Job	Truss	Truss Type	Qty	Ply	
860330	002	GESI	1	1	A_MGE_e125947_7/3/2014 4:07:29 PM Job Reference (optional)

Boise Structural Solutions, Biddeford, ME 04005, SAMANTHA TURBIDE

7.530 s Jul 14 2014 MiTek Industries, Inc. Tue Sep 16 17:25:01 2014 Page 1

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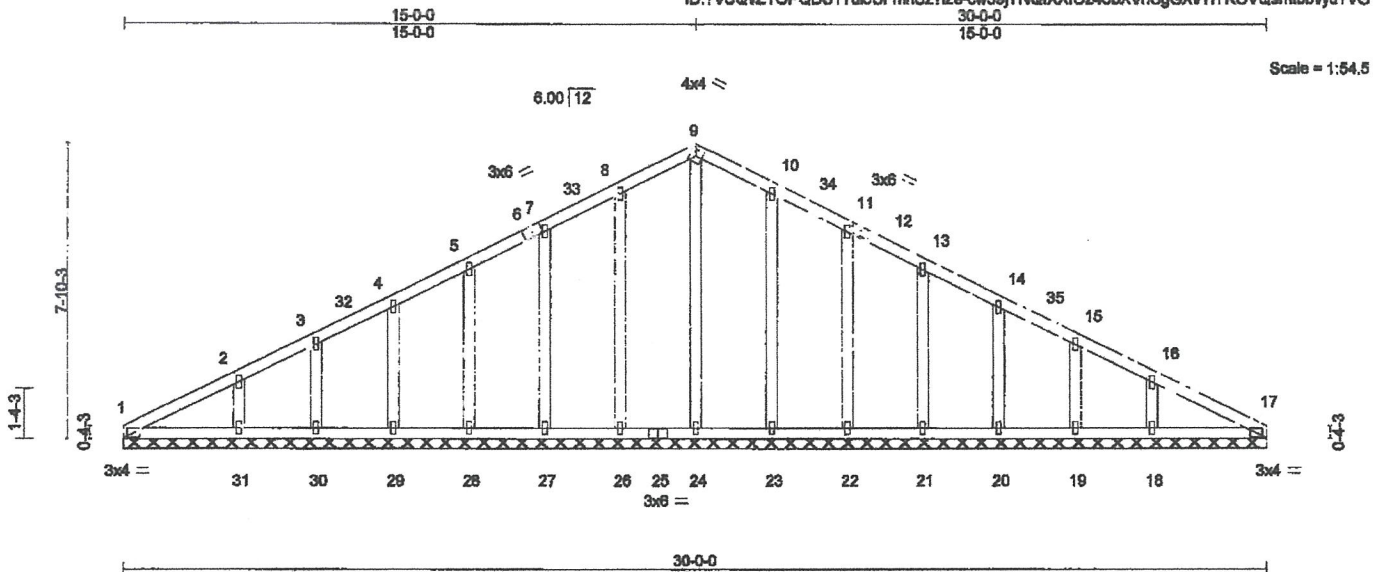

Plate Offsets (X,Y)-- [6:0-3-0,Edge], [12:0-3-0,Edge]

LOADING (psf)		SPACING-	2-0-0	CSI		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	46.2	Plates Increase	1.15	TC	0.16	Vert(LL)	n/a	-	n/a	999	MT20	169/123
(Ground Snow=60.0)		Lumber Increase	1.15	BC	0.09	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.32	Horz(TL)	0.01	17	n/a	n/a		
BCLL	0.0 *	Code IBC2009/TPI2007		(Matrix)							Weight: 119 lb	FT = 0%
BCDL	10.0											

LUMBER-
TOP CHORD 2x4 SPF-S No.2
BOT CHORD 2x4 SPF 1650F 1.5E
WEBS 2x4 SPF-S No.2
OTHERS 2x4 SPF-S No.2

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. All bearings 30-0-0.
(lb) - Max Horz 1=-189(LC 7)
Max Uplift All uplift 100 lb or less at joint(s) 1, 26, 27, 28, 23, 22, 21 except 29=-103(LC 9), 30=-116(LC 9), 31=-237(LC 9), 20=-103(LC 10), 19=-116(LC 10), 18=-237(LC 10)
Max Grav All reactions 250 lb or less at joint(s) 1, 17, 24, 30, 19 except 26=372(LC 13), 27=366(LC 13), 28=311(LC 13), 29=278(LC 1), 31=408(LC 1), 23=372(LC 14), 22=366(LC 14), 21=311(LC 14), 20=278(LC 1), 18=408(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 7-33=-97/251, 8-33=-26/257, 8-9=-101/343, 9-10=-101/343, 10-34=-26/257, 11-34=-97/251
WEBS 8-26=-332/170, 7-27=-326/145, 5-28=-272/127, 2-31=-314/242, 10-23=-332/170, 11-22=-326/145, 13-21=-272/127, 16-18=-314/242

NOTES- (13)
1) Wind: ASCE 7-05; 120mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 12-0-0, Exterior(2) 12-0-0 to 15-0-0, Interior(1) 18-0-0 to 27-0-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
3) TCLL: ASCE 7-05; Pg= 60.0 psf (ground snow); Pf=46.2 psf (flat roof snow); Category II; Exp C; Partially Exp.; Ct=1.1
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
6) All plates are 1.5x4 MT20 unless otherwise indicated.
7) Gable requires continuous bottom chord bearing.
8) Gable studs spaced at 2-0-0 oc.
9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 26, 27, 28, 23, 22, 21 except (jt=lb) 29=103, 30=116, 31=237, 20=103, 19=116, 18=237.
12) This truss is designed in accordance with the 2009 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
13) Drawing prepared exclusively for manufacturing by Boise Structural Solutions

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	
660330	001	FINK	14	1	A_PMT_e125947_7/3/2014 4:07:29 PM Job Reference (optional)

Boise Structural Solutions, Biddeford, ME 04005, SAMANTHA TURBIDE

7.530 s Jul 14 2014 MiTek Industries, Inc. Tue Sep 16 17:24:35 2014 Page 1

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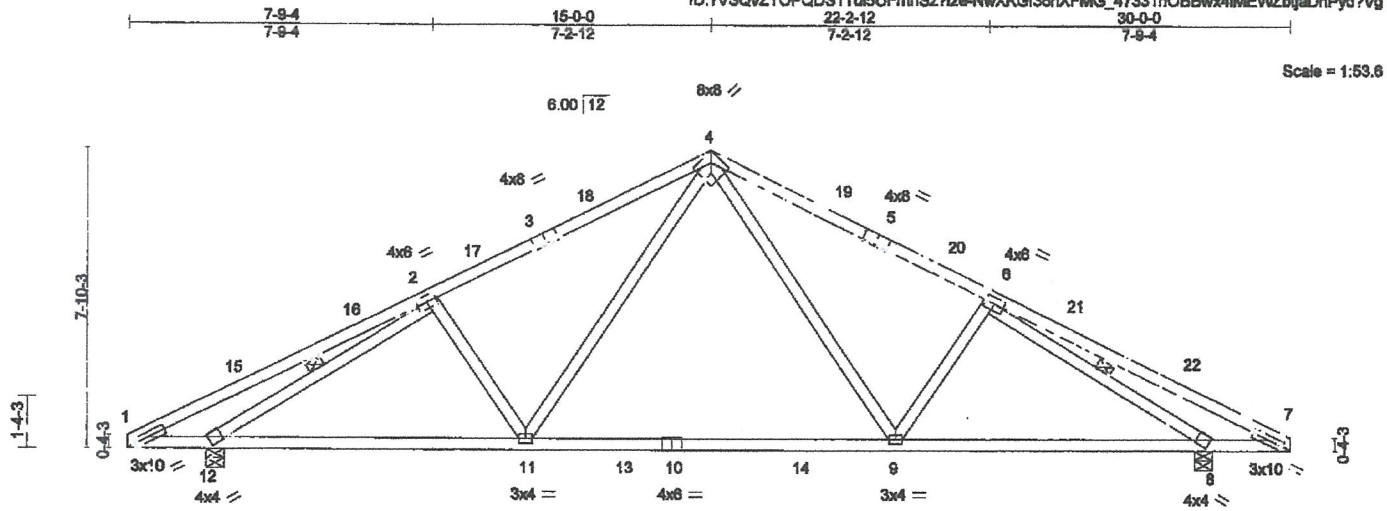


Plate Offsets (X,Y)-- [1:0-2-9,0-1-8], [3:0-4-0,Edge], [4:0-2-12,Edge], [5:0-4-0,Edge], [7:0-2-9,0-1-8]

LOADING (psf)		SPACING-	2-0-0	CSI		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	46.2	Plates Increase	1.15	TC	0.89	Vert(LL)	-0.39	9-11	>800	240	MT20	169/123
(Ground Snow=60.0)		Lumber Increase	1.15	BC	0.80	Vert(TL)	-0.68	9-11	>460	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.80	Horz(TL)	0.08	8	n/a	n/a		
BCLL	0.0 *	Code IBC2009/TPI2007		(Matrix)							Weight: 112 lb	FT = 0%
BCDL	10.0											

LUMBER-
TOP CHORD 2x4 SPF 1650F 1.5E *Except* T1,T4: 2x4 SPF 2100F 1.8E
BOT CHORD 2x4 SPF 1650F 1.5E
WEBS 2x4 SPF-S No.2
WEDGE
Left: 2x3 SPF No.2, Right: 2x3 SPF No.2

BRACING-
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 2-12, 6-8

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 12=1986/0-5-8 (min. 0-3-2), 8=1986/0-5-8 (min. 0-3-2)
Max Horz 12=189(LC 8)
Max Uplift12=-695(LC 9), 8=-695(LC 10)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-15=-318/182, 15-16=-299/210, 2-16=-281/419, 2-17=-2225/863, 3-17=-2075/875, 3-18=-2038/880, 4-18=-2008/898, 4-19=-2008/898, 5-19=-2038/880, 5-20=-2075/875, 6-20=-2225/863, 6-21=-281/419, 21-22=-299/210, 7-22=-318/182
BOT CHORD 1-12=-187/416, 11-12=-558/2041, 11-13=-288/1498, 10-13=-288/1498, 10-14=-288/1498, 9-14=-288/1498, 8-9=-558/2041, 7-8=-187/416
WEBS 2-11=-557/344, 4-11=-197/752, 4-9=-197/752, 6-9=-557/344, 2-12=-2695/1180, 6-8=-2695/1180

NOTES- (9)
1) Wind: ASCE 7-05; 120mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 12-0-0, Exterior(2) 12-0-0 to 15-0-0, Interior(1) 18-0-0 to 27-0-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-05; Pg= 60.0 psf (ground snow); Pf=46.2 psf (flat roof snow); Category II; Exp C; Partially Exp.; Ct=1.1
3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=695, 8=695.
8) This truss is designed in accordance with the 2009 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
9) Drawing prepared exclusively for manufacturing by Boise Structural Solutions

LOAD CASE(S) Standard

Boise Cascade Building Materials Distribution
Boise Structural Solutions
20 Pomerleau St.
Biddeford, Me 04005
Tel: 877-291-5276
Fax: 877-782-0999

Customer: MEWE00 - HILLSIDE LUMBER - ENAP 0067
781 COUNTY ROAD
WESTBROOK, ME04092

Contact: JOE HALL
Email: joe.hall@hillsidelumber.com
Phone: Fax:

Job Name: LEDUE

PORTLAND, ME

Prepared By:	Date Quoted:	Delivery Date:	Last Revised:	Price Protected Until:
SAMANTHA EXT 2761	07/03/2014		09/16/2014	09/23/2014

ROOF TRUSSES Designed per: IBC2009/TPI2007 Code.

PROFILE	LBL	QTY / PLY	OVRALL LGTH / WEIGHT	NET SPAN	PITCH TOP	PITCH BOT	TYPE	SPC	OVERHANG LEFT	OVERHANG RIGHT	CUT	LOADING TLL-TDL-BLL-BDL / Heel Height Left	LOADING Heel Height Right	CANTILEVER LEFT	CANTILEVER RIGHT	BRG SIZE LEFT	BRG SIZE RIGHT
 A660330-0001	001	14	30-00-00	26-00-00	6.00	0.00	FINK	24	00-00-00	00-00-00	P	46.2-10-0-10 Gmd Snow=60		02-00-00	02-00-00	00-05-08	00-05-08
		1	112 lbs									00-04-03	00-04-03			BRG#:	2
 A660330-0002	002	1	30-00-00	30-00-00	6.00	0.00	GESI	24	00-00-00	00-00-00	P	46.2-10-0-10 Gmd Snow=60		00-00-00	00-00-00	30-00-00	00-00-00
		1	118 lbs									00-04-03	00-04-03			BRG#:	1

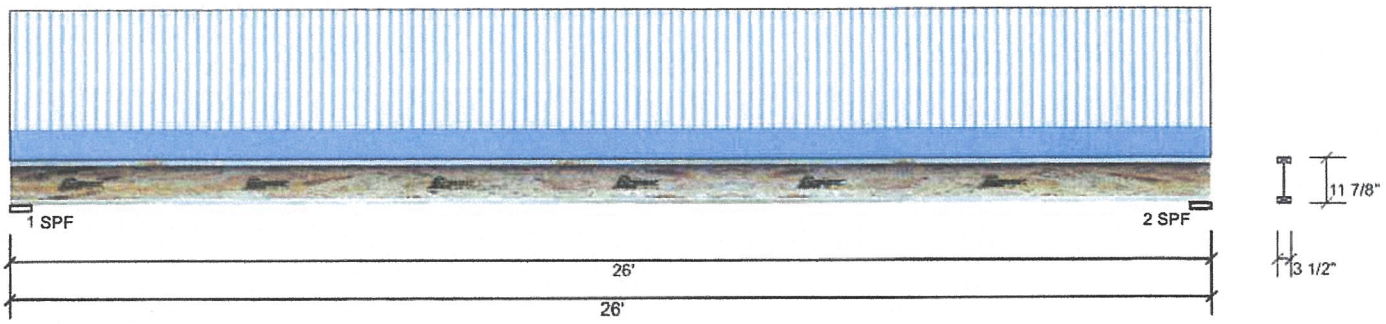
Total Weight: 1686 lbs

Report Date/Time: 9/16/2014 5:27:18 PM

NORDIC ENGINEERED WOOD	Client		Shipping 15 JEANNE COURT PORTLAND, ME
Project Name:	Job#:	Quantity 1	Description:

FLOOR JOIST NI-90x 11.875" - PASSED

9/16/2014 12:37 PM
Page 1 of 1
Designer: JLP



Type:	Joist	Application:	Floor
Spacing:	12" o.c.	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC 2012 / IRC
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
Temperature:	Temp <= 100°F		

Reactions lb (PLF)

Brg	Live	Dead	Snow	Wind	Const
1	520 (520)	130 (130)	0 (0)	0 (0)	0 (0)
2	520 (520)	130 (130)	0 (0)	0 (0)	0 (0)

Bearings

Bearing	Input Length	In Analysis	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	1.750"	37%	130 / 520	650	L	D+L
2 - SPF	5.500"	1.750"	37%	130 / 520	650	L	D+L

Analysis	Actual	Location	Allowed	Capacity	Load Comb.	Ld. Case
Moment	3978 ft-lb	13'	9465 ft-lb	0.420 (42%)	D+L	L
Unbraced	796 ft-lb	13'	796 ft-lb	1.000 (100%)	D	Uniform
Shear	631 lb	4 5/8"	2055 lb	0.307 (31%)	D+L	L
LL Defl inch	0.642 (L/471)	13' 1/16"	0.841 (L/360)	0.760 (76%)	L	L
TL Defl inch	0.803 (L/377)	13' 1/16"	1.261 (L/240)	0.640 (64%)	D+L	L

Design OK.

Design Notes

1 Top flange must be laterally braced at a maximum of 9' o.c.
2 Bottom flange unbraced.

ID	Load Type	Location	Trib Width	Dead	Live	Snow	Wind	Const.	Comments
1	Uniform			10 PSF	40 PSF	0 PSF	0 PSF	0 PSF	

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application and to verify the dimensions and loads.

Lumber

1 Dry service conditions unless noted otherwise.
2 Joist not to be treated with fire retardant or corrosive chemicals.

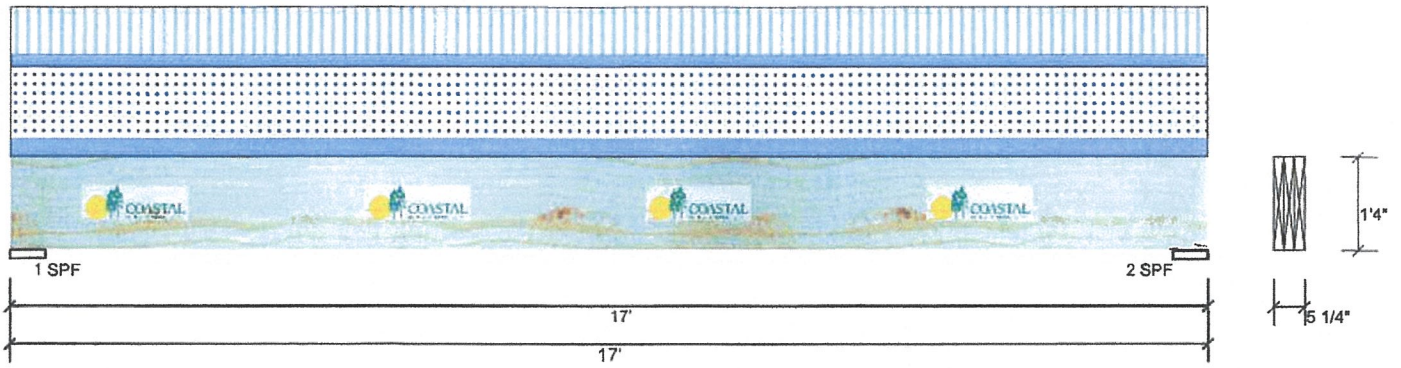
Handling & Installation

1 I-Joist flanges must not be cut or drilled.
2 Refer to latest copy of the I-Joist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details.
3 Damaged I-Joists must not be used.
4 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
5 Provide lateral support at bearing points to avoid lateral displacement and rotation.
6 Web stiffeners for point load as shown. Minimum point load bearing length>= 3.5 inches.
7 For flat roofs provide proper drainage to prevent ponding.

Coastal Forest Products
451 South River Rd, NH
USA
03110

COASTAL FOREST PRODUCTS, INC.

COASTAL FOREST PRODUCTS, INC.	Client		Shipping 15 JEANNE COURT PORTLAND, ME
Project Name:	Job#:	Quantity 1 (3pcs.)	Description:
GARAGE DOOR HEADER 2.0E CP-LAM 1.750" X 16.000" 3-Ply - PASSED			9/16/2014 12:45 PM Page 1 of 1 Designer: JLP



Type:	Girder	Application:	Floor
Plies:	3	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC 2012 / IRC
Deflection LL:	360	Load Sharing:	Yes
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
Temperature:	Temp <= 100°F		

Reactions

Brg	Live	Dead	Snow	Wind	Const
1	4280	2854	6420	0	0
2	4280	2854	6420	0	0

Bearings

Bearing	Input Length	In Analysis	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	6.000"	5.000"	98%	2854 / 8025	10879	L	D+0.75(L+S)
2 - SPF	6.000"	5.000"	98%	2854 / 8025	10879	L	D+0.75(L+S)

Analysis	Actual	Location	Allowed	Capacity	Load Comb.	Ld. Case
Moment	44530 ft-lb	8'6"	65339 ft-lb	0.682 (68%)	D+0.75(L+S)	L
Unbraced	44530 ft-lb	8'6"	61625 ft-lb	0.723 (72%)	D+0.75(L+S)	L
Shear	9087 lb	15'4 1/2"	18354 lb	0.495 (50%)	D+0.75(L+S)	L
LL Defl inch	0.445 (L/443)	8'6 1/16"	0.547 (L/360)	0.810 (81%)	0.75(L+S)	L
TL Defl inch	0.603 (L/327)	8'6 1/16"	0.821 (L/240)	0.730 (73%)	D+0.75(L+S)	L

Design OK

Design Notes

1 Girders are designed to be supported on the bottom edge only.
2 Multiple plies must be fastened together as per manufacturer's details.
3 Top loads must be supported equally by all plies.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Const.	Comments
1	Uniform		13-0-0	Top	15 PSF	0 PSF	60 PSF	0 PSF	0 PSF	
2	Uniform		13-0-0	Top	10 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
	Self Weight				22 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1 Dry service conditions unless noted otherwise
2 LVL not to be treated with fire retardant or corrosive chemicals

Handling & Installation

1 LVL beams must not be cut or drilled
2 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values and code approvals
3 Damaged Beams must not be used
4 Design assumes top edge is laterally restrained
5 Provide lateral support at bearing points to avoid lateral displacement and rotation
6 For flat roofs provide proper drainage to prevent ponding

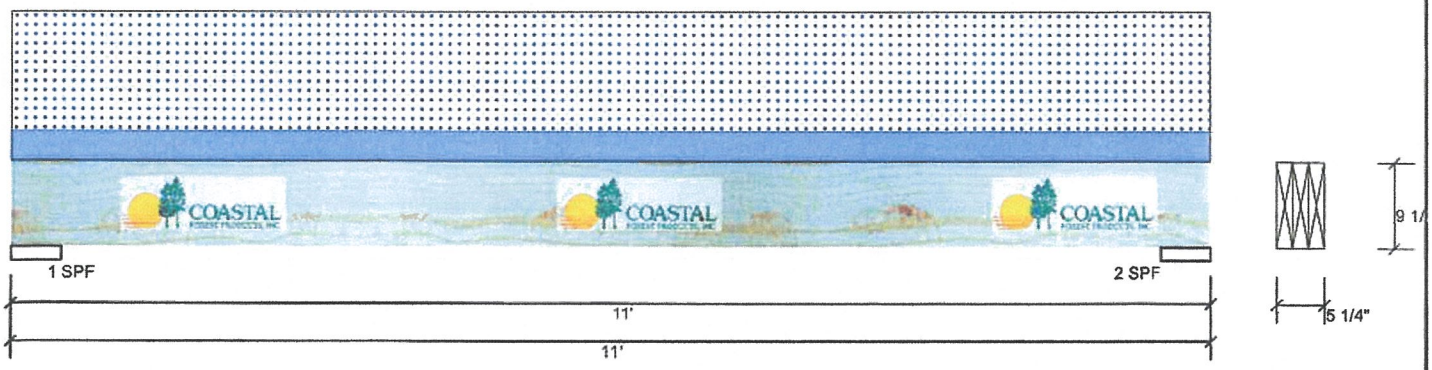
Coastal Forest Products
451 South River Rd, NH
USA
03110

COASTAL FOREST PRODUCTS, INC.

COASTAL FOREST PRODUCTS INC.	Client		Shipping 15 JEANNE COURT PORTLAND, ME
Project Name:	Job#:	Quantity 1 (3pcs.)	Description:

HEADER 2.0E CP-LAM 1.750" X 9.500" 3-Ply - PASSED

9/16/2014 12:43 PM
Page 1 of 1
Designer: JLP



Type:	Girder	Application:	Floor
Plies:	3	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC 2012 / IRC
Deflection LL:	360	Load Sharing:	Yes
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
Temperature:	Temp <= 100°F		

Reactions

Brg	Live	Dead	Snow	Wind	Const
1	0	1075	4035	0	0
2	0	1075	4035	0	0

Bearings

Bearing	Input Length	In Analysis	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	2.500"	92%	1075 / 4035	5110	L	D+S
2 - SPF	5.500"	2.500"	92%	1075 / 4035	5110	L	D+S

Analysis	Actual	Location	Allowed	Capacity	Load Comb.	Ld. Case
Moment	13080 ft-lb	5'6"	25566 ft-lb	0.512 (51%)	D+S	L
Unbraced	13080 ft-lb	5'6"	25181 ft-lb	0.519 (52%)	D+S	L
Shear	4302 lb	1'1 3/4"	10898 lb	0.395 (39%)	D+S	L
LL Defl inch	0.262 (L/471)	5'6"	0.343 (L/360)	0.770 (77%)	S	L
TL Defl inch	0.332 (L/372)	5'6"	0.515 (L/240)	0.650 (65%)	D+S	L

Design OK.

Design Notes

1 Girders are designed to be supported on the bottom edge only.
2 Multiple plies must be fastened together as per manufacturer's details.
3 Top loads must be supported equally by all plies.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Const.	Comments
1	Uniform		13-0-0	Top	15 PSF	0 PSF	60 PSF	0 PSF	0 PSF	
	Self Weight				13 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application and to verify the dimensions and loads

Lumber

1 Dry service conditions unless noted otherwise
2 LVL not to be treated with fire retardant or corrosive chemicals

Handling & Installation

1 LVL beams must not be cut or drilled
2 Refer to manufacturers product information regarding installation requirements, multi-ply fastening details, beam strength values and code approvals
3 Damaged Beams must not be used
4 Design assumes top edge is laterally restrained
5 Provide lateral support at bearing points to avoid lateral displacement and rotation
6 For flat roofs provide proper drainage to prevent ponding

Coastal Forest Products
451 South River Rd, NH
USA
03110

COASTAL FOREST PRODUCTS, INC.