

Location: Combination Roof And Floor Beam 1  
 Combination Roof And Floor Beam  
 [2012 International Building Code(AISC 14th Ed ASD)]  
 A36 W8x21 x 11.5 FT  
 Section Adequate By: 56.7%  
 Controlling Factor: Deflection

DEFLECTIONS		Center
Live Load	0.18	IN L/752
Dead Load	0.06	in
Total Load	0.24	IN L/568
Live Load Deflection Criteria: L/480		Total Load Deflection Criteria: L/240

REACTIONS		A	B
Live Load	5854 lb	5854 lb	
Dead Load	1903 lb	1903 lb	
Total Load	7757 lb	7757 lb	
Bearing Length	0.70 in	0.70 in	

BEAM DATA		Center
Span Length	11.5	ft
Unbraced Length-Top	0	ft
Roof Pitch	0	:12
Floor Duration Factor	0.00	
Roof Duration Factor	0.00	

**STEEL PROPERTIES**  
 W8x21 - A36

**Properties:**

Yield Stress:	Fy =	36	ksi
Modulus of Elasticity:	E =	29000	ksi
Depth:	d =	8.28	in
Web Thickness:	tw =	0.25	in
Flange Width:	bf =	5.27	in
Flange Thickness:	tf =	0.4	in
Distance to Web Toe of Fillet:	k =	0.7	in
Moment of Inertia About X-X Axis:	Ix =	75.3	in <sup>4</sup>
Section Modulus About X-X Axis:	Sx =	18.2	in <sup>3</sup>
Plastic Section Modulus About X-X Axis:	Zx =	20.4	in <sup>3</sup>

**Design Properties per AISC 14th Edition Steel Manual:**

Flange Buckling Ratio:	FBR =	6.59
Allowable Flange Buckling Ratio:	AFBR =	10.79
Web Buckling Ratio:	WBR =	27.52
Allowable Web Buckling Ratio:	AWBR =	106.72
Controlling Unbraced Length:	Lb =	0 ft
Limiting Unbraced Length - for lateral-torsional buckling:	Lp =	5.25 ft
Nominal Flexural Strength w/ safety factor:	Mn =	36647 ft-lb
Controlling Equation:	F2-1	
Web height to thickness ratio:	h/tw =	27.52
Limiting height to thickness ratio for eqn. G2-2:	h/tw-limit =	63.58
Cv Factor:	Cv =	1
Controlling Equation:	G2-2	
Nominal Shear Strength w/ safety factor:	Vn =	29808 lb

**Controlling Moment:** 22301 ft-lb

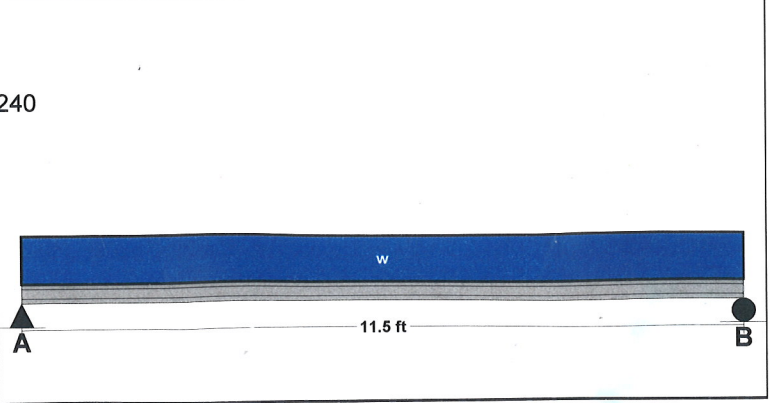
5.75 ft from left support  
 Created by combining all dead and live loads.

**Controlling Shear:** 7757 lb

At support.  
 Created by combining all dead and live loads.

Comparisons with required sections:	Req'd	Provided
Moment of Inertia (deflection):	48.04 in <sup>4</sup>	75.3 in <sup>4</sup>
Moment:	22301 ft-lb	36647 ft-lb

**LOADING DIAGRAM**



**ROOF LOADING**

	Side 1	Side 2
Roof Live Load	RLL = 47 psf	0 psf
Roof Dead Load	RDL = 10 psf	0 psf
Roof Tributary Width	RTW = 14 ft	0 ft

**FLOOR LOADING**

	Side 1	Side 2
Floor Live Load	FLL = 40 psf	40 psf
Floor Dead Load	FDL = 10 psf	10 psf
Floor Tributary Width	FTW = 7 ft	2 ft
Wall Load	WALL = 80 plf	

**BEAM LOADING**

Roof Uniform Live Load:	wL-roof =	658 plf
Roof Uniform Dead Load:	wD-roof =	140 plf
Floor Uniform Live Load:	wL-floor =	360 plf
Floor Uniform Dead Load:	wD-floor =	90 plf
Beam Self Weight:	BSW =	21 plf
Combined Uniform Live Load:	wL =	1018 plf
Combined Uniform Dead Load:	wD =	331 plf
Combined Uniform Total Load:	wT =	1349 plf
Controlling Total Design Load:	wT-cont =	1349 plf

**NOTES**