GENERAL NOTES

1. THE NOTES ON THESE DRAWINGS ARE NOT INTENDED TO REPLACE SPECIFICATIONS. SEE SPECIFICATIONS FOR REQUIREMENTS IN ADDITION TO GENERAL NOTES. INCONSISTENCIES BETWEEN THESE DRAWINGS AND THE SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT PRIOR TO PROCEEDING WITH THE AFFECTED PORTION OF THE WORK.

2. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH JOB SPECIFICATIONS AND ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND SITE DRAWINGS. CONSULT THESE DRAWINGS FOR LOCATIONS AND DIMENSIONS OF OPENINGS, CHASES, INSERTS, REGLETS, SLEEVES, DEPRESSIONS, AND OTHER DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS.

3. ALL DIMENSIONS, EXISTING CONDITIONS, AND AS-BUILT CONDITIONS MUST BE VERIFIED IN THE FIELD. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK.

4. THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE ONLY AFTER THE STRUCTURAL WORK CONTAINED IN THE S- DRAWINGS IS COMPLETED. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO INSURE THE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS OR TIEDOWNS. SUCH MATERIAL SHALL REMAIN THE PROPERTY OF THE CONTRACTOR AFTER COMPLETION OF THE PROJECT.

5. SECTIONS AND DETAILS SHOWN ON ANY STRUCTURAL DRAWINGS SHALL BE CONSIDERED TYPICAL FOR SIMILAR CONDITIONS AS DETERMINED BY THE STRUCTURAL ENGINEER. THE STRUCTURAL ENGINEER RESERVES THE RIGHT TO INTERPRET DETAILS TO ADDRESS OTHER PROJECT CONDITIONS.

6. PROVIDE AND INSTALL NECESSARY MATERIAL TO CONNECT ELEVATOR SUPPORT BEAMS AND GUIDE RAILS. LOCATION AND SIZE OF MEMBERS AND ANY INSERTS REQUIRED SHALL BE DETERMINED BY THE ELEVATOR MANUFACTURER.

7. THE CONTRACTOR SHALL SUBMIT COMPLETE SHOP DRAWINGS FOR ALL PARTS OF THE WORK, INCLUDING DESCRIPTION OF SHORING, AND CONSTRUCTION METHODS AND SEQUENCING WHERE APPLICABLE. NO PERFORMANCE OF THE WORK INCLUDING, BUT NOT LIMITED TO, DEMOLITION OF EXISTING STRUCTURE, OR FABRICATION OR ERECTION OF NEW STRUCTURAL ELEMENTS, SHALL COMMENCE WITHOUT REVIEW OF THE SHOP DRAWINGS BY THE ARCHITECT AND ENGINEER. SUBMIT ONE COPY AND ONE SEPIA. COPY WILL BE REVIEWED AND SEPIA WILL BE RETURNED. FOR SHOP DRAWINGS AND SUBMITTALS REQUIRED, REFERENCE THE PROJECT SPECIFICATIONS.

8. ALL APPLICABLE FEDERAL, STATE, AND MUNICIPAL REGULATIONS SHALL BE FOLLOWED, INCLUDING THE FEDERAL DEPARTMENT OF LABOR OCCUPATIONAL SAFETY AND HEALTH ACT.

9. IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE (2003 EDITION, SECTION 1704.1), A STATEMENT OF SPECIAL INSPECTIONS IS REQUIRED AS A CONDITION FOR PERMIT ISSUANCE BY THE LOCAL CODE OFFICIAL. THIS STATEMENT SHALL INCLUDE A COMPLETE LIST OF MATERIALS AND WORK REQUIRING SPECIAL INSPECTIONS, THE INSPECTIONS TO BE PERFORMED AND A LIST OF THE INDIVIDUALS, APPROVED AGENCIES AND FIRMS INTENDED TO BE RETAINED FOR CONDUCTING SUCH INSPECTIONS.

10. REFERENCE THE PROJECT SPECIFICATIONS FOR ALL TESTING REQUIREMENTS.

DESIGN LOADS

1. BUILDING CODE:
   INTERNATIONAL BUILDING CODE, 2003 EDITION
   ASCE 7-02 MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES.

2. DESIGN FLOOR LIVE LOADS:
   OFFICES: 50 PSF + 20 PSF PARTITION ALLOWANCE
   LIGHT STORAGE: 125 PSF
   CORRIDORS ABOVE FIRST FLOOR: 80 PSF
   STAIRS: 100 PSF

3. DESIGN ROOF SNOW LOAD:
   GROUND SNOW LOAD (Pg): 60 PSF
   SNOW EXPOSURE FACTOR (Ce): 1.0
   SNOW LOAD IMPORTANCE FACTOR (Is): 1.0
   SNOW LOAD THERMAL FACTOR (Ct): 1.1
   FLAT ROOF SNOW LOAD (PF): 46.2 PSF + DRIFT

4. DESIGN WIND LOAD:
   BASIC WIND SPEED: 100 MPH
   WIND LOAD IMPORTANCE FACTOR (Iw): 1.00
   WIND EXPOSURE: B
   INTERNAL PRESSURE COEFFICIENT: ±0.18
   COMPONENTS & CLADDING LOADS PER ASCE 7-02

5. DESIGN SEISMIC LOADS:
   EQUIVALENT LATERAL FORCE PROCEDURE
   SEISMIC USE GROUP: I
   SEISMIC IMPORTANCE FACTOR (Ie):1.0
   MAPPED SPECTRAL RESPONSE ACCELERATIONS:
   Ss: 0.369
   S1: 0.098
   SEISMIC SITE CLASS: E
   SPECTRAL RESPONSE COEFFICIENTS:
   Sds: 0.521
   Sd1: 0.229
   SEISMIC DESIGN CATEGORY: D

   BASIC SEISMIC FORCE RESISTING SYSTEM:
   STEEL ECCENTRICALLY BRACED FRAMES NON-MOMENT FRAMES AT COLUMNS AWAY FROM LINKS

   RESPONSE MODIFICATION FACTOR (R): 7

   SEISMIC RESPONSE COEFFICIENT (Cs), X: 0.036
   Y: 0.038

   ANALYSIS: EQUIVALENT FORCE PROCEDURE

6. SYSTEM & COMPONENTS REQUIRING SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE: REFER TO SPECIFICATION SECTION 01400

FOUNDATION NOTES (SOIL SUPPORTED)

1. FOUNDATIONS HAVE BEEN DESIGNED IN ACCORDANCE WITH A REPORT ENTITLED "GEOTECHNICAL ENGINEERING SERVICES, PROPOSED MEDICAL BUILDING, SEWALL STREET, PORTLAND, MAINE", PREPARED BY S.W. COLE ENGINEERING INC., DATED 02/17/2005. THE RECOMMENDATIONS OF THE REPORT ARE PART OF THIS WORK. REFER TO THIS REPORT FOR SPECIFIC RECOMMENDATIONS.

2. FOUNDATION DESIGN IS BASED ON SHALLOW MAT FOOTINGS BEARING ON CRUSHED STONE ON GEOTEXTILE FABRIC OVER SUITABLE NATIVE STIFF CLAY PER THE GEOTECHNICAL REPORT.REFER TO THIS REPORT FOR SPECIFIC BEARING RECOMMENDATIONS.

3. ALLOWABLE BEARING CAPACITY 1,100 PSF MAX (750 PSF AVE) PER SUPPLEMENTAL MEMO BY S.W. COLE ENGINEERING, INC. DATED 05/12/2005.

4. EXTEND BOTTOM OF EXTERIOR FOOTINGS AT LEAST 3.5 FEET BELOW THE FINAL EXTERIOR GRADE FOR PROTECTION AGAINST FROST. FOOTINGS SHALL BEAR ON A 12" MIN LAYER OF COMPACTED CRUSHED STONE.

5. NO FILL FOR BUILDING SUPPORT SHALL BE PLACED UNTIL SUBGRADES HAVE BEEN OBSERVED AND APPROVED BY THE GEOTECHNICAL ENGINEER.

6. REFERENCE THE GEOTECHNICAL REPORT FOR ALL EXCAVATION, BACKFILL, COMPACTION, CONSTRUCTION DEWATERING AND PERMANENT DRAINAGE REQUIREMENTS.

7. SOILS EXPOSED AT THE BASE OF ALL SATISFACTORY FOUNDATION EXCAVATIONS SHOULD BE PROTECTED AGAINST ANY DETRIMENTAL CHANGE IN CONDITION, SUCH AS DISTURBANCE FROM RAIN OR FROST. SURFACE RUNOFF SHOULD BE DRAINED AWAY FROM THE EXCAVATIONS AND NOT BE ALLOWED TO POND. FOUNDATION EXCAVATIONS AND SHOULD BE ADEQUATELY PROTECTED FROM RAINFALL OR FREEZING CONDITIONS. GROUNDWATER SHOULD BE ANTICIPATED FOR EXCAVATIONS AND APPROPRIATE DEWATERING MEASURES SHALL BE EMPLOYED.

8. EXCAVATIONS FOR BUILDING CONSTRUCTION SHALL BE IN ACCORDANCE WITH OSHA REQUIREMENTS. BRACED EXCAVATIONS SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF MAINE. DO NOT UNDERMINE EXISTING FOUNDATIONS OF ANY ADJACENT STRUCTURES. REFER TO THE GEOTECHNICAL REPORT FOR ADDITIONAL AND/OR MORE SPECIFIC REQUIREMENTS.

CONCRETE NOTES

1. CONCRETE WORK SHALL CONFORM TO "ACI MANUAL OF CONCRETE PRACTICE", LATEST EDITION. THIS PUBLICATION IS AVAILABLE THROUGH THE AMERICAN CONCRETE INSTITUTE (248) 848-3800.

2. ALL CONCRETE SHALL HAVE A 28-DAY COMPRESSIVE STRENGTH OF 3,000 PSI, U.N.O. EXTERIOR SLAB-ON-GRADE SHALL HAVE A 28-DAY COMPRESSIVE STRENGTH OF 4,500 PSI. ADDITIONAL CONCRETE MIX PERFORMANCE DATA INCLUDING AIR CONTENT, WATER-CEMENT RATIO, AIR CONTENT, AGGREGATE SIZE, SLUMP, ETC. HAS BEEN INCLUDED IN THE PROJECT SPECIFICATIONS. SEE THE SPECIFICATIONS FOR ADDITIONAL REQUIRMENTS.

3. CONCRETE SHALL NOT BE PLACED IN WATER OR ON FROZEN GROUND.

4. PROVIDE PVC SLEEVES WHERE PIPES PASS THROUGH EXTERIOR CONCRETE, OR SLABS.

5. REINFORCING BARS SHALL CONFORM TO ASTM A615 GRADE 60 DEFORMED BARS AND SHALL BE DETAILED, FABRICATED AND PLACED IN ACCORDANCE WITH ACI 315, LATEST EDITION.

6. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185 AND BE PROVIDED IN FLAT SHEETS.

7. FIBER REINFORCEMENT SHALL BE TYPE III SYNTHETIC VIRGIN HOMOPOLYMER POLYPROPYLENE FIBERS CONFORMING TO ASTM C1116.

8. MINIMUM CONCRETE PROTECTIVE COVERING FOR REINFORCEMENT, UNLESS NOTED OTHERWISE, SHALL BE AS FOLLOWS:

   A) SURFACES CAST AGAINST AND PERMANENTLY IN CONTACT WITH EARTH, 3.0"
   B) FORMED SURFACES IN CONTACT WITH EARTH OR EXPOSED TO WEATHER
      #5 BARS, 5/8" DIAMETER WIRE, AND SMALLER, 1.5"
      #6 THROUGH #11 BARS, 2.0"
   C) SURFACES NOT IN CONTACT WITH EARTH OR EXPOSED TO WEATHER
      WALLS, SLABS, JOISTS #11 BARS AND SMALLER, 1.0"
      BEAMS, GIRDERS, AND COLUMNS; ALL REINFORCEMENT, 1.5"

9. REINFORCEMENT SHALL BE CONTINUOUS AROUND CORNERS AND AT INTERSECTIONS. PROVIDE LAPPED BARS AT NECESSARY SPLICES OR HOOKED BARS AT DISCONTINUOUS ENDS. PROVIDE TENSION LAP SPLICES PER THE SCHEDULE ON DWG S2.1, FOR ALL REINFORCING UNLESS OTHERWISE SHOWN ON PLAN.

10. WELDING OF REINFORCEMENT IS NOT PERMITTED.

11. FOR ALL OPENINGS IN CONCRETE WALLS AND SLABS, PROVIDE SUPPLEMENTAL REINFORCING AROUND OPENING AS SHOWN ON THE CONTRACT DOCUMENTS TYPICAL DETAILS.

12. CONSTRUCTION/CONTRACTION JOINTS SHOWN ON DRAWINGS ARE MANDATORY. OMISSIONS, ADDITIONS, OR CHANGES SHALL NOT BE MADE EXCEPT WITH THE SUBMITTAL OF A WRITTEN REQUEST TOGETHER WITH DRAWINGS OF THE PROPOSED JOINT LOCATIONS FOR APPROVAL OF THE STRUCTURAL ENGINEER. WHERE CONSTRUCTION JOINTS ARE NOT SHOWN, OR WHEN ALTERNATE LOCATIONS ARE PROPOSED, DRAWINGS SHOWING LOCATION OF CONSTRUCTION AND CONTROL JOINTS AND CONCRETE PLACING SEQUENCE SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW PRIOR TO PREPARATION OF THE REINFORCEMENT SHOP DRAWINGS. CONCRETE SHALL BE PLACED WITHOUT HORIZONTAL CONSTRUCTION JOINTS EXCEPT WHERE SHOWN OR NOTED. VERTICAL CONSTRUCTION JOINTS AND STOPS IN CONCRETE BEAMS/GRADE BEAMS SHALL BE MADE AT MIDSPAN OR AT POINTS OF MINIMUM SHEAR, UNLESS NOTED OTHERWISE.

13. SPACING OF CONSTRUCTION/CONTRACTION JOINTS, UNLESS NOTED OTHERWISE SHALL BE AS FOLLOWS:
   A) FOOTINGS AND WALLS MAX LENGTH 40'-0" NOR 15'-0" FROM ANY CORNER**
   B) SLABS ON GRADE SEE FOUNDATION PLAN

14. ANCHOR RODS SHALL BE HEADED RODS CONFORMING TO ASTM F1554, GRADE 36 KSI WELDABLE STEEL, UNLESS NOTED OTHERWISE ON DRAWINGS. ANCHOR RODS THAT ARE TO BE IN CONTACT WITH PRESSURE TREATED LUMBER SHALL BE HOT-DIPPED GALVANIZED.

15. ALL GROUT BENEATH BASE PLATES & BEARING PLATES SHALL BE "5-STAR" 5000-PSI NON-SHRINK GROUT BY U.S. GROUT CORP. AND

STRUCTURAL STEEL NOTES

1. STRUCTURAL STEEL FABRICATION, ERECTION, AND CONNECTION DESIGN SHALL CONFORM TO AISC "SPECIFICATION FOR THE DESIGN FABRICATION, AND ERECTION OF STRUCTURAL STEEL" 9TH EDITION, AND THE "CODE OF STANDARD PRACTICE, LATEST EDITION.

2. STRUCTURAL STEEL: STEEL PLATES, SHAPES, AND BARS, CONFORM TO ASTM A36 UNLESS NOTED OTHERWISE (U.N.O.). STRUCTURAL STEEL SHAPES DESIGNATED ON THE DRAWINGS FOR WIDE-FLANGE SECTIONS: ASTM A992 (ASTM A572 GRADE 50 WITH SPECIAL REQUIREMENTS PER AISC TECHNICAL BULLETIN #3 DATED MARCH, 1997)

3. STRUCTURAL TUBING: CONFORM TO ASTM A500 GRADE B46 KSI.

4. FIELD CONNECTIONS SHALL BE BOLTED USING 3/4" DIAMETER ASTM A325N HIGH STRENGTH BOLTS (U.N.O.) EXCEPT WHERE SLIP CRITICAL CONNECTIONS ARE REQUIRED AND NOTED BY A325 (SC) ON THE DRAWINGS. PROVIDE SLIP CRITICAL (SC) CONNECTIONS AT ALL MOMENT CONNECTIONS, BRACED FRAMES, RELIEVING ANGLES AND AS OTHERWISE NOTED. USE A490 BOLTS WHERE INDICATED.

5. WHERE WELDING IS INDICATED, ALL WELDING SHALL CONFORM TO AWS D1.1-LATEST EDITION. ELECTRODES SHALL BE CONFORM TO AWS A5.1 E70XX SERIES WITH PROPER ROD TO PRODUCE OPTIMUM WELD (LOW HYDROGEN).

6. SEE CONCRETE NOTES AND DRAWINGS FOR ANCHOR BOLT INFORMATION, TYP.

7. PROVIDE 3/8" MINIMUM STIFFENER PLATES EACH SIDE OF BEAM WEB AT BEAMS FRAMING OVER COLUMNS AND AT BEAMS SUPPORTING COLUMNS ABOVE.

8. PROVIDE 1/4" THICK LEVELING PLATE UNDER ALL COLUMN BASE PLATES UNLESS OTHERWISE NOTED. LEVELING PLATES SHALL BE SET AND GROUTED PRIOR TO ERECTING COLUMNS.

9. PROVIDE ALL MISCELLANEOUS ANGLES, PLATES, ANCHORS, BOLTS, ETC., SHOWN ON ARCHITECTURAL DRAWINGS FOR SUPPORT OF BLOCKING, PARAPETS, FINISHES, ETC. COORDINATE WITH MISCELLANEOUS METAL FABRICATOR TO ENSURE COMPLETE COVERAGE OF ALL ITEMS.

10. PROVIDE L 4 x 4 x 1/4 SLAB SUPPORT ANGLE AS REQUIRED AT COLUMNS WHERE STRUCTURAL MEMBERS DO NOT FRAME IN AT ALL FOUR SIDES.

OPEN WEB STEEL JOISTS

1. DESIGN, DETAIL, FABRICATE AND ERECT STEEL JOISTS IN ACCORDANCE WITH THE LATEST EDITION OF STANDARD SPECIFICATIONS OF THE STEEL JOIST INSTITUTE (SJI)

2. HANGERS FOR DUCTS, PIPES, UNITS, ETC., MUST BE ATTACHED TO JOISTS AT PANEL POINTS ONLY. EXCEPT WHERE DESIGNATED ON "SP" JOISTS, IN NO CASE SHALL ANY SINGLE POINT LOAD EXCEED 300 LBS.

3. PROVIDE BRIDGING AND BRIDGING ANCHORAGE IN ACCORDANCE WITH SJI SPECIFICATIONS. PROVIDE ADDITIONAL BRIDGING FOR UPLIFT FOR ROOF JOISTS WHERE REQUIRED BY SJI SPECIFICATIONS.

METAL DECK

1. THE METAL ROOF AND FLOOR DECK SHALL BE FORMED OF STEEL SHEETS CONFORMING TO ASTM STANDARD A611.

2. FLOOR AND ROOF DECK SHALL BE AS NOTED ON THE DRAWINGS (OR EQUIVALENT).

3. FOR DECK ATTACHMENTS, PENETRATIONS AND ACCESSORIES, REFER TO SPECIFICATIONS.

MASONRY NOTES

1. ALL MASONRY CONSTRUCTION SHALL CONFORM TO ACI 530.1-02.

2. ALL CONCRETE MASONRY UNITS SHALL BE ASTM C90 GRADE N, TYPE I STANDARD WEIGHT BLOCKS INCLUDING STRETCHERS AND CORNER BLOCKS. MINIMUM PRISM STRENGTH OF BLOCK SHALL BE F'M = 1500 PSI IN 28 DAYS.

3. MORTAR SHALL CONFORM TO ASTM SPECIFICATION C270, TYPE M OR S

4. GROUT SHALL CONFORM TO ASTM-C476

5. REINFORCING FOR BOND BEAMS, LINTEL BLOCKS AND VERTICAL WALL REINFORCING SHALL BE BILLET STEEL CONFORMING TO ASTM A615, GRADE 60

6. HORIZONTAL JOINT REINFORCING SHALL BE DUR-O-WAL TRUSS DESIGN, STANDARD CLASS MILL GALVANIZED WITH 3/16" DIAMETER SIDE RODS AND 9 GAUGE CROSS TIES, UNO. REINFORCING SHALL BE PLACED IN MASONRY WALLS AT EVERY SECOND BLOCK COURSE.

7. CONCRETE MASONRY UNITS SHALL BE LAID IN RUNNING BOND UNLESS OTHERWISE NOTED. PROVIDE FULL MORTAR COVERAGE ON ALL WEBS AND FACE SHELLS. PROVIDE CORNER BLOCKS AND END BLOCKS TO FINISH ALL 90 DEGREE CORNERS AND WALL OPENINGS.

8. PROVIDE BOND BEAMS AT WALL PENETRATIONS AS SHOWN IN SECTIONS.

9. STANDARD LAP LENGTH OF GRADE 60 MASONRY REINFORCING BARS SHALL BE 48 BAR DIAMETERS. PROVIDE CORNER BARS TO MATCH HORIZONTAL REINFORCEMENT

10. CELLS TO BE GROUTED SHALL BE 2-CELL BLOCK. ALIGN CELLS TO MAINTAIN A CLEAR UNOBSTRUCTED, CONTINUOUS VERTICAL CHASE. CELLS MUST BE KEPT CLEAN OF PROTRUSIONS OR FINS OF MORTAR. FILL CELLS OF MASONRY UNITS AND WALL CAVITIES WHERE INDICATED WITH 2500 PSI GROUT. MAXIMUM GROUT LIFT WITHOUT CLEAN-OUTS SHALL BE 4'-0". HIGH LIFT GROUTING SHALL CONFORM TO CODE REQUIREMENTS WITH A MINIMUM CEMENT CONTENT OF 8 SACKS PER CUBIC YARD. SUPPORT ALL VERTICAL BARS IN CENTER OF GROUTED CELLS WITH VERTICAL BAR POSITIONER.

11. FIELD PENETRATIONS THROUGH BLOCK WALLS SHALL NOT BE MADE THROUGH BOND BEAMS, LINTELS OR GROUTED CELLS.

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JOB NO.
1270

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TITLE
GENERAL NOTES

SHEET
S1.0

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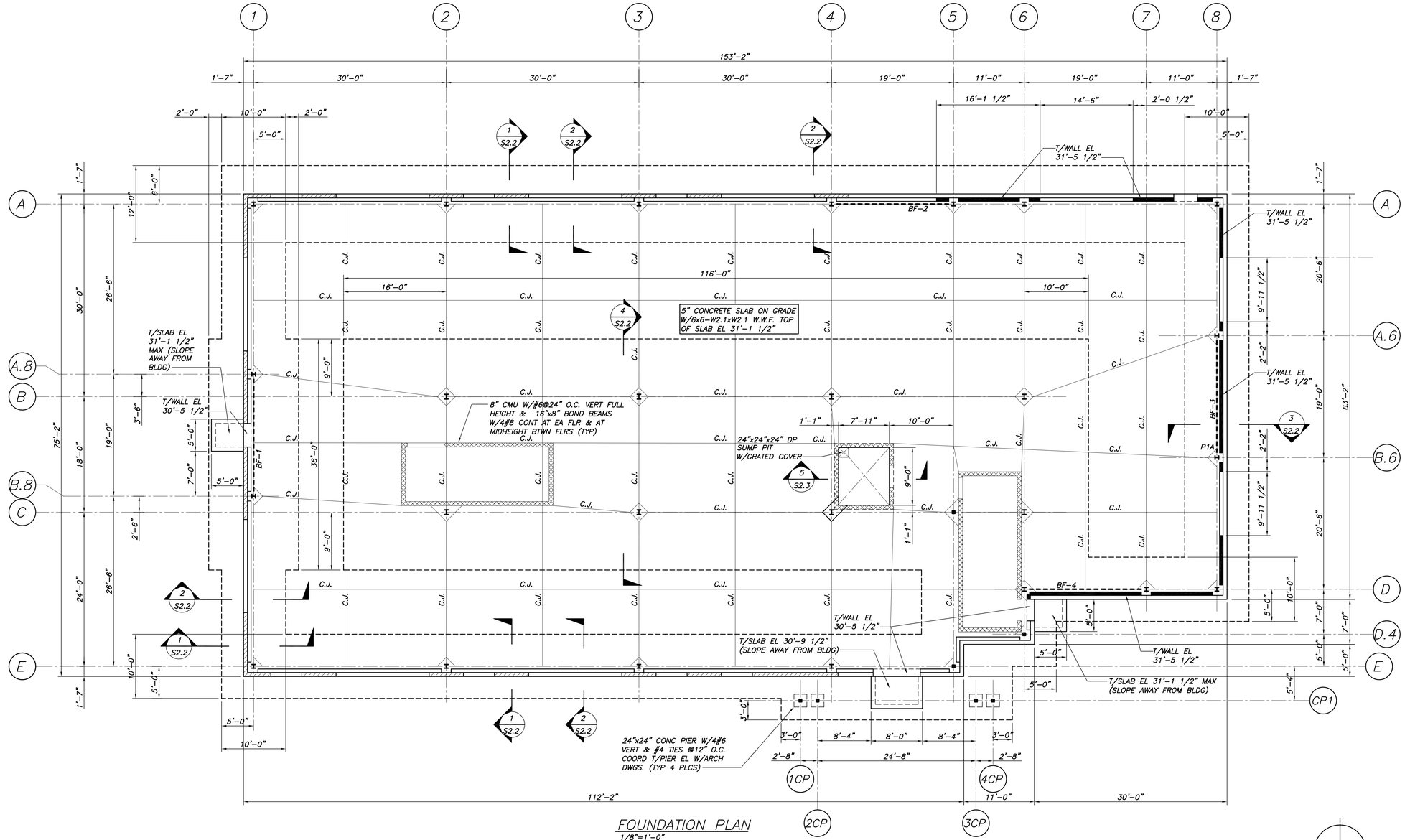

FOUNDATION PLAN
1/8"=1'-0"

NOTES:

1. SEE DWG S1.6 FOR BASE PLATES & ANCHOR BOLT INFORMATION.
2. SEE SECTIONS & DETAILS FOR TOP OF FOOTING ELEVATIONS.
3. C.J. INDICATES CONTRACTION/CONSTRUCTION JOINT.
4. SLABS ON GRADE ARE TO BE WET CURED PER ACI 308, LATEST.
5. SEE ARCH DWGS FOR SILL ANGLE REQUIREMENTS AT ELEVATOR
6. TOP OF WALL EL 31'-1 1/2" U.N.O.
7. FOR ENTRY SLABS, SEE TYP ENTRANCE SLAB DETAIL DWG S2.3

PROJECT NORTH

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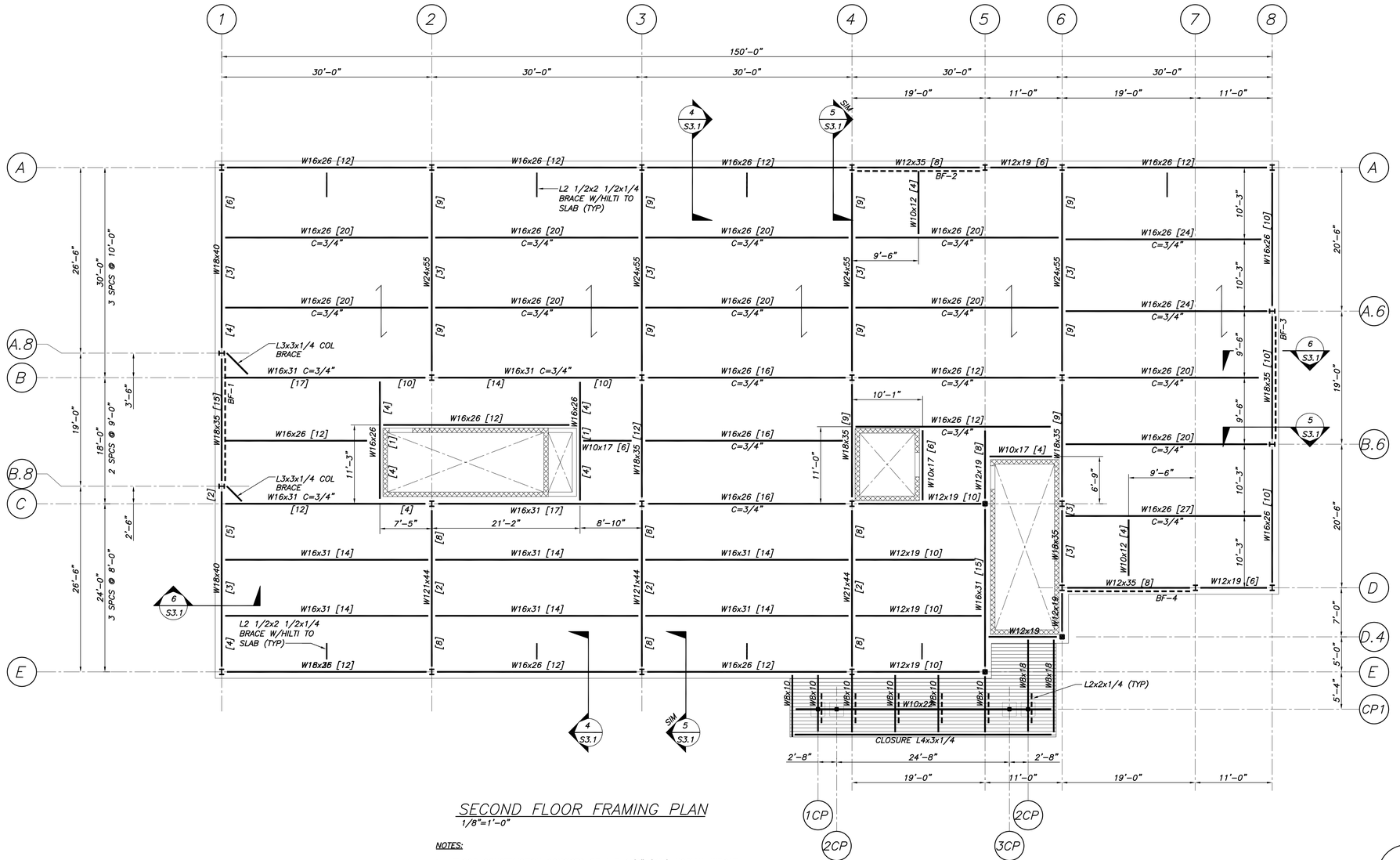

SECOND FLOOR FRAMING PLAN

1/8"=1'-0"

NOTES:

1. FLOOR CONSTRUCTION CONSISTS OF A 5 1/2" (MIN) NORMAL WEIGHT CONCRETE SLAB ON 3VLI 20GA GALVANIZED METAL DECK, (2 1/2" CONCRETE FILL OVER 3" DECK), W/6x6-W1.4xW1.4 W.W.F. PROVIDE ADDL CONC AS REQD TO ACHIEVE A LEVEL FLOOR FINISH.
2. TOP OF 5 1/2" CONC SLAB EL 45'-1 1/2".
3. TOP OF STEEL EL 44'-8" U.N.O.
4. [22] ETC INDICATES 3/4"x4 1/2" LG HEADED SHEAR STUDS SPACED UNIFORMLY ALONG BEAM OR BETWEEN CONCENTRATED LOADS.
5. INDICATES BRACING. SEE BRACING ELEVATIONS DWG S.1.6.
6. ALL STAIR STRUCTURES SHALL BE DESIGNED AND FABRICATED AS PER SPECIFICATIONS.
7. INDICATES DECK SPAN DIRECTION.
8. SEE ARCH DWGS FOR POUR STOP/SILL ANGLE REQUIREMENTS AT ELEVATOR.

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SHEET
S1.2

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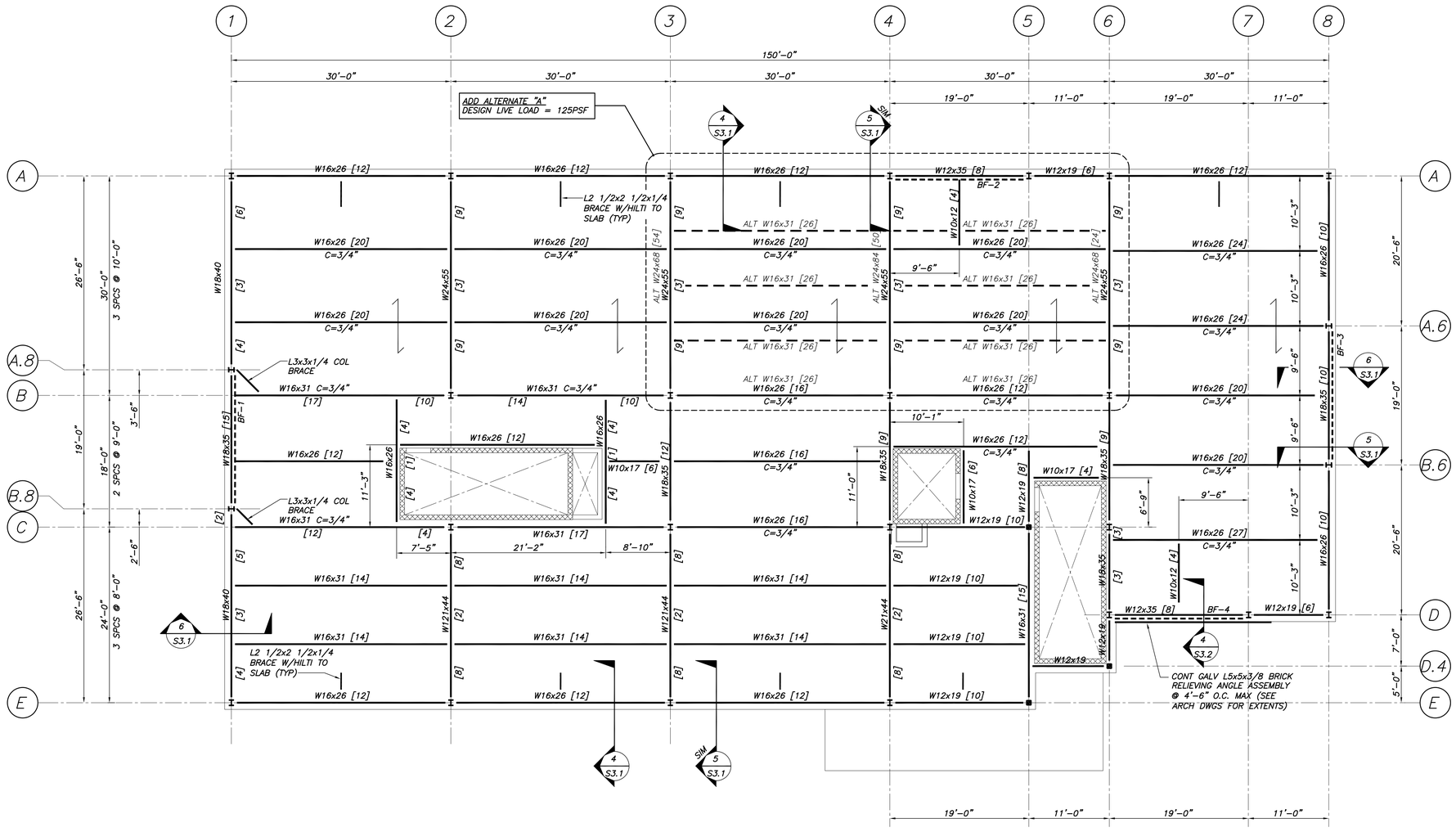

THIRD FLOOR FRAMING PLAN
1/8"=1'-0"

NOTES:

1. FLOOR CONSTRUCTION CONSISTS OF A 5 1/2" (MIN) NORMAL WEIGHT CONCRETE SLAB ON 3VLI 20GA GALVANIZED METAL DECK, (2 1/2" CONCRETE FILL OVER 3" DECK), W/6x6–W1.4xW1.4 W.W.F. PROVIDE ADDL CONC AS REQD TO ACHIEVE A LEVEL FLOOR FINISH.
2. TOP OF 5 1/2" CONC SLAB EL 59'–1 1/2".
3. TOP OF STEEL EL 58'–8" U.N.O.
4. [22] ETC INDICATES 3/4"x4 1/2" LG HEADED SHEAR STUDS SPACED UNIFORMLY ALONG BEAM OR BETWEEN CONCENTRATED LOADS.
5. INDICATES BRACING. SEE BRACING ELEVATIONS DWG S.1.6.
6. ALL STAIR STRUCTURES SHALL BE DESIGNED AND FABRICATED AS PER SPECIFICATIONS.
7. INDICATES DECK SPAN DIRECTION.
8. SEE ARCH DWGS FOR POUR STOP/SILL ANGLE REQUIREMENTS AT ELEVATOR.

PROJECT NORTH

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SCALE: NOTED

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TITLE
THIRD FLOOR FRAMING PLAN

SHEET
S1.3

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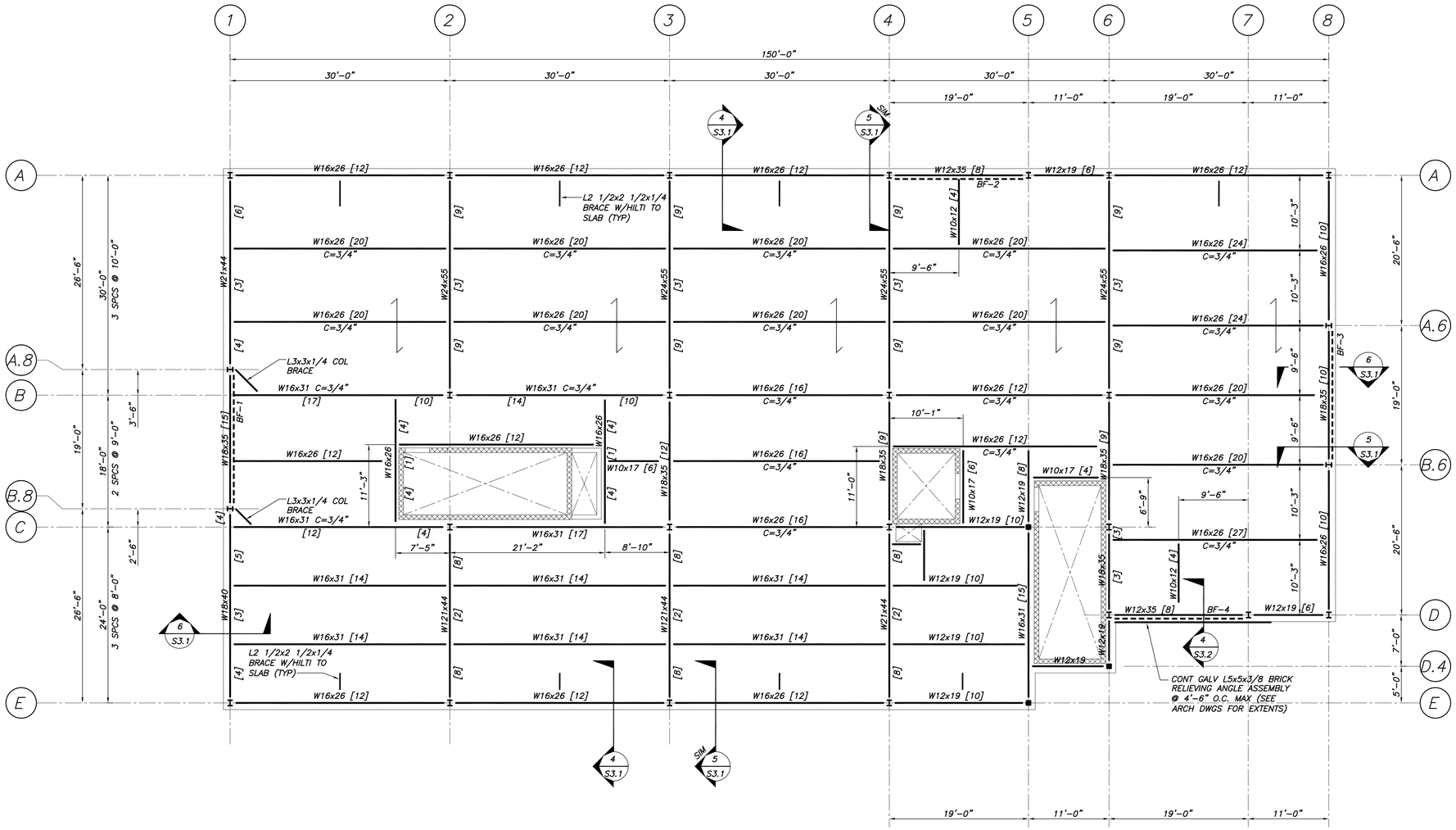

FOURTH FLOOR FRAMING PLAN

1/8"=1'-0"

NOTES:

1. FLOOR CONSTRUCTION CONSISTS OF A 5 1/2" (MIN) NORMAL WEIGHT CONCRETE SLAB ON 3VLI 20GA GALVANIZED METAL DECK, (2 1/2" CONCRETE FILL OVER 3" DECK), W/6x6-W1.4xW1.4 W.W.F. PROVIDE ADDL CONC AS REQD TO ACHIEVE A LEVEL FLOOR FINISH.
2. TOP OF 5 1/2" CONC SLAB EL 73'-1 1/2".
3. TOP OF STEEL EL 72'-8" U.N.O.
4. [22] ETC INDICATES 3/4"x4 1/2" LG HEADED SHEAR STUDS SPACED UNIFORMLY ALONG BEAM OR BETWEEN CONCENTRATED LOADS.
5. INDICATES BRACING. SEE BRACING ELEVATIONS DWG S.1.6.
6. ALL STAIR STRUCTURES SHALL BE DESIGNED AND FABRICATED AS PER SPECIFICATIONS.
7. INDICATES DECK SPAN DIRECTION.
8. SEE ARCH DWGS FOR POUR STOP/SILL ANGLE REQUIREMENTS AT ELEVATOR.

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SCALE: NOTED

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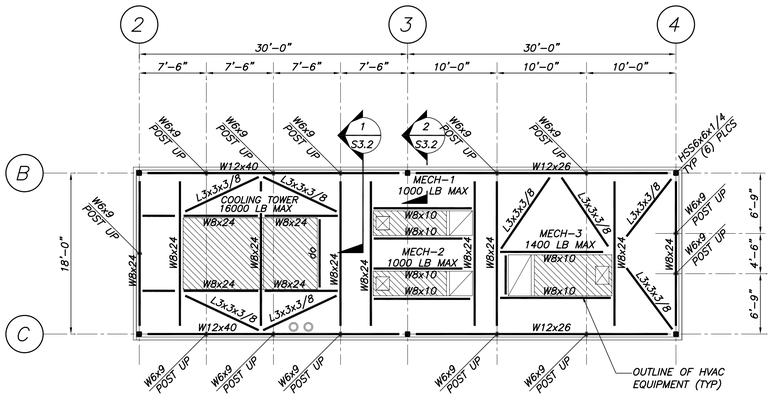
TITLE
FOURTH FLOOR FRAMING PLAN

SHEET
S1.4

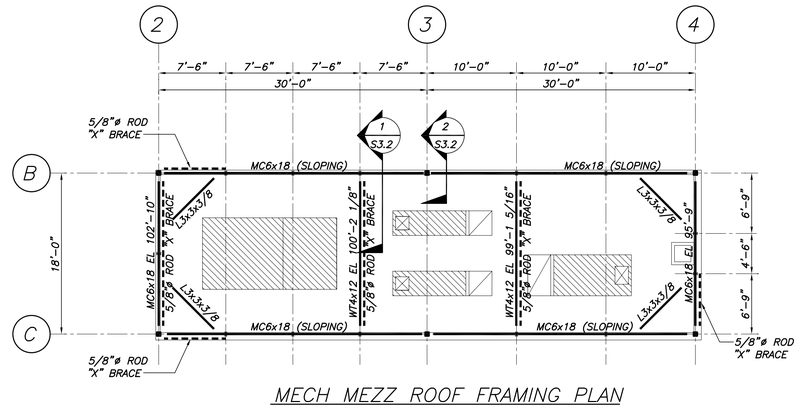


MECH MEZZ FRAMING PLAN

1/8"=1'-0"

NOTES:

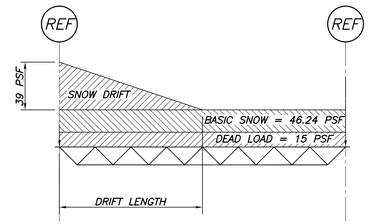
1. TOP OF STEEL EL 91'-6" U.N.O. BY (+8"), (-6"), ETC
2. HOT DIPPED GALV ALL EXPOSED ROOF STEEL AT MECH MEZZ.



MECH MEZZ ROOF FRAMING PLAN

1/8"=1'-0"

1. FOR TOP OF STEEL EL, SEE PLAN.
2. HOT DIPPED GALV ALL EXPOSED ROOF STEEL AT MECH MEZZ..

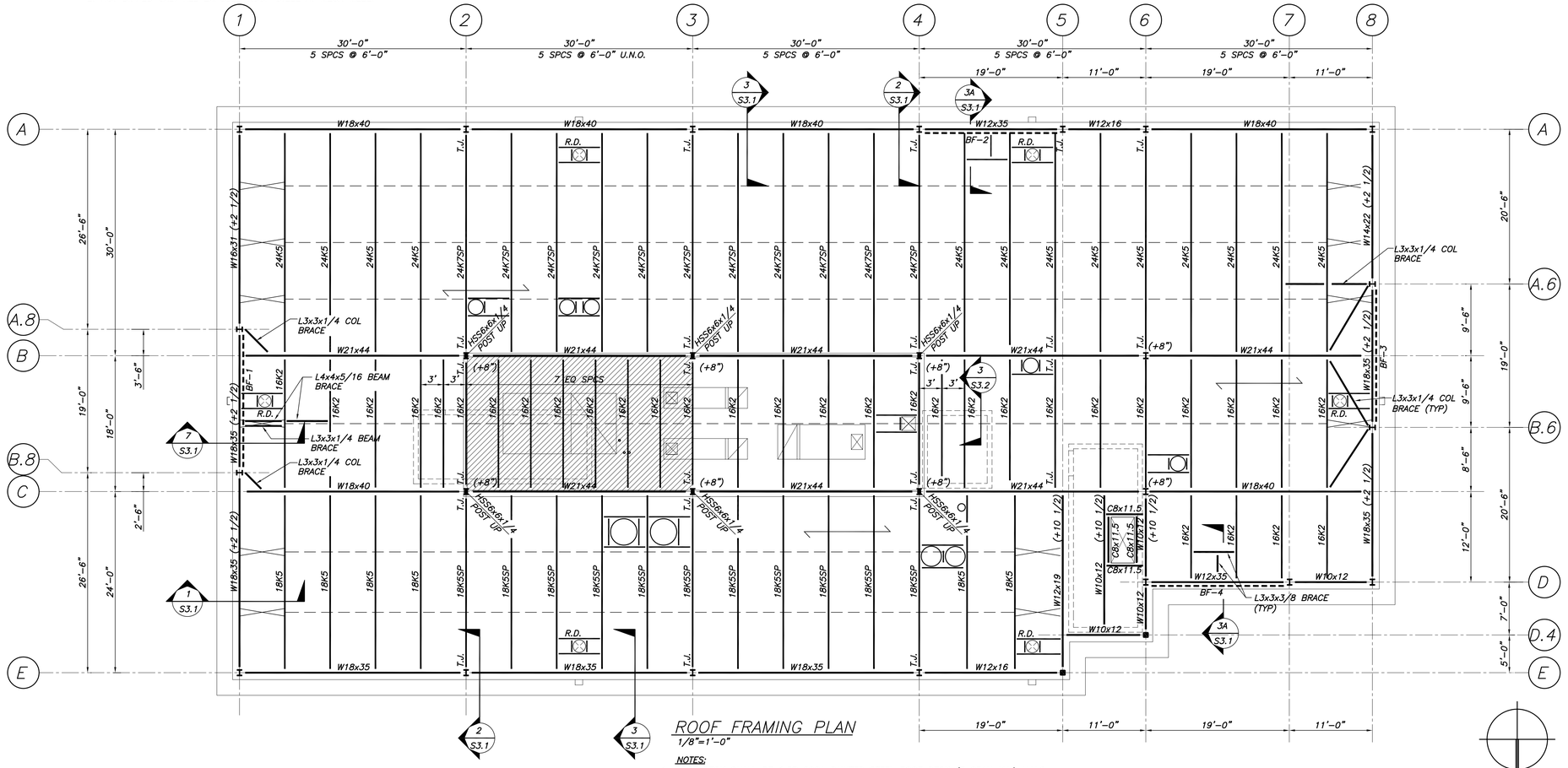


KSP JOIST LOADING DIAGRAM

N.T.S.

NOTES:

1. SEE PLAN FOR JOIST SPAN AND SPACING.
2. JOIST SUPPLIER SHALL DESIGN KSP JOISTS FOR THE LOADS INDICATED IN TABLE BELOW.



ROOF FRAMING PLAN

1/8"=1'-0"

NOTES:

1. ROOF DECK SHALL BE 1.5B 20GA PAINTED STEEL ROOF DECK (3 SPAN MIN) U.N.O.
2. TOP OF STEEL EL 87'-1 1/2" U.N.O. BY (+8"), (-6"), ETC
3. R.D. INDICATES ROOF DRAIN. SEE TYP OPENING IN DECK DETAIL, DWG S3.X.
4. BF-1 INDICATES BRACING. SEE BRACING ELEVATIONS DWG S1.6.
5. ALL STAIR STRUCTURES SHALL BE DESIGNED AND FABRICATED AS PER SPECIFICATIONS.
6. INDICATES DECK DIRECTION.
7. INDICATES AREA LOADED WITH CONCRETE PAVERS NOT EXCEEDING 35 PSF.
8. JOIST MANUF SHALL DESIGN "SP" JOISTS FOR THE LOADING INDICATED IN THE "SP" JOIST LOADING DIAGRAM THIS DWG.

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TITLE
ROOF FRAMING PLAN

SHEET
S1.5

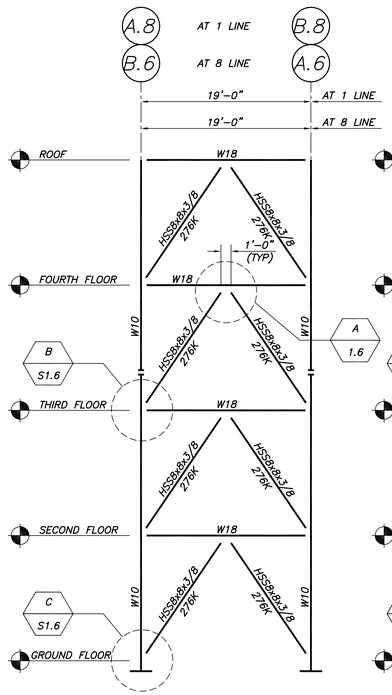
COLUMN SCHEDULE

COLUMN MARK	A–1	A–2	A–4	A–8	B–2	B–6	C–5	C–6	D.4–6	D–8	E–5	1CP
		A–3	A–5	E–1	B–3							2CP
		A–6	A.8–1		B–4							3CP
		E–2	B.8–1		C–2							4CP
		E–3	A.6–8		C–3							
		E–4	B.6–8		C–4							
			D–6									
PENTHOUSE ROOF			D–7									
					HSS6x6x1/4							
ROOF												
4th FLOOR	W8x24	W10x33	W10x45	W8x24	W10x39	W10x33		W10x33	HSS5x5x1/4	W8x24	HSS5x5x1/4	
3rd FLOOR												
2nd FLOOR	W8x35	W10x45	W10x68	W8x31	W10x68	W10x68	HSS6x6x5/16	W10x39	HSS6x6x1/4	W8x24	HSS6x6x5/16	HSS4x4x1/4
1st FLOOR												
BOT OF BASE PL ELEVATION U.N.O.	EL 29'–8 1/2"	EL 29'–8 1/2"	EL 29'–9 1/2"	EL 29'–8 1/2"	EL 29'–8 1/2"	EL 29'–8 1/2"	EL 29'–8 1/2"	EL 29'–8 1/2"	EL 29'–8 1/2"	EL 29'–8 1/2"	EL 29'–8 1/2"	EL
BASE PL TYPE	C	B	E	C	A	A	D	A	D	C	D	D

SEE COL SPLICE DETAILS DWG S3.3

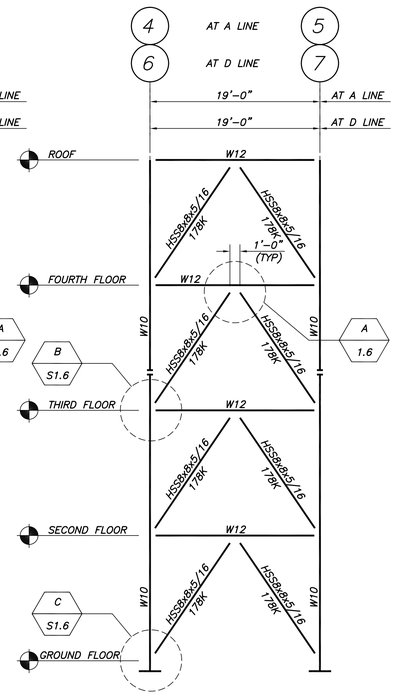
4'–0"

NOTE:
PROVIDE 1/2"CAP PL's AT W COLS & 3/8" CLOSURE PL's AT HSS COLS.



BF–1 AT 1 LINE

BF–3 AT 8 LINE



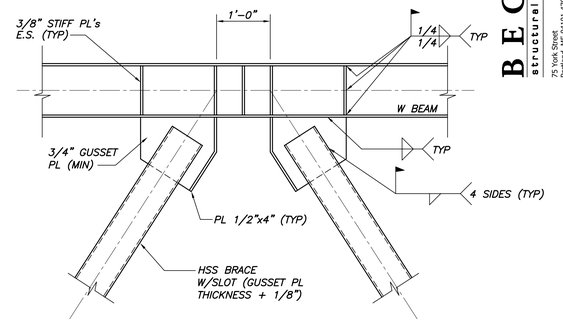
BF–2 AT A LINE

BF–4 AT D LINE

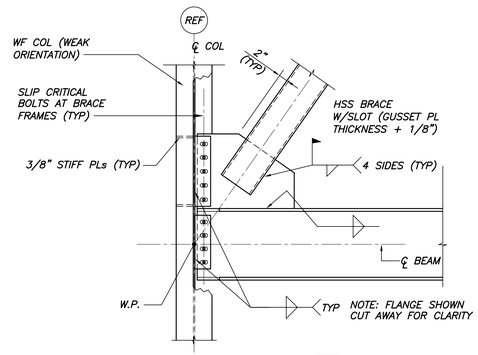
BRACING ELEVATIONS
N.T.S.

NOTES:

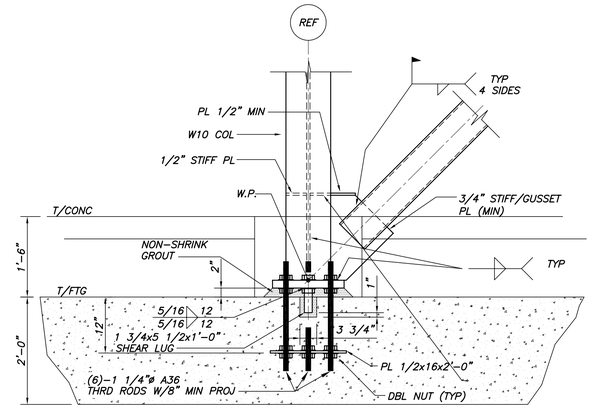
1. SEE DWG S1.0 FOR SEISMIC DESIGN PARAMETERS.
2. BRACING CONNECTIONS TO BE DESIGNED PER AISC 341–02 SEISMIC PROVISIONS.
3. ALL BRACES CLASSIFIED AS: ECCENTRIC BRACED FRAME.
4. "262K", ETC INDICATES CONNECTION "REQUIRED STRENGTH" (FACTORED LOAD) AS DEFINED IN AISC 341–02 SEISMIC PROVISIONS.
5. CONNECTION DESIGN CALCULATIONS TO BE STAMPED BY A STRUCTURAL ENGINEER LICENSED IN THE STATE OF MAINE AND SHALL INCLUDE THE FOLLOWING:
   A. GEOMETRY NECESSARY FOR UNIFORM FORCE METHOD CALCULATIONS.
   B. ALL APPLICABLE FAILURE MODE CHECKS.



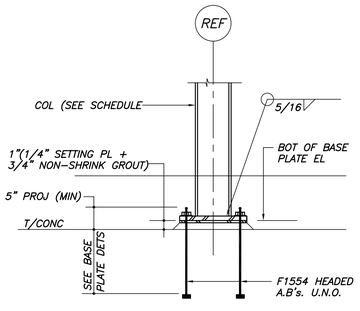
DETAIL
3/4"=1'–0"
A S1.6



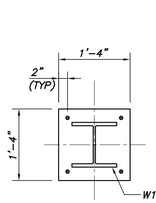
DETAIL
3/4"=1'–0"
B S1.6



DETAIL
3/4"=1'–0"
C S1.6

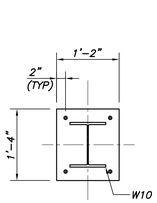


TYP COL BASE DETAIL U.N.O.
N.T.S.



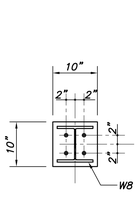
BASE PLATE TYPE "A"

PL 1 1/2x16x1'–4"
W/(4)–1 5/16"ø HOLES
FOR (4)–3/4"ø HEADED
A.B'S. W/9" EMBED



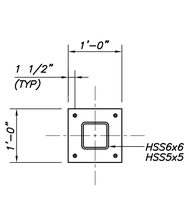
BASE PLATE TYPE "B"

PL 1 1/4x14x1'–4"
W/(4)–1 5/16"ø HOLES
FOR (4)–3/4"ø HEADED
A.B'S. W/9" EMBED



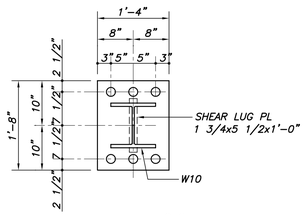
BASE PLATE TYPE "C"

PL 3/4x10x0'–10"
W/(4)–1 5/16"ø HOLES
FOR (4)–3/4"ø HEADED
A.B'S. W/9" EMBED



BASE PLATE TYPE "D"

PL 3/4x12x1'–0"
W/(4)–1 5/16"ø HOLES
FOR (4)–3/4"ø HEADED
A.B'S. W/9" EMBED



BASE PLATE TYPE "E"

PL 1 3/4x16x1'–18"
W/(4)–2 1/16"ø HOLES
FOR (4)–1 1/4"ø A36 THRD
RODS EXTENDED INTO FTG.

BASE PLATE DETAILS
3/4"=1'–0"

NOTE: SEE FRAMING PLANS AND BRACING ELEVATIONS FOR GUSSET PLATE SIZE & ORIENTATION.

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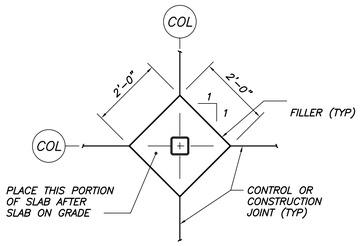
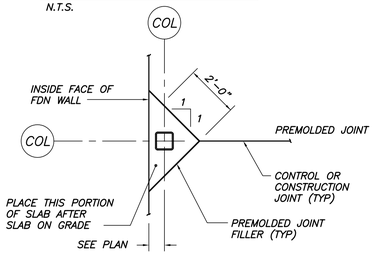
TITLE
COL SCHED &
BRACED FRAME
ELEVATIONS

SHEET
S1.6



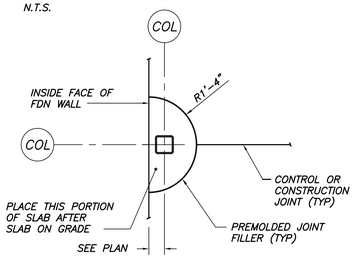
TYP SLAB ON GRADE DETAIL AT INTERIOR COLUMN

N.T.S.



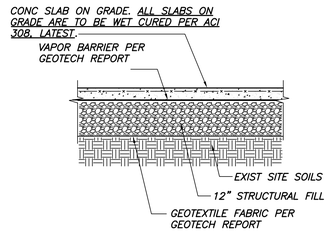
TYP SLAB ON GRADE DETAIL AT EXTERIOR COLUMN

N.T.S.



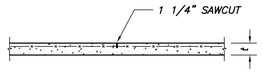
ALTERNATE ISOLATION JOINT SLAB ON GRADE DETAIL AT EXTERIOR COLUMN

N.T.S.



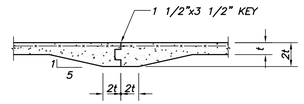
TYP SLAB DETAIL

N.T.S.



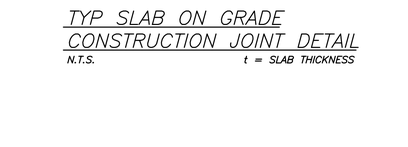
TYP SLAB ON GRADE CONTRACTION JOINT DETAIL

N.T.S. t = SLAB THICKNESS



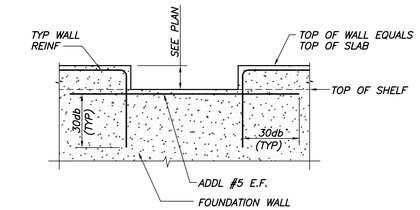
TYP SLAB ON GRADE CONSTRUCTION JOINT DETAIL

N.T.S. t = SLAB THICKNESS



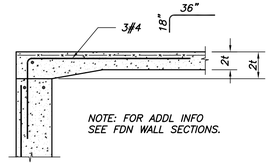
TYP CONSTRUCTION JOINT IN WALL

N.T.S. t = WALL THICKNESS



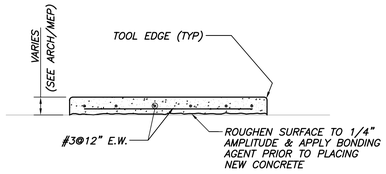
TYP WALL DEPRESSION DETAIL @ DOOR

N.T.S.



TYP SLAB DETAIL @ DOOR

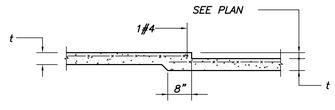
N.T.S. t = SLAB THICKNESS



NOTE: PAD SIZE AND LOCATION TO BE COORD W/MECH.

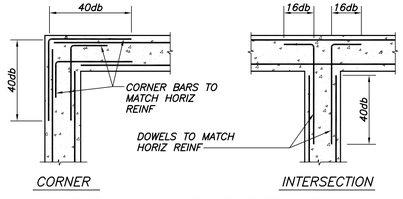
TYP EQUIPMENT HOUSEKEEPING PAD DETAIL

N.T.S. t = SLAB THICKNESS



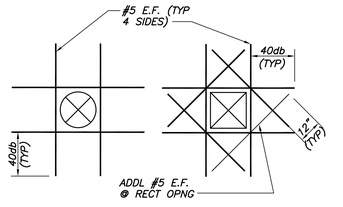
TYP SLAB ON GRADE DEPRESSION DETAIL

N.T.S. t = SLAB THICKNESS



TYP WALL REINF DETAILS

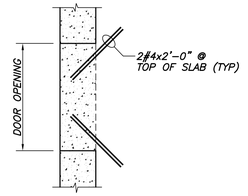
N.T.S.



TYP OPENING IN WALL OR SLAB

N.T.S.

REBAR LAP SPLICE TABLE	
BAR SIZE	LAP LENGTH (CLASS B)
	3,000 PSI
#3	21"
#4	28"
#5	35"
#6	42"
#7	50"
#8	66"



NOTE: PROVIDE 2#4x2'-0" (TOP) IN SLAB @INSIDE CORNERS, SEE PLAN.

TYP SLAB CORNER DETAIL @ DOOR

N.T.S.

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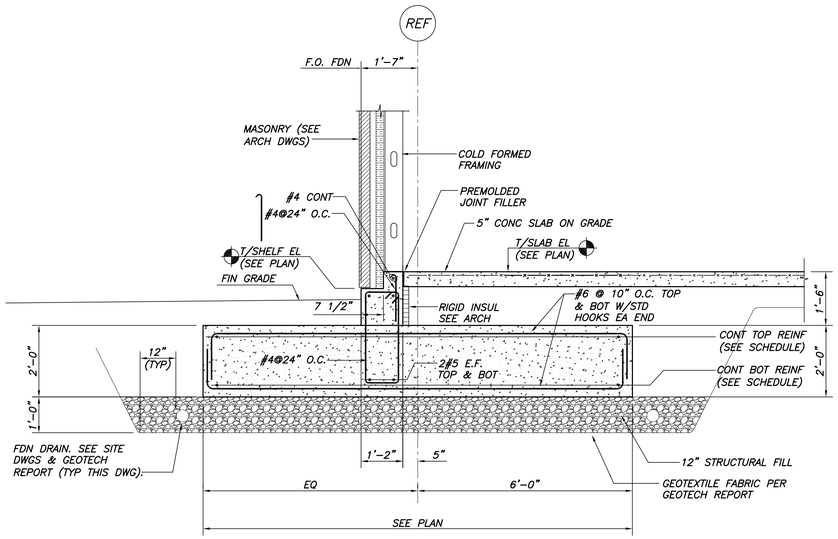
19 MAY 2005

TITLE
CONCRETE SECTIONS AND DETAILS

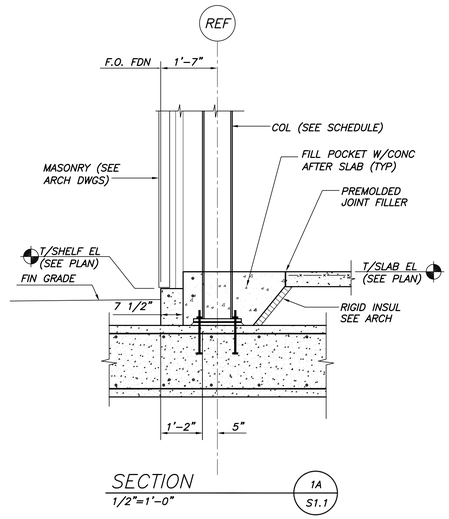
SHEET
S2.1

PERMIT SET
19 MAY 2005

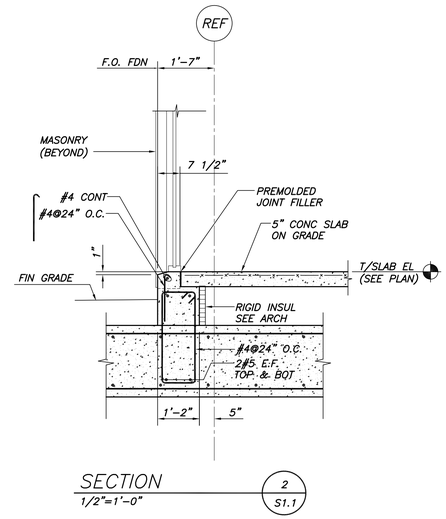


STRIP FOOTING SCHEDULE			
FOOTING WIDTH	FOOTING THK.	TOP REINF	BOT REINF
10'-0"	2'-0"	7#8	7#8
12'-0"	2'-0"	8#8	14#8
14'-0"	2'-0"	10#8	14#8

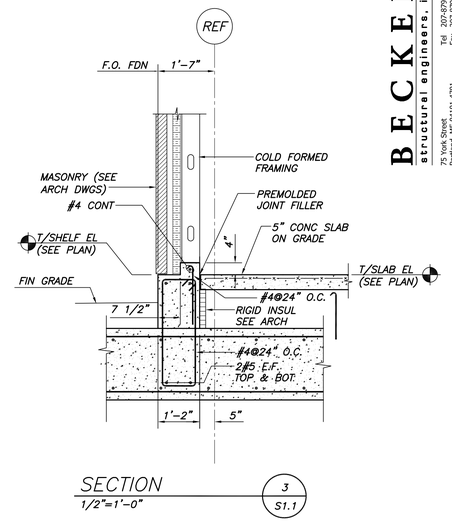
SECTION 1 / S1.1
1/2"=1'-0"



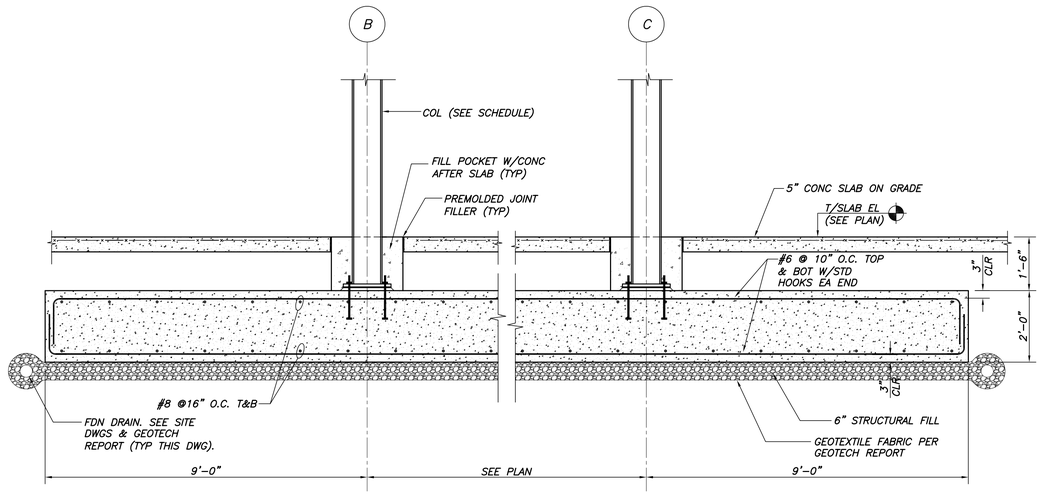
SECTION 1A / S1.1
1/2"=1'-0"



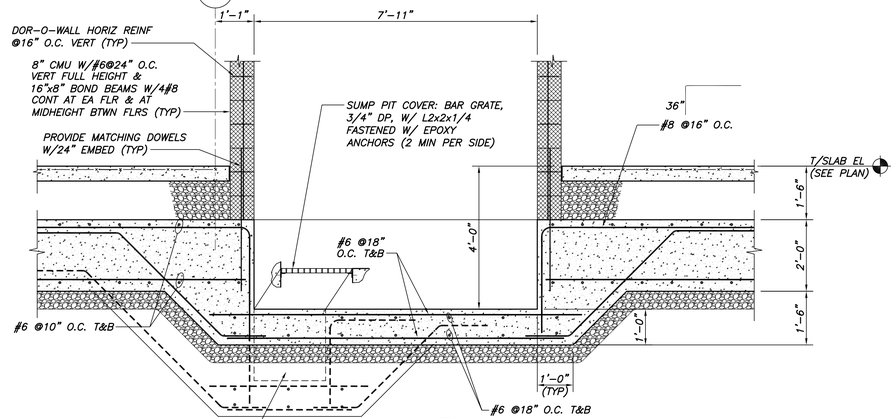
SECTION 2 / S1.1
1/2"=1'-0"



SECTION 3 / S1.1
1/2"=1'-0"



SECTION 4 / S1.1
1/2"=1'-0"



NOTES:
1. G.C. TO COORD SUMP PIT SIZE & LOCATION W/APPROVED ELEVATOR SHOP DWGS.
2. G.C. TO COORD HYDRAULICS BONDOUT W/ APPROVED ELEVATOR SHOP DWGS.

SECTION 5 / S1.1
1/2"=1'-0"

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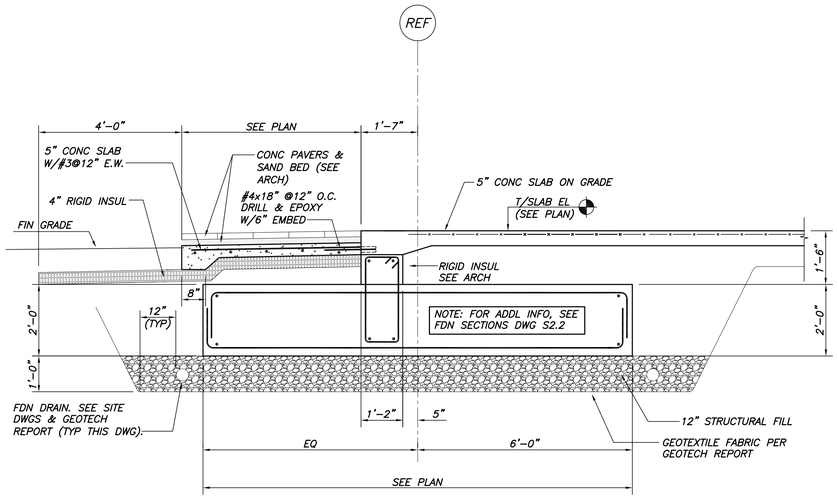
TITLE
CONCRETE SECTIONS AND DETAILS

SHEET
S2.2



TYP ENTRANCE SLAB DETAIL
1/2"=1'-0"

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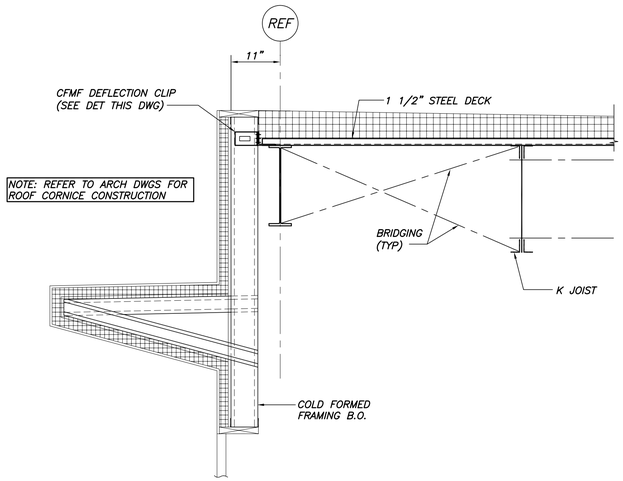
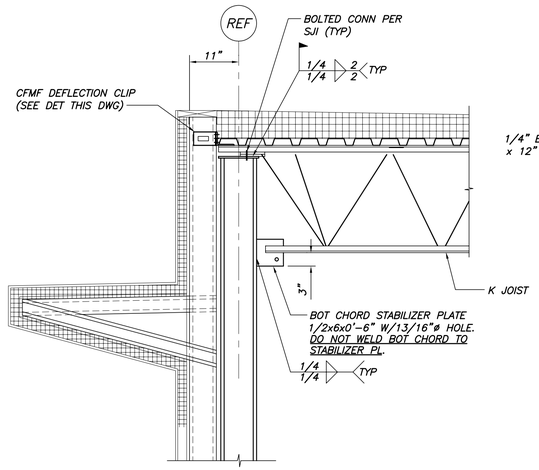
19 MAY 2005

TITLE
CONCRETE SECTIONS AND DETAILS

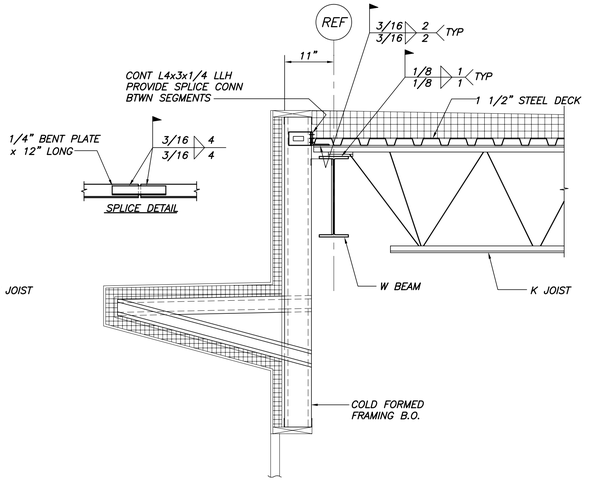
SHEET
S2.3



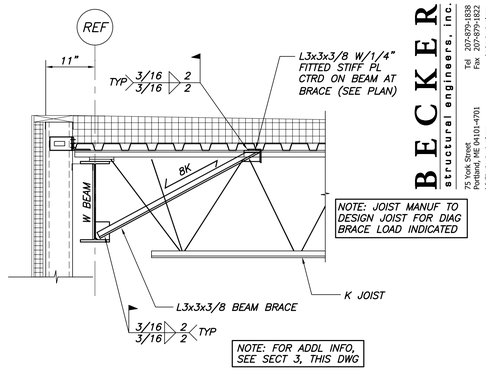
SECTION 1 S1.5
3/4"=1'-0"



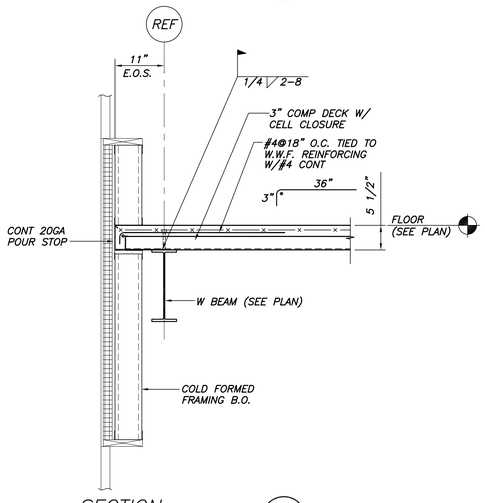
SECTION 2 S1.5
3/4"=1'-0"



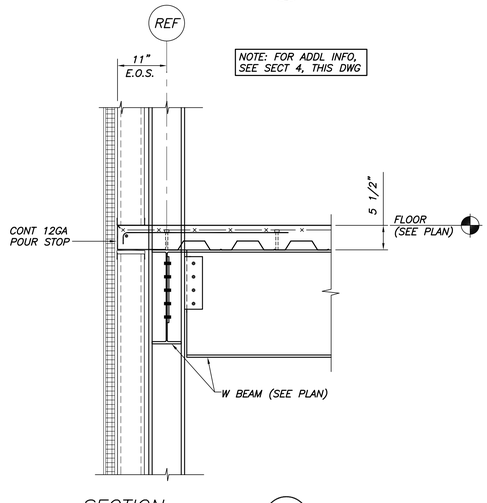
SECTION 3 S1.5
3/4"=1'-0"



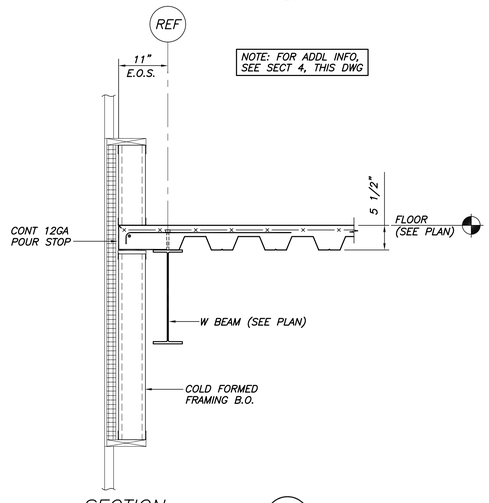
SECTION 3A S1.5
3/4"=1'-0"



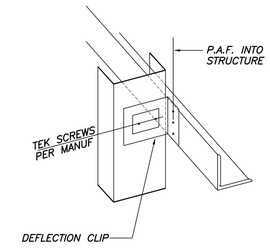
SECTION 4 S1.2–S1.4
3/4"=1'-0"



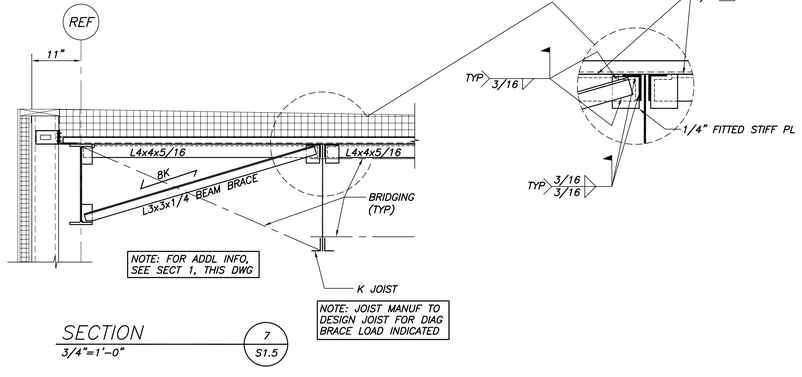
SECTION 5 S1.2–S1.4
3/4"=1'-0"



SECTION 6 S1.2–S1.4
3/4"=1'-0"



TYP EXTERIOR CFMF DEFLECTION CLIP
N.T.S.
NOTE: SUBMIT DESIGN PER SPEC.



SECTION 7 S1.5
3/4"=1'-0"

PERMIT SET
19 MAY 2005

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50 Sewall Street
PORTLAND, MAINE

JOB NO.
1270

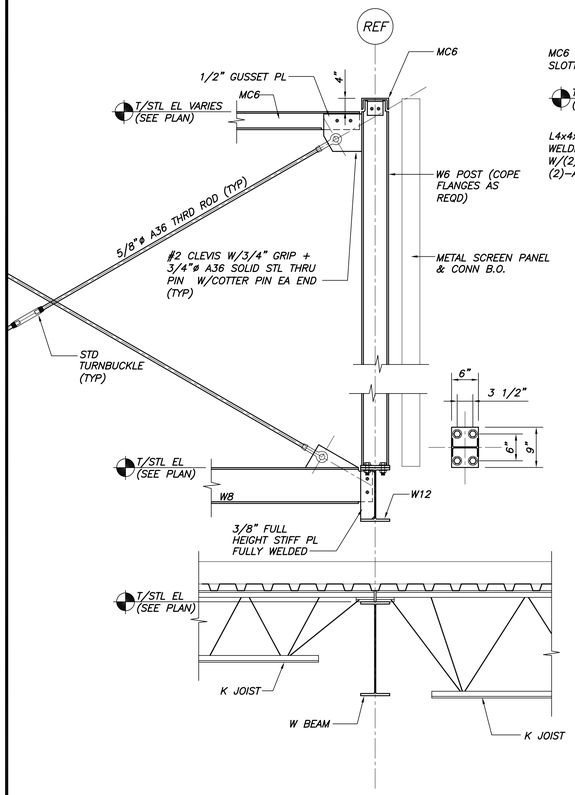
DRWN. APP/SMB

SCALE: NOTED

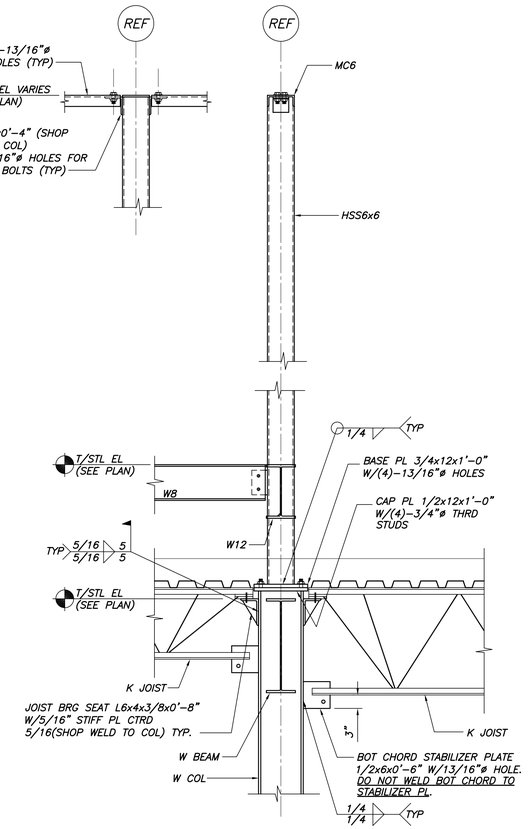
ISSUE
PERMIT SET
19 MAY 2005

TITLE
FRAMING
SECTIONS AND
DETAILS

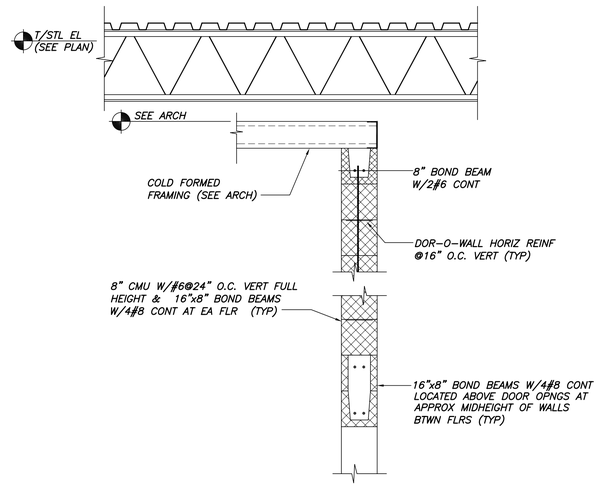
SHEET
S3.1



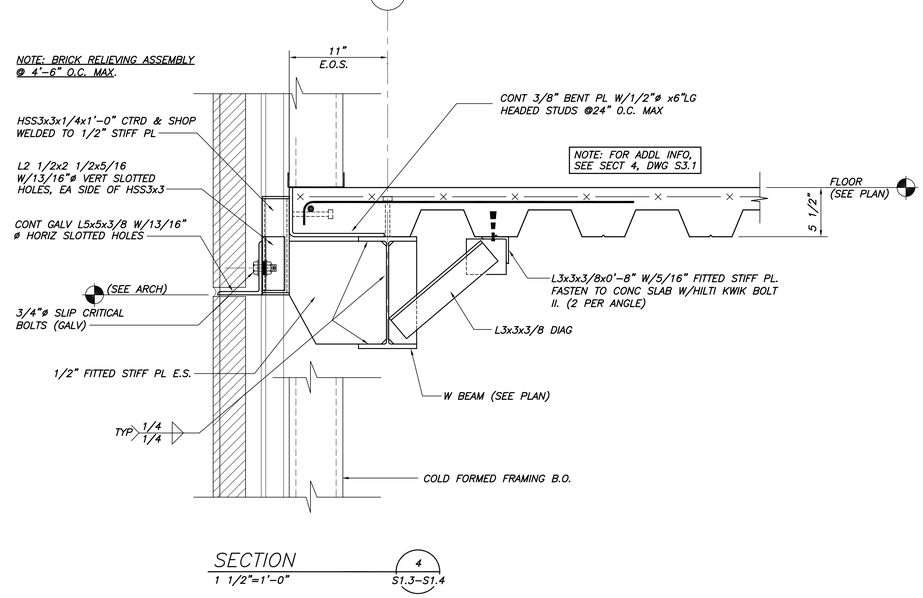
SECTION 1 S1.5
3/4"=1'-0"



SECTION 2 S1.5
3/4"=1'-0"



SECTION 3 S1.5
3/4"=1'-0"



SECTION 4 S1.3-S1.4
1 1/2"=1'-0"

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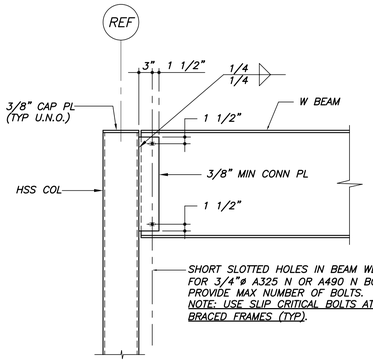
19 MAY 2005

TITLE
FRAMING SECTIONS AND DETAILS

SHEET
S3.2



TYP BEAM TO HSS COL CONN U.N.O.

N.T.S.

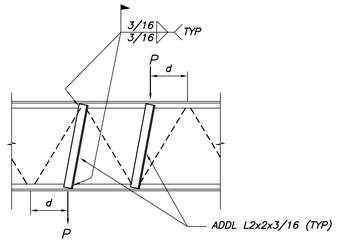
BEAM SIZE	DESIGN REACTION (WORKING LOAD)	MINIMUM NO BOLTS 1 SIDED CONNECTION	MINIMUM NO BOLTS 2 SIDED CONNECTION
W8	8K	2	2
W12	16k	3	2
W14	24K	3	3
W16	29K	4	3
W18	32K**	5	3
W21x44	45K	5	4

** 46K AT BRACE FRAME W18's.

SIMPLE SHEAR BEAM CONNECTIONS

SIMPLE SHEAR CONNECTIONS NOTES:

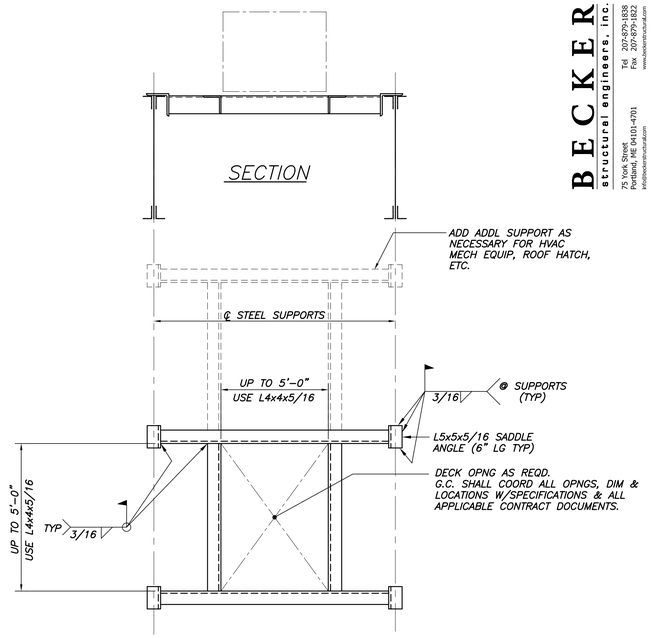
1. SIMPLE SHEAR CONNECTIONS SHALL BE SELECTED FROM THE AISC "MANUAL OF STEEL CONSTRUCTION – ALLOWABLE STRESS DESIGN, NINTH EDITION" USING THE ABOVE REFERENCED REACTIONS AND CRITERIA. REACTIONS INDICATED ARE UNFACTORED. MORE BOLTS THAN REFERENCED IN THE "MINIMUM" SECTIONS ABOVE MAY BE REQUIRED.
2. CONNECTIONS ARE SUBJECT TO REVIEW ON THE STEEL SHOP DRAWINGS.
3. ALL BOLTS SHALL BE A325 OR A490 FOR SIMPLE SHEAR CONNECTIONS, MIN 3/4"ø. MINIMUM WELD SIZE SHALL BE 5/16". MINIMUM MATERIAL SIZE FOR PLATES OR ANGLES SHALL BE 3/8"
4. ONE SIDED CONNECTIONS INCLUDE SINGLE PLATES AND SINGLE ANGLE CONNECTIONS.
5. TWO SIDED CONNECTIONS INCLUDE DOUBLE ANGLE AND END PLATE CONNECTIONS.



TYP JOIST REINF DETAIL

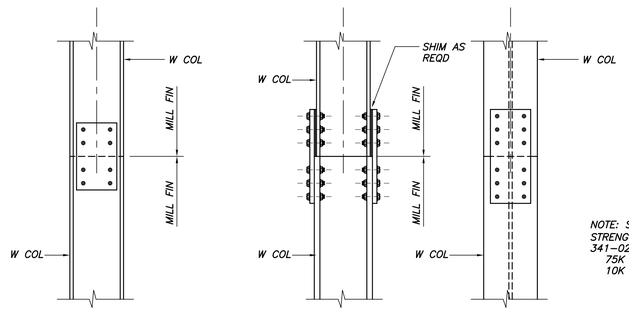
N.T.S.

1. FOR CONCENTRATED LOADS LOCATED A DISTANCE "d", 4" OR MORE FROM PANEL POINT, PROVIDE ADDITIONAL WEB MEMBERS AS SHOWN.
2. P = 300# MAX LOAD.



PLAN OF TYP OPENING IN ROOF DECK

N.T.S. TYPICAL @ FANS, ROOF DRAINS & OTHER MECH & HVAC EQUIP U.N.O.



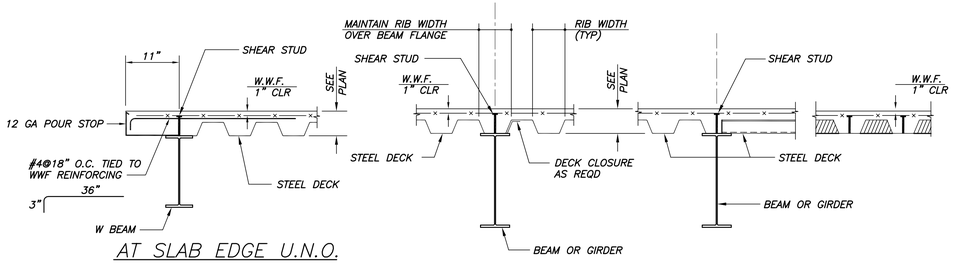
TYP SPLICE

SPLICE DETAIL AT BRACING

NOTE: SPLICE SHALL BE DESIGNED FOR THE "REQUIRED STRENGTH" (FACTORED LOAD) AS REFERENCED IN AISC 341-02 SEISMIC PROVISIONS.
75K COMPRESSION
10K TENSION

TYP COL SPLICE DETAILS

N.T.S.



AT SLAB EDGE U.N.O.

TYP COMPOSITE DECK DETAILS

N.T.S.

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