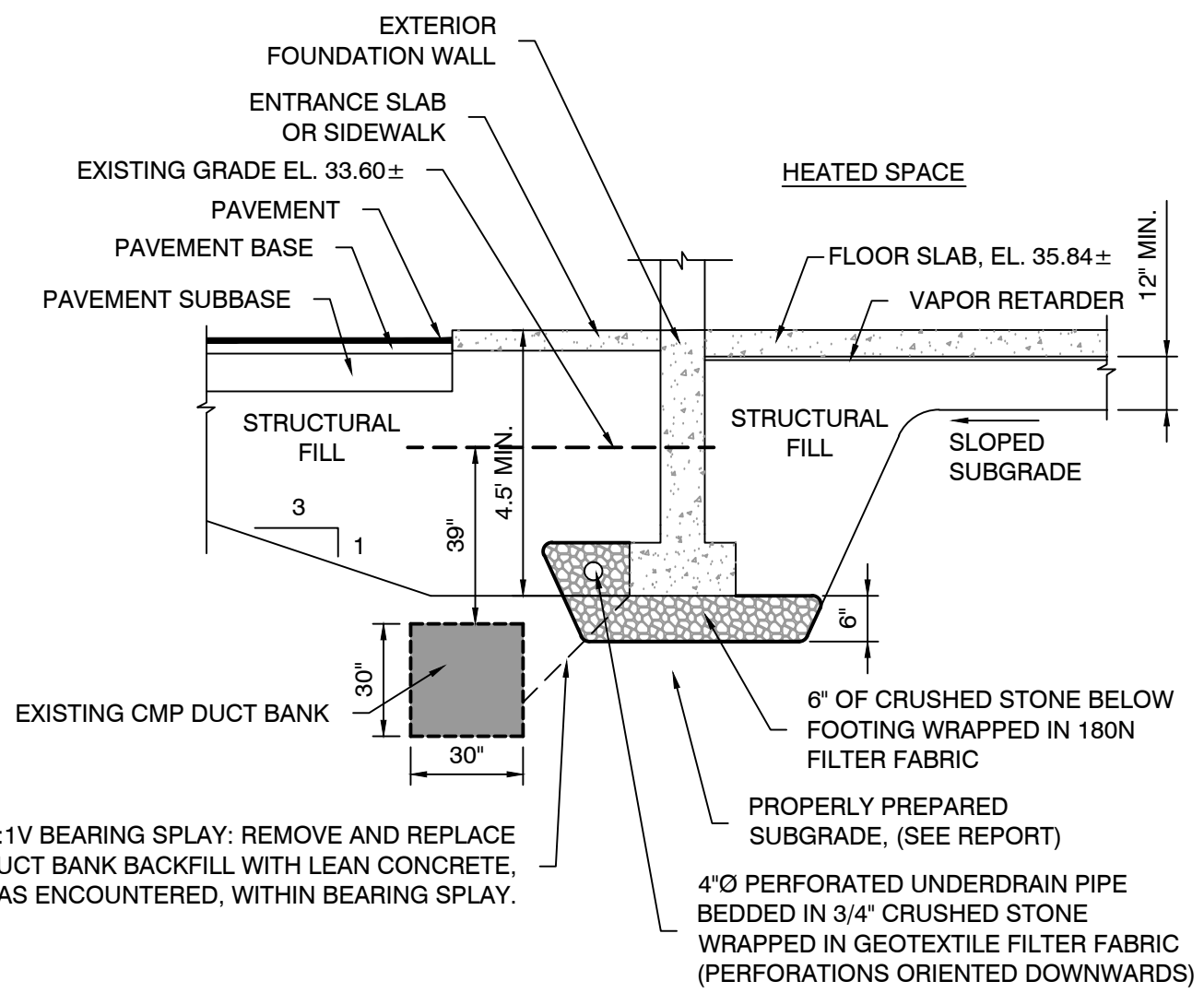
Comments:

EMW

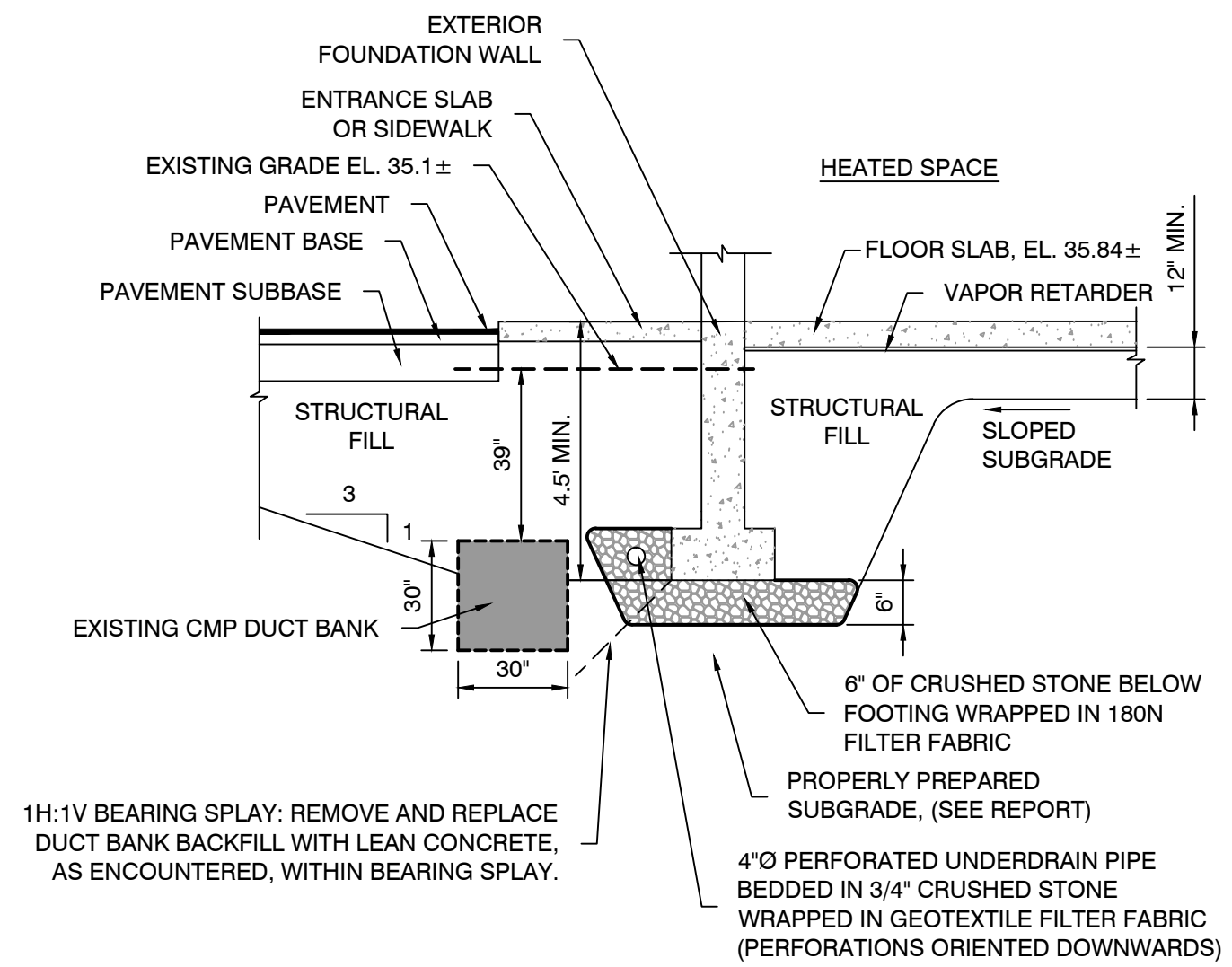
Reviewed By



R:\2013\13-0392\CAD\Drawings\13-0392 Sheet 10 UD.dwg, 6/12/2013 3:08:37 PM, 1:1, CEM, S.W. Cole Engineering, Inc.



**SOUTHWEST BUILDING CORNER**




**NORTHWEST BUILDING CORNER**

**NOTE:**

1. UNDERDRAIN INSTALLATION AND MATERIAL GRADATION RECOMMENDATIONS ARE CONTAINED WITHIN THIS REPORT.
2. DETAIL IS PROVIDED FOR ILLUSTRATIVE PURPOSES ONLY, NOT FOR CONSTRUCTION.

| NO. | DATE       | DESCRIPTION   | BY  |
|-----|------------|---|-----|
| 1   | 06/12/2013 | REVISED STONE BEDDING AND UNDERDRAIN, ADD CMP DUCT BANK | CEM |



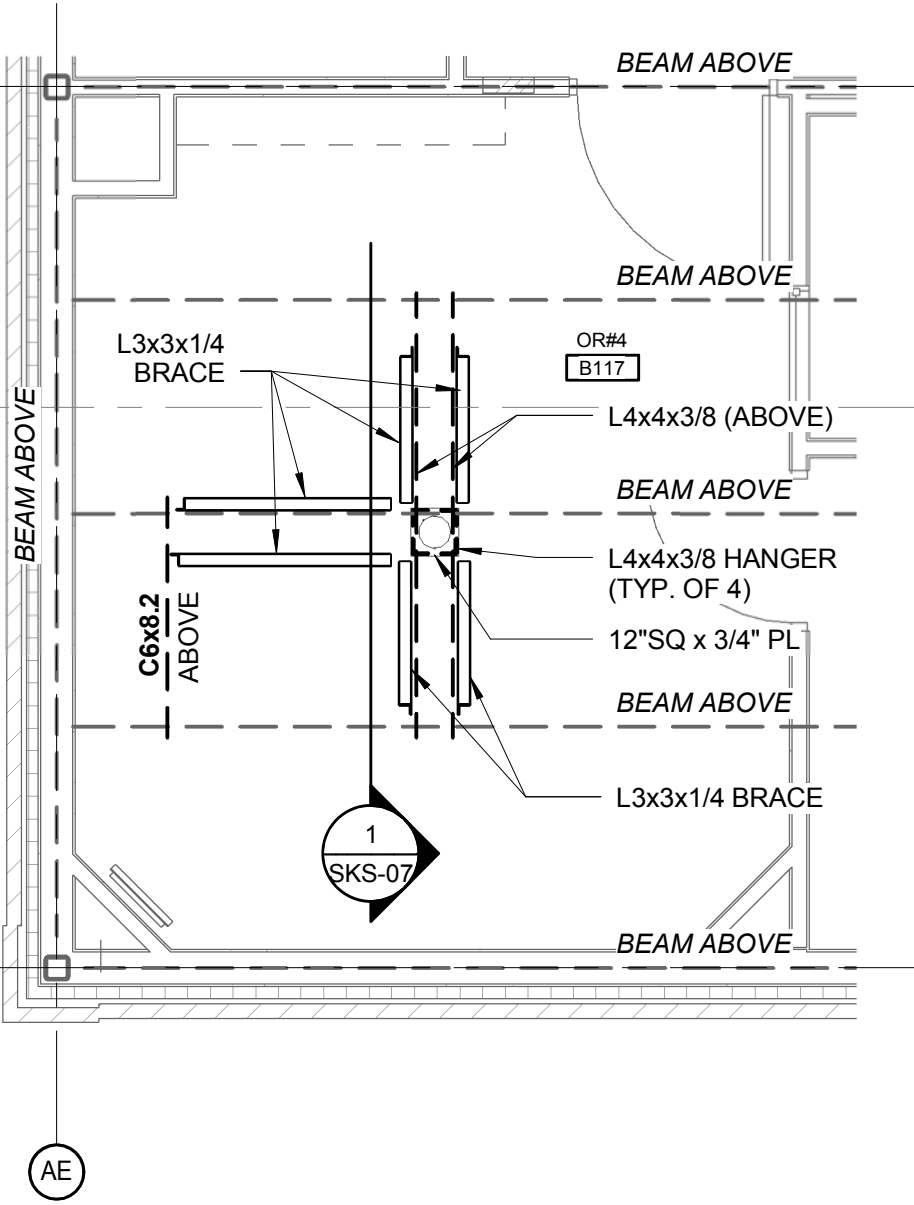
**EYE CARE MEDICAL GROUP**  
**UNDERDRAIN DETAIL**  
PROPOSED BUILDING ADDITION AND PARKING LOT  
53 SEWALL STREET  
PORTLAND, MAINE

|          |            |        |              |
|----------|------------|--------|--------------|
| Job No.: | 13-0392    | Scale: | Not to Scale |
| Date:    | 06/07/2013 | Sheet: | 10           |

1.09

1.04

1.01



AE

1  
SKS-07

ADDENDUM-2



ARCHITECTURE  
INTERIOR DESIGN  
PLANNING

49 DARTMOUTH STREET  
PORTLAND, MAINE 04101  
www.pdtarchs.com

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**E.M.G.-PHASE 2-ADDTION & RENOVATION**  
53 Sewall Street, Portland, Maine 04102

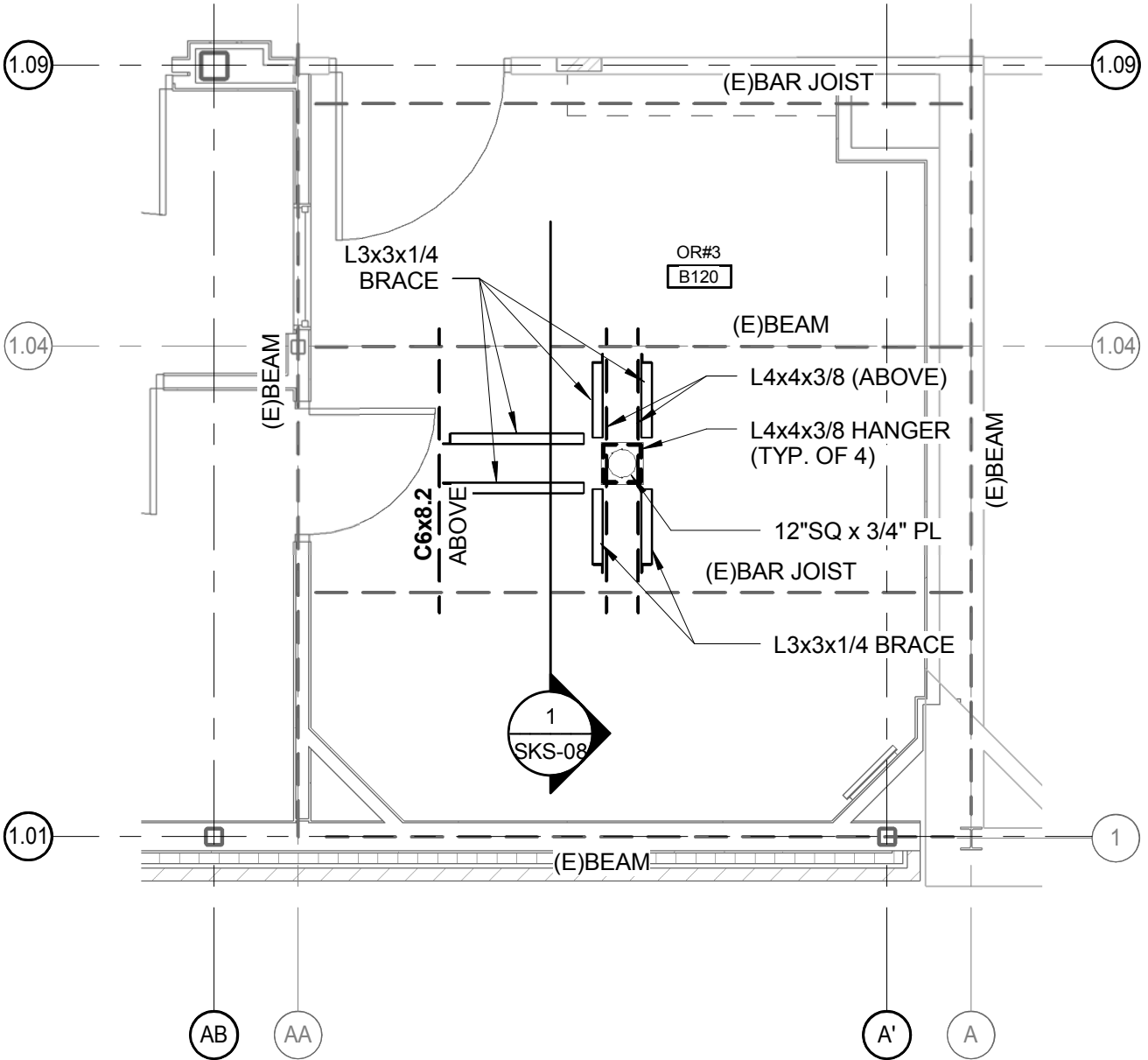
TITLE  
OR#4 LIGHT SUPPORT FRAMING  
PLAN

JOB # 12084  
DATE 07-12-2013  
SCALE 1/4" = 1'-0"

SHEET  
SKS-05

Checker

N:\Projects\2012\12064 - Eyecare Medical Group-Addition and Renovation\00 Drawing Files\12064\_S.rvt



ADDENDUM-2



ARCHITECTURE  
INTERIOR DESIGN  
PLANNING

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PORTLAND, MAINE 04101  
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E.M.G.-PHASE 2-ADDTION & RENOVATION  
53 Sewall Street, Portland, Maine 04102

TITLE  
OR#3 LIGHT SUPPORT FRAMING  
PLAN

|       |              |
|-------|--------------|
| JOB # | 12084        |
| DATE  | 07-12-2013   |
| SCALE | 1/4" = 1'-0" |

SHEET  
SKS-06



(1) 3/4" Ø A325  
BOLT PER HANGER  
(4) TOTAL

L4x4x3/8

TOP OF ROOF SLAB  
16' - 4"

BEAM - SEE PLAN

(E) BEAM

FRAMING PLAN  
14' - 4 1/2"

2 1/2" EACH  
ANGLE  
EDGE TYP.  
EACH END

1/8

3/16

3/16

TYP.

(E)  
JOIST

1/4

1/4

4

4

TYP. EA. BEAM

EACH END  
AND 1 1/2"  
RETURN, TYP.

1/8

1/4" x 2" FLAT PLATE  
TYP. PROVIDE AS SHOWN  
ALL FOUR SIDES  
ANGLE BRACES - SEE PLAN

DUCTWORK - COORD.  
w/MECH. DWGS.

1/2" Ø ERECTION BOLTS

12" SQ x 3/4" ANCHOR  
PLATE

3/8" WELD PLATE FOR  
BRACE CONN.

L4x4x3/8 HANGER  
(4-TOTAL)

OR LIGHT - SEE  
ELECTRICAL DWGS.

ADDENDUM-2



ARCHITECTURE  
INTERIOR DESIGN  
PLANNING

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PORTLAND, MAINE 04101  
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53 Sewall Street, Portland, Maine 04102

TITLE  
TYPICAL LIGHT SUPPORT DETAIL -  
REFER TO SKS-06

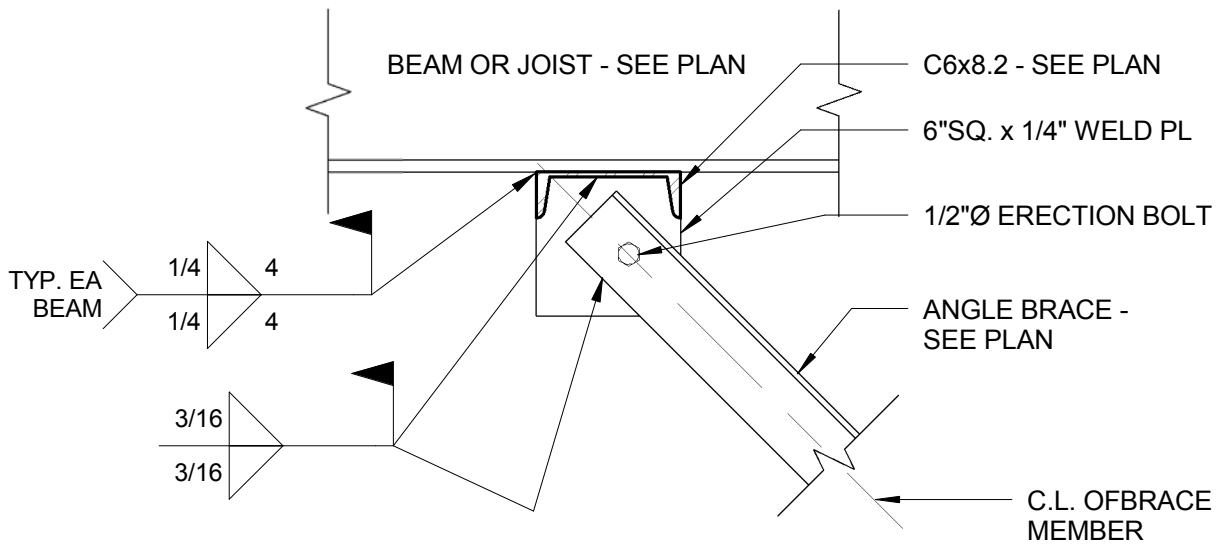
JOB # 12084

DATE 07-12-2013

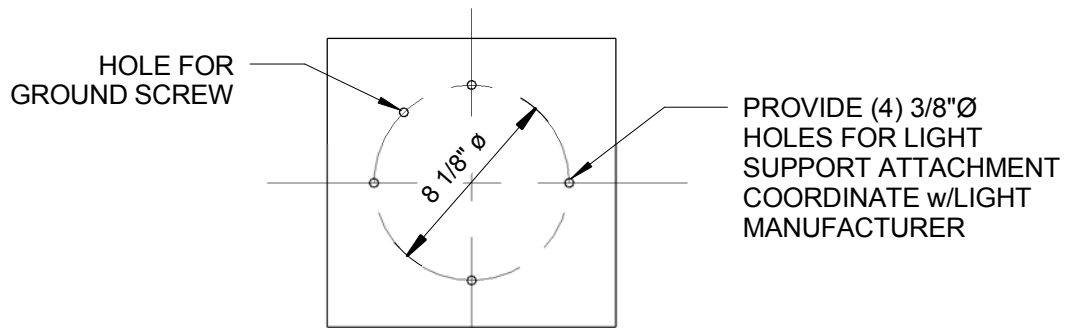
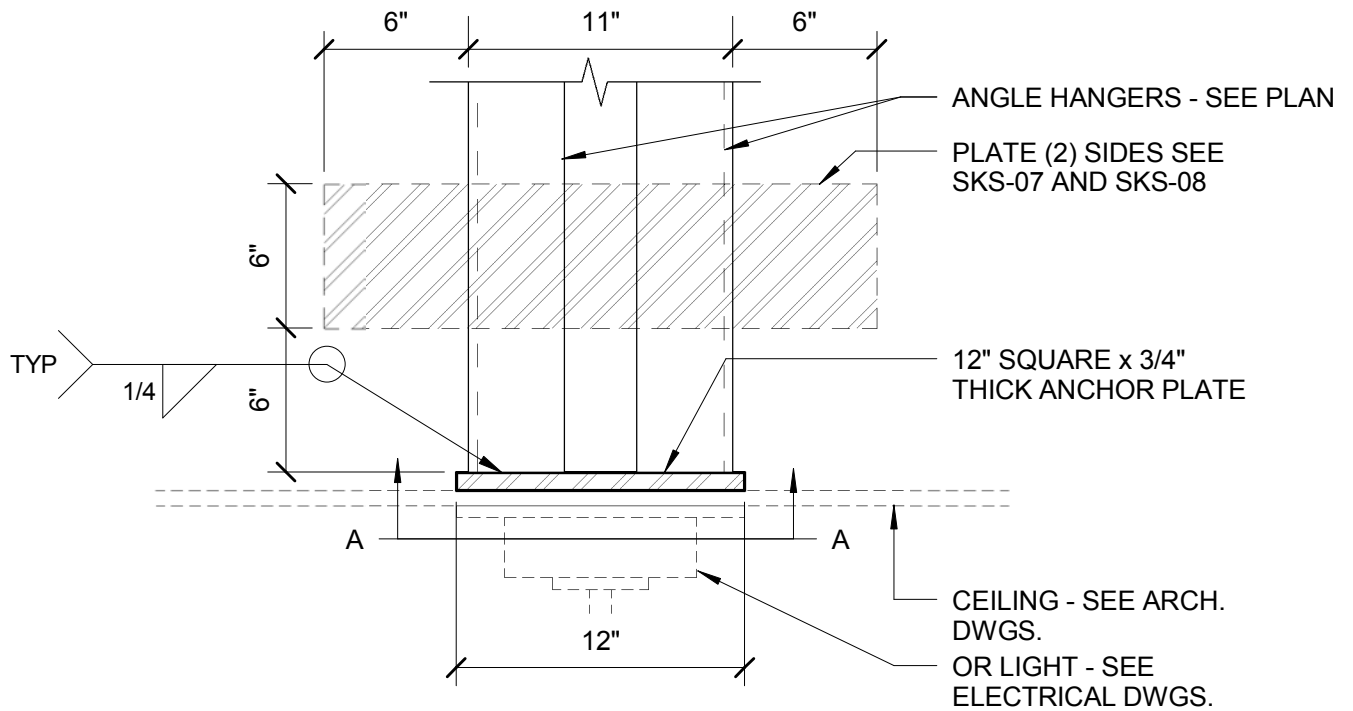
SCALE 1/2" = 1'-0"

SHEET

SKS-08



**TYPICAL DETAIL OF BRACE PARALLEL TO BEAM/JOIST**



**DETAIL A-A**

**ADDENDUM-2**



ARCHITECTURE  
INTERIOR DESIGN  
PLANNING

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**E.M.G.-PHASE 2-ADDTION & RENOVATION**

53 Sewall Street, Portland, Maine 04102

TITLE  
TYPICAL LIGHT SUPPORT DETAILS-  
REFER TO SKS-05 AND SKS-06

JOB # 12084

DATE 07-12-2013

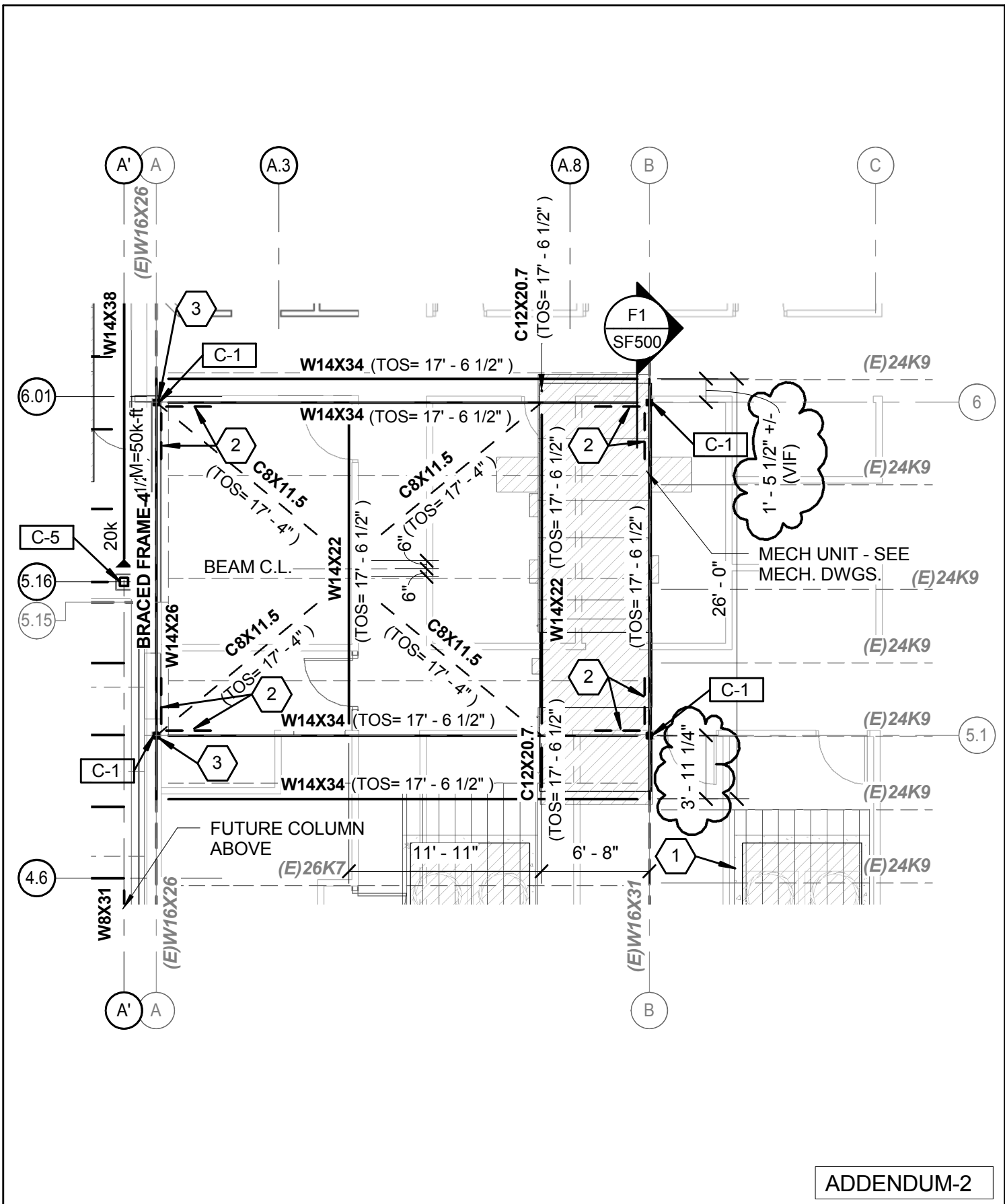
SCALE 1 1/2" = 1'-0"

SHEET

**SKS-09**

Checker

N:\Projects\2012\12064 - Eyecare Medical Group-Addition and Renovation\00 Drawing Files\12064\_S.rvt



**ADDENDUM-2**



ARCHITECTURE  
INTERIOR DESIGN  
PLANNING

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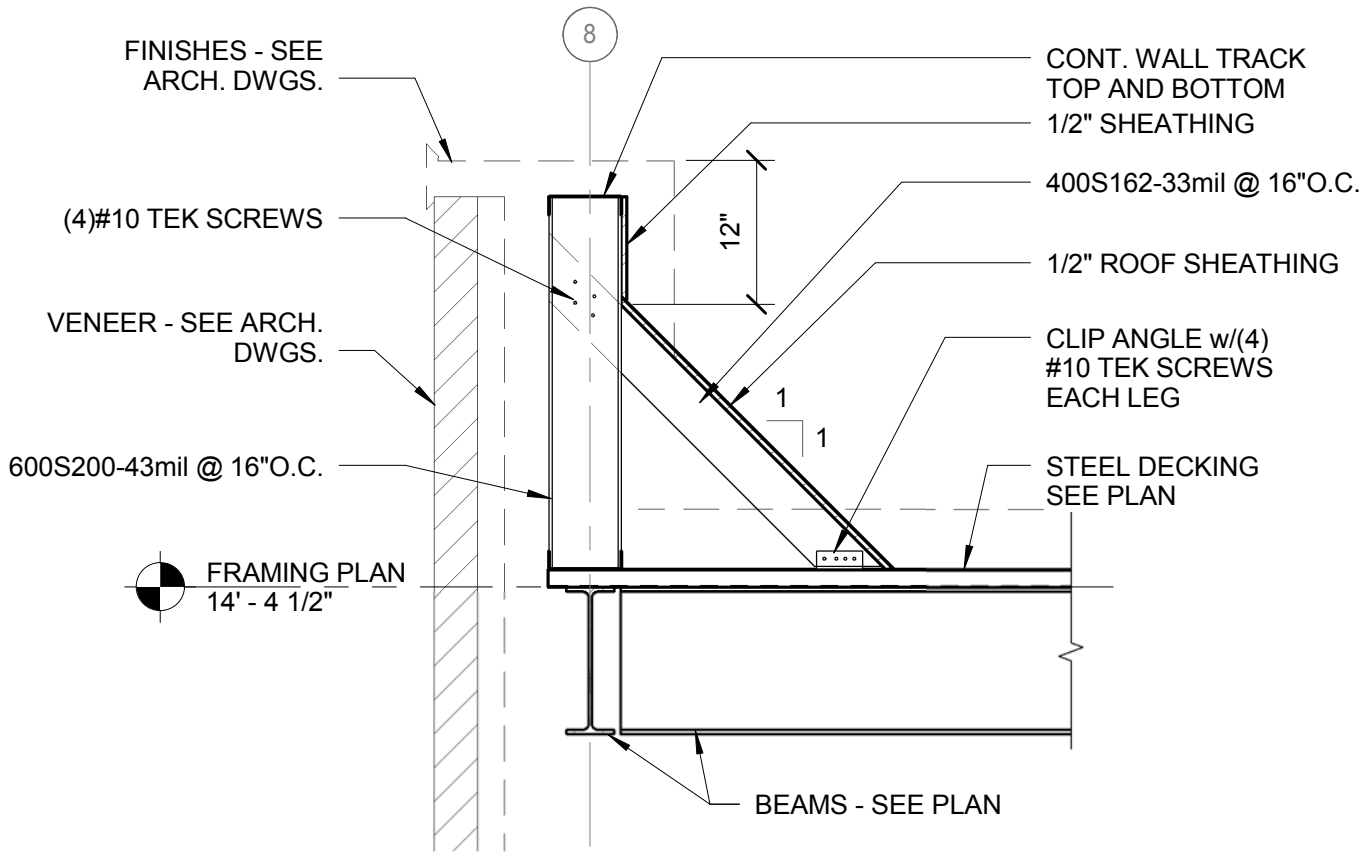
TITLE  
REVISED PARTIAL ROOF FRAMING  
PLAN

JOB # 12084  
DATE 07-12-2013  
SCALE 1/8" = 1'-0"

SHEET  
SKS-10

Checker

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NOTE: DETAIL APPLIES ALONG LINE-8 BETWEEN LINES A AND C COORDINATE w/ARCHITECTURAL DRAWINGS

ADDENDUM-2



ARCHITECTURE  
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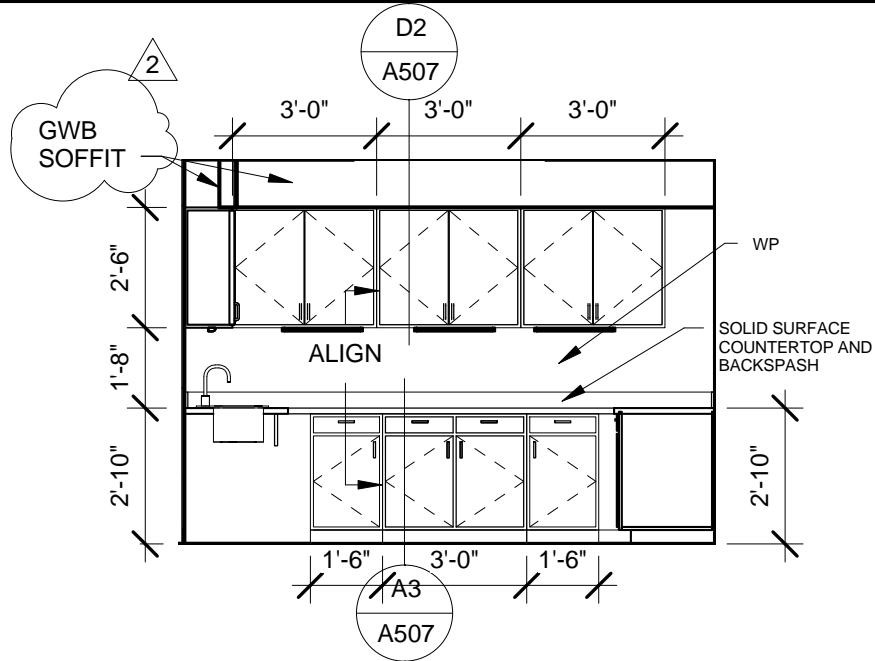
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53 Sewall Street, Portland, Maine 04102

TITLE  
DETAIL AT PARAPET ALONG LINE-8

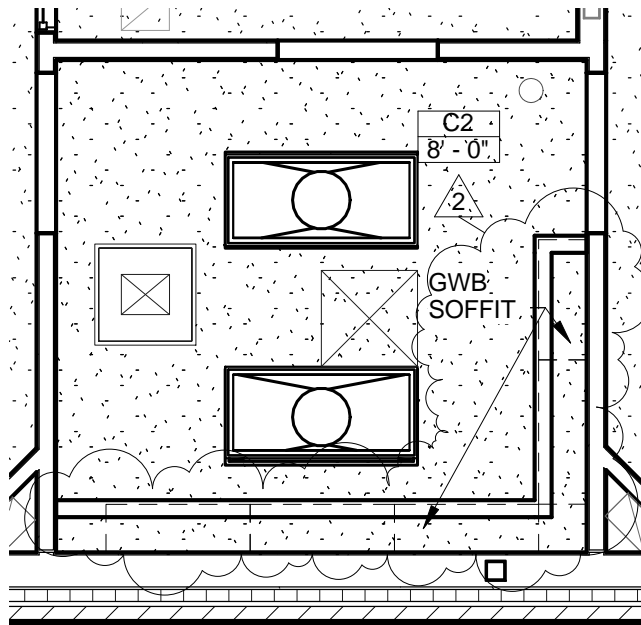
JOB # 12084  
DATE 07-12-2013  
SCALE 3/4" = 1'-0"

SHEET  
SKS-11





1 Autoclave South - ADD Soffit  
1/4" = 1'-0"



2 CEILING PLAN "B" - Autoclave Soffits  
1/4" = 1'-0"

ADDENDUM 2



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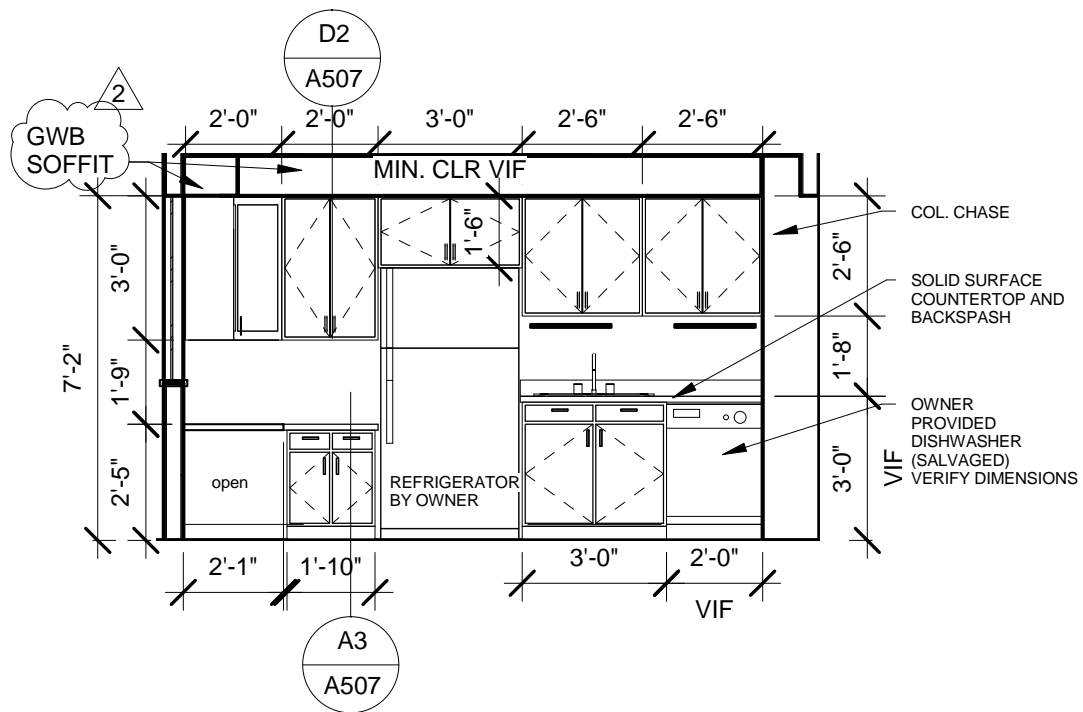
TITLE  
Add soffits at Autoclave B119

JOB # 12084

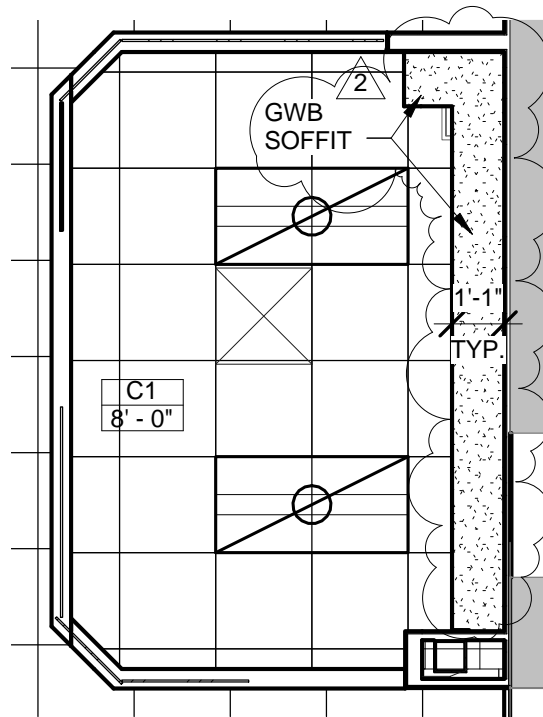
DATE 7-15-2013 06/25/13

SCALE 1/4" = 1'-0"

SHEET  
SKA-1



2 Nurse Station East - Add Soffit  
 1/4" = 1'-0"



1 CEILING PLAN "B" - Nurse Soffit  
 1/4" = 1'-0"

ADDENDUM 2



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TITLE  
 Add soffit at Nurse B108

JOB # 12084

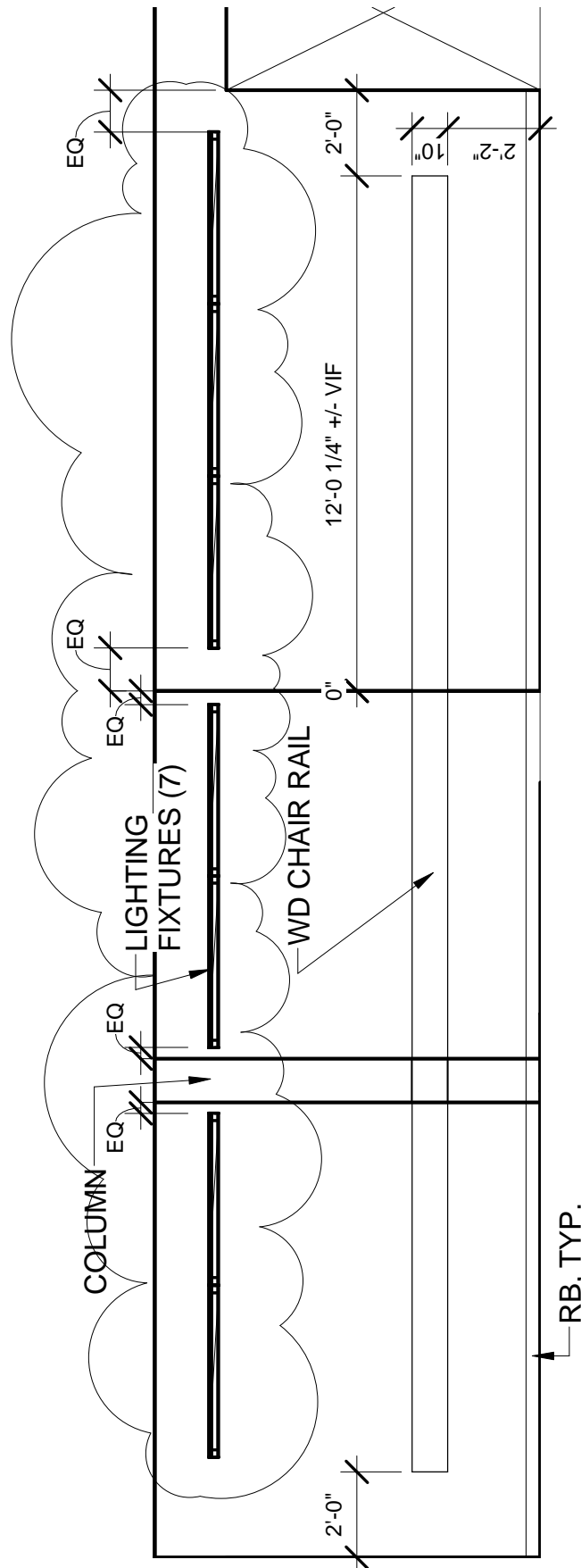
DATE 7-15-2013

SCALE 1/4" = 1'-0"

SHEET  
 SKA-2

Checker

C:\Users\klem\Documents\EMG-12084-Central\_REDESIGN\_ph2-3\_klein.rvt



1 Waiting Room East -Lighting  
 1/4" = 1'-0"

ADDENDUM 2



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E. M. G.-PHASE 2-ADDITION & RENOVATION

53 Sewall Street, Portland, ME 04102

TITLE  
 Waiting Room Lighting Clarification

JOB # 12084

DATE 7-15-2013

SCALE 1/4" = 1'-0"

SHEET  
 SKA-3

Checker

C:\Users\klein\Documents\EMG-12084-Central\_REDESIGN\_ph2-3\_klein.nvt

6" SPRINKLER & 4" CW EXTEND 5'-0" FROM BUILDING, SEE SITE PLANS FOR CONT. COORDINATE LOCATION WITH SITE PLANS.

INV. 4.37'

INV. 4.14'

2

4" CW UP  
TRANSITION TO  
3" ABOVE SLAB  
6" SPRINKLER UP

P-1

P-2

FCO

(E) 3" W

PROVIDE TEMPORARY SUPPORT FOR ELEVATED PIPING. COORDINATE ELEVATION TO BE JUST BELOW FUTURE STEEL FRAMING. AS AN OPTION CONTRACTOR MAY PROVIDE TEMPORARY SERVICE FOR EXISTING SPRINKLER AND INSTALL NEW SERVICE WITH FIT-UP.

KEYED NOTES:

- 1 FIXTURE DESIGNATION IS FOR REFERENCE ONLY COORDINATING ROUGH-IN & SIZING REQUIREMENTS. FIXTURES WILL BE FURNISHED AND INSTALLED WITHIN THE FIT-UP PACKAGE (TYP.).
- 2 EXTEND 3" CW SERVICE INTO EXISTING SPRINKLER ROOM (RUN ABOVE CEILING) AND CONNECT, TEMPORARILY, TO EXISTING 2" SERVICE. FIT-UP PACKAGE WILL UPGRADE THIS SERVICE TO MEET ADDITIONAL DEMAND.
- 3 PIPING SHOWN FOR REFERENCE. INSTALLATION WILL BE COMPLETED IN FIT-UP PACKAGE.

**ADDENUM-2**



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TITLE  
REVISED UNDER SLAB DOMESTIC  
WATER AND SPRINKLER ENTRANCE

JOB # 12084

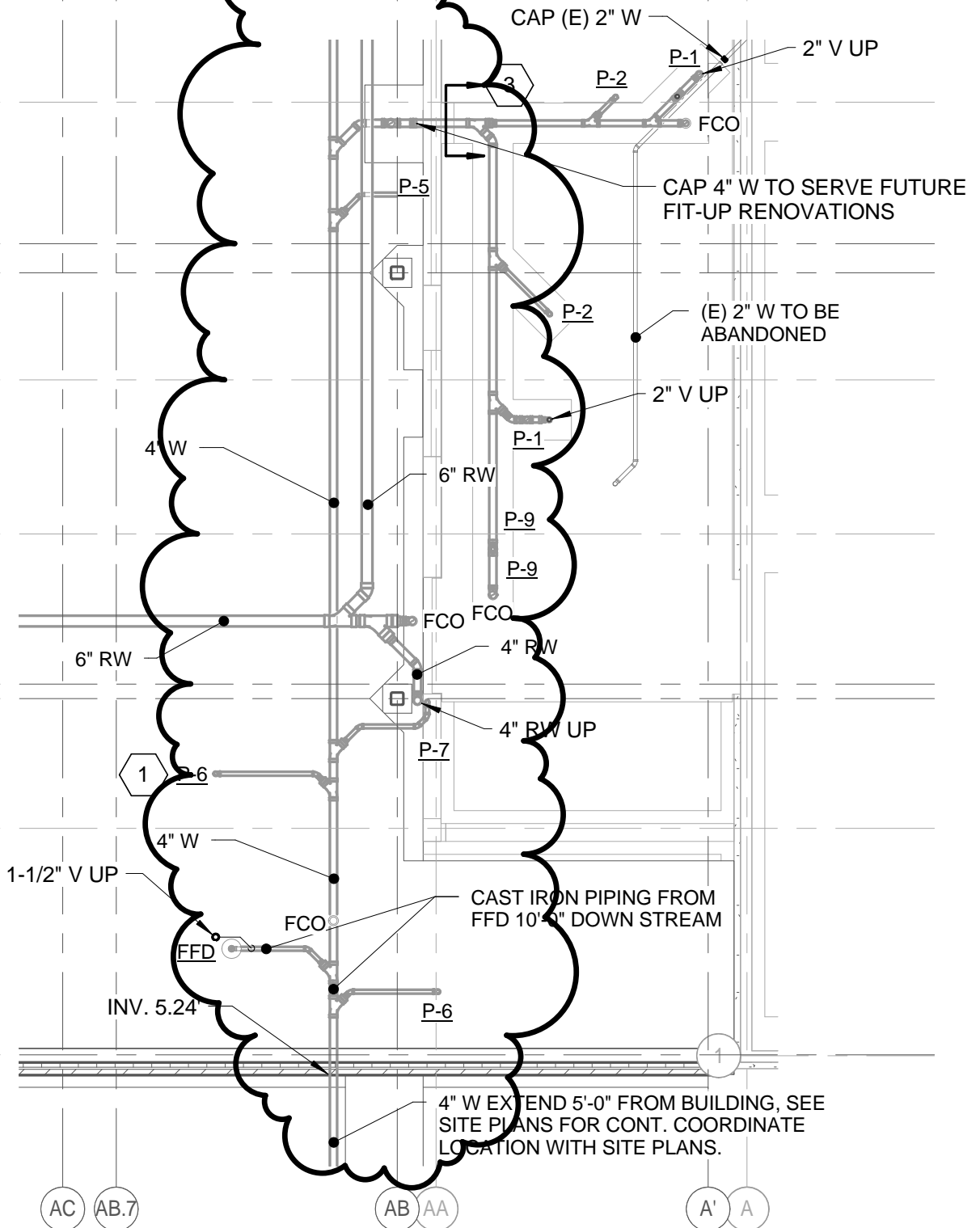
DATE 07-12-2013

SCALE 1/8" = 1'-0"

SHEET  
SKP-1

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**ADDENUM-2**



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**E.M.G.-PHASE 2-ADDTION & RENOVATION**

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**TITLE**  
DELETED UNDER SLAB DOMESTIC  
WATER AND SPRINKLER ENTRANCE

|       |              |
|-------|--------------|
| JOB # | 12084        |
| DATE  | 07-12-2013   |
| SCALE | 1/8" = 1'-0" |

**SHEET**  
SKP-2

Checker

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