

FORESIDE ARCHITECTS, LLC
ARCHITECTURE/INTERIOR DESIGN/PLANNING

April 25, 2007

Ms. Barbara A. Barhydt
Development Review Services Manager
Planning & Development Department
City of Portland
Portland, ME 04101-3509

Re: Seaside Nursing Facility – New Porte Cochere Addition

Dear Ms. Barhydt,

Attached please find completed application, drawings and fee payment for a minor site plan review for the above referenced project located at 850 Baxter Boulevard, Portland.

1. The proposed project includes documentation for the design and construction of a new Porte Cochere Structure and associated sitework. The project will include approximately 1,200 square feet of footprint area.
2. We have however assumed for the purposes of City of Portland Zoning and Planning Approvals, that our approach will include application for “administrative approval action”, based upon prior contact with Marge Schumuckel, of the City of Portland’s Zoning Department.
3. The design and construction will be based on State and Local code requirements as provided in the International Building Code, 2003 and NFPA Life Safety Code, and as required by the City of Portland and the State of Maine.

Yours truly,



Mark J. Burnes, AIA
President
Foreside Architects, LLC

attachments

/jpl
cc: file



Site Plan Application

Department of Planning and Development
Portland Planning Board

Address of Proposed Development: *850 BAXTER BOULEVARD* Zone: *R5*

Project Name: *SEASIDE NURSING AND REHABILITATION FACILITY*

Existing Building Size: *38,000* sq. ft.

Proposed Building Size: *38,000* sq. ft.

Existing Acreage of Site: *162,000* sq. ft.

Proposed Acreage of Site: *162,000* sq. ft.

Tax Assessor's Chart, Block & Lot:

Chart#	Block #	Lot#
<i>166</i>	<i>A</i>	<i>010001</i>

Property Owners Mailing address:

*FIRST ATLANTIC HEALTHCARE
222 ST. JOHN'S STREET
PORTLAND, ME. 04101*

Telephone #: *874-2700*

Cell Phone #:

Consultant/Agent Contact Name and mailing address, Telephone # and Cell Phone #:

*FORESIDE ARCHITECTS LLC
251 US ROUTE 1, SUITE 3
FALMOUTH, ME 04105
ATTN: MARK BURNES, AIA*

ATTN: *CRAIG COFFIN, COO*
Applicant's Name/Mailing Address:

Telephone #:

Cell Phone #:

Fee For Service Deposit (all applications) (\$200.00)

Proposed Development (check all that apply)

- New Building Building Addition Change of Use Residential Office Retail
 Manufacturing Warehouse/Distribution Parking lot
 Subdivision (\$500.00) + amount of lots _____ (\$25.00 per lot) \$ _____ + major site plan fee if applicable
 Site Location of Development (\$3,000.00)
(except for residential projects which shall be \$200.00 per lot _____)
 Traffic Movement (\$1,000.00) Storm water Quality (\$250.00)
 Section 14-403 Review (\$400.00 + \$25.00 per lot)
 Other _____

Major Development (more than 10,000 sq. ft.)

- Under 50,000 sq. ft. (\$500.00)
 50,000 - 100,000 sq. ft. (\$1,000.00)
 Parking Lots over 100 spaces (\$1,000.00)
 100,000 - 200,000 sq. ft. (\$2,000.00)
 200,000 - 300,000 sq. ft. (\$3,000.00)
 Over 300,000 sq. ft. (\$5,000.00)
 After-the-fact Review (\$1,000.00 + applicable application fee)

~ Please see next page ~

Minor Site Plan Review

Less than 10,000 sq. ft. (\$400.00)

After-the-fact Review (\$1,000.00 + applicable application fee)

Plan Amendments

Planning Staff Review (\$250.00)

Planning Board Review (\$500.00)

Who billing will be sent to:

FIRST ATLANTIC HEALTHCARE
222 ST. JOHN'S STREET
PORTLAND, ME 04101

Submittals shall include (7) separate **folded** packets of the following:

- a. copy of application
- b. cover letter stating the nature of the project
- c. site plan containing the information found in the attached sample plans checklist
- d. 1 set of 11x17 plans

Section 14-522 of the Zoning Ordinance outlines the process which is available on our web site: portlandmaine.gov

I hereby certify that I am the Owner of record of the named property, or that the owner of record authorizes the proposed work and that I have been authorized by the owner to make this application as his/her authorized agent. I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in this application is issued, I certify that the Code Official's authorized representative shall have the authority to enter all areas covered by this permit at any reasonable hour to enforce the provisions of the codes applicable to this permit

This application is for site review only; a Building Permit application and associated fees will be required prior to construction.

Signature of Applicant::

Al J. Burns, AIA
AGENT FOR FIRST ATLANTIC HEALTHCARE

Date:

4/25/07

THE POCHEBIT CO., INC.

171 Warren Avenue
 PORTLAND, MAINE 04103

(207) 797-3369
 FAX (207) 797-3299

TO CITY OF PORTLAND

LETTER OF TRANSMITTAL

DATE	7/26/07	JOB NO.
ATTENTION	JEANNE B.	
RE:	SEASIDE 850 BAXTER BLVD	

WE ARE SENDING YOU Attached Under separate cover via _____ the following items:

Shop drawings Prints Plans Samples Specifications
 Copy of letter Change order _____

COPIES	DATE	NO.	DESCRIPTION
1			SET OF STAMPED DRAWINGS
1			SPECIAL INSPECTIONS

THESE ARE TRANSMITTED as checked below:

For approval Approved as submitted Resubmit _____ copies for approval
 For your use Approved as noted Submit _____ copies for distribution
 As requested Returned for corrections Return _____ corrected prints
 For review and comment _____
 FOR BIDS DUE _____ PRINTS RETURNED AFTER LOAN TO US

REMARKS _____

LET ME KNOW IF YOU NEED
 ANY THINGS - ELSE OR WHEN
 PERMITS IS READY, THANKS

COPY TO _____

SIGNED: MIKE WHITE

**GEOTECHNICAL ENGINEERING SERVICES
PROPOSED DROP-OFF CANOPY
SEASIDE NURSING HOME
850 BAXTER BOULEVARD
PORTLAND, MAINE**

06-1209

December 18, 2006

Prepared for:

Forside Architects, LLC
Attention: Mark Burnes
5 Johnson Road
Falmouth, ME 04105

Prepared by:



286 Portland Road
Gray, ME 04039

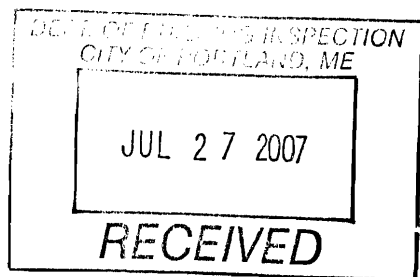


TABLE OF CONTENTS

1.0 INTRODUCTION.....	1
1.1 Scope of Work.....	1
1.2 Proposed Construction.....	1
2.0 EXPLORATION AND TESTING.....	2
2.1 Explorations.....	2
2.2 Laboratory Testing.....	2
3.0 SITE AND SUBSURFACE CONDITIONS.....	2
3.1 Site Conditions.....	2
3.2 Subsurface Conditions.....	2
3.3 Groundwater.....	3
3.4 Site Seismic and Design Frost Depth.....	3
4.0 EVALUATION AND RECOMMENDATIONS.....	3
4.1 General Findings.....	3
4.2 Pile Options.....	4
4.2.1 Shallow Bearing.....	4
4.2.2 Deep Bearing.....	4
4.3 Pile Capacity Estimates.....	5
4.4 Excavation Work.....	6
4.5 Backfill and Compaction Requirements.....	6
4.6 Weather Considerations.....	7
4.7 Construction Observation and Testing.....	7
5.0 CLOSURE.....	8

Attachment A - Limitations

Sheet 1 – Exploration Location Plan

Sheets 2 through 4 – Exploration Logs

Sheet 5 – Key to Notes & Symbols

Sheets 6 and 7 – Gradation Reports



• Geotechnical Engineering • Field & Lab Testing • Scientific & Environmental Consulting

06-1209

December 18, 2006

Foreside Architects, LLC
Attention: Mark Burnes
5 Johnson Road
Falmouth, ME 04105

Subject: Geotechnical Engineering Services
Drop-Off Canopy
Seaside Nursing Home
850 Baxter Boulevard
Portland, Maine

Dear Mark:

In accordance with our Agreement dated October 17, 2006, we have made a subsurface investigation for the proposed canopy at 850 Baxter Boulevard in Portland, Maine. The contents of this report are subject to the limitations set forth in Attachment A.

1.0 INTRODUCTION

1.1 Scope of Work

The purpose of the investigation was to obtain subsurface soils information at the location of the proposed canopy in order to provide recommendations relative to foundation design. The investigation included the making of two test borings, soils laboratory testing, and a geotechnical evaluation of the findings as they relate to the proposed construction.

1.2 Proposed Construction

Based on the information provided, we understand plans call for construction of a 20 by 20± foot canopy constructed about 10 feet from the existing main entrance. We understand the canopy will likely be steel-framed and supported at the corners with steel columns. We understand the canopy may be attached to a short 10± foot wide connector roof. We understand that differential settlements up to about .004 in/in is acceptable. We anticipate finish pavement grade will match existing pavement grade.

GRAY, ME OFFICE

286 Portland Road, Gray, ME 04039-9586 ■ Tel (207) 657-2866 ■ Fax (207) 657-2840 ■ E-Mail infogray@swcole.com ■ www.swcole.com

Other offices in Augusta, Bangor, and Caribou, Maine & Somersworth, New Hampshire

2.0 EXPLORATION AND TESTING

2.1 Explorations

On November 8, 2006, Great Works Test Boring, Inc. of Rollinsford, New Hampshire made two test borings at the site. S. W. COLE ENGINEERING, INC. selected the exploration locations and established them in the field based on measurements from existing site features. The approximate locations of the explorations are shown on the "Exploration Location Plan," attached as Sheet 1. Sheet 1 is based on a plan provided by Foreside Architects. Logs of the test borings are attached as Sheets 2 through 4 and a key to the notes and symbols used on the logs is attached as Sheet 5.

2.2 Laboratory Testing

Visual classification was performed on samples recovered from the test borings. The results of laboratory moisture content and strength testing are shown on the exploration logs. The results of two soil gradation analyses are attached as Sheets 6 and 7.

3.0 SITE AND SUBSURFACE CONDITIONS

3.1 Site Conditions

The site of the proposed canopy is located on the westerly side of the existing nursing home structure adjacent to the main entrance. The ground surface is either paved or occupied by landscape areas and is relatively flat. The existing structure is reportedly pile supported. A 24" by 24" wood box culvert reportedly exists beneath the proposed canopy site, although its depth is not known. The approximate location is shown on a 1969 drawing of the facility provided to us by Foreside Architects.

3.2 Subsurface Conditions

In general, the borings encountered about 4 inches of bituminous asphalt pavement overlying several inches of brown sand with gravel base fill. The base fill overlies black miscellaneous fill. The black miscellaneous fill consists of silt and sand mixed with ash, pieces of wood, brick, glass, metal and organics. A stiff to medium olive-brown to gray-

brown silty clay was encountered at depths of 14 and 15 feet at borings B-1 and B-2, respectively. This stiff to medium silty clay appears to be about 9 feet thick. The stiff to medium silty clay transitions to soft gray silty clay at about 23 feet below the ground surface. Boring B-1 was advanced through the soft silty clay with a rod probe. A refusal surface (possible bedrock) was encountered at a depth of 77± feet. Boring B-2 was terminated at a depth of 17 feet.

Refer to the attached boring logs for a more detailed description of the subsurface findings.

3.3 Groundwater

Free groundwater was encountered at the borings at depths of about 7 feet at the time of exploration work. It is possible that groundwater elevations are affected by tidal action and it should be anticipated that groundwater will fluctuate seasonally and during periods of heavy precipitation and/or snowmelt.

3.4 Site Seismic and Design Frost Depth

According to the 2003 International Building Code, utilizing standard penetration testing values obtained at the borings, we interpret the subsurface conditions to correspond to a soil site class E. The design-freezing index for the Portland area is about 1,250-Fahrenheit degree-days, which corresponds to a frost penetration on the order of 4.5 feet.

4.0 EVALUATION AND RECOMMENDATIONS

4.1 General Findings

Based on the subsurface findings and our understanding of the proposed project, it is our opinion that the existing fill soils are not suitable for support of the proposed structure utilizing spread footings.

The 15± foot thick miscellaneous fill soil underlying the site is loose and contains a variety of materials such as ash, organics and pieces of glass, metal, brick, etc. This material has

poor bearing characteristics and will not consolidate uniformly; causing differential movement between the corner columns. Due to the variable matrix of the fill, estimation of post-construction settlement beneath spread footings is difficult, but is expected to be beyond tolerable limits. It is our opinion that a large, reinforced concrete mat foundation, although providing a lower contact pressure by spreading out the canopy load, would also settle (non-uniformly) due to the loose, variable nature of the fills. It is our opinion that densification of the existing fill would have little affect due to the type of fill material and presence of groundwater.

Other geotechnical concerns include the presence of potentially contaminated fill soils, groundwater and the presence of soft, compressible, silty clay with depth.

Thus, based on the findings at the explorations and project information provided, it is our opinion that the proposed canopy be supported utilizing pile foundations.

4.2 Pile Options

4.2.1 Shallow Bearing

Based on the findings at the two explorations, it appears that a 9± foot thick layer of stiff to medium silty clay exists beneath the miscellaneous fill. In our opinion, a driven displacement pile (such as timber, concrete or steel pipe piles) could be driven into the stiff silty clay zone to a depth of about 20 feet below the existing ground surface. Another option to consider is helical piles, also spun into the stiffer silty clay.

4.2.2 Deep Bearing

Steel H-pile driven to end bearing is also an option to consider. Based on the findings at Boring B-1, it appears that a steel H-pile would need to be driven to about 75 feet below the existing ground surface. The resulting pile capacities would likely be well above the capacity needed.

4.3 Pile Capacity Estimates

For this project, it appears that driven timber displacement piles or Helical piles are appropriate. For a timber displacement pile driven into the stiff to medium silty clay zone to a depth of about 25 feet below the existing ground surface, we estimate an axial download working capacity on the order of 5.5 kips per pile. This estimate assumes a factor of safety of 2.5. For the helical piles, we have contacted an installation contractor (Solid Earth Technologies, Inc. of Amherst, New Hampshire) who estimates capacities on the order of 12 kips per pile could be achieved using a helical tip of 10" – 12' – 14' and a grouted column. They indicate that this estimate assumes a factor of safety of 2.

For either timber displacement piles or the helical piles, we estimate that post-construction settlement due to consolidation of the underlying soft clay could be on the order of 1 inch at each column.

The above capacities are estimates only. Actual pile capacity estimates will need to be assessed by the pile contractor. A reduction in pile capacity will need to be considered, due to soil downdrag, if surficial loads, such as new fills, are placed on the site.

For any pile option, it is likely that some piles may encounter cobbles and/or debris at depth and could be damaged or shifted during driving. Thus the project should account for the potential for piles out of plumb, out of position and/or capacity reduction, due to damage.

Uplift capacity can be resisted with concrete pile caps and piles. Uplift capacity of the piles will be affected by the pile spacing, pile type, splices and actual depths required to achieve capacity. S. W. COLE ENGINEERING, INC. can review uplift capacity estimates prepared by the pile contractors.

Pile caps and foundations exposed to freezing temperatures should extend at least 4.5 feet from finished grade. Piles should be spaced a minimum center-to-center distance of at least 3 pile diameters, but no less than 30 inches. Piles in groups should be driven

from the interior outward to help preclude excessively hard driving conditions of the interior piles due to soil densification.

We recommend that lateral loads be resisted by passive earth pressures acting on the pile caps. Passive lateral resistance acting on grade beams and pile caps backfilled with compacted Structural Fill should consider a total unit weight of granular backfill (γ_t) of 125 pcf, an angle of internal friction of 30 degrees with an ultimate passive lateral earth pressure coefficient (K_p) of 3.0.

4.4 Excavation Work

Excavation work will encounter uncontrolled miscellaneous fills. The on-site fill soils are not suitable for reuse below pile caps or backfill against pile caps.

Groundwater and wet soil conditions will likely be encountered in the foundation excavations deeper than about 7 feet below existing grades. In our opinion, ditching with sump and pump dewatering techniques should be adequate to control groundwater in shallow foundation excavations. It should be anticipated, however, that heavy rains and/or higher than normal tides will affect groundwater levels and may require significant sumping and pumping or other means of dewatering. We recommend a 12-inch layer of crushed stone be placed beneath all pile caps to act as a drainage media from which to sump and pump. Excavations must be properly shored and/or sloped in accordance with OSHA trenching regulations to prevent sloughing and caving of the sidewalls during construction.

4.5 Backfill and Compaction Requirements

As previously mentioned, the on-site fills are not suitable for reuse below or adjacent to pile caps. We recommend that a 12-inch thick layer of compacted crushed stone be provided below all pile caps. Compacted granular fill against all pile caps should be clean granular material meeting the gradation for Structural Fill. We recommend at least 4 feet of compacted Structural Fill be placed around each pile cap to help provide lateral support.

Structural Fill	
Sieve Size	Percent Finer by Weight
4 inch	100
3 inch	90 to 100
¼ inch	25 to 90
No. 40	0 to 30
No. 200	0 to 5

We recommend that a layer of woven geotextile, such as Mirafi 160N, be placed on top of the crushed stone prior to placing the Structural Fill. Fill should be placed in horizontal lifts and be compacted. Lift thickness should be such that desired density is achieved throughout the lift thickness with 3 to 5 passes of the compaction equipment. Structural fill should be compacted to at least 95 percent of its maximum dry density as determined by the Modified Proctor (ASTM-D1557). Backfill below pile-supported foundations should be compacted crushed stone.

4.6 Weather Considerations

Subgrades and foundations must be protected from freezing conditions. Fill soils and concrete must not be placed on frozen soil and once placed, the soil beneath the structure must be protected from freezing. Further, the existing uncontrolled fill is moisture sensitive and as such subgrades will be susceptible to disturbance, particularly during wet conditions. Consequently, site work and construction activities should take appropriate measures to protect exposed subgrades, particularly when wet.

4.7 Construction Observation and Testing

We recommend that S. W. COLE ENGINEERING, INC. be retained to provide consultation and testing services for the excavation and foundation phases of construction. This is to observe compliance with the design recommendations, drawings and specifications and to allow design changes in the event that subsurface conditions are found to differ from those anticipated prior to the start of construction.



06-1209
December 18, 2006

S. W. COLE ENGINEERING, INC. is available to provide vibration monitoring, pile installation monitoring, and testing of soils, concrete, steel, masonry, fireproofing and asphalt.

5.0 CLOSURE

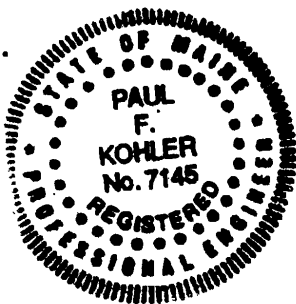
We request the opportunity to review the sitework and foundation design drawing to confirm that our recommendations have been appropriately interpreted and implemented. It has been a pleasure to be of assistance to you with this phase of your project. We look forward to working with you as the design progresses and during the construction phase.

Sincerely,

S. W. COLE ENGINEERING, INC.

A handwritten signature in black ink, appearing to read "Paul F. Kohler", is written over the printed name.

Paul F. Kohler, P. E.
Vice President



c: Ethan Rhile – Becker Structural
Craig Coffin – 1st Atlantic Corp.

PFK:pfb

ATTACHMENT A

Limitations

This report has been prepared for the exclusive use of Foreside Architects, LLC for specific application to the proposed Drop-Off Canopy at the Seaside Nursing Home in Portland, Maine. S. W. COLE ENGINEERING, INC. has endeavored to conduct the work in accordance with generally accepted soil and foundation engineering practices. No warranty, expressed or implied, is made.

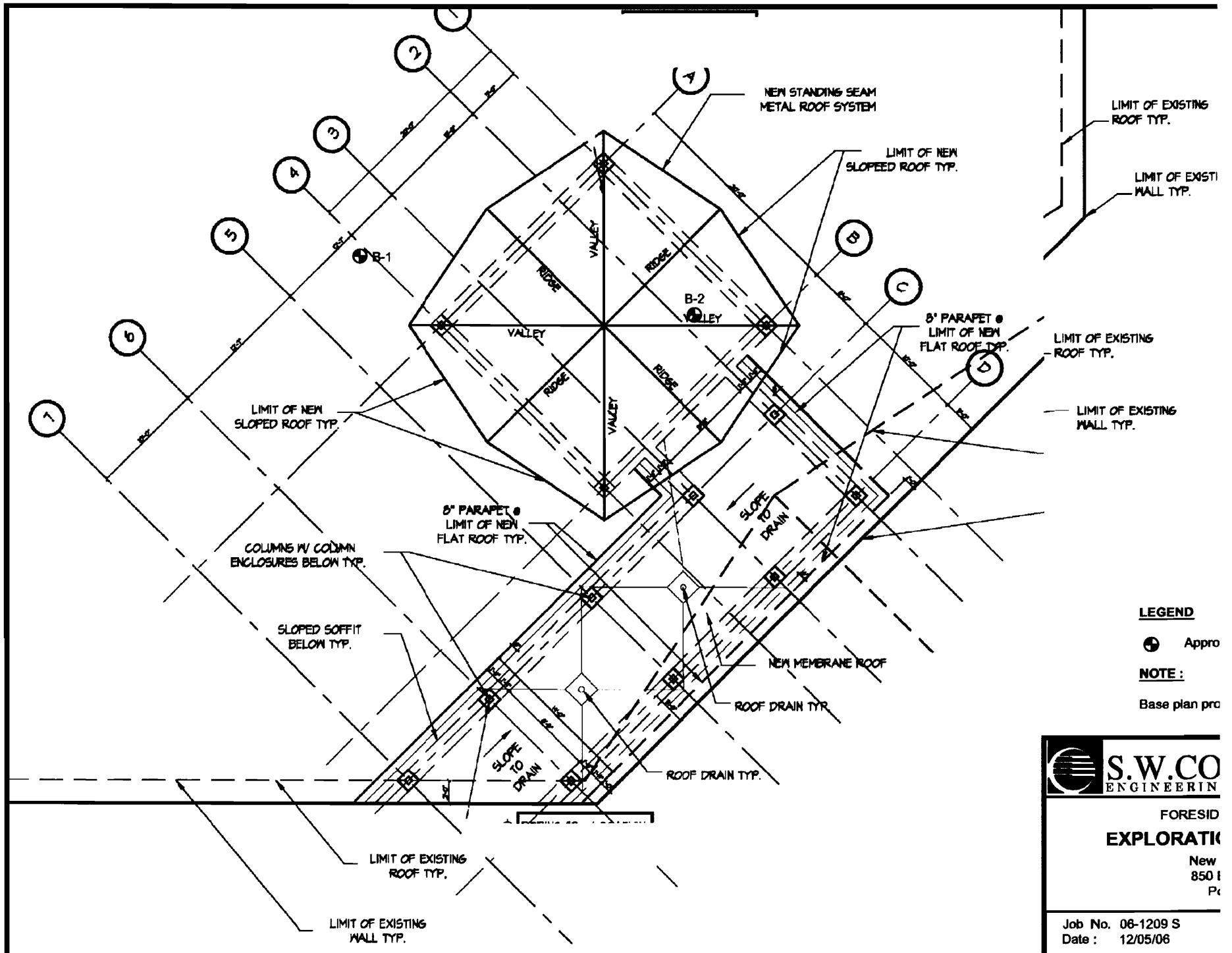
The soil profiles described in the report are intended to convey general trends in subsurface conditions. The boundaries between strata are approximate and are based upon interpretation of exploration data and samples.

The analyses performed during this investigation and recommendations presented in this report are based in part upon the data obtained from subsurface explorations made at the site. Variations in subsurface conditions may occur between explorations and may not become evident until construction. If variations in subsurface conditions become evident after submission of this report, it will be necessary to evaluate their nature and to review the recommendations of this report.

Observations have been made during exploration work to assess site groundwater levels. Fluctuations in water levels will occur due to variations in rainfall, temperature, and other factors.

S. W. COLE ENGINEERING, INC.'s scope of work has not included the investigation, detection, or prevention of any Biological Pollutants at the project site or in any existing or proposed structure at the site. The term "Biological Pollutants" includes, but is not limited to, molds, fungi, spores, bacteria, and viruses, and the byproducts of any such biological organisms.

Recommendations contained in this report are based substantially upon information provided by others regarding the proposed project. In the event that any changes are made in the design, nature, or location of the proposed project, S. W. COLE ENGINEERING, INC. should review such changes as they relate to analyses associated with this report. Recommendations contained in this report shall not be considered valid unless S. W. COLE ENGINEERING, INC. reviews the changes.





BORING LOG

BORING NO.: B-1
 SHEET: 2 OF 2
 PROJECT NO.: 06-1209
 DATE START: 11/8/2006
 DATE FINISH: 11/8/2006
 ELEVATION: NOT AVAILABLE
 SWC REP.: MPL

PROJECT / CLIENT: NEW DROP OFF CANOPY - SEASIDE NURSING HOME / FORESIDE ARCHITECTS
 LOCATION: 850 BAXTER BOULEVARD PORTLAND, MAINE
 DRILLING CO.: GREAT WORKS TEST BORING DRILLER: WAYNE MACPHERSON

CASING: TYPE CASING SIZE I.D. 4" HAMMER WT. HYDRAULIC PUSH HAMMER FALL
 SAMPLER: SS 1 3/8" 140 lbs 30"
 CORE BARREL:

WATER LEVEL INFORMATION
 SOILS APPEAR SATURATED BELOW 7FT.±

CASING BLOWS PER FOOT	SAMPLE			DEPTH @ BOT	SAMPLER BLOWS PER FOOT				DEPTH	STRATA & TEST DATA		
	NO.	PEN.	REC.		0-6	6-12	12-18	18-24				
										GRAY SILTY CLAY cont. ROD PROBE WITH HYDRAULIC PUSH TO 76.5'		
									76.5'			
									77.1'			
										100 BLOWS / 0.6'		
										BOTTOM OF EXPLORATION		
										PROBE REFUSAL (POSSIBLE BEDROCK)		
SAMPLES: D = SPLIT SPOON C = 2" SHELBY TUBE S = 3" SHELBY TUBE U = 3.5" SHELBY TUBE				SOIL CLASSIFIED BY: <input type="checkbox"/> DRILLER - VISUALLY <input checked="" type="checkbox"/> SOIL TECH. - VISUALLY <input checked="" type="checkbox"/> LABORATORY TEST				REMARKS: STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY BETWEEN SOIL TYPES AND THE TRANSITION MAY BE GRADUAL.				<div style="border: 1px solid black; border-radius: 50%; width: 20px; height: 20px; display: flex; align-items: center; justify-content: center;">3</div>
BORING NO.: <u>B-1</u>												



BORING LOG

BORING NO.: **B-2**
 SHEET: **1 OF 1**
 PROJECT NO.: **06-1209**
 DATE START: **11/8/2006**
 DATE FINISH: **11/8/2006**
 ELEVATION: **NOT AVAILABLE**
 SWC REP.: **MPL**

PROJECT / CLIENT: **NEW DROP OFF CANOPY - SEASIDE NURSING HOME / FORESIDE ARCHITECTS**
 LOCATION: **850 BAXTER BOULEVARD PORTLAND, MAINE**
 DRILLING CO.: **GREAT WORKS TEST BORING** DRILLER: **WAYNE MACPHERSON**

CASING: TYPE **CASING** SIZE I.D. **4"** HAMMER WT. **HYDRAULIC PUSH** HAMMER FALL
 SAMPLER: **SS** **1 3/8"** **140 lbs** **30"**
 CORE BARREL:

WATER LEVEL INFORMATION
 SOILS APPEAR SATURATED BELOW 7FT.±

CASING METHOD	SAMPLE				SAMPLER BLOWS PER				DEPTH @ BOT	0-6	6-12	12-18	18-24	SOIL TYPE
	NO.	PEN.	REC.	DEPTH	0-6	6-12	12-18	18-24						
HYD.														0.3' 4"± ASPHALT PAVEMENT
PUSH	1D	24"	15"	2.3'	3	4	5	4						0.8' TAN SAND AND GRAVEL (FILL) - MEDIUM DENSE BECOMING
	2D	24"	20"	4.3'	7	6	7	8						BLACK SILTY SAND WITH ASH (FILL), TRACE GRAVEL, ORGANIC MATERIAL LOOSE ~ PIECES OF WOOD, ORGANIC MATERIAL
	3D	24"	8"	7.0'	2	1	1	1						PIECES OF METAL, DEBRIS
	4D	24"	12"	9.0'	2	5	7	4						
	5D	24"	1"	12.0'	1	1	1	1						
														15.0'
	6D	24"	24"	17.0'	WOH	7	12	15						17.0' Sv = 2.5 ksf - STIFF - w = 28.0% OLIVE BROWN SILTY CLAY qp = 5+/- ksf BOTTOM OF EXPLORATION

SAMPLES:
 D = SPLIT SPOON
 C = 2" SHELBY TUBE
 S = 3" SHELBY TUBE
 U = 3.5" SHELBY TUBE

SOIL CLASSIFIED BY:

DRILLER - VISUALLY
 SOIL TECH. - VISUALLY
 LABORATORY TEST

REMARKS:

STRATIFICATION LINES REPRESENT THE
 APPROXIMATE BOUNDARY BETWEEN SOIL TYPES
 AND THE TRANSITION MAY BE GRADUAL.

4

BORING NO.: **B-2**

KEY TO THE NOTES & SYMBOLS

Test Boring and Test Pit Explorations

All stratification lines represent the approximate boundary between soil types and the transition may be gradual.

Key to Symbols Used:

w	-	water content, percent (dry weight basis)
q _u	-	unconfined compressive strength, kips/sq. ft. - based on laboratory unconfined compressive test
S _v	-	field vane shear strength, kips/sq. ft.
L _v	-	lab vane shear strength, kips/sq. ft.
q _p	-	unconfined compressive strength, kips/sq. ft. based on pocket penetrometer test
O	-	organic content, percent (dry weight basis)
W _L	-	liquid limit - Atterberg test
W _P	-	plastic limit - Atterberg test
WOH	-	advance by weight of hammer
WOM	-	advance by weight of man
WOR	-	advance by weight of rods
HYD	-	advance by force of hydraulic piston on drill
RQD	-	Rock Quality Designator - an index of the quality of a rock mass. RQD is computed from recovered core samples.
γ _T	-	total soil weight
γ _B	-	buoyant soil weight

Description of Proportions:

0 to 5% TRACE
5 to 12% SOME
12 to 35% "Y"
35+% AND

REFUSAL: Test Boring Explorations - Refusal depth indicates that depth at which, in the drill foreman's opinion, sufficient resistance to the advance of the casing, auger, probe rod or sampler was encountered to render further advance impossible or impracticable by the procedures and equipment being used.

REFUSAL: Test Pit Explorations - Refusal depth indicates that depth at which sufficient resistance to the advance of the backhoe bucket was encountered to render further advance impossible or impracticable by the procedures and equipment being used.

Although refusal may indicate the encountering of the bedrock surface, it may indicate the striking of large cobbles, boulders, very dense or cemented soil, or other buried natural or man-made objects or it may indicate the encountering of a harder zone after penetrating a considerable depth through a weathered or disintegrated zone of the bedrock.

Report of Gradation

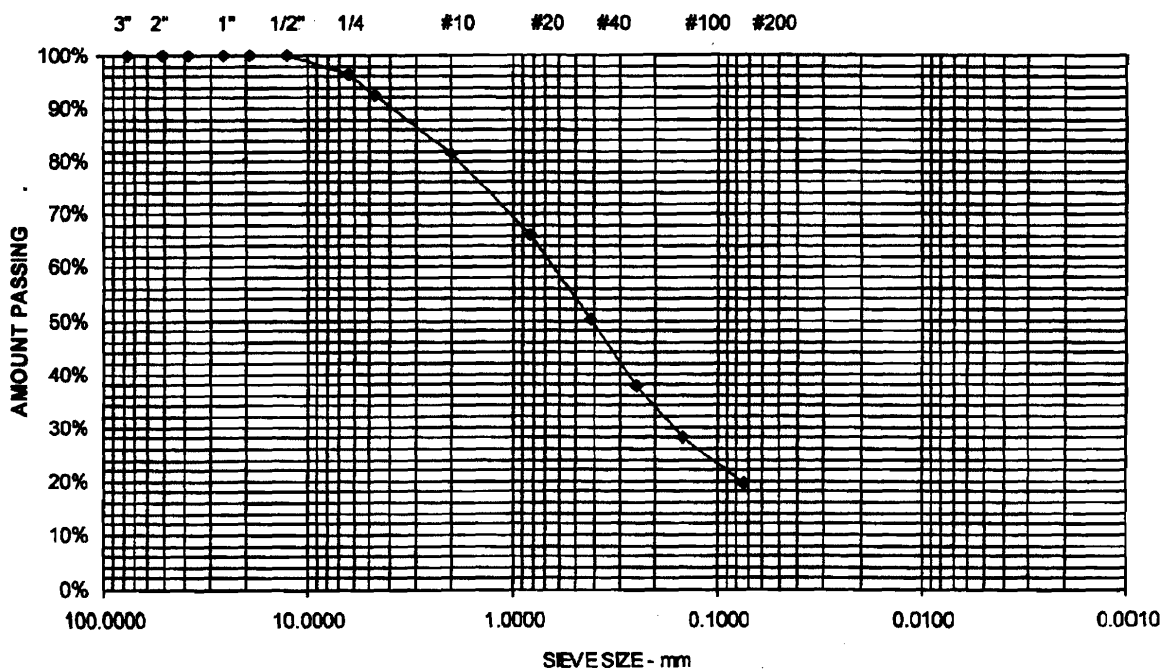
ASTM C-117 & C-136

Project Name PORTLAND - 850 BAXTER BOULEVARD - NEW DROP-OFF
CANOPY - GEOTECHNICAL ENGINEERING SERVICES
Client FORESIDE ARCHITECTS
Exploration B-1 2D
Material Source 2.3 - 4.3

Project Number 06-1209
Lab ID 5984G
Date Received 11/13/2006
Date Complete 11/14/2006
Tested By JUSTIN BISSON

STANDARD DESIGNATION (mm/um)	SIEVE SIZE	AMOUNT PASSING (%)	
150 mm	6"	100	
125 mm	5"	100	
100 mm	4"	100	
75 mm	3"	100	
50 mm	2"	100	
38.1 mm	1-1/2"	100	
25.0 mm	1"	100	
19.0 mm	3/4"	100	
12.5 mm	1/2"	100	
6.3 mm	1/4"	97	
4.75 mm	No. 4	92	7.6% Gravel
2.00 mm	No. 10	81	
850 um	No. 20	66	
425 um	No. 40	50	72.9% Sand
250 um	No. 60	38	
150 um	No. 100	28	
75 um	No. 200	19.4	19.4% Fines

BLACK SILTY SAND, SOME ASH, SOME GRAVEL (FILL)



Comments:

Report of Gradation

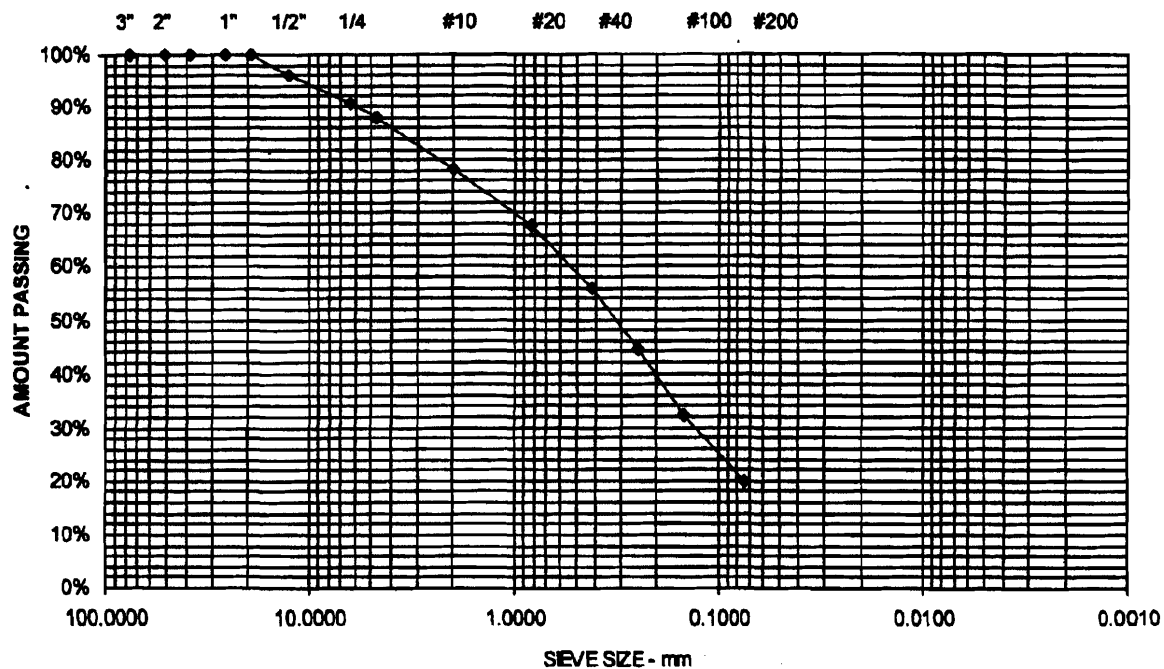
ASTM C-117 & C-136

Project Name PORTLAND - 850 BAXTER BOULEVARD - NEW DROP-OFF
CANOPY - GEOTECHNICAL ENGINEERING SERVICES
Client FORESIDE ARCHITECTS
Exploration B-2 2D
Material Source 2.3 - 4.3

Project Number 06-1209
Lab ID 5988G
Date Received 11/13/2006
Date Complete 11/14/2006
Tested By JUSTIN BISSON

<u>STANDARD DESIGNATION (mm/µm)</u>	<u>SIEVE SIZE</u>	<u>AMOUNT PASSING (%)</u>	
150 mm	6"	100	
125 mm	5"	100	
100 mm	4"	100	
75 mm	3"	100	
50 mm	2"	100	
38.1 mm	1-1/2"	100	
25.0 mm	1"	100	
19.0 mm	3/4"	100	
12.5 mm	1/2"	96	
6.3 mm	1/4"	91	
4.75 mm	No. 4	88	12.2% Gravel
2.00 mm	No. 10	78	
850 µm	No. 20	68	
425 µm	No. 40	56	67.7% Sand
250 µm	No. 60	44	
150 µm	No. 100	32	
75 µm	No. 200	20.1	20.1% Fines

BLACK SILTY SAND, SOME ASH, SOME GRAVEL (FILL)



Comments:

THE POCHEBIT CO., INC.

171 Warren Avenue
 PORTLAND, MAINE 04103

(207) 797-3369
 FAX (207) 797-3299

TO

CITY OF PORTLAND

LETTER OF TRANSMITTAL

DATE	7/26/07	JOB NO.
ATTENTION	JEANNE R.	
RE:	SEASIDE 850 BAXTER BLVD	



PLANNING & DEVELOPMENT DEPARTMENT
 Inspection Services Division

WE AI

Jeanie Bourke, Inspection Services Division Director
 jmb@portlandmaine.gov

er via _____ the following items:

Plans Samples Specifications

389 Congress Street • Portland, Maine 04101-3509
 www.portlandmaine.gov • Ph(207)874-8715 • Fx874-8716

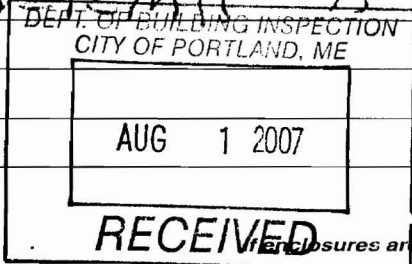
COP	DESCRIPTION
1	SET OF SEAMPED DRAWINGS
1	SPECIAL INSPECTIONS

THESE ARE TRANSMITTED as checked below:

- For approval
- For your use
- As requested
- For review and comment
- FOR BIDS DUE _____
- Approved as submitted
- Approved as noted
- Returned for corrections
- Resubmit _____ copies for approval
- Submit _____ copies for distribution
- Return _____ corrected prints
- PRINTS RETURNED AFTER LOAN TO US

REMARKS _____

LET ME KNOW IF YOU NEED
 ANY THINGS ELSE OR WHEN
 PERMIT IS READY, THANKS



COPY TO _____

SIGNED: _____

MIKE WHITE

If enclosures are not as noted, kindly notify us at once.

Project: Seaside Elderly Care Entry Canopy
Date Prepared: 07/25/2007

Structural Statement of Special Inspections

Project: *Seaside Elderly Care Entry Canopy*

Location: *850 Baxter Blvd, Portland, Maine*

Owner: *First Atlantic Corporation*

This *Statement of Special Inspections* encompass the following discipline: **Structural**

This *Statement of Special Inspections* is submitted as a condition for permit issuance in accordance with the Special Inspection and Structural Testing requirements of the Building Code. It includes a schedule of Special Inspection services applicable to this project as well as the name of the Structural Special Inspection Coordinator (SSIC) and the identity of other approved agencies to be retained for conducting these inspections and tests

The Structural Special Inspection Coordinator shall keep records of all Structural inspections and shall furnish inspection reports to the Building Code Official (BCO) and the Structural Registered Design Professional in Responsible Charge (SRDP). Discovered discrepancies shall be brought to the immediate attention of the Contractor for correction. If such discrepancies are not corrected, the discrepancies shall be brought to the attention of the Building Official and the Structural Registered Design Professional in Responsible Charge. The Special Inspection program does not relieve the Contractor of his or her responsibilities.

Interim reports shall be submitted to the Building Official and the Structural Registered Design Professional in Responsible Charge at an interval determined by the SSIC and the BCO.

A *Final Report of Special Inspections* documenting completion of all required Special Inspections, testing and correction of any discrepancies noted in the inspections shall be submitted to the BCO prior to issuance of a Certificate of Use and Occupancy.

Job site safety and means and methods of construction are solely the responsibility of the Contractor.

Interim Report Frequency: Upon request of Building Official or per attached schedule.

Prepared by:

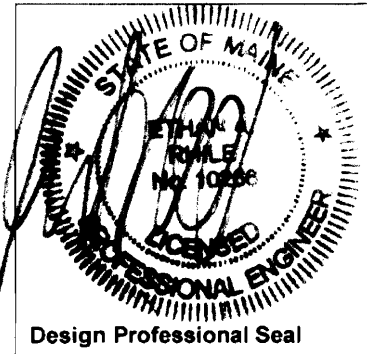
Ethan A. Rhile, P.E.

(type or print name of the Structural Registered Design Professional in Responsible Charge)

Signature

07 25 2007

Date



Owner's Authorization:

Signature

8/1/07

Date

Building Code Official's Acceptance:

Signature

Date

Project: Seaside Elderly Care Entry Canopy
Date Prepared: 07/25/2007

Structural Statement of Special Inspections (Continued)

List of Agents

Project: Seaside Elderly Care Entry Canopy

Location: 850 Baxter Blvd

Owner: First Atlantic Corporation

This Statement of Special Inspections encompass the following discipline: **Structural**

(Note: Statement of Special Inspections for other disciplines may be included under a separate cover)

This Statement of Special Inspections / Quality Assurance Plan includes the following building systems:

- Soils and Foundations
- Cast-in-Place Concrete
- Precast Concrete System
- Masonry Systems
- Structural Steel
- Cold Formed Metal Trusses
- Special Cases

Special Inspection Agencies	Firm	Address, Telephone, e-mail
1. STRUCTURAL Special Inspections Coordinator (SSIC)	Becker Structural Engineers	75 York Street Portland, ME 04101 (207) 879-1838 ethan@beckerstructural.com
2. Special Inspector (SI 1)	Same	Same
3. Special Inspector (SI 2)		
4. Testing Agency (TA 1)	S. W. Cole Engineering	286 Portland Road Gray, ME 04039 (207) 657-2866 rdomingo@swcole.com
5. Testing Agency (TA 2)		
6. Other (O1)		

Note: The inspectors and testing agencies shall be engaged by the Owner or the Owner's Agent, and not by the Contractor or Subcontractor whose work is to be inspected or tested. Any conflict of interest must be disclosed to the Building Official, prior to commencing work.

Project: Seaside Elderly Care Entry Canopy
Date Prepared: 07/25/2007

Structural Statement of Special Inspections (Continued)

Final Report of Special Inspections (SSIC/SI 1)

[To be completed by the Structural Special Inspections Coordinator (SSIC/SI 1). Note that all Agent's Final Reports must be received prior to issuance.]

Project:
Location:
Owner:
Owner's Address:

Architect of Record: _____
(name) *(firm)*
Structural Registered Design
Professional in Responsible Charge: _____
(name) *(firm)*

To the best of my information, knowledge and belief, the Special Inspections required for this project, and itemized in the *Statement of Special Inspections* submitted for permit, have been performed and all discovered discrepancies have been reported and resolved.

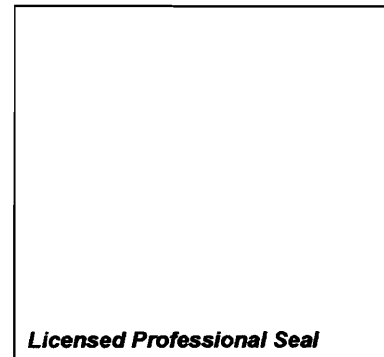
Interim reports submitted prior to this final report form a basis for and are to be considered an integral part of this final report.

Respectfully submitted,
Structural Special Inspection Coordinator

(Type or print name)

(Firm Name)

Signature Date



Project: Seaside Elderly Care Entry Canopy
Date Prepared: 07/25/2007

Structural Statement of Special Inspections (Continued)

Special Inspector's/Agent's Final Report

Project:

Special Inspector or
Agent:

(name)

(firm)

Designation:

To the best of my information, knowledge and belief, the Special Inspections or testing required for this project, and designated for this Inspector/Agent in the *Statement of Special Inspections* submitted for permit, have been performed and all discovered discrepancies have been reported and resolved.

Interim reports submitted prior to this final report form a basis for and are to be considered an integral part of this final report.

Respectfully submitted,
Special Inspector or Agent:

(Type or print name)

Signature

Date

**Licensed Professional Seal or
Certification Number**

Structural Schedule of Special Inspections

Qualifications of Inspectors and Testing Technicians

The qualifications of all personnel performing Special Inspection and testing activities are subject to the approval of the Building Official. The credentials of all inspectors and testing technicians shall be provided to the Special Inspector for their records. *NOTE VERIFICATION THAT QUALIFIED INDIVIDUALS ARE AVAILABLE TO PERFORM STIPULATED TESTING AND/OR INSPECTION SHOULD BE PROVIDED PRIOR TO SUBMITTING STATEMENT. AGENT QUALIFICATIONS IN SCHEDULE ARE SUGGESTIONS ONLY; FINAL QUALIFICATIONS ARE SUBJECT TO THE DISCRETION OF THE REGISTERED DESIGN PROFESSIONAL PREPARING THE SCHEDULE.*

Key for Minimum Qualifications of Inspection Agents:

When the Registered Design Professional in Responsible Charge or Special Inspector of Record deems it appropriate that the individual performing a stipulated test or inspection have a specific certification, license or experience as indicated below, such requirement shall be listed below and shall be clearly identified within the schedule under the Agent Qualification Designation.

PE/SE Structural Engineer – a licensed SE or PE specializing in the design of building structures
PE/GE Geotechnical Engineer – a licensed PE specializing in soil mechanics and foundations
EIT Engineer-In-Training – a graduate engineer who has passed the Fundamentals of Engineering examination

Experienced Testing Technician

ETT Experienced Testing Technician – An Experienced Testing Technician with a minimum 5 years experience with the stipulated test or inspection

American Concrete Institute (ACI) Certification

ACI-CFTT Concrete Field Testing Technician – Grade 1
ACI-CCI Concrete Construction Inspector
ACI-LTT Laboratory Testing Technician – Grade 1&2
ACI-STT Strength Testing Technician

American Welding Society (AWS) Certification

AWS-CWI Certified Welding Inspector
AWS/AISC-SSI Certified Structural Steel Inspector

American Society of Non-Destructive Testing (ASNT) Certification

ASNT Non-Destructive Testing Technician – Level II or III.

International Code Council (ICC) Certification

ICC-SMSI Structural Masonry Special Inspector
ICC-SWSI Structural Steel and Welding Special Inspector
ICC-SFSI Spray-Applied Fireproofing Special Inspector
ICC-PCSI Prestressed Concrete Special Inspector
ICC-RCSI Reinforced Concrete Special Inspector

National Institute for Certification in Engineering Technologies (NICET)

NICET-CT Concrete Technician – Levels I, II, III & IV
NICET-ST Soils Technician - Levels I, II, III & IV
NICET-GET Geotechnical Engineering Technician - Levels I, II, III & IV

Other

Project: Seaside Elderly Care Entry Canopy
 Date Prepared: 07/25/2007

Structural Schedule of Special Inspections
SOILS & FOUNDATION CONSTRUCTION

VERIFICATION AND INSPECTION	Y/N	EXTENT: CONTINUOUS, PERIODIC, SUBMITTAL, OR NONE	COMMENTS	AGENT	AGENT QUALIFICATION	TASK COMPLETED
IBC Section 1704.7, 1704.8, 1704.9						
1. Verify existing soil conditions, fill placement and load bearing requirements						
a. Prior to placement of prepared fill, determine that the site has been prepared in accordance with the approved soils report.	Y	P	IBC 1704.7.1	TA1	PE/GE, EIT or ETT	
b. During placement and compaction of fill material, verify material being used and maximum lift thickness comply with the approved soils report.	Y	P	IBC 1704.7.2	TA1	PE/GE, EIT or ETT	
c. Test in-place dry density of compacted fill complies with the approved soils report.	Y	p	IBC 1704.7.2	TA1	PE/GE, EIT or ETT	
2. Pile foundations:						
a. Observe and record procedures for static load testing of piles.	N	Load Testing not required due to pile capacity				
b. Observe and record procedures for dynamic load testing of piles.	N					
c. Record installation of each pile and results of load test. Include cutoff and tip elevations of each pile relative to permanent reference.		C		TA1	PE/GE, EIT or ETT	
d. Test welded splices of steel piles	N					
3. Pier foundations: Verify installation of pier foundations for buildings assigned to Seismic Design Category C, D, E or F.	N					
a. Verify pier diameter and length	N					
b. Verify pier embedment (socket) into bedrock	N					
c. Verify suitability of end bearing strata	N					

Project: Seaside Elderly Care Entry Canopy
 Date Prepared: 07/25/2007

Structural Schedule of Special Inspections CONCRETE CONSTRUCTION

VERIFICATION AND INSPECTION	Y/N	EXTENT: CONTINUOUS, PERIODIC, SUBMITTAL, OR NONE	COMMENTS	AGENT	AGENT QUALIFICATION	TASK COMPLETED
IBC Section 1704.4						
1. Inspection of reinforcing steel, including prestressing tendons, and placement	Y	P	ACI 318: 3.5, 7.1-7.7	SI1	PE/SE or EIT	
2. Inspection of reinforcing steel welding in accordance with Table 1704.3, Item 5B	N		Welding of Reinf Not Allowed			
3. Inspect bolts to be installed in concrete prior to and during placement of concrete where allowable loads have been increased	N	Loads not increased				
4. Verifying use of required design mix	Y	P	ACI 318: Ch 4, 5.2-5.4	SI1	PE/SE or EIT	
5. At time fresh concrete is sampled to fabricate specimens for strength test, perform slump and air content test and temperature	Y	C	ASTM C 172 ASTM C 31 ACI 318: 5.6, 5.8	TA1	ACI-CFTT or ACI-STT	
6. Inspection of concrete and shotcrete placement for proper application techniques	N					
7. Inspection for maintenance of specified curing temperature and techniques	Y	P	ACI 318: 5.11-5.13	SI1	PE/SE or EIT	
8. Inspection of Prestressed Concrete						
a. Application of prestressing force.	N					
b. Grouting of bonded prestressing tendons in seismic force resisting system	N					
9. Erection of precast concrete members	N					
10. Verification of in-situ concrete strength, prior to stressing of tendons in post-tensioned concrete and prior to removal of shores and forms beams and structural slabs	N					

Project: Seaside Elderly Care Entry Canopy
 Date Prepared: 07/25/2007

Structural Schedule of Special Inspections
MASONRY CONSTRUCTION – LEVEL 1 (NON-ESSENTIAL FACILITY)

VERIFICATION AND INSPECTION IBC Section 1704.5	Y/N	EXTENT: CONTINUOUS, PERIODIC, SUBMITTAL, OR NONE	COMMENTS	AGENT	AGENT QUALIFICATION	TASK COMPLETED
1. As masonry construction begins, the following shall be verified to ensure compliance:		No Structural Masonry				
a. Proportions of site-prepared mortar.						
b. Construction of mortar joints.						
c. Location of reinforcement and connectors.						
d. Prestressing technique.						
e. Grade and size of prestressing tendons and anchorages.						
2. The inspection program shall verify:						
a. Size and location of structural elements.						
b. Type, size and location of anchors, including other details of anchorage of masonry to structural members, frames or other construction.						
c. Specified size, grade and type of reinforcement.						
d. Welding of reinforcing bars.						
e. Protection of masonry during cold weather (temperature below 40°F) or hot weather (temperature above 90°F).						
f. Application and measurement of prestressing force.						
3. Prior to grouting, the following shall be verified to ensure compliance:						
a. Grout space is clean.						
b. Placement of reinforcement and connectors and prestressing tendons and anchorages.						
c. Proportions of site-prepared grout and prestressing grout for bonded tendons.						
d. Construction of mortar joints.						
4. Grout placement shall be verified to ensure compliance with code and construction document provisions.						
a. Grouting of prestressing bonded tendons.						
5. Preparation of any required grout specimens, mortar specimens and/or prisms shall be observed.						
6. Compliance with required inspection provisions of the construction documents and the approved submittals shall be verified.						

Project: Seaside Elderly Care Entry Canopy
 Date Prepared: 07/25/2007

Structural Schedule of Special Inspections
MASONRY CONSTRUCTION – LEVEL 2 (ESSENTIAL FACILITY)

VERIFICATION AND INSPECTION IBC Section 1704.5	Y/N	EXTENT: CONTINUOUS, PERIODIC, SUBMITTAL, OR NONE	COMMENTS	AGENT	AGENT QUALIFICATION	TASK COMPLETED
1. From the beginning of masonry construction, the following shall be verified to ensure compliance:		No Structural Masonry				
a. Proportions of site-mixed mortar, grout and prestressing grout for bonded tendons.						
b. Placement of masonry units and construction of mortar joints.						
c. Placement of reinforcement, connectors and prestressing tendons and anchorages.						
d. Grout space prior to grouting.						
e. Placement of grout.						
f. Placement of prestressing grout.						
2. The inspection program shall verify:						
a. Size and location of structural elements.						
b. Type, size and location of anchors, including other details of anchorage of masonry to structural members, frames or other construction.						
c. Specified size, grade and type of reinforcement.						
d. Welding of reinforcement.						
e. Protection of masonry during cold weather and (temperature below 40°F) or hot weather (temperature above 90°F).						
f. Application and measurement of prestressing force.						
3. Preparation of any required grout specimens, mortar specimens and/or prisms shall be observed.						
4. Compliance with required inspection provisions of the construction documents and the approved submittals shall be verified.						

Project: Seaside Elderly Care Entry Canopy
Date Prepared: 07/25/2007

Structural Schedule of Special Inspections - STEEL CONSTRUCTION

VERIFICATION AND INSPECTION	Y/N	EXTENT: CONTINUOUS, PERIODIC, SUBMITTAL, OR NONE	COMMENTS	AGENT	AGENT QUALIFICATION	TASK COMPLETED
IBC Section 1704.3						
1. Material verification of high-strength bolts, nuts and washers:						
a. Identification markings to conform to ASTM standards specified in the approved construction documents.	Y	S	Applicable ASTM material specifications; AISC 335, Section A3.4; AISC LRFD, Section A3.3	SI1	PE/SE or EIT	
b. Manufacturer's certificate of compliance required.	Y	S		SI1	PE/SE or EIT	
2. Inspection of high-strength bolting						
a. Bearing-type connections.	Y	P	AISC LRFD Section M2.5	TA1	AWS/AISC-SSI	
b. Slip-critical connections.	Y	C or P (method dependent)	IBC Sect 1704.3.3	TA1	AWS/AISC-SSI	
3. Material verification of structural steel (IBC Sect 1708.4):						
a. Identification markings to conform to ASTM standards specified in the approved construction documents.	Y	S	ASTM A 6 or ASTM A 568 IBC Sect 1708.4	SI1	PE/SE or EIT	
b. Manufacturers' certified mill test reports.	Y	S	ASTM A 6 or ASTM A 568 IBC Sect 1708.4	SI1	PE/SE or EIT	
4. Material verification of weld filler materials:						
a. Identification markings to conform to AWS specification in the approved construction documents.	Y	S	AISC, ASD, Section A3.6; AISC LRFD, Section A3.5	SI1	PE/SE or EIT	
b. Manufacturer's certificate of compliance required.	Y	S		SI1	PE/SE or EIT	
5. Submit current AWS D1.1 welder certificate for all field welders who will be welding on this project.	Y	S	AWS D1.1	SI1	PE/SE or EIT	
6. Inspection of welding (IBC 1704.3.1):						
a. Structural steel:						
1) Complete and partial penetration groove welds.	Y	C	AWS D1.1	TA1	AWS-CWI	
2) Multipass fillet welds.	Y	C		TA1	AWS-CWI	
3) Single-pass fillet welds > 5/16"	Y	C		TA1	AWS-CWI	
4) Single-pass fillet welds < 5/16"	Y	P		TA1	AWS-CWI	
5) Floor and deck welds.	N					
b. Reinforcing steel (IBC Sect 1903.5.2):						
1) Verification of weldability of reinforcing steel other than ASTM A706.	N					
2) Reinforcing steel-resisting flexural and axial forces in intermediate and special moment frames, and boundary elements of special reinforced concrete shear walls and shear reinforcement.	N					
3) Shear reinforcement.	N					
4) Other reinforcing steel.	N					
7. Inspection of steel frame joint details for compliance (IBC Sect 1704.3.2) with approved construction documents:						
a. Details such as bracing and stiffening.	Y	P		SI1	PE/SE or EIT	
b. Member locations.	Y	P		SI1	PE/SE or EIT	
c. Application of joint details at each connection.	Y	P		SI1	PE/SE or EIT	

Project: Seaside Elderly Care Entry Canopy
Date Prepared: 07/25/2007

Structural Schedule of Special Inspection Services
FABRICATION AND IMPLEMENTATION PROCEDURES – STRUCTURAL STEEL

VERIFICATION AND INSPECTION IBC Section 1704.2	Y/N	EXTENT: CONTINUOUS, PERIODIC, SUBMITTAL, OR NONE	COMMENTS	AGENT	AGENT QUALIFICATION	TASK COMPLETED
1. Fabrications Procedures: Review of fabricator's written procedural and quality control manuals and periodic auditing of fabrication practices by an approved special inspection agency. At the completion of fabrication, the approved fabricator shall submit a certificate of compliance to the building code official stating that the work was performed in accordance with the approved construction documents. -OR- 2. AISC Certification	Y	S	Fabricator shall submit one of the two qualifications	SI1	PE/SE or EIT	
3. At completion of fabrication, the approved fabricator shall submit a certificate of compliance to the building code official stating that the work was performed in accordance with the approved construction documents.	Y	S	IBC 1704.2.2	SI1	PE/SE or EIT	

Project: Seaside Elderly Care Entry Canopy
 Date Prepared: 07/25/2007

Structural Schedule of Special Inspection Services
FABRICATION AND IMPLEMENTATION PROCEDURES – COLD FORMED METAL TRUSSES

VERIFICATION AND INSPECTION IBC Section 1704.2	Y/N	EXTENT: CONTINUOUS, PERIODIC, SUBMITTAL, OR NONE	COMMENTS	AGENT	AGENT QUALIFICATION	TASK COMPLETED
1. Fabrications Procedures: Review of fabricator's written procedural and quality control manuals and periodic auditing of fabrication practices by an approved special inspection agency. At the completion of fabrication, the approved fabricator shall submit a certificate of compliance to the building code official stating that the work was performed in accordance with the approved construction documents.	Y	S	Fabricator shall submit one of the two qualifications	SI1	PE/SE or EIT	
3. At completion of fabrication, the approved fabricator shall submit a certificate of compliance to the building code official stating that the work was performed in accordance with the approved construction documents	Y	S	IBC 1704.2.2	SI1	PE/SE or EIT	

Project: Seaside Elderly Care Entry Canopy
 Date Prepared: 07/25/2007

Structural Schedule of Special Inspections
WOOD CONSTRUCTION

VERIFICATION AND INSPECTION IBC Section 1704.6	Y/N	EXTENT: CONTINUOUS, PERIODIC, SUBMITTAL, OR NONE	COMMENTS	AGENT	AGENT QUALIFICATION	TASK COMPLETED
1. Fabrication of high-load diaphragms						
a. Verify wood structural panel sheathing for grade and thickness		No wood construction				
b. Verify the nominal size of framing members at adjoining panel edges						
b. Verify the nail or staple diameter and length						
b. Verify the number of fastener lines						
b. Verify the spacing between fasteners in each line and at edge margins						
2. Load Tests for Joist Hangers: Provide evidence of manufacturer's load test in accordance with ASTM D1761 including the vertical load bearing capacity, torsional moment capacity, and deflection characteristics when there is no calculated procedure recognized by the code.						

Structural Schedule of Special Inspections
SEISMIC RESISTANCE - STRUCTURAL

VERIFICATION AND INSPECTION	Y/N	EXTENT: CONTINUOUS, PERIODIC, SUBMITTAL, OR NONE	COMMENTS	AGENT	AGENT QUALIFICATION	TASK COMPLETED
IBC Section 1707						
1. Special inspections for seismic resistance. Special inspection as specified in this section is required for the following:			Seismic Design Category: C			
a. The seismic-force-resisting systems in structures assigned to Seismic Design Category C, D, E or F	Y	P	IBC 1707.1	SI1	PE/SE or EIT	
2. Structural steel: Continuous special inspection for structural welding in accordance with AISC 341.	N					
3. Structural wood:						
a. Continuous special inspection during field gluing operations of elements of the seismic-force-resisting system.	N					
b. Periodic special inspections for nailing, bolting, anchoring and other fastening of components within the seismic-force-resisting system, including drag struts, braces and hold-downs	N					
4. Cold-formed steel framing: Periodic special inspections during welding operations of elements of the seismic-force-resisting system. Periodic special inspections for screw attachment, bolting, anchoring and other fastening of components within the seismic-force-resisting system, including struts, braces, and hold-downs	Y	P		SI1	PE/SE or EIT	
4. Seismic isolation system. Provide periodic special inspection during the fabrication and installation of isolator units and energy dissipation devices if used as part of the seismic isolation system	N					

Project: Seaside Elderly Care Entry Canopy
 Date Prepared: 07/25/2007

Quality Assurance Plan – Seismic and Wind

QUALITY ASSURANCE FOR SEISMIC RESISTANCE CHECK LIST [IBC 1705]

Seismic Design Category **C**

FOR SEISMIC DESIGN CATEGORY C OR HIGHER:

Structural:

- The seismic-force-resisting systems
 - Steel Braced Frames and associated connections/anchorage
 - Steel Moment Frames and associated connections
 - Shear walls: CMU Wood Concrete
 - Diaphragms: Floor Roof
 - Other:

QUALITY ASSURANCE FOR WIND RESISTANCE CHECK LIST [IBC 1706]

Wind Exposure Category

REQUIRED	NOT REQUIRED	NOT APPLICABLE	QUALITY ASSURANCE PLAN REQUIREMENTS (A Quality Assurance Plan is required where indicated below)
<input type="checkbox"/>	<input checked="" type="checkbox"/>	<input type="checkbox"/>	In wind exposure Categories A and B, where the 3-second-gust basic wind speed is 120 miles per hour (mph) (52.8 <i>m/sec</i>) or greater.
<input type="checkbox"/>	<input checked="" type="checkbox"/>	<input type="checkbox"/>	In wind exposure Categories C and D, where the 3-second-gust basic wind speed is 110 mph (49 <i>m/sec</i>) or greater.

Prepared by:

Building Code Official's Acceptance:

Signature

Date

Signature

Date

Project: Seaside Elderly Care Entry Canopy
Date Prepared: 07/25/2007

Contractor's Statement of Responsibility

Each contractor responsible for the construction or fabrication of a system or component designated in the Quality Assurance Plan must submit a Statement of Responsibility. The Statement of Responsibility is required for Seismic Design Category C or higher. Make additional copies of this form as required.

Project: Seaside Elderly Care Entry Canopy

Contractor's Name:

Address:

License No.:

Description of designated building systems and components included in the Statement of Responsibility:

Contractor's Acknowledgment of Special Requirements

I hereby acknowledge that I have received, read, and understand the Quality Assurance Plan and Special Inspection program.

I hereby acknowledge that control will be exercised to obtain conformance with the construction documents approved by the Building Official.

Signature

Date

Contractor's Provisions for Quality Control

Procedures for exercising control within the contractor's organization, the method and frequency of reporting and the distribution of reports is attached to this Statement.

Identification and qualifications of the person(s) exercising such control and their position(s) in the organization are attached to this Statement.

SEASIDE NURSING FACIL

850 BAXTER BOULEVARD
PORTLAND, MAINE

OWNER:

FIRST ATLANTIC HEALTHCARE
222 ST. JOHN'S STREET
PORTLAND, ME 04101

ARCHITECT

FORESIDE ARCHITECTS, LLC
PO BOX 66736
FALMOUTH, ME 04105

TEL. 781-3344

CIVIL ENGINEER/
LANDSCAPE ARCHITECTS

LAND-USE CONSULTANTS
966 RIVERSIDE STREET
PORTLAND, ME 04103

TEL. 878-3313

STRUCTURAL ENGINEER

BECKER STRUCTURAL ENGINEERS
75 YORK STREET
PORTLAND, ME 04101

TEL. 879-1838

GENERAL CONTRACTOR/
CONSTRUCTION MANAGER

TO BE DETERMINED

ELECTRICAL ENGINEER

DESIGN / BUILD
SUBCONTRACTOR TO THE
SELECTED GENERAL
CONTRACTOR

MECHANICAL ENGINEER
AND FIRE SUPPRESSION

DESIGN / BUILD
SUBCONTRACTOR TO THE
SELECTED GENERAL
CONTRACTOR

LIST

T-1

SITE

SHEE

SHEE

STRU

S-1

S-2

ARCH

A-1

A-2

A-3

A-4

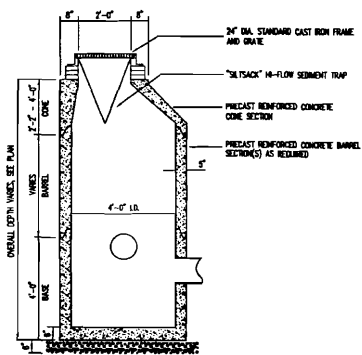
A-5

A-5:

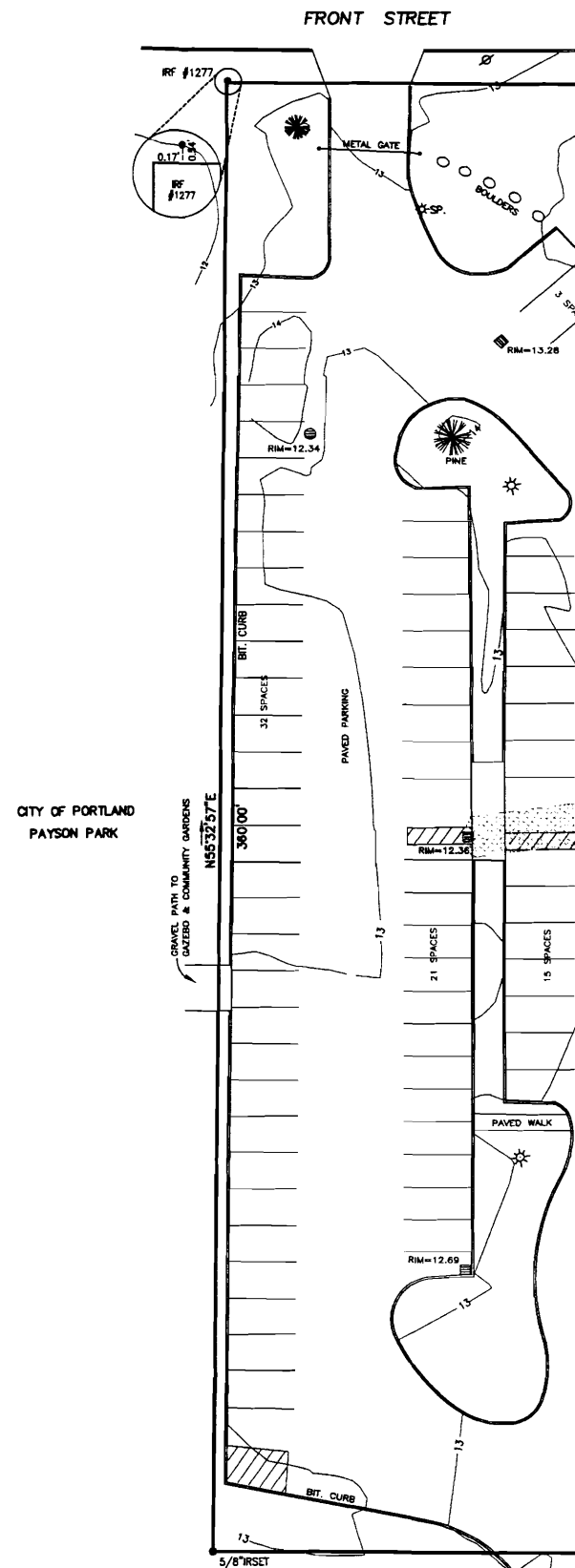
A-6

SITE DEMOLITION NOTES:

- PROJECT IS LOCATED WITHIN THE CITY OF PORTLAND, MAINE.
- TOPOGRAPHIC INFORMATION FROM A GROUND SURVEY BY OWN HASKELL, INC, DATED MARCH, 2007.
- THE PROJECT IS SUBJECT TO MINOR SITE PLAN REVIEW APPROVAL FROM THE CITY OF PORTLAND.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR NOTIFYING "DIG-SAFE" AND LOCAL UTILITY COMPANIES AT LEAST 3 BUSINESS DAYS, BUT NOT MORE THAN 30 CALENDAR DAYS, PRIOR TO THE COMMENCEMENT OF ANY EXCAVATION, OR AS OTHERWISE REQUIRED BY MAINE STATE LAW.
- THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ALL MEANS, METHODS AND TECHNIQUES EMPLOYED TO PERFORM THE WORK SHOWN ON THESE PLANS.
- ALL WORK SHALL COMPLY WITH ALL LOCAL, STATE AND FEDERAL SAFETY REGULATIONS.
- ALL WORK SHALL BE IN CONFORMANCE WITH THE CITY OF PORTLAND AND ALL UTILITY COMPANIES STANDARDS.
- CONTRACTOR SHALL VERIFY LOCATIONS AND DEPTHS OF ALL UTILITIES WITH THE RESPECTIVE COMPANY PRIOR TO THE START OF CONSTRUCTION. IF ANY DISCREPANCIES OR CONFLICTS ARE FOUND, THE CONTRACTOR SHALL NOTIFY THE ENGINEER AND OWNER PRIOR TO PROCEEDING.
- THE CONTRACTOR SHALL SECURE ALL NECESSARY CONSTRUCTION ACTIVITY PERMITS FOR THE WORK SHOWN ON THESE PLANS PRIOR TO CONSTRUCTION.
- NO BLASTING WILL BE ALLOWED WITHIN 500 FT OF ANY UTILITY WITHOUT THE NOTIFICATION AND APPROVAL OF THE APPROPRIATE UTILITY COMPANY. NO LEDGE BLASTING WILL BE PERMITTED WITHIN A UTILITY COMPANY EASEMENT UNTIL WRITTEN APPROVAL FROM THE UTILITY IS GIVEN. BLASTING OPERATIONS SHALL BE IN ACCORDANCE WITH MAINE DEPT OF ENVIRONMENTAL PROTECTION BLASTING REQUIREMENTS AND SITE LOCATION OF DEVELOPMENT APPROVAL ORDER.
- ALL PAVEMENT CUTS SHALL BE SAW CUT TO RESULT IN CLEAN EDGES. A TACK COAT SHALL BE APPLIED ALONG THE CUT EDGES AND NEW PAVEMENT BUTTED TO IT, UNLESS OTHERWISE DIRECTED BY THE ENGINEER.
- THE CONTRACTOR SHALL BE RESPONSIBLE TO RECLAIM OR PROPERLY DISPOSE OF ALL REMOVED BITUMINOUS MATERIALS.
- ALL EROSION & SEDIMENT CONTROL MEASURES SHALL BE INSTALLED & MAINTAINED IN ACCORDANCE WITH THE "MAINE EROSION AND SEDIMENT CONTROL HANDBOOK FOR CONSTRUCTION: BEST MANAGEMENT PRACTICES, MARCH 1991" AND AS OTHERWISE SPECIFIED OR INDICATED ON DRAWINGS.
- ALL LOAM SHALL REMAIN THE PROPERTY OF THE OWNER UNLESS OTHERWISE INDICATED.
- IF REQUIRED, REMOVE EXISTING LARGE STONES ALONG PAVEMENT EDGE STORE ON-SITE AND REPLACE.
- CORDON WORK AREAS AS DIRECTED BY OWNER. PROVIDE ACCESS AND EGRESS AS DIRECTED BY OWNER/ BUILDING ARCHITECT.



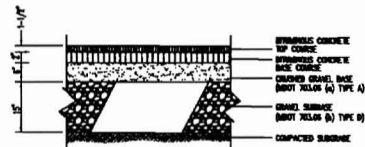
① CATCH BASIN WITH "SILTSACK" TEMPORARY SEDIMENT TRAP



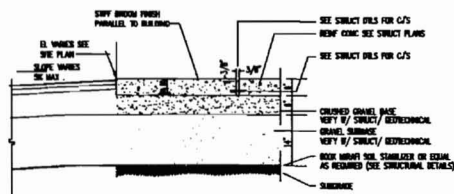
EXISTING CONDITION

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- THE CONTRACTOR SHALL BE RESPONSIBLE TO RECLAIM OR PROPERLY DISPOSE OF ALL REMOVED BITUMINOUS MATERIALS.
- CONDUIT SHALL BE USED FOR ELECTRIC, TELEPHONE AND TV IN ACCORDANCE WITH THE RESPECTIVE COMPANIES REQUIREMENTS.
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- REFER TO ARCHITECTURAL PLANS FOR BUILDING DIMENSIONS.
- ALL SIDEWALKS SHALL INCLUDE HC ACCESSIBLE RAMPS AT ALL INTERSECTIONS & DRIVEWAYS.



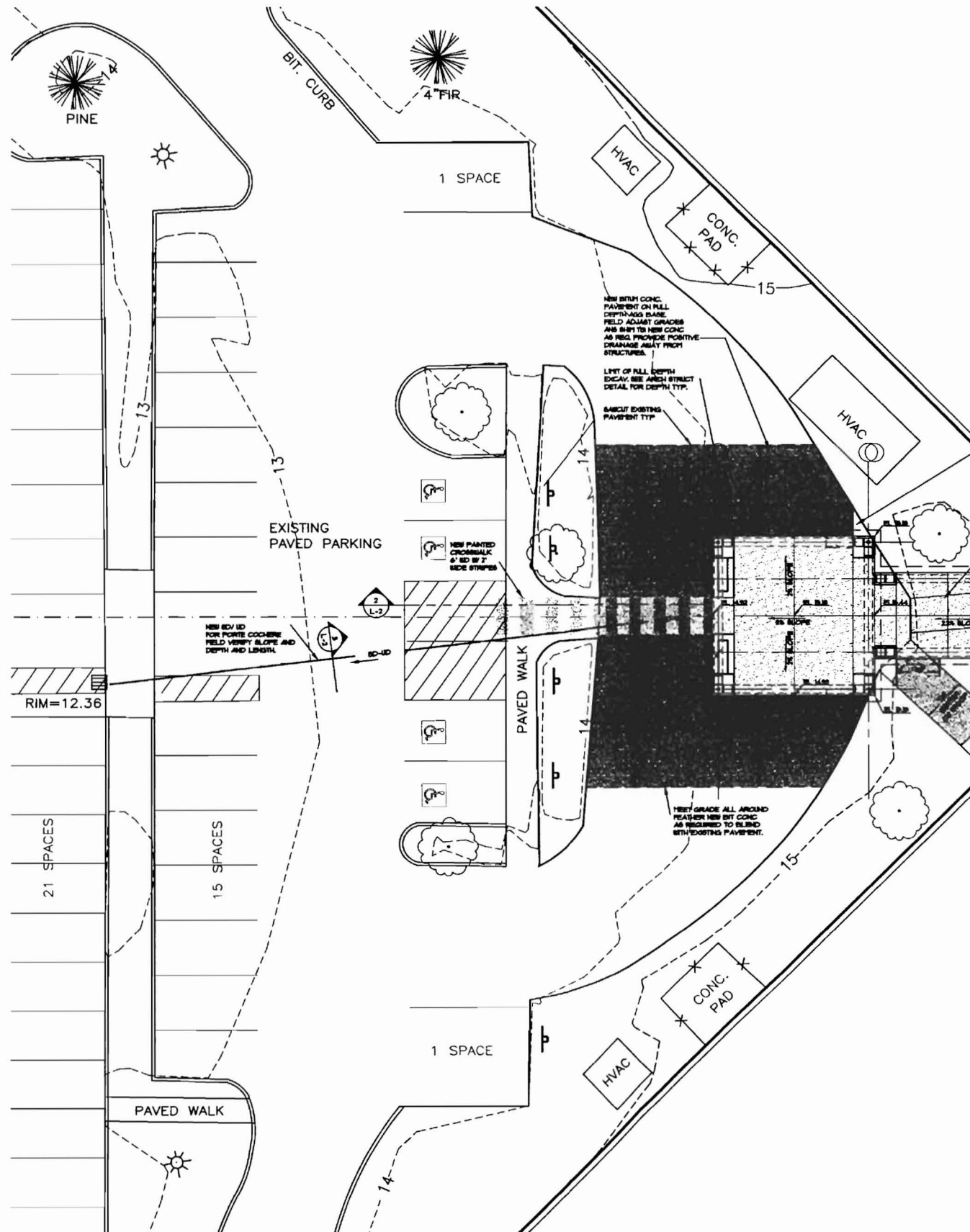
3 BITUMINOUS CONCRETE DRIVE & PARKING



NOTES:

- SEE ARCH AND STRUCT PLANS FOR CONC. DESIGN & DLS
- DO NOT PLACE CONCRETE FORMS STRAIGHT TO NEW CONC. APRON
- SEE ARCH PLAN DRAWINGS FOR EXPANSION JOINT LOCATIONS

2 BITUM. CONC. TRANSITION TO CONC. APRON

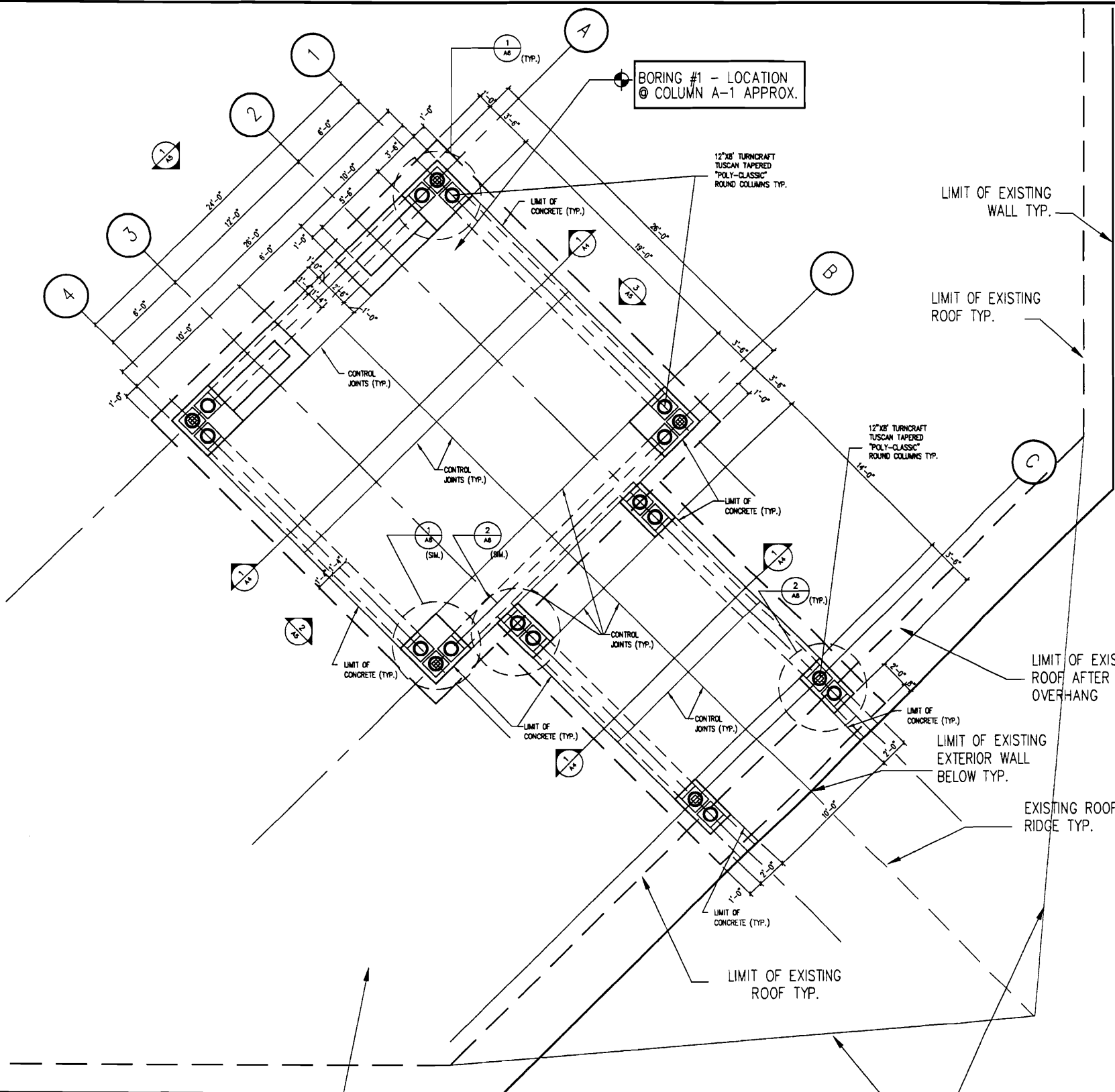


SITE PLAN

GRAPHIC SCALE



1 inch = 10 ft



PARTITION LEGEND

	EXISTING WALLS TO
	NEW INT PARTITION TO INTERIOR WALL SECTIONS FOR WALL

GRAPHIC LEGEND

DRAWING TITLE	6 DRAWING TITLE
INTERIOR ELEVATION MARKER	A3.1 7
EXTERIOR ELEVATION MARKER	1 A2.1
ROOM NAME & NUMBER	EXEC 105
DOOR TAG	104
FLOOR ELEVATION MARKER	
SECTION INDICATOR	3 A4.1
COLUMN & GRID	3
WINDOW TAG	A



PLAN NORTH

NOTE:
THE EXISTING CONDITIONS PLANS REPRRI ARE PROVIDED AS A CONCEPTUAL TOOL OF HELPING THE GENERAL CONTRACTOR SUB-CONTRACTOR TO PLAN THE SELEC PROCESS. HAVING HAD ACCESS TO THE DURING THE BIDDING PROCESS, IT IS A GENERAL CONTRACTOR AND THEIR SUBE RESPONSIBLE FOR THEIR CHOSEN M NECESSARY FOR THE COMPLETION OF 1 PRODUCT REPRESENTED WITHIN THESE 1 DOCUMENTS.

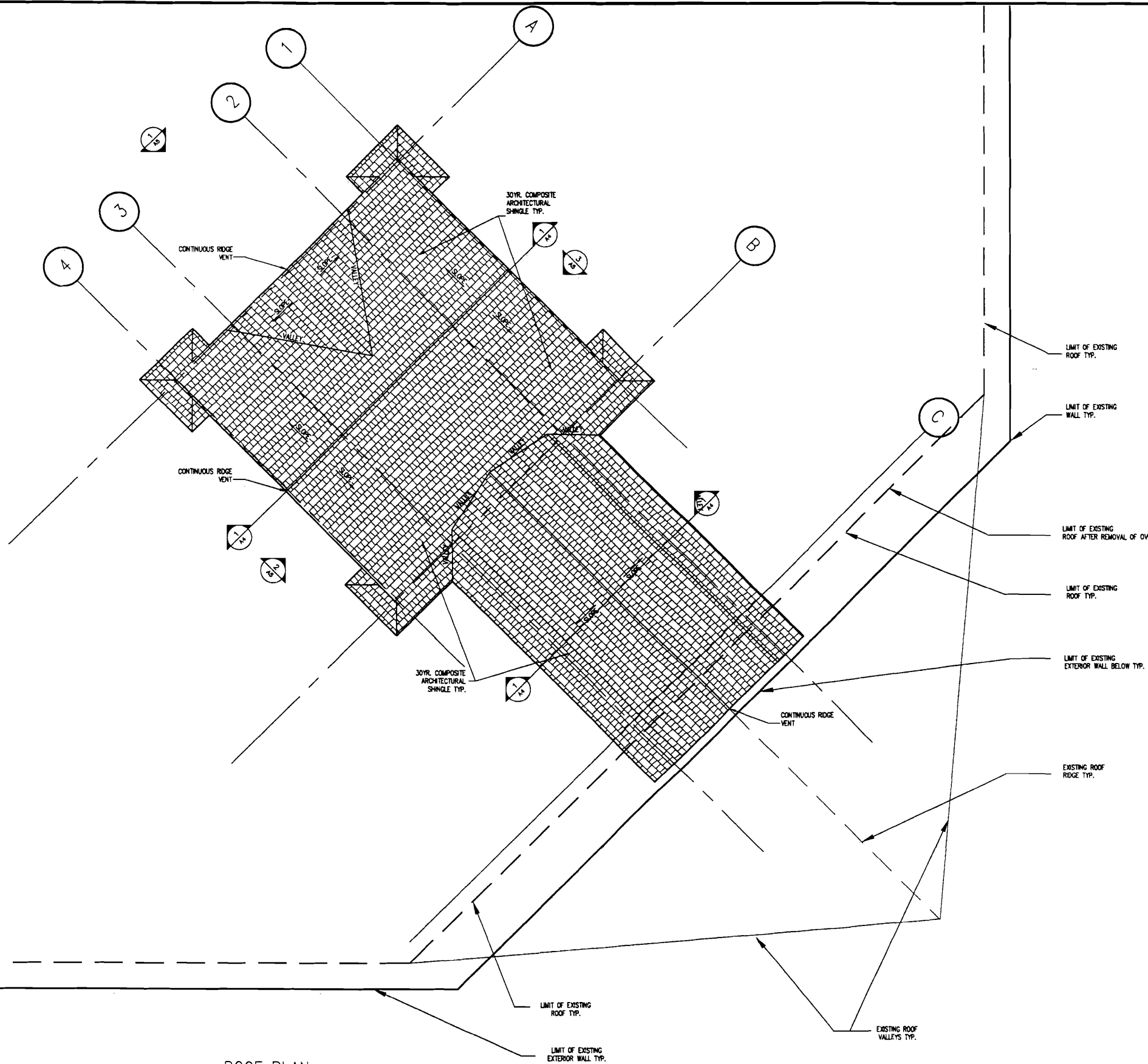
LIMIT OF EXISTING EXTERIOR WALL TYP.

FLOOR PLAN

1

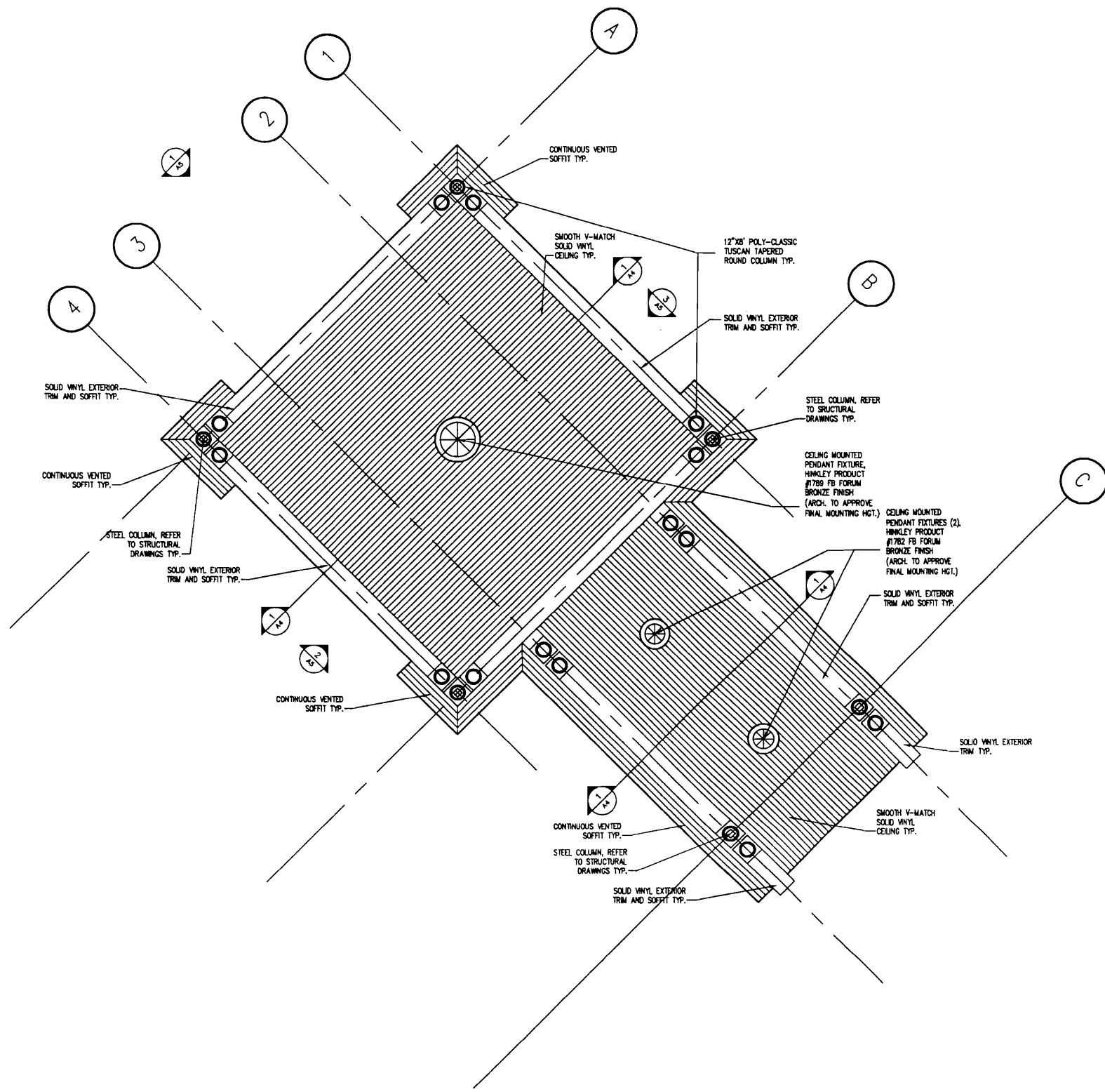
BORING #2 - LOCATION @ COLUMN C-6 APPROX.

SCALE: 1/4"=1'-0"



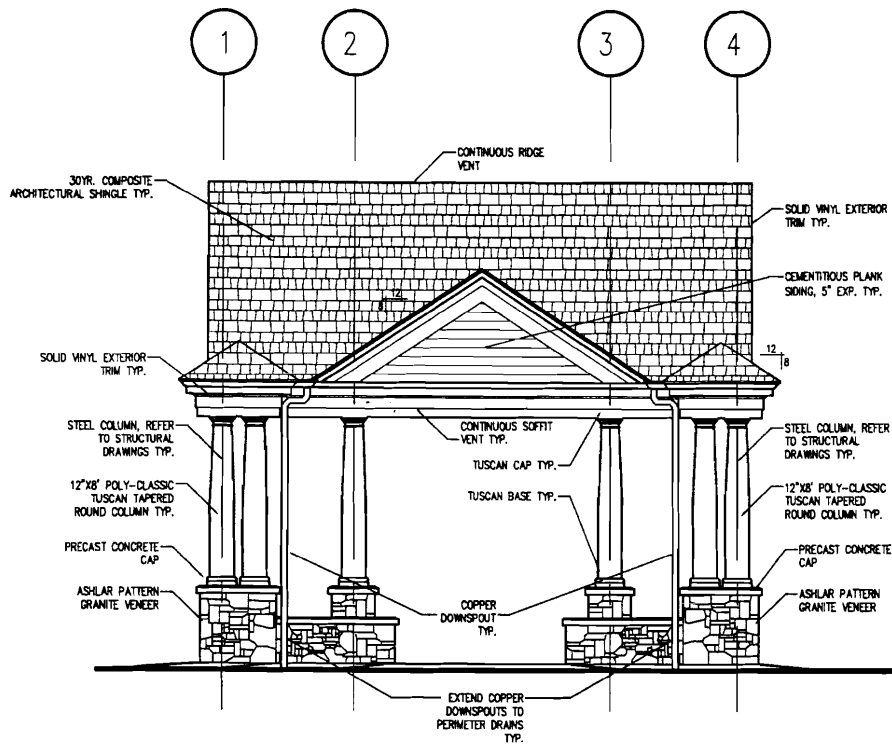
1 ROOF PLAN

SCALE: 1/8" = 1'-0"

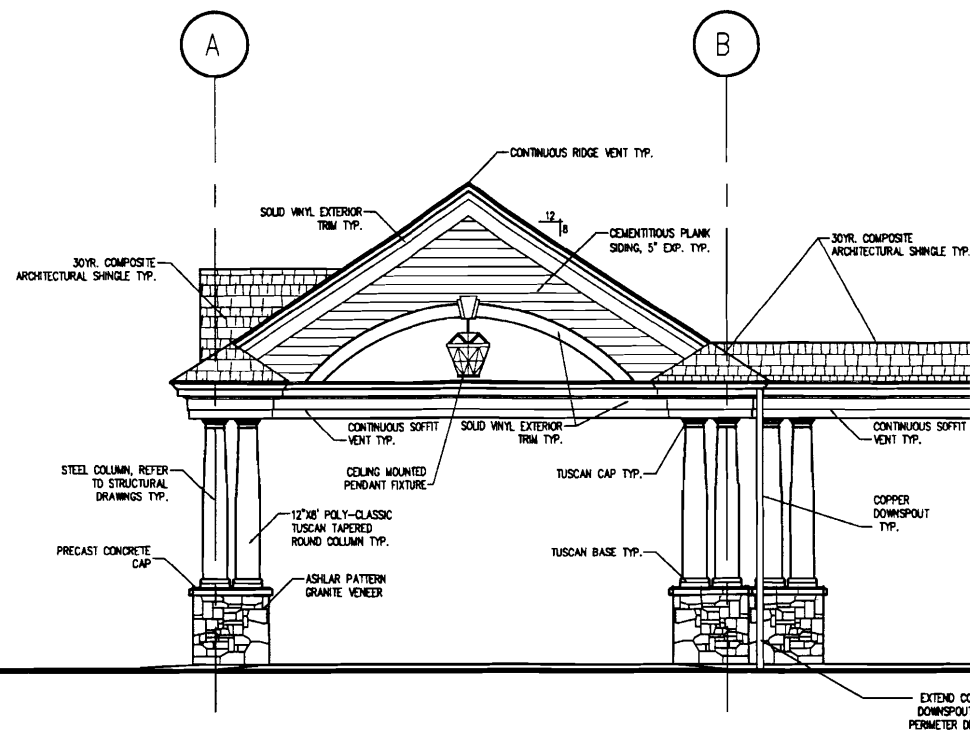


1 REFLECTED CEILING PLAN

SCALE: 1/4" = 1'-0"

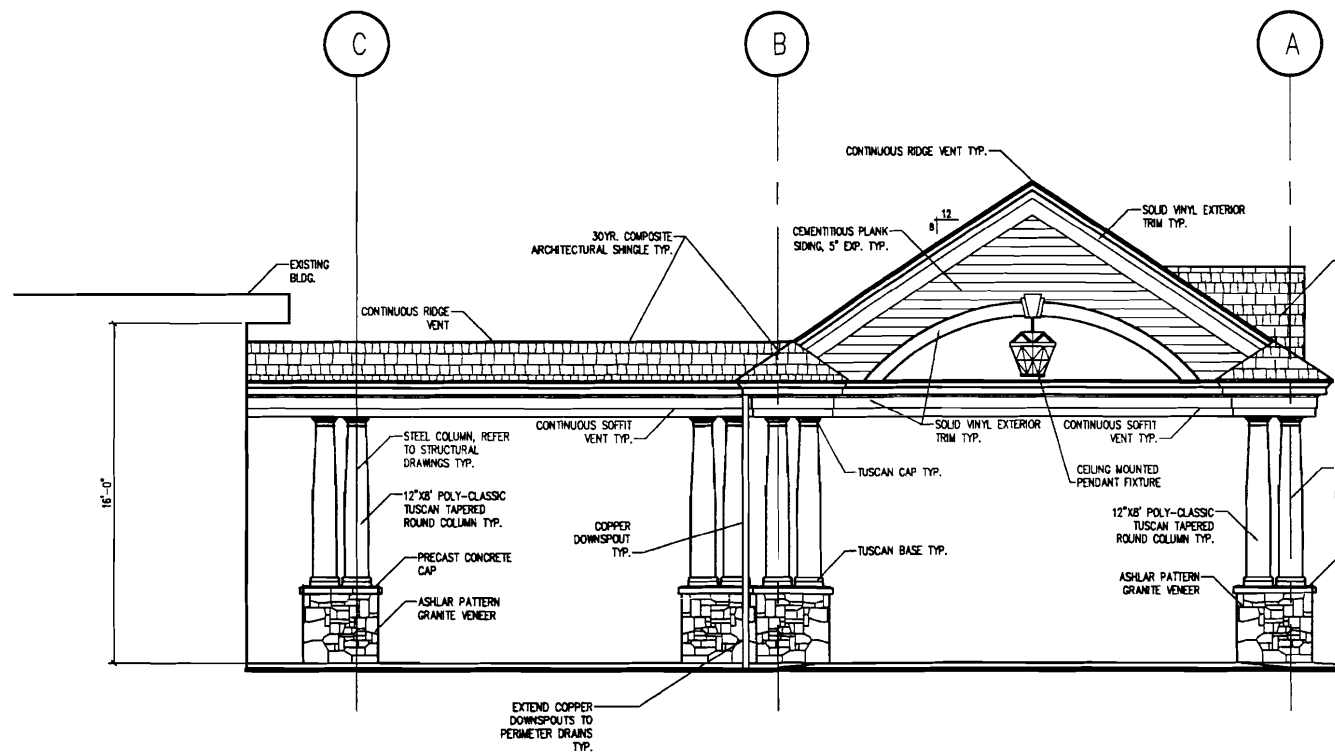


1 WEST ELEVATION



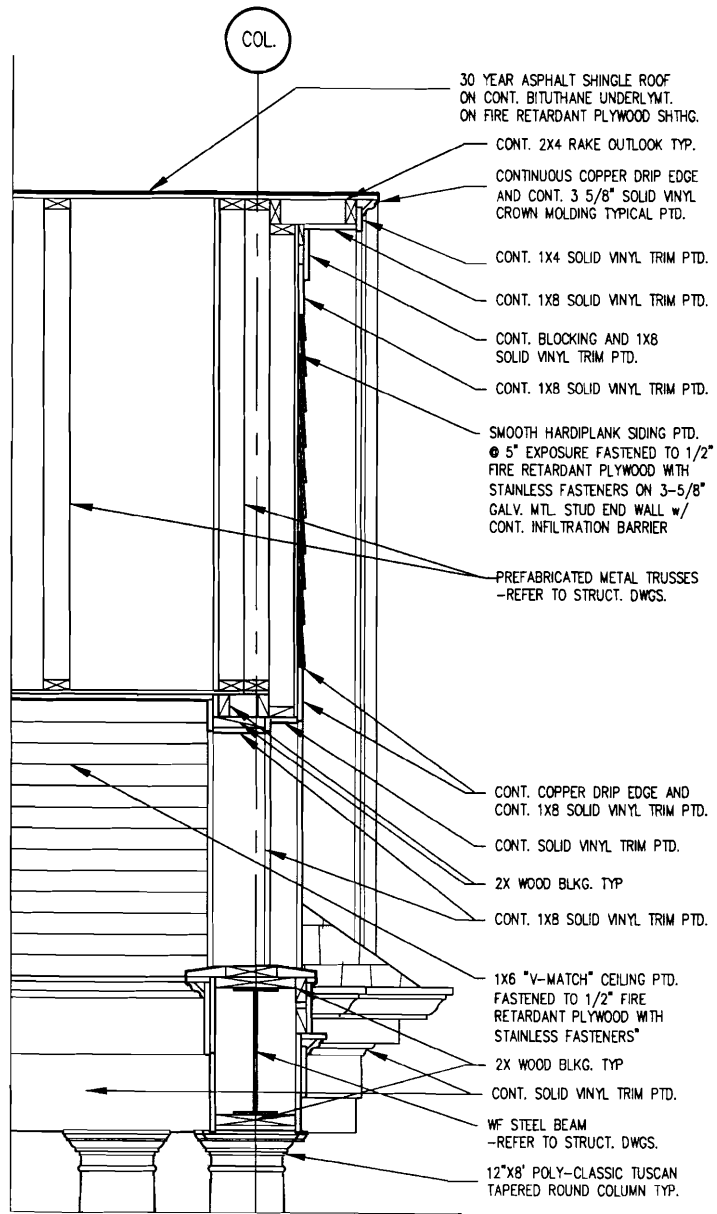
2 SOUTH ELEVATION

SCALE: 1/4"=1'-0"



3 NORTH ELEVATION

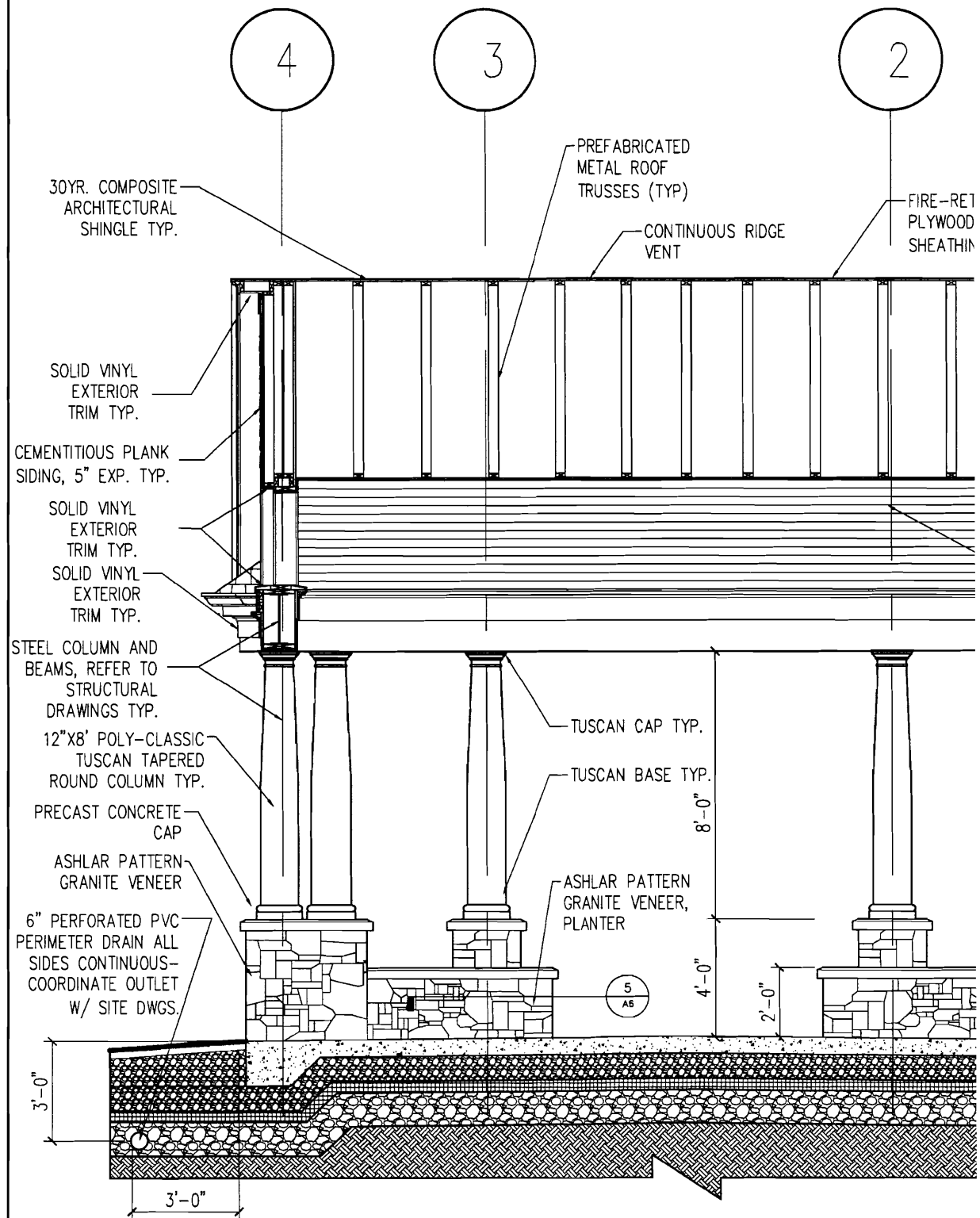
SCALE: 1/4"=1'-0"



1
A5

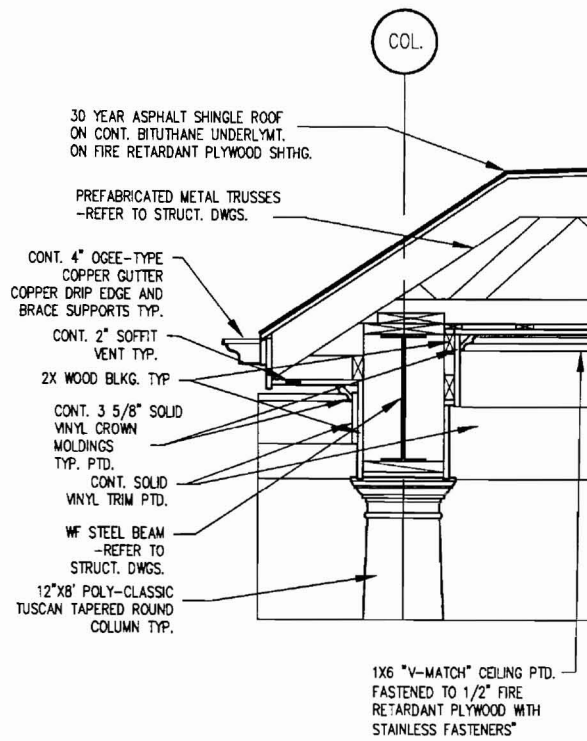
DETAIL @ RAKE

1"=1'-0"



SECTION A - A

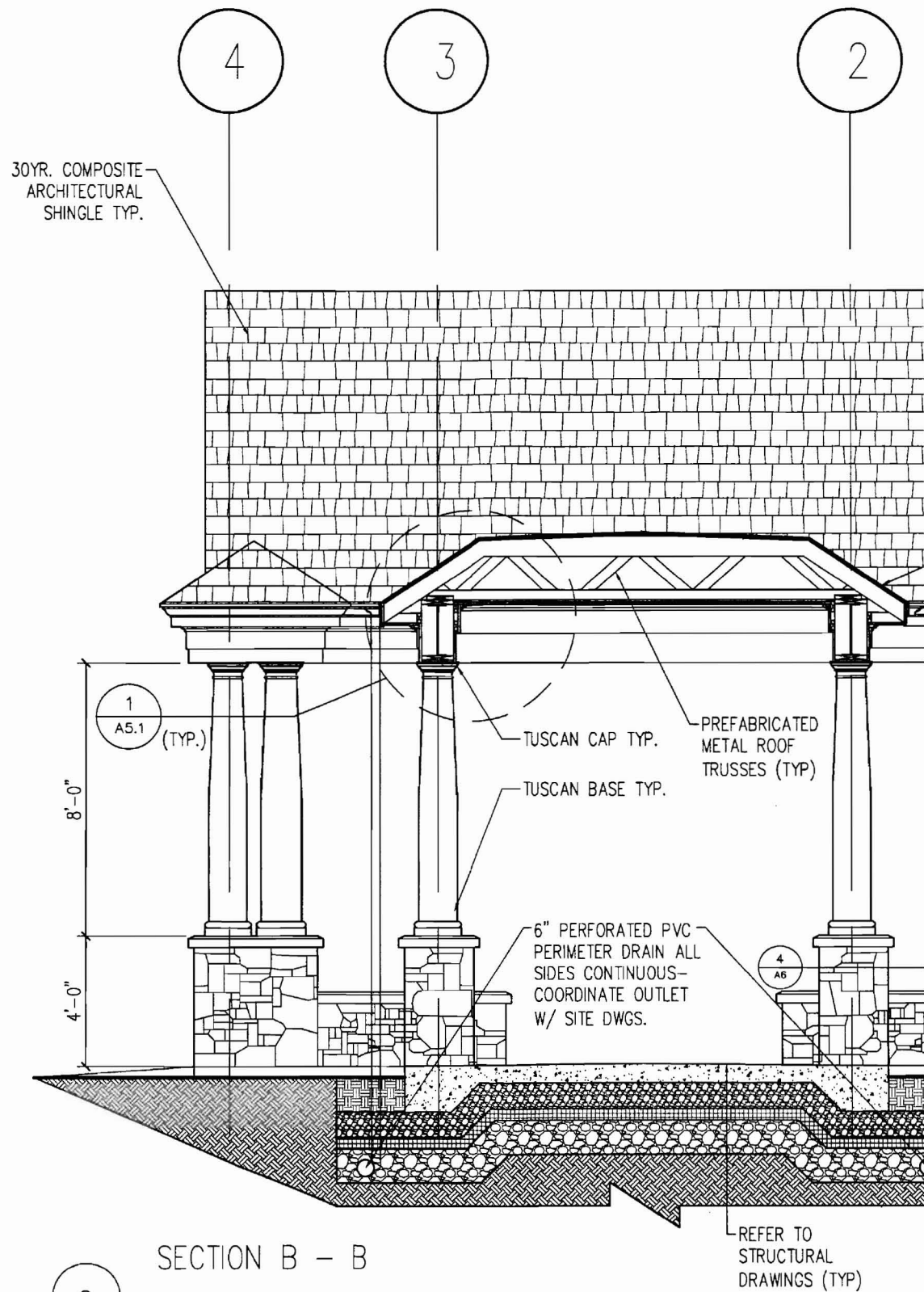
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1
A5.1

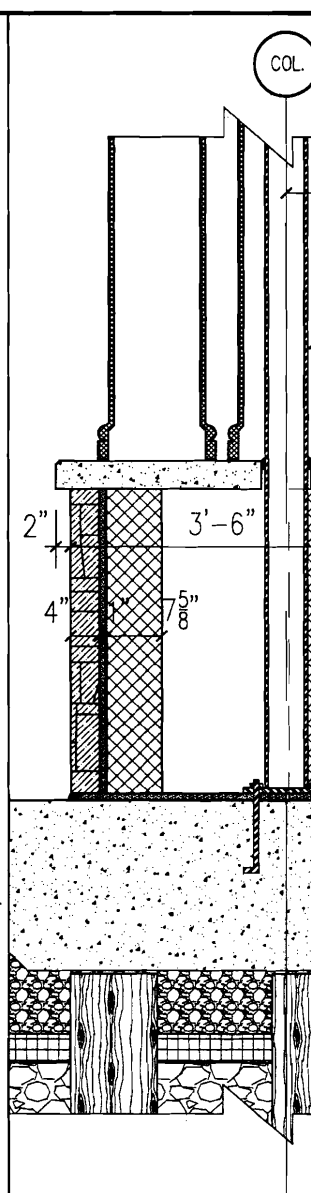
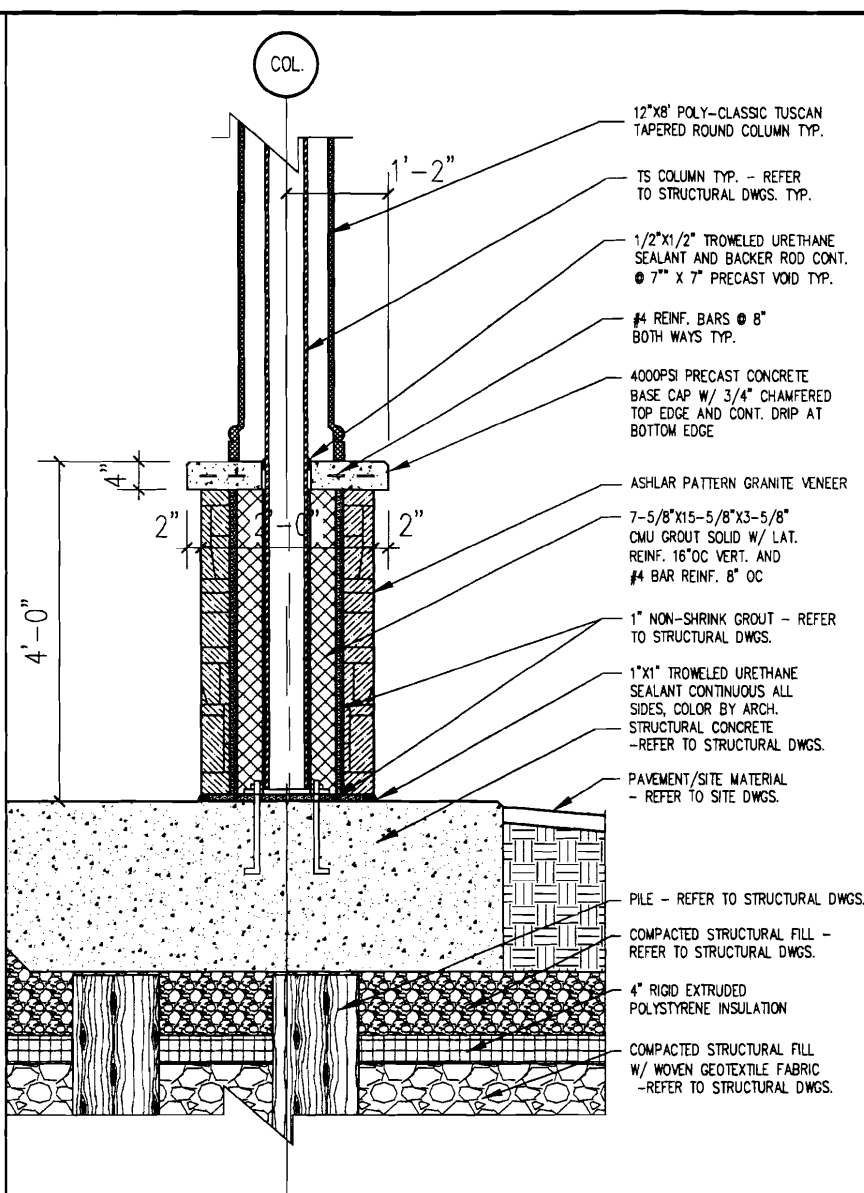
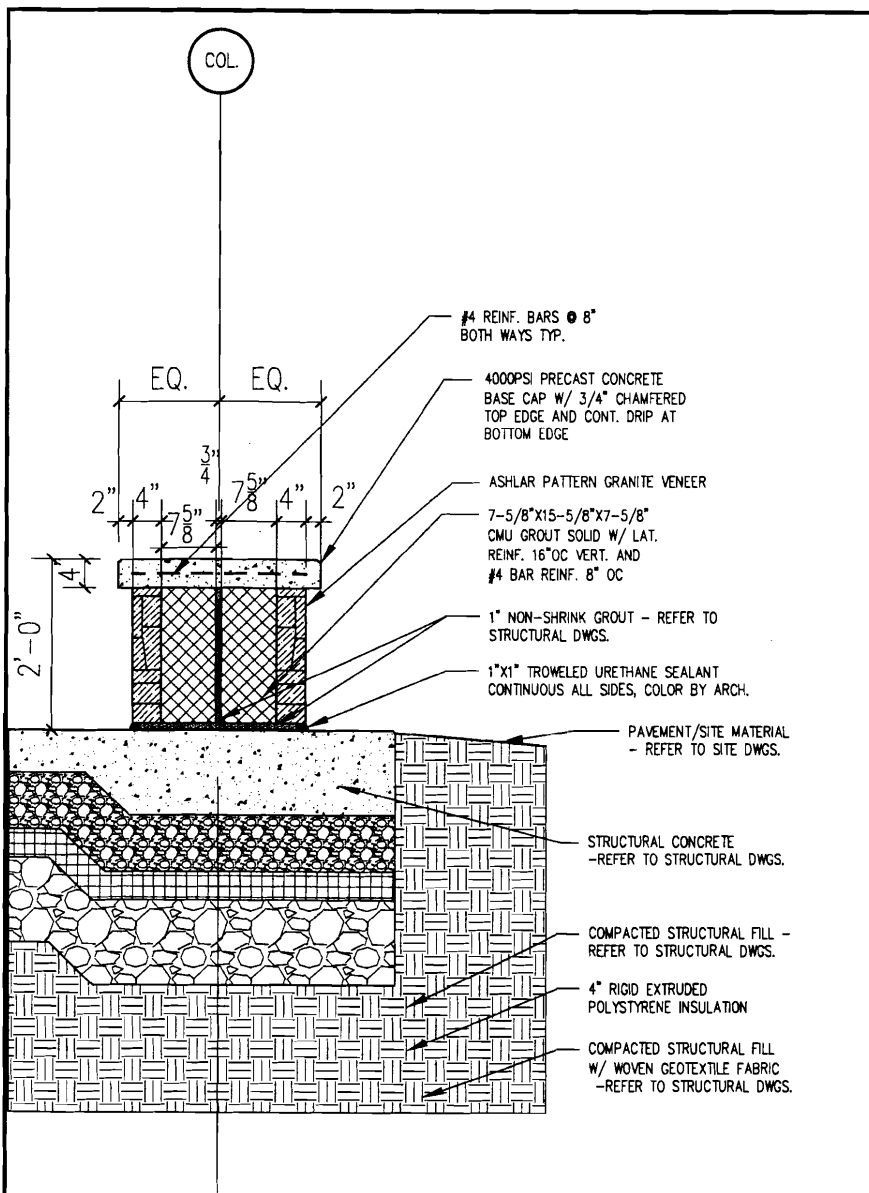
DETAIL @ EAVE

1"=1'-0"



SECTION B - B

2



4 DETAIL @ COLUMN BASE

3 DETAIL @

1"=1'-0"

